## Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

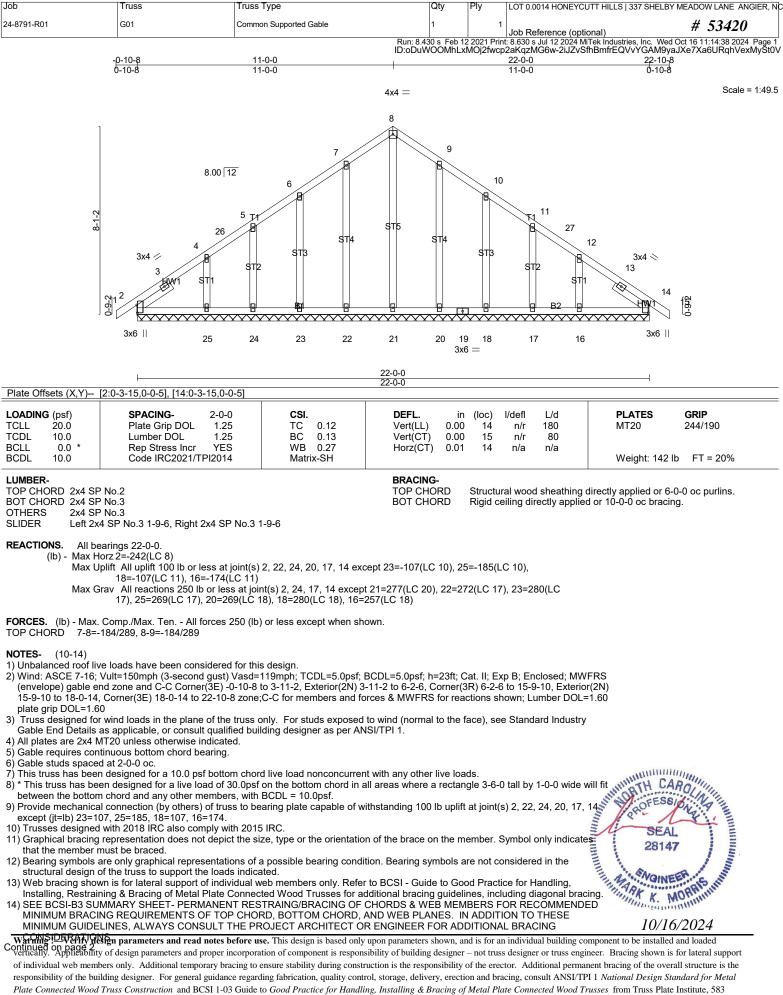
AST #: 53420 JOB: 24-8791-R01 JOB NAME: LOT 0.0014 HONEYCUTT HILLS Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2018 as well as IRC 2021. 40 Truss Design(s)

Trusses:

G01, G02, J01, J01A, J02, J04, J05, J06, J08, J09, J10, J11, J12, P02, R01, R02, R07, R08, R09, R10, R11, R12, R13, R14, R14A, R15, R16, R17, V01, V02, V03, V04, V05, V06, V07, V08,



#### Warning !--- Verify design parameters and read notes before use.

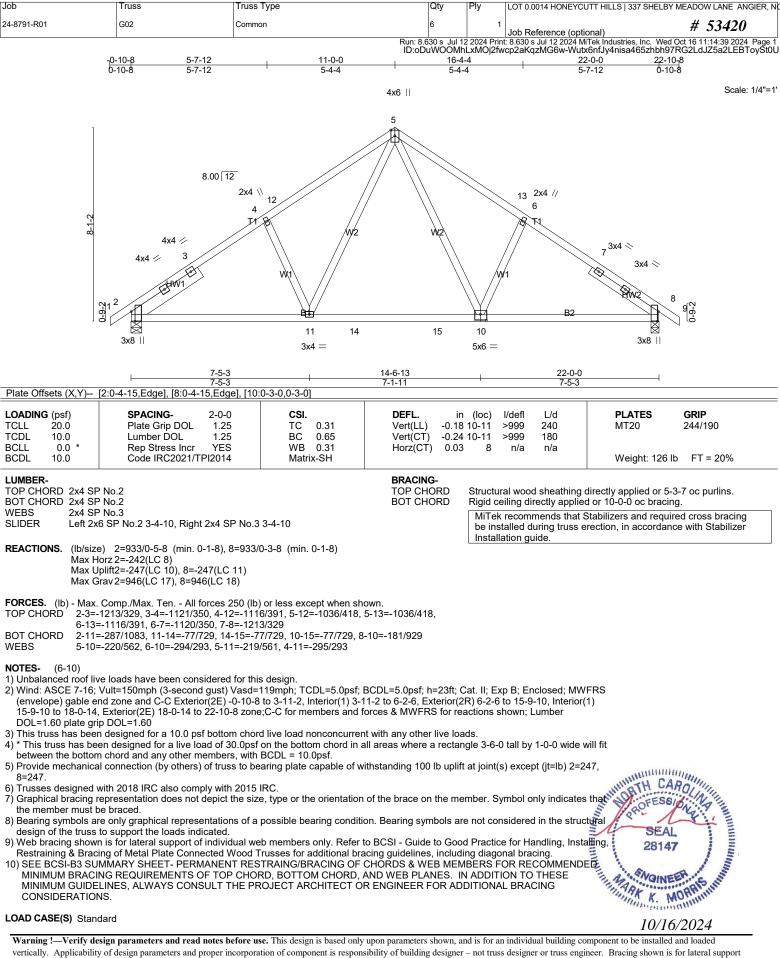


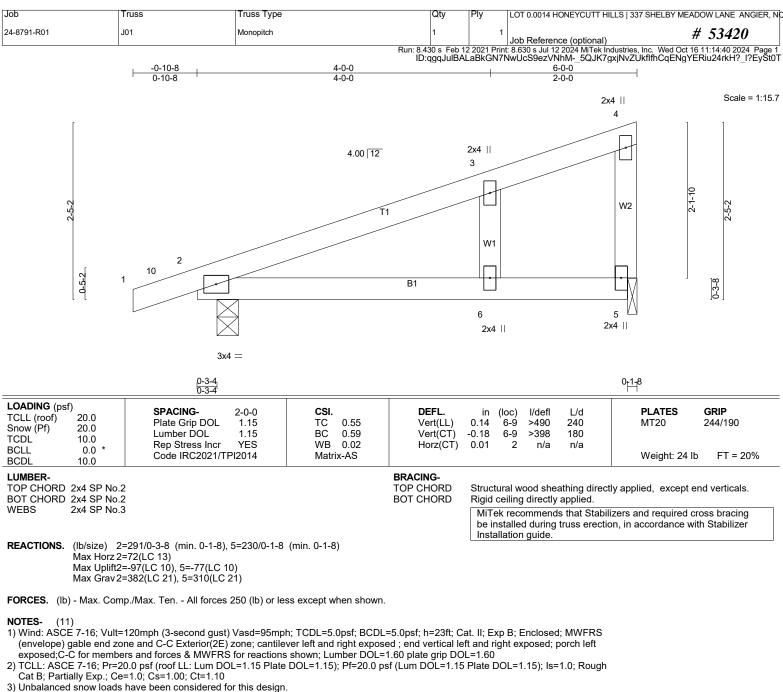
D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0014 HONEYCUTT HILLS   337 SHELB	Y MEADOW LANE ANGIER, NC
24-8791-R01	G01	Common Supported Gable	1	1	Job Reference (optional)	# 53420
					t: 8.630 s Jul 12 2024 MiTek Industries, Inc. We cp2aKqzMG6w-Wutx6nfJy4nisa465zhbh9	

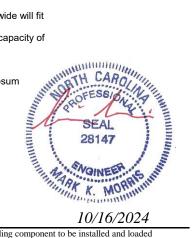
LOAD CASE(S) Standard



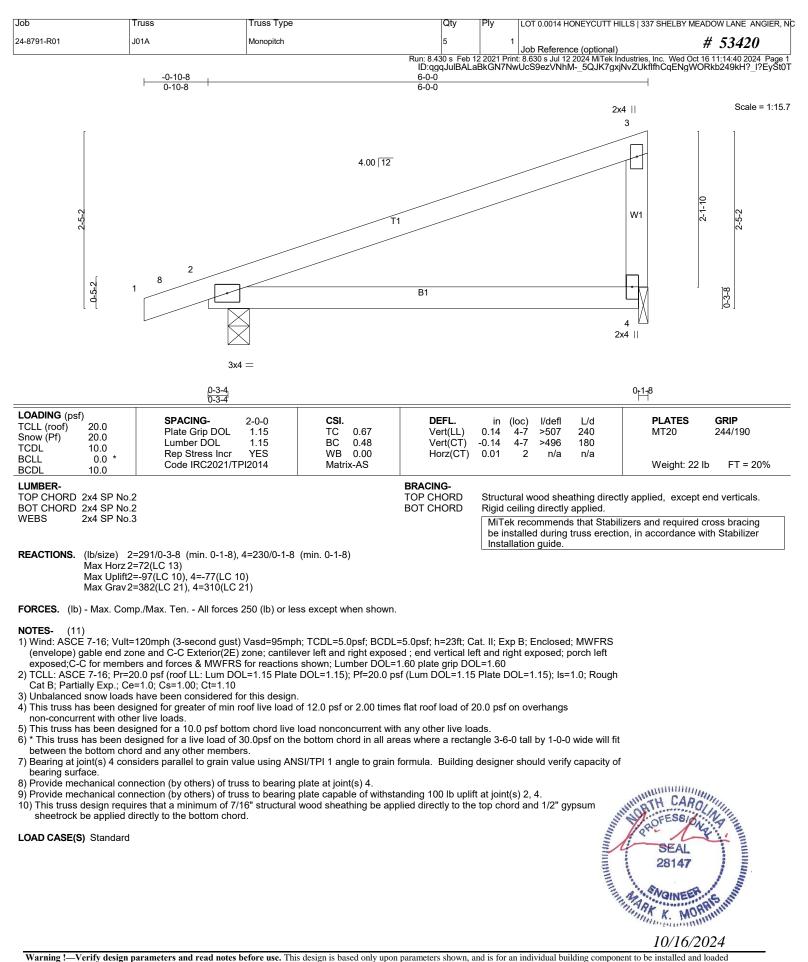




- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

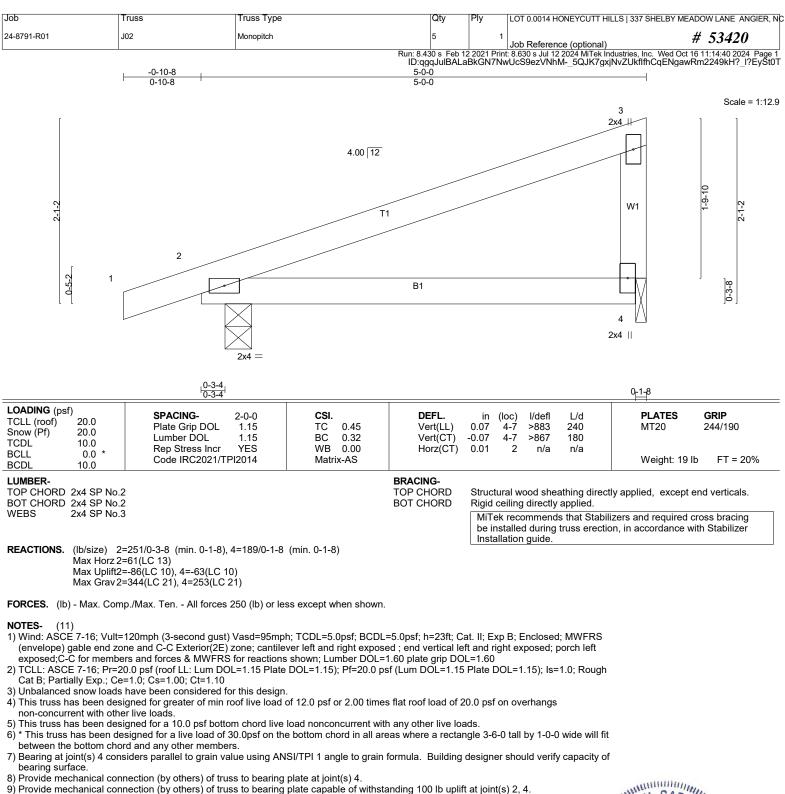


LOAD CASE(S) Standard



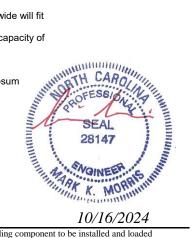
LOAD CASE(S) Standard

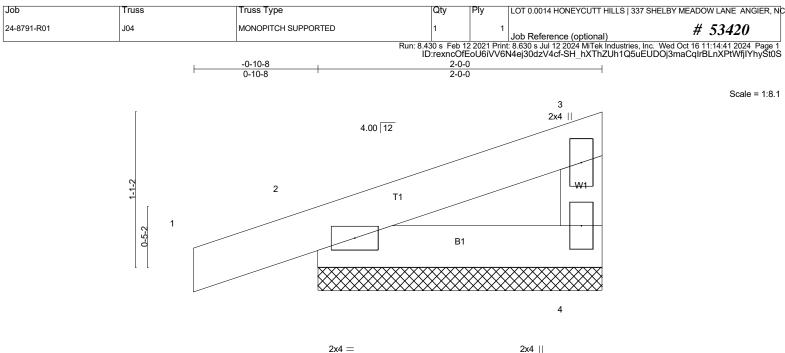
# 10/16/2024



10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard





Installation guide.

LOADING (psf)           TCLL (roof)         20.0           Snow (Pf)         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	<b>CSI.</b> TC 0.09 BC 0.06 WB 0.00 Matrix-P	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d 0.00 1 n/r 180 0.00 1 n/r 80 0.00 4 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 8 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire end verticals. Rigid ceiling directly applied or	ctly applied or 2-0-0 oc purlins, except 10-0-0 oc bracing.
					ilizers and required cross bracing tion, in accordance with Stabilizer

REACTIONS. (lb/size) 4=62/2-0-0 (min. 0-1-8), 2=139/2-0-0 (min. 0-1-8) Max Horz 2=27(LC 11) Max Uplift4=-9(LC 14), 2=-41(LC 10) Max Grav 4=76(LC 21), 2=180(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

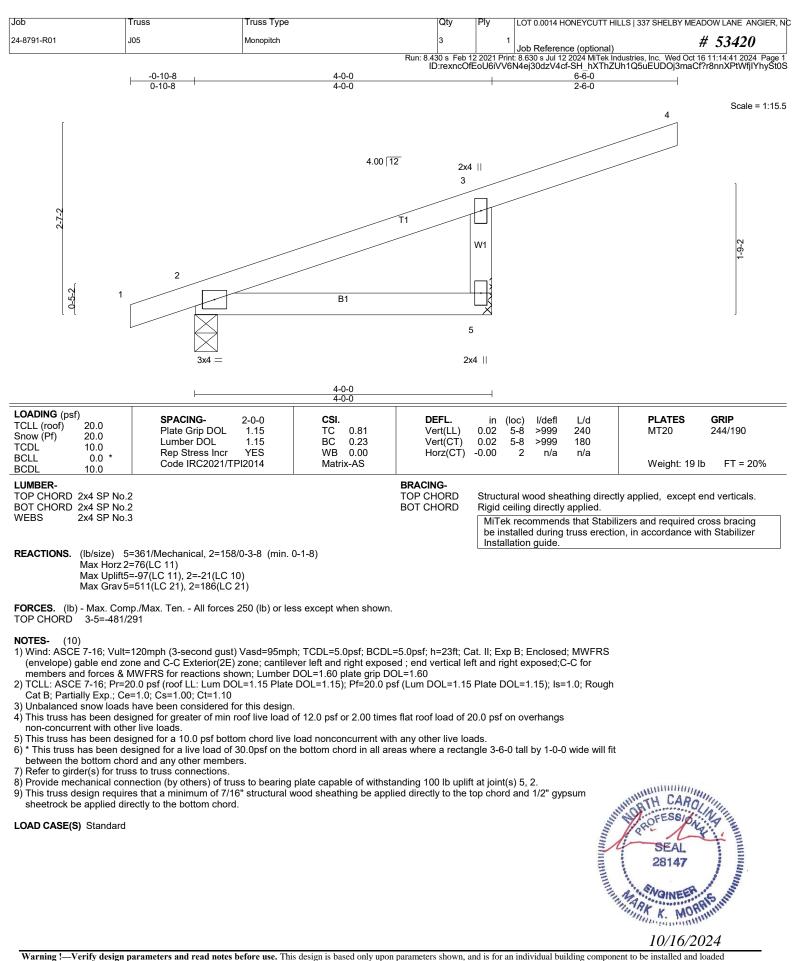
NOTES-

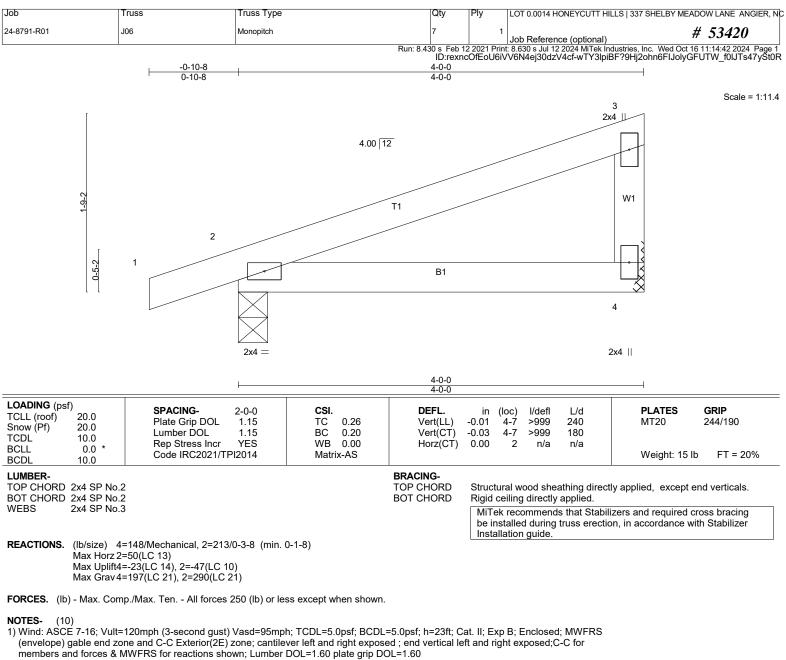
- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads. 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 9) between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

LOAD CASE(S) Standard



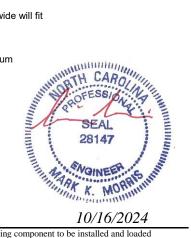


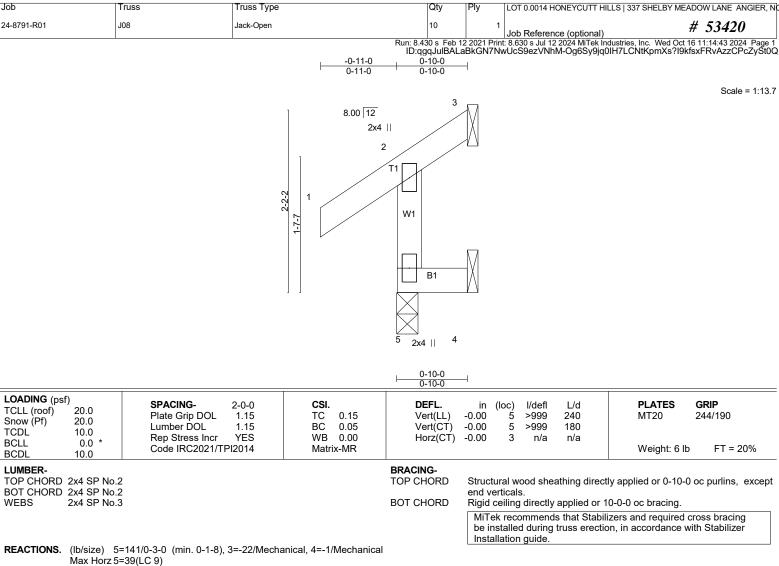


- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.

LOAD CASE(S) Standard

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.





Max Horz 5=39(LC 9) Max Uplift3=-63(LC 18), 4=-29(LC 9)

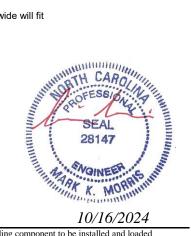
Max Grav 5=203(LC 18), 3=12(LC 10), 4=33(LC 10)

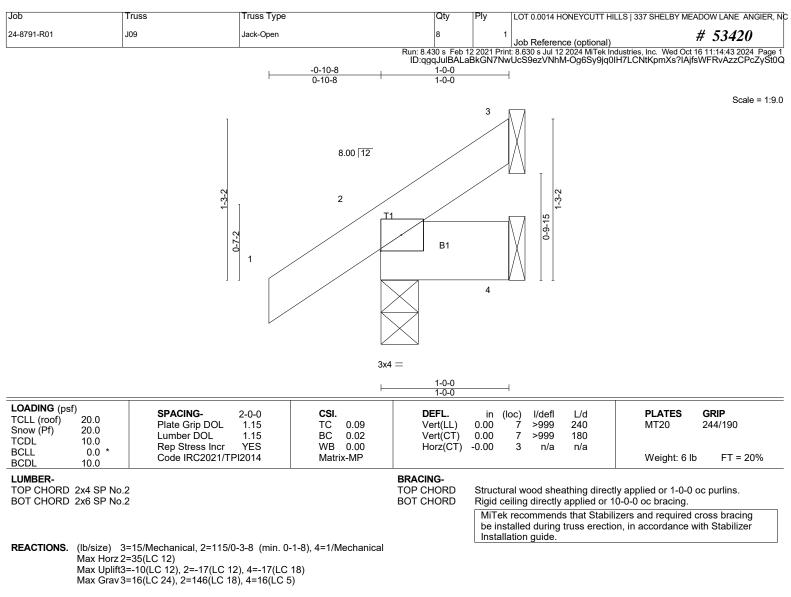
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.

LOAD CASE(S) Standard





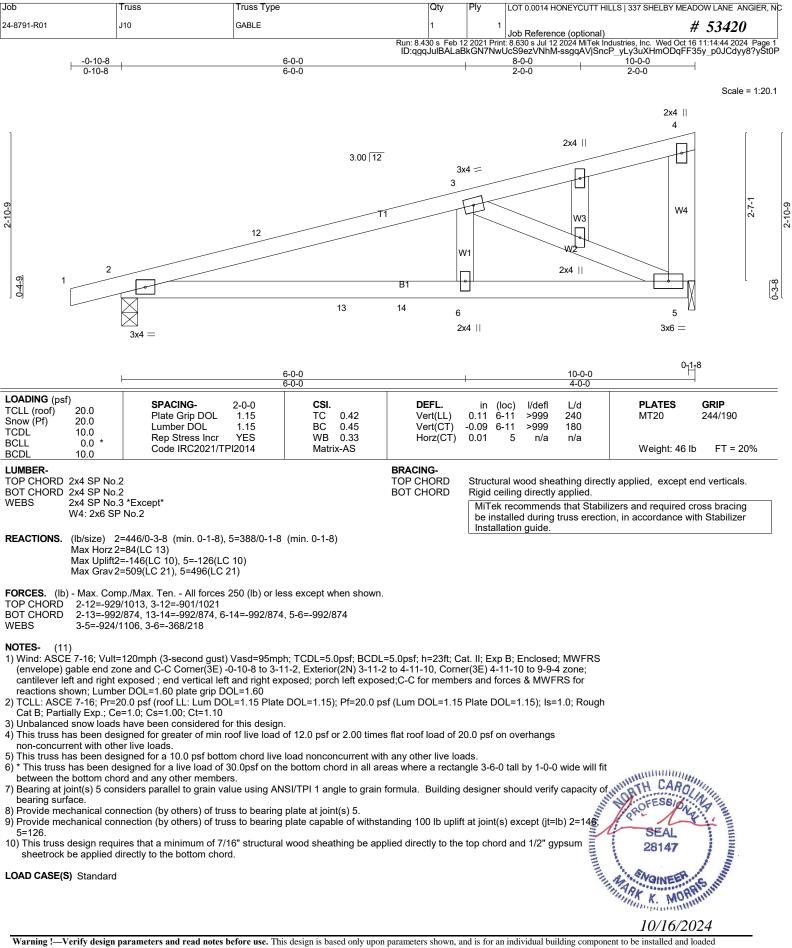
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

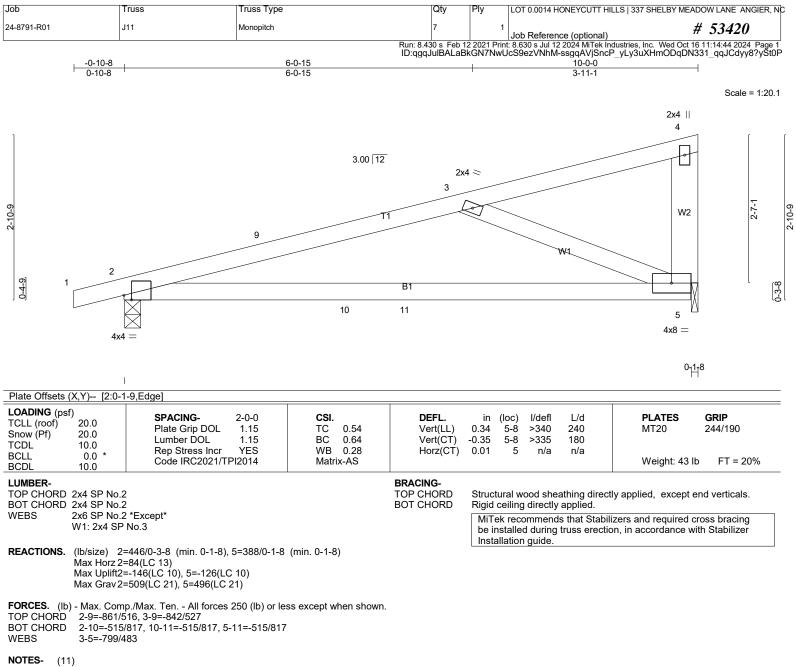
#### NOTES- (8)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.

LOAD CASE(S) Standard







- 1) Wind: ASCÉ 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 4-11-10, Exterior(2E) 4-11-10 to 9-9-4 zone; cantilever left and right exposed; end vertical left and right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- between the bottom chord and any other members.
   7) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of CESSION (S) Provide mechanical connection (by others) of truss to bearing plate at joint(c) 5.

- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=146 5=126
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

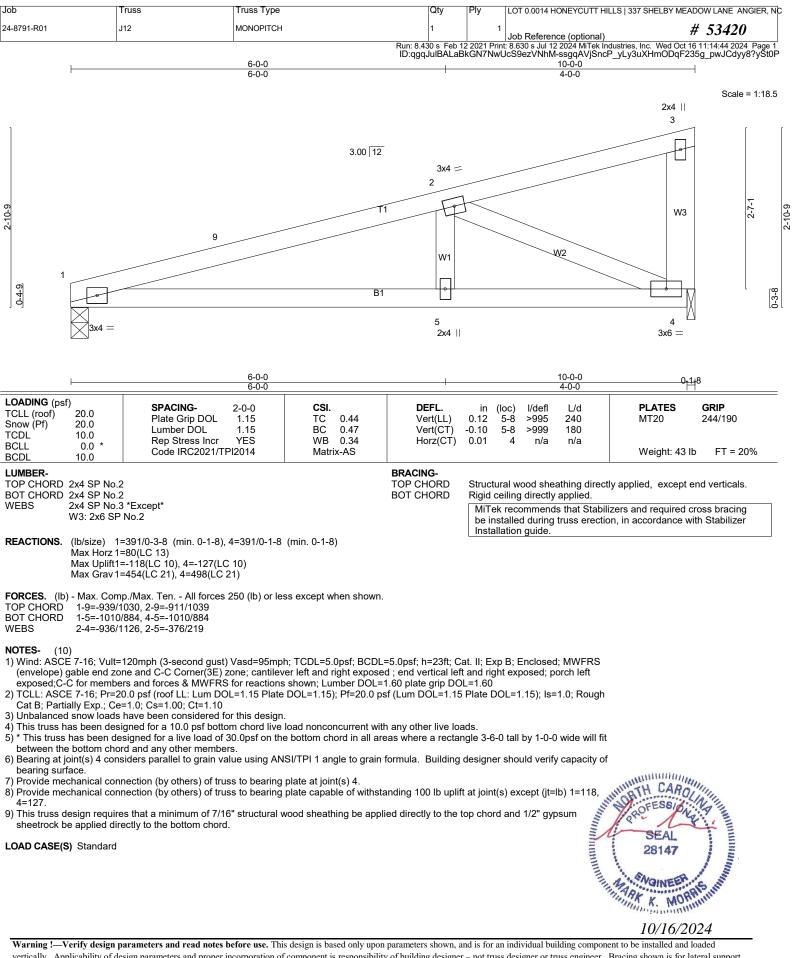
MORRESUM MORRESUM Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

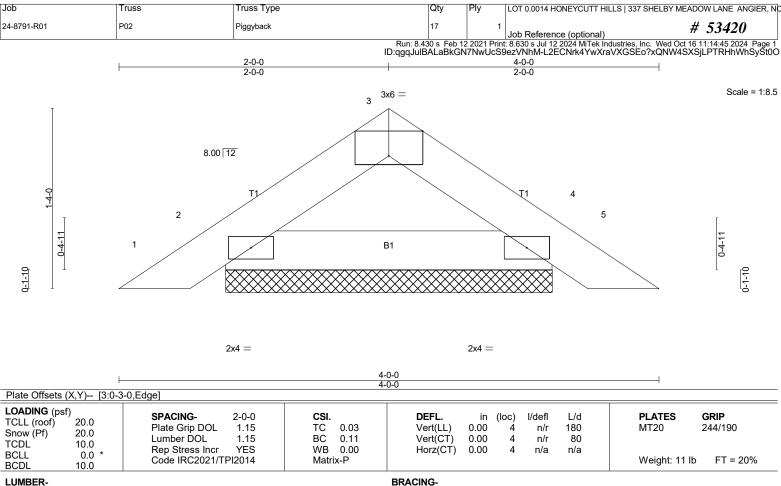
SEAL

28147

NOINEE K. MORRIS

10/16/2024





TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

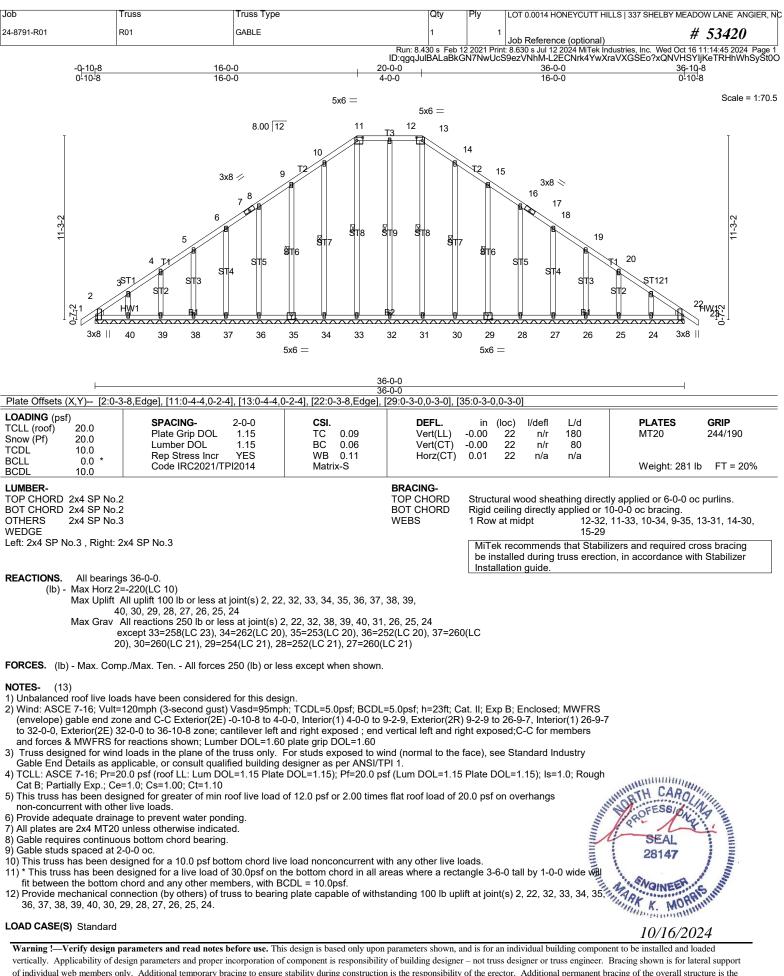
- REACTIONS. (lb/size) 2=128/2-5-0 (min. 0-1-8), 4=128/2-5-0 (min. 0-1-8) Max Horz 2=-23(LC 10) Max Uplift2=-16(LC 12), 4=-16(LC 13)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.

#### NOTES-(10)

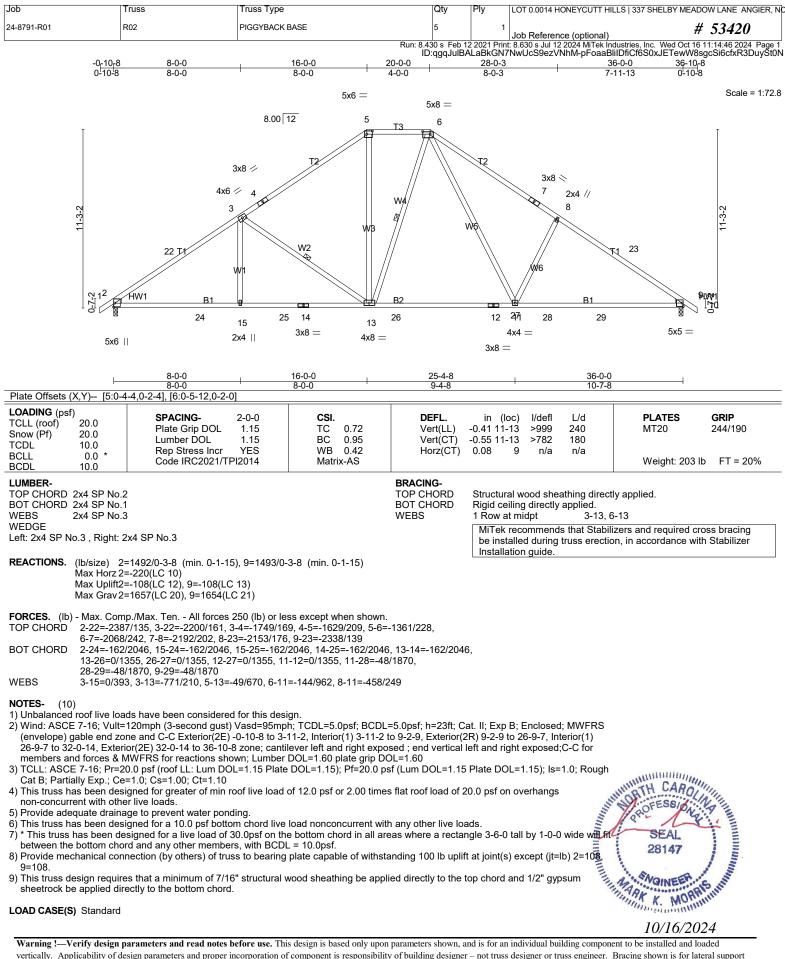
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

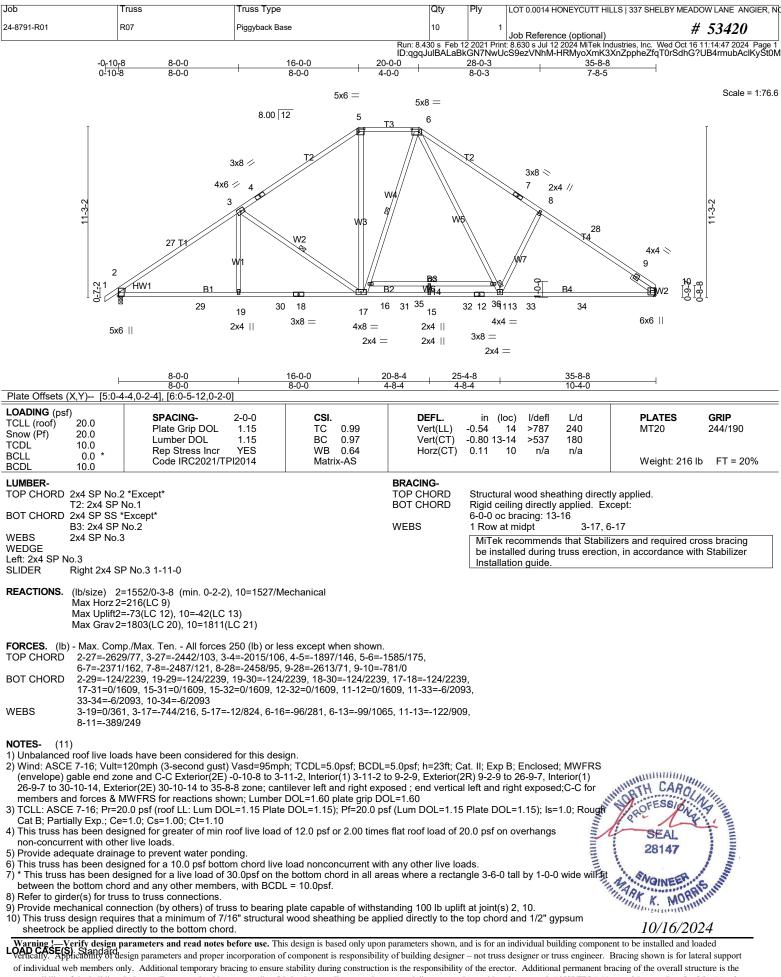
#### LOAD CASE(S) Standard



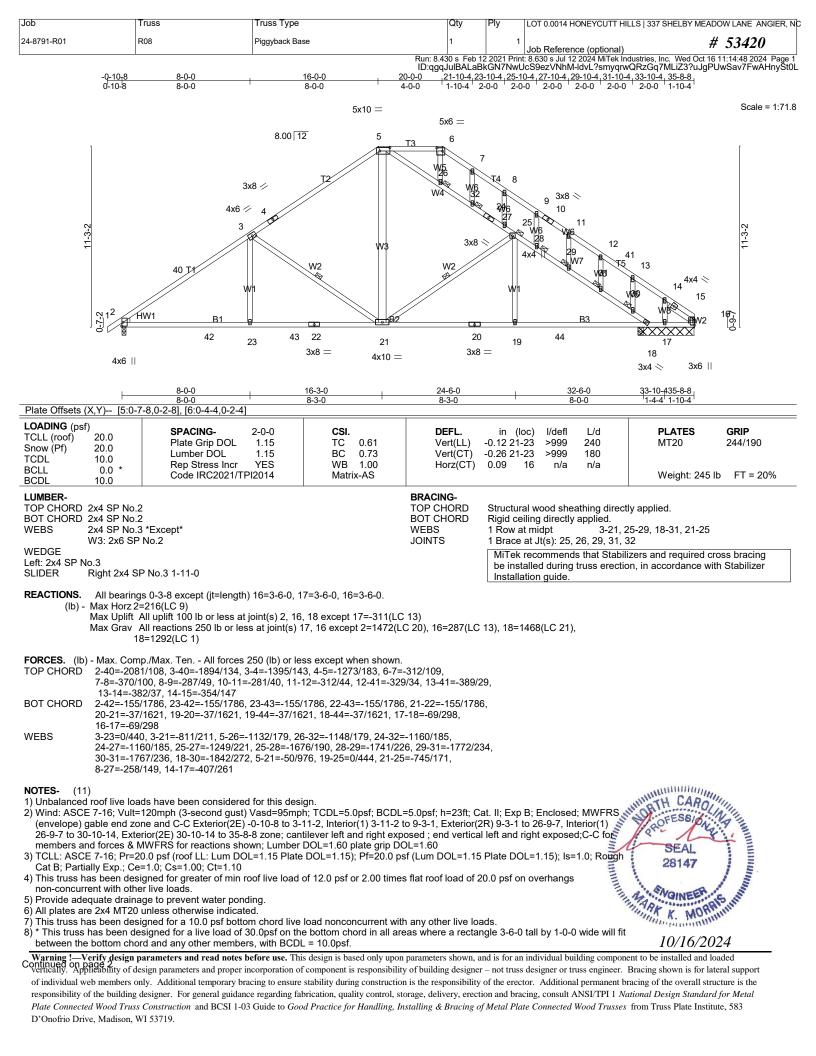


of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	LOT 0.0014 HONEYCUTT HILLS   337 SHELBY MEADOW LANE ANGIER, N
24-8791-R01	R08	Piggyback Base	1	1	Job Reference (optional) # 53420
					t: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Oct 16 11:14:48 2024 Page 2 JcS9ezVNhM-IdvL?smyqrwQRzGq7MLiZ3?uJgPUwSav7FwAHnySt0L

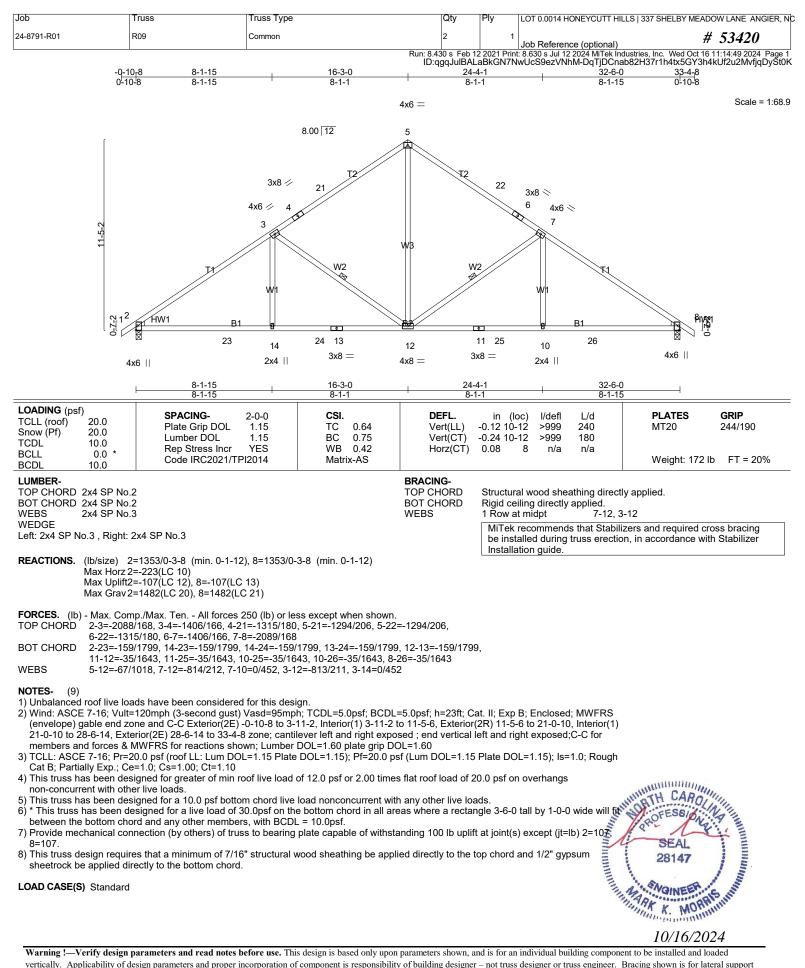
NOTES- (11)

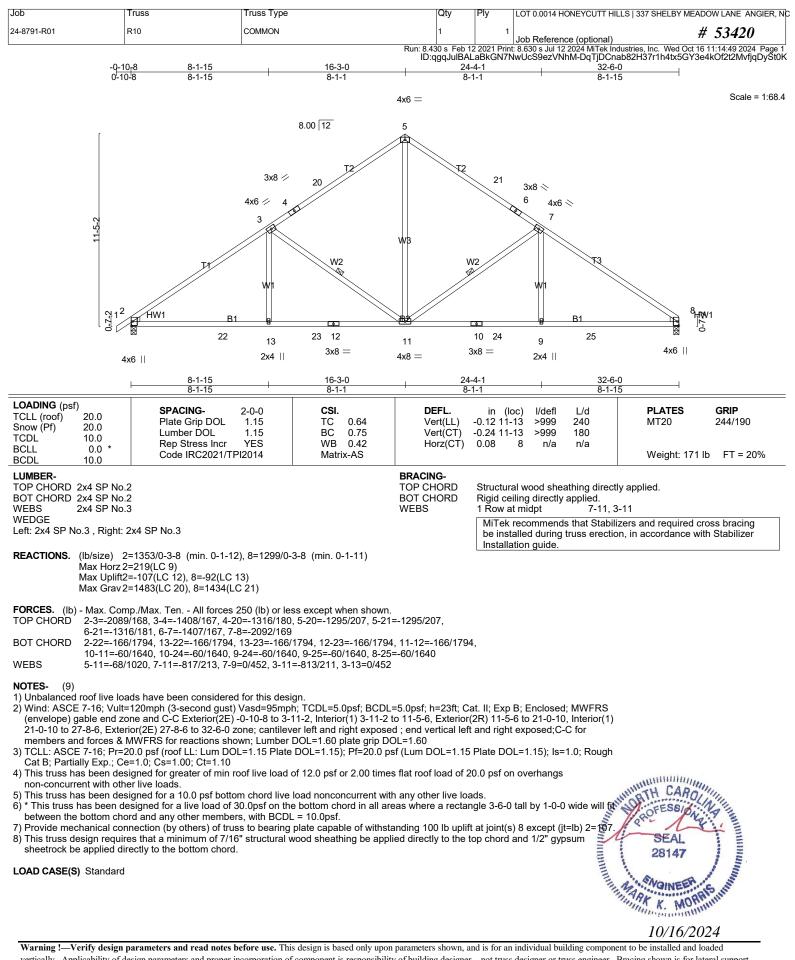
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 16, 18, 16 except (jt=lb) 17=311.

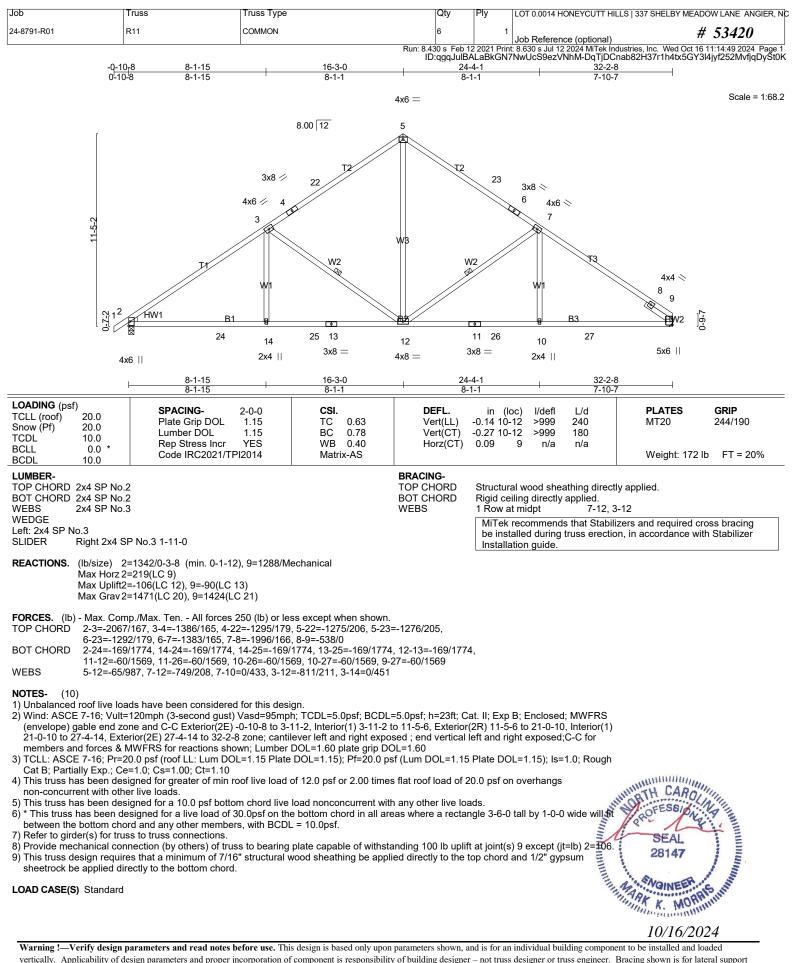
10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

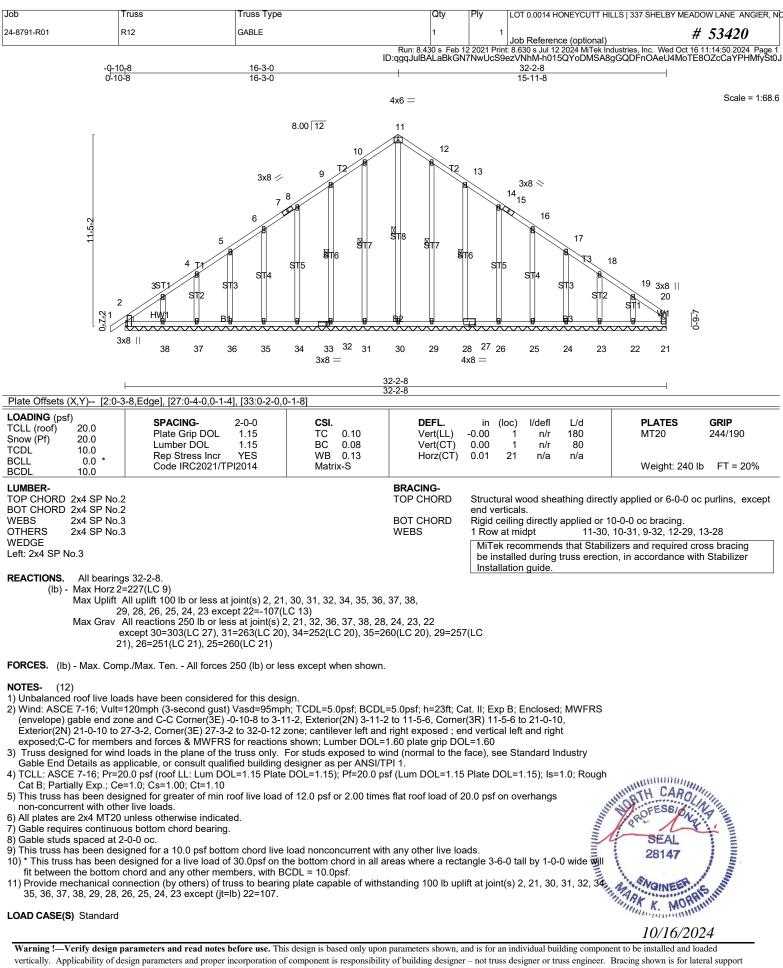
LOAD CASE(S) Standard



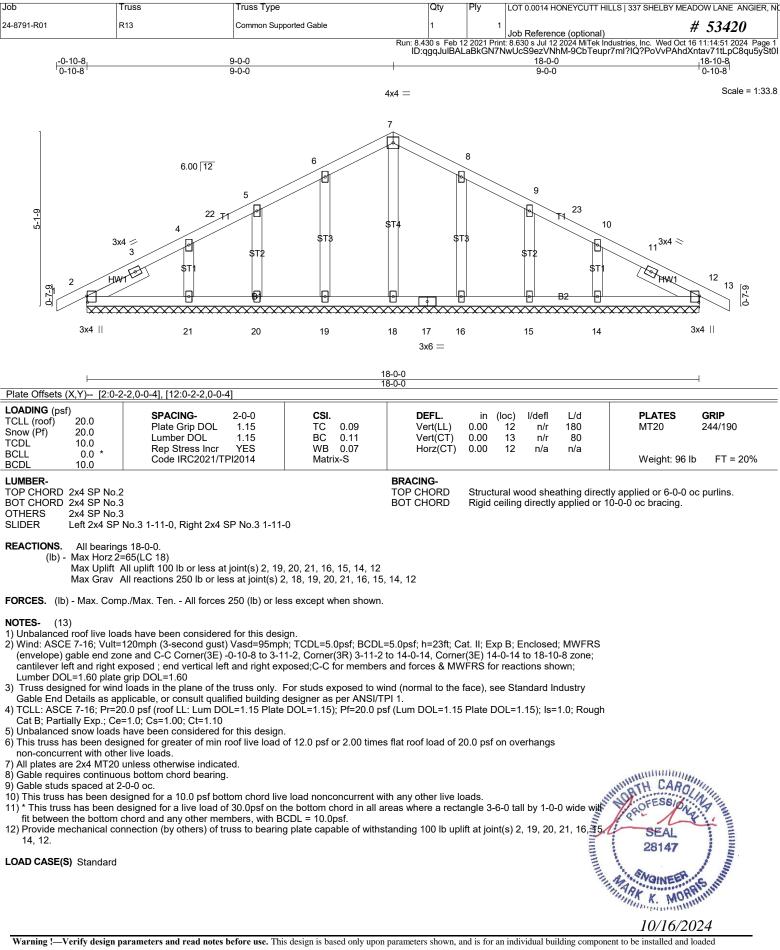


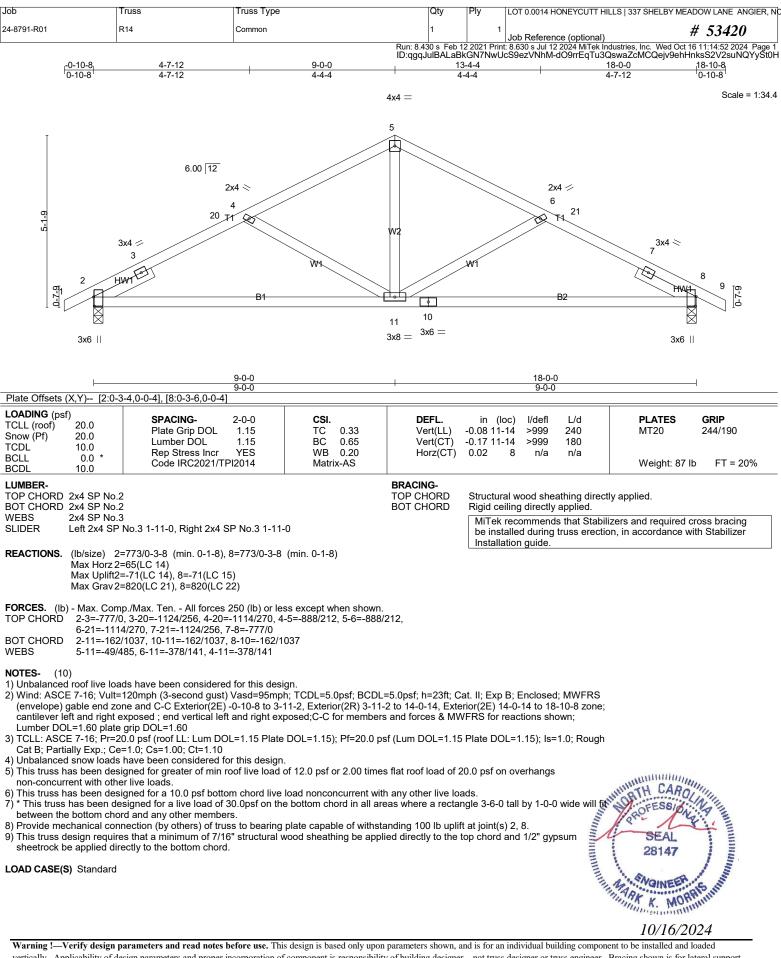


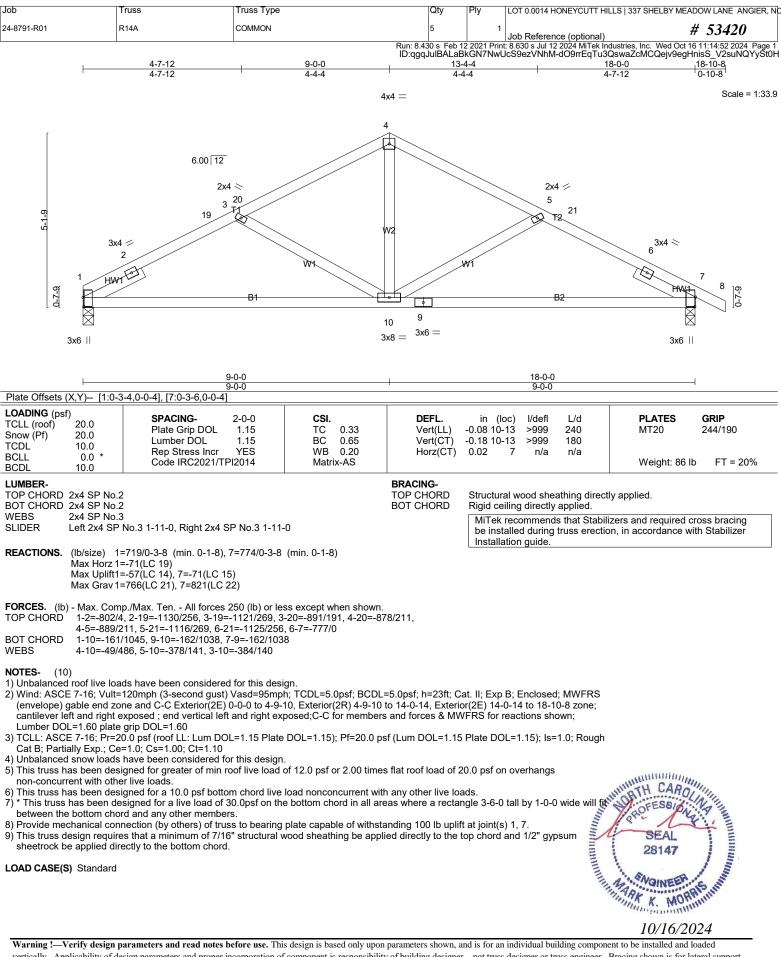




vertically. Applicability of design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be instanted and toaded vertically. Applicability of design parameters and read notes before use. This design is obased only upon parameters shown, and is for an individual building component to be instanted and toaded of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.







4-8791-R01	Truss R15		COMMON GIRDER	1	<b>3</b> Jo	h Reference (antional)		# 53420
				Run: 8.430 s Feb ID:aaa.luIR		bb Reference (optional) 630 s Jul 12 2024 MiTek I UcS9ezVNhM-6bjE2ac		
			0-2-8 3-3-0 5-3-0 0-2-8 3-0-8 2-0-0	7-3-0 9-3-0 11-3-0 13-3 2-0-0 2-0-0 2-0-0 2-0-0 2-0-	-0 15-3-0 í í	18-3-8 18 <sub>7</sub> 6-0 3-0-8 0-2-8	1011 1 J 1100 1117 100 1	
				4x6				Scale = 1:70.9
		11-2-10 1-11-10 09	$12.00 \overline{12}$ $12.00 \overline{12}$ $10 = \begin{array}{c} 5 \\ 7 \\ 1 \\ 39 \\ 7 \\ 25 \\ 26 \\ 24 \\ 56 \\ 26 \\ 24 \\ 56 \\ 24 \\ 56 \\ 26 \\ 24 \\ 56 \\ 24 \\ 56 \\ 26 \\ 24 \\ 56 \\ 24 \\ 56 \\ 26 \\ 24 \\ 56 \\ 24 \\ 56 \\ 26 \\ 24 \\ 56 \\ 24 \\ 24 \\ 24 \\ 24 \\ 24 \\ 24 \\ 24 \\ 2$	<sup>27</sup> 28 29 23 22 30	14 14 14 14 14 14 14 15 14 14 15 15 16 18 18 18 18 18 18 18 18 18 18	j←] 32 20	2-2-2	
Plate Offsets (	X,Y) [1:0-6-8,0-1-7	], [19:0-6-8,0-1-7	5x6 = 8x8 = <u>4-8-6</u> <u>7], [21:0-3-8,0-4-8], [23:0-4-8]</u>	7x8 = <u>9-3-0 13-9-10</u> <u>4-6-10 4-6-10</u>	8x8 = + 18- 4-8	5x6 =		
LOADING (psf TCLL (roof) Snow (Pf) TCDL BCLL	) 20.0 20.0 10.0 0.0 * R	PACING- late Grip DOL umber DOL ep Stress Incr ode IRC2021/TF	2-0-0 <b>CSI.</b> 1.15 TC ( 1.15 BC ( NO WB (	D.76 <b>DEFL.</b> D.48 Vert(LL) D.96 Horz(CT)	in (loc) -0.06 21-23 -0.12 21-23 0.02 20	>999 240 >999 180	PLATES MT20 Weight: 649	<b>GRIP</b> 244/190
BCDL	10.0			BRACING-			Weight. 043	10 11 - 20%
TOP CHORD BOT CHORD	2x4 SP No.2 *Excep T2,T3: 2x6 SP No.2 2x6 SP DSS 2x4 SP No.3	t*		TOP CHORD BOT CHORD JOINTS	end vertica Rigid ceilin	wood sheathing direc ls. g directly applied or Jt(s): 11, 16, 4, 13, 8	10-0-0 oc bracing.	oc purlins, except
REACTIONS.	(lb/size) 25=8257/0 Max Horz 25=226(L0 Max Uplift25=-460(L Max Grav 25=9484(1	C 7) .C 11), 20=-414(		1-12)				
FORCES. (Ib) TOP CHORD BOT CHORD	1-2=-5388/264, 2-5 12-14=-5186/366, 1-3=-4157/495, 3-4 9-11=-1659/120, 1 18-19=-4147/503 25-26=-260/1352, 2	5=-5178/314, 5-7 14-17=-5178/316 =-4157/495, 4-6 1-13=-1659/120, 26-27=-260/1352	=-1659/120, 6-8=-1659/12 13-15=-1659/120, 15-16= 2, 24-27=-260/1352, 24-28	98, 10-12=-5026/400, -6814/388, 19-20=-6825/38 0, 8-9=-1659/120, -1659/120, 16-18=-4147/50 =-616/7748, 28-29=-616/77	03, 748,			
WEBS	31-32=-92/1324, 20 11-23=-393/6381,	0-32=-92/1324 16-23=-2659/409		=-547/7776, 21-31=-92/132 -2686/410, 4-24=-160/149 197/6758, 2-3=-145/253	,			
Top chords Bottom chor Webs conne	b be connected toget connected as follows ds connected as follo ected as follows: 2x4	: 2x4 - 1 row at 0 ows: 2x6 - 2 row - 1 row at 0-9-0		-		S) spotion Divite -1		in,
<ol> <li>All loads are connections</li> <li>Unbalanced</li> <li>Wind: ASCE (envelope) g</li> <li>TCLL: ASCE Cat B; Partia</li> <li>Provide ade</li> <li>All plates are</li> <li>This truss has between the</li> </ol>	e considered equally have been provided roof live loads have 7-16; Vult=120mph jable end zone; canti 57-16; Pr=20.0 psf (i ally Exp.; Ce=1.0; Cs quate drainage to pro- e 2x4 MT20 unless or as been designed for has been designed for bottom chord and ar	applied to all plie to distribute onl been considered (3-second gust) lever left and rig roof LL: Lum DC =1.00; Ct=1.10 event water pond therwise indicate a 10.0 psf botto or a live load of 3 ny other membe	es, except if noted as front y loads noted as (F) or (B), d for this design. Vasd=95mph; TCDL=5.0p ht exposed ; end vertical le L=1.15 Plate DOL=1.15); F ding. ed. m chord live load nonconc 30.0psf on the bottom chor rs.	(F) or back (B) face in the l unless otherwise indicated osf; BCDL=5.0psf; h=23ft; C eft and right exposed; Lumb Pf=20.0 psf (Lum DOL=1.1 current with any other live lo d in all areas where a recta 1 angle to grain formula. B d only upon parameters shown, esponsibility of building design construction is the responsibility o control, storage, delivery, erect <i>Handling, Installing &amp; Bracing</i>	LOAD CASE( d. Cat. II; Exp B; per DOL=1.60 5 Plate DOL= bads. angle 3-6-0 tal	S) section. Ply to ply Enclosed; MWFRS plate grip DOL=56 1.15); Is=1.0; Rough Il by 1-0-0 wide will fi	SEAL 28147	A BUILDING
10) D	joint(s) 25, 20 consid	lers parallel to g	rain value using ANSI/TPI	1 angle to grain formula. B	uilding desigi	ner should verify	10/16/2	024
capacity of	bearing surface.						10/10/20	<i>JZ</i> <b>4</b>

D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0014 HONEYCUTT HILLS   337 SHELBY	MEADOW LANE ANGIER, NC
24-8791-R01	R15	COMMON GIRDER	1	3	Job Reference (optional)	# 53420
					t: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed S9ezVNhM-anHcGwrjQhga9uj_UdS6oKFu	

#### NOTES- (14)

- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 25=460, 20=414.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 27 lb down and 60 lb up at 0-4-11, and 27 lb down and 60 lb up at 18-1-5 on top chord, and 1779 lb down and 54 lb up at 0-1-12, 29 lb down and 31 lb up at 2-3-4, 1771 lb down and 62 lb up at 2-3-12, 29 lb down and 31 lb up at 4-3-4, 1771 lb down and 62 lb up at 4-3-12, 29 lb down and 31 lb up at 6-3-4, 1771 lb down and 62 lb up at 4-3-12, 29 lb down and 31 lb up at 6-3-4, 1771 lb down and 62 lb up at 4-3-4, 1771 lb down and 62 lb up at 4-3-12, 29 lb down and 31 lb up at 6-3-12, 29 lb down and 31 lb up at 10-3-4, 1771 lb down and 62 lb up at 10-3-12, 29 lb down and 31 lb up at 10-3-12, 29 lb down and 31 lb up at 10-3-12, 29 lb down and 31 lb up at 12-3-4, 1771 lb down and 62 lb up at 14-3-4, 1771 lb down and 62 lb up at 10-3-12, 29 lb down and 31 lb up at 12-3-12, 29 lb down and 31 lb up at 14-3-4, 1771 lb down and 62 lb up at 14-3-12, and 29 lb down and 31 lb up at 16-3-4, and 1771 lb down and 62 lb up at 16-3-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

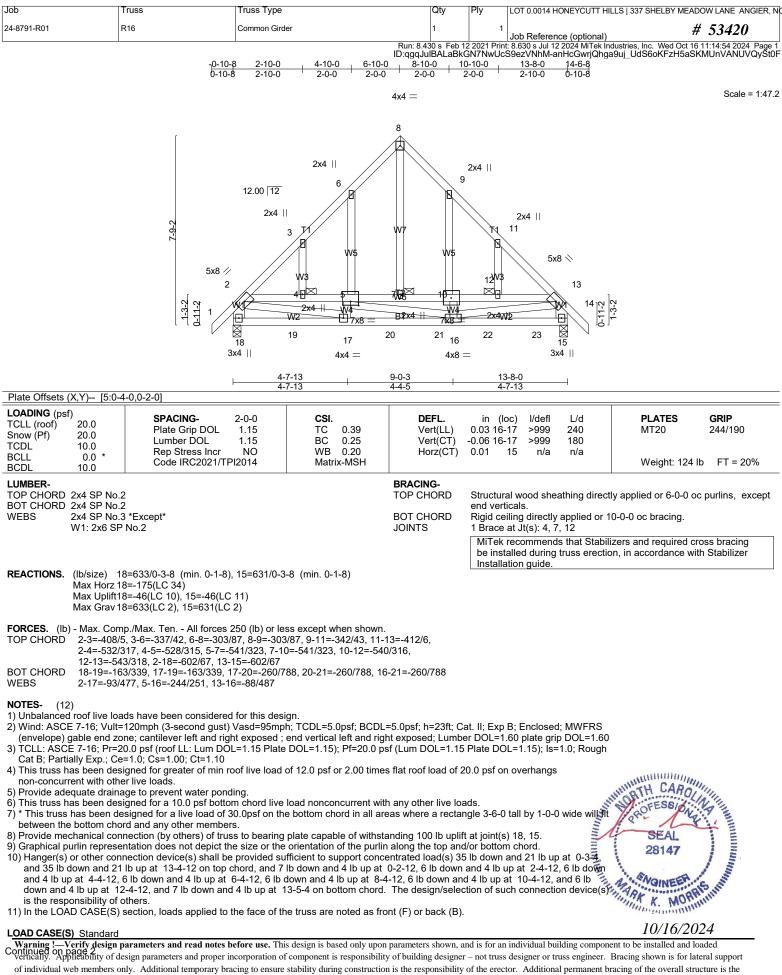
Uniform Loads (plf)

Vert: 1-10=-60, 10-19=-60, 20-25=-20, 1-19=-6(F)

Concentrated Loads (lb)

Vert: 22=-1503(F=4, B=-1507) 1=20(F) 19=20(F) 25=-1515(B) 26=-1503(F=4, B=-1507) 27=-1503(F=4, B=-1507) 28=-1503(F=4, B=-1507) 29=-1503(F=4, B=-1507) 30=-1503(F=4, B=-1507) 31=-1503(F=4, B=-1507) 32=-1503(F=4, B=-1507) 32=-1503(F





responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0014 HONEYCUTT HILLS   337 SHE	LBY MEADOW LANE ANGIER, NC
24-8791-R01	R16	Common Girder	1	1	Job Reference (optional)	# 53420
		Pup: 1	2 130 c Eob 1	2 2021 Drin	t: 8 630 c. Jul 12 2024 MiTek Industries Inc. V	Nod Oct 16 11:14:54 2024 Page 2

Run: 8.430 s Feb 12 2021 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Oct 16 11:14:54 2024 Page 2 ID:qqqJuIBALaBkGN7NwUcS9ezVNhM-anHcGwrjQhga9uj\_UdS6oKFzH5aSKMUnVANUVQySt0F

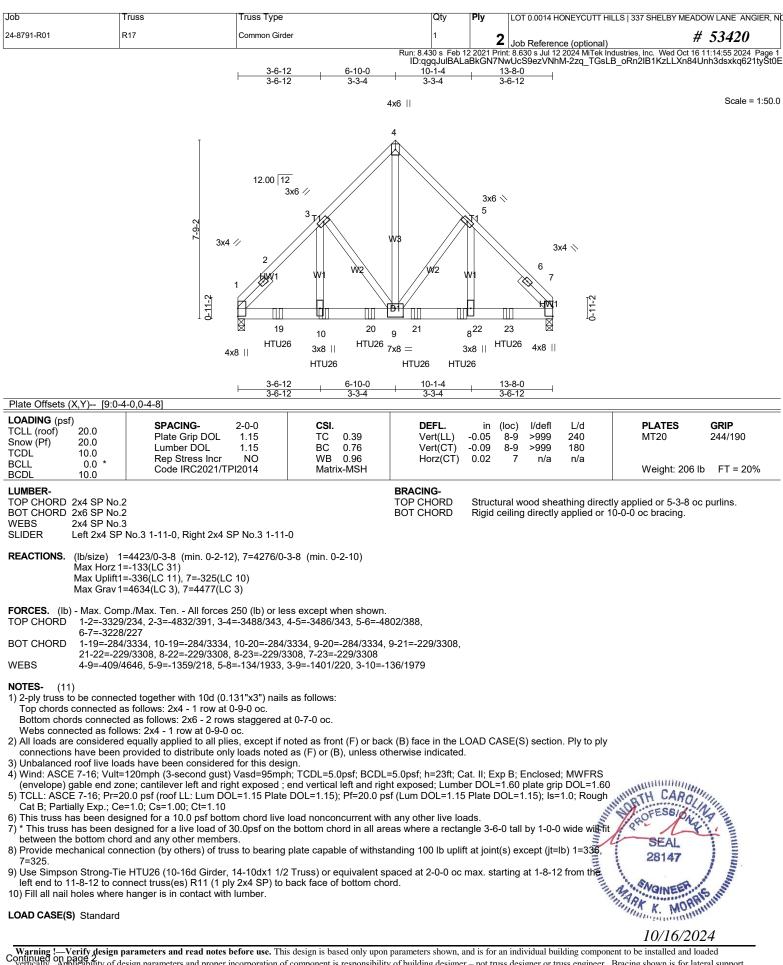
#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-8=-60, 8-13=-60, 13-14=-60, 15-18=-20, 2-13=-8(F) Concentrated Loads (lb)

Vert: 18=4(F) 15=4(F) 17=4(F) 19=4(F) 20=4(F) 21=4(F) 22=4(F) 23=4(F)





Job	Truss	Truss Type	Qty	Ply	LOT 0.0014 HONEYCUTT HILLS   337	SHELBY MEADOW LANE ANGIER, NC
24-8791-R01	R17	Common Girder	1	2	Job Reference (optional)	# 53420
		·	Run: 8 430 s Feb 12	2 2021 Prin	t 8 630 s Jul 12 2024 MiTek Industries Ir	nc. Wed Oct 16 11:14:55 2024 Page 2

un: 8.430 s Feb 12 2021 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Oct 16 11:14:55 2024 Page 2 ID:qgqJuIBALaBkGN7NwUcS9ezVNhM-2zq\_TGsLB\_oRn2IB1KzLLXn84Unh3dsxkq621tySt0E

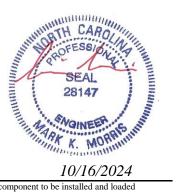
#### LOAD CASE(S) Standard

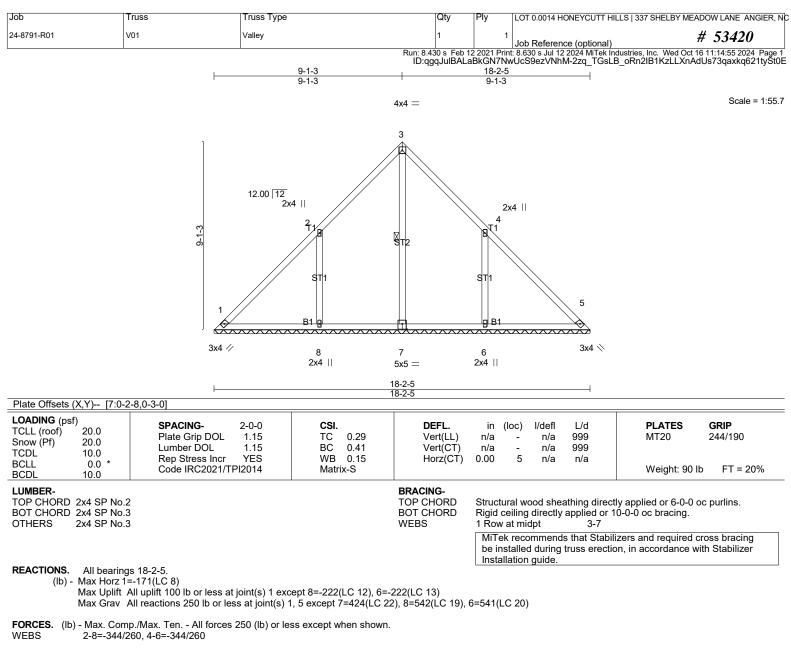
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 11-15=-20

Concentrated Loads (lb)

Vert: 10=-1268(B) 19=-1268(B) 20=-1268(B) 21=-1268(B) 22=-1268(B) 23=-1268(B)





NOTES- (8)

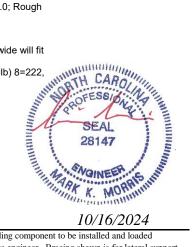
1) Unbalanced roof live loads have been considered for this design.

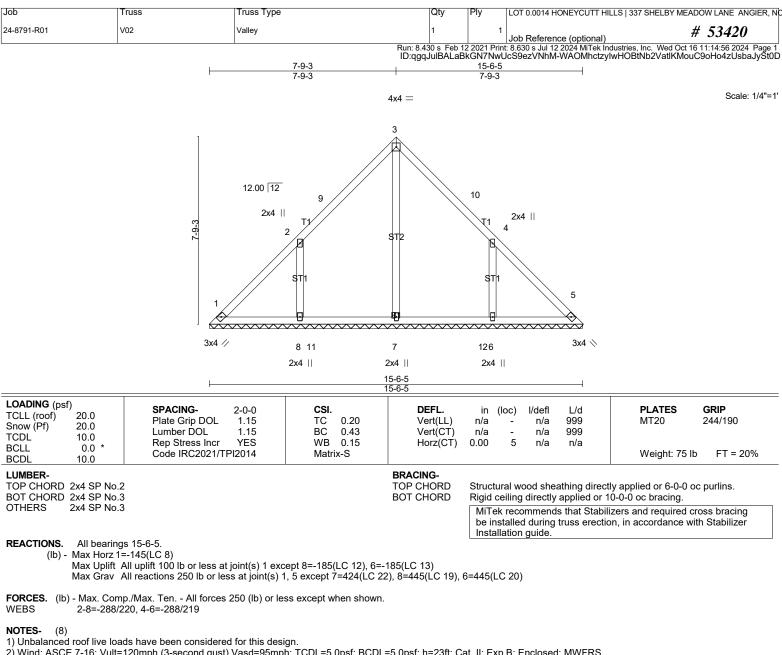
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-3, Exterior(2R) 5-1-3 to 13-1-3, Exterior(2E) 13-1-3 to 17-10-2 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=222, 6=222.

### LOAD CASE(S) Standard





- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 10-4-8, Exterior(2E) 10-4-8 to 15-2-2 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

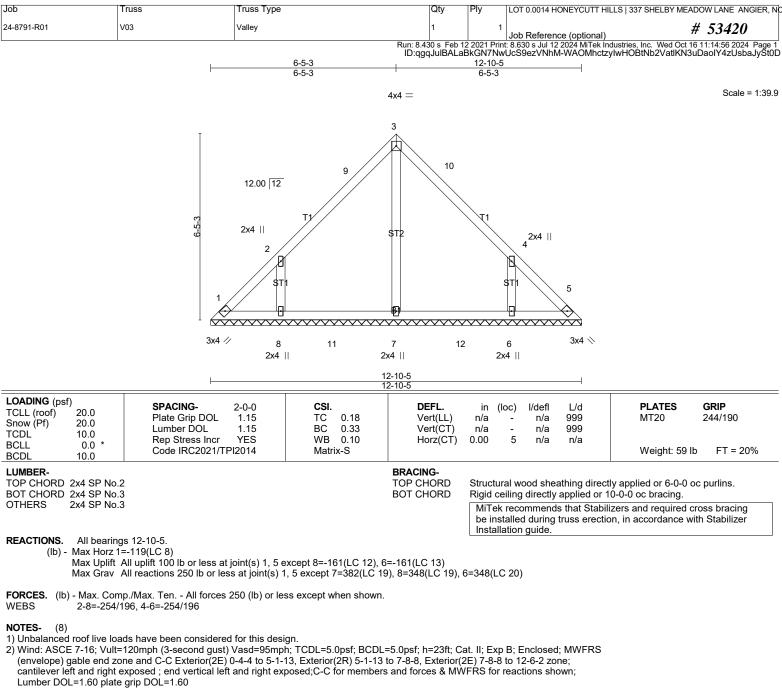
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=185, 6=185.

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LOAD CASE(S) Standard
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3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

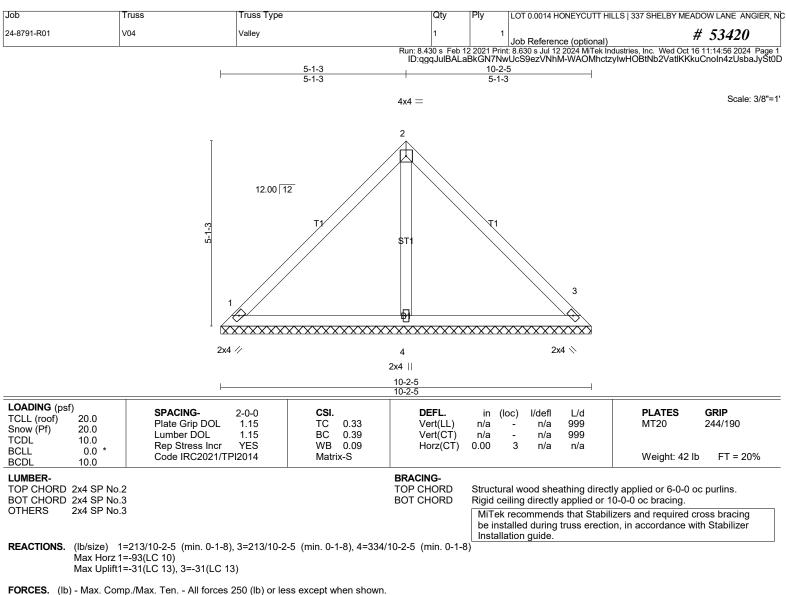
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=161, 6=161.

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LOAD CASE(S) Standard
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NOTES-(8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed: MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

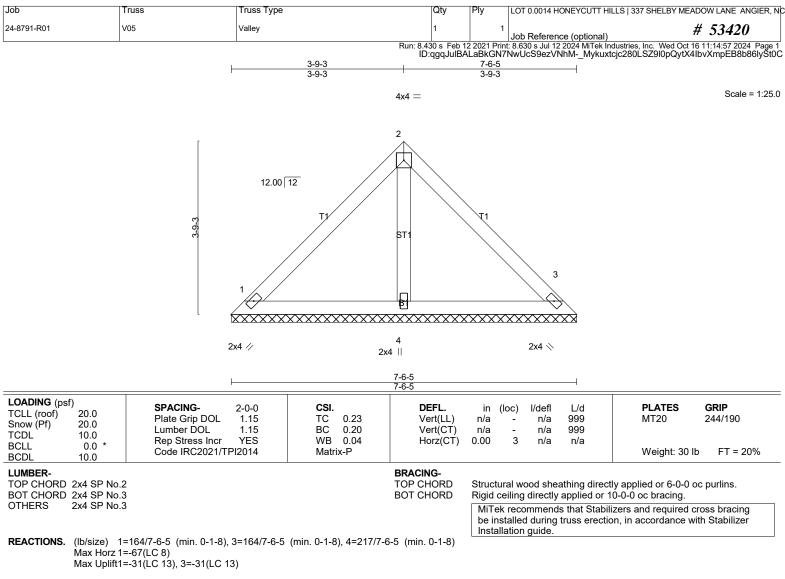
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed: MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

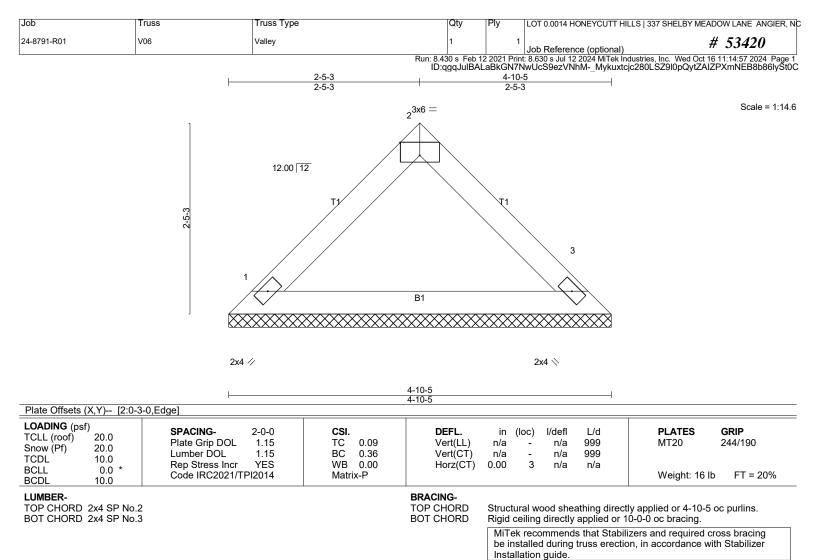
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 1=166/4-10-5 (min. 0-1-8), 3=166/4-10-5 (min. 0-1-8) Max Horz 1=-41(LC 10) Max Uplift1=-8(LC 12), 3=-8(LC 12)

#### **NOTES-** (8)

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

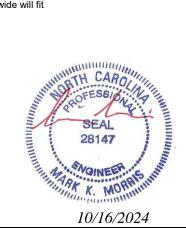
4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

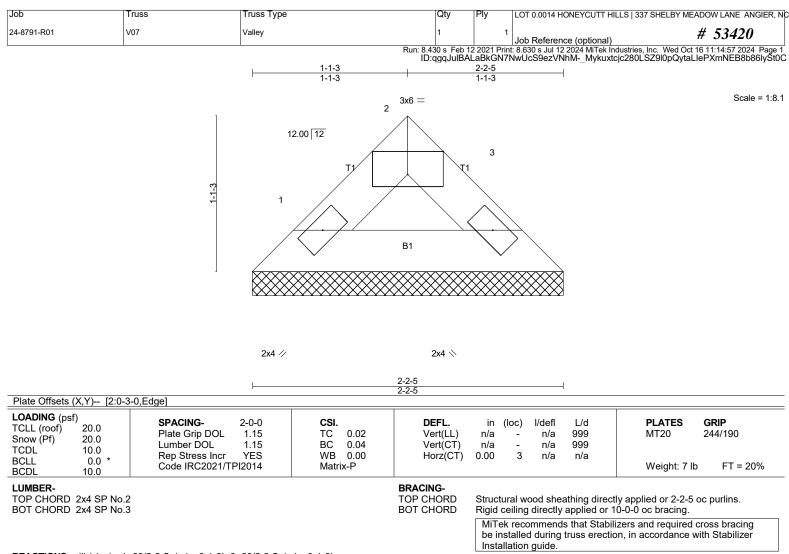
LOAD CASE(S) Standard



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

<sup>1)</sup> Unbalanced roof live loads have been considered for this design.

<sup>2)</sup> Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



REACTIONS. (Ib/size) 1=60/2-2-5 (min. 0-1-8), 3=60/2-2-5 (min. 0-1-8) Max Horz 1=15(LC 11) Max Uplift1=-3(LC 12), 3=-3(LC 13)

#### NOTES- (8)

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

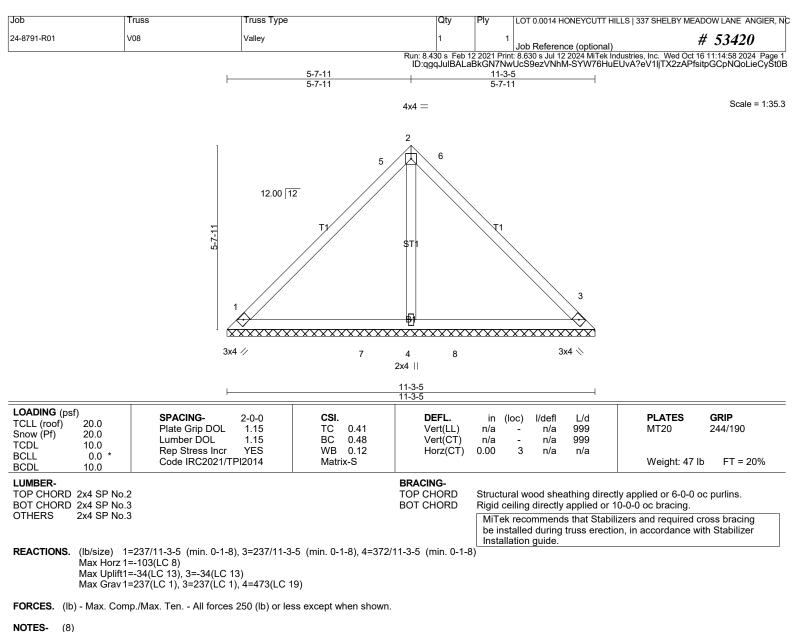
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 6-1-8, Exterior(2E) 6-1-8 to 10-11-2 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Gable requires continuous bottom chord bearing.

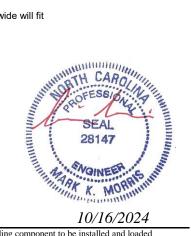
This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

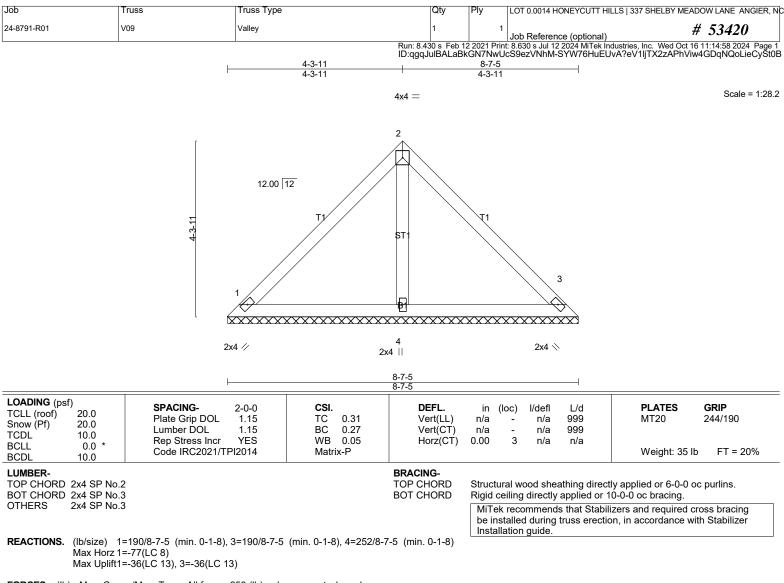
6)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

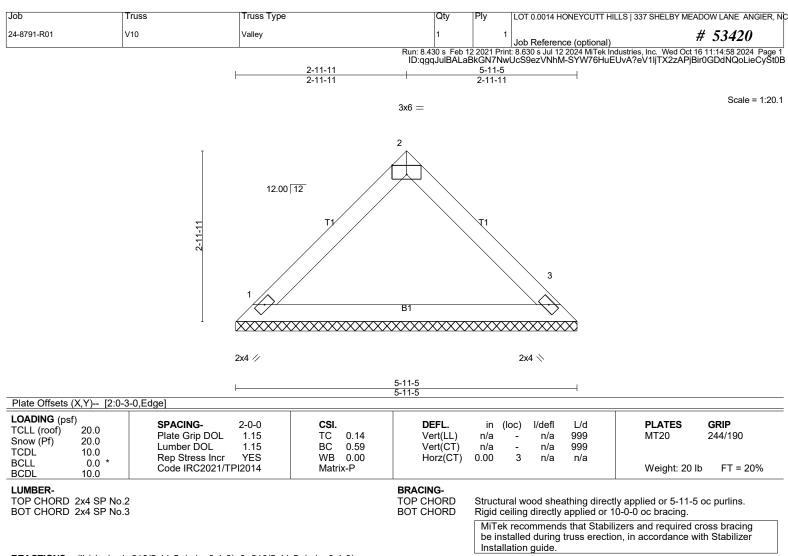
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 1=210/5-11-5 (min. 0-1-8), 3=210/5-11-5 (min. 0-1-8) Max Horz 1=-51(LC 8) Max Uplift1=-10(LC 12), 3=-10(LC 12)

#### NOTES- (8)

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

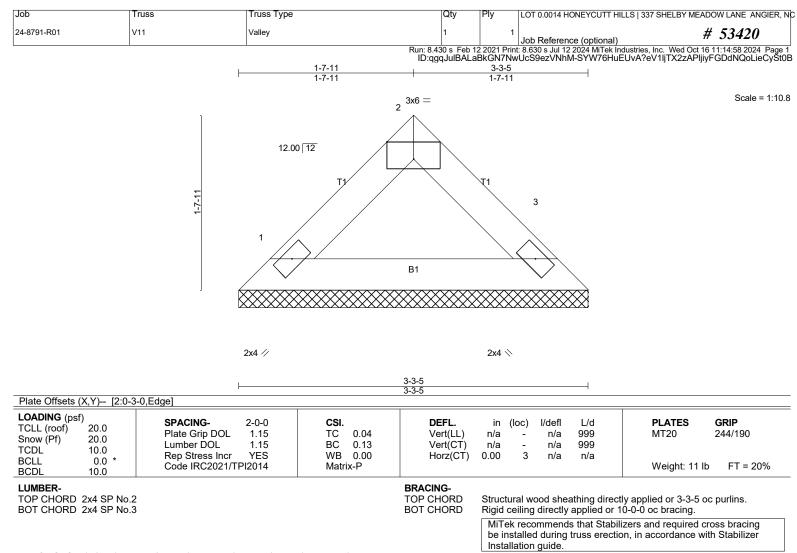
LOAD CASE(S) Standard



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

<sup>1)</sup> Unbalanced roof live loads have been considered for this design.

<sup>2)</sup> Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



REACTIONS. (lb/size) 1=103/3-3-5 (min. 0-1-8), 3=103/3-3-5 (min. 0-1-8) Max Horz 1=25(LC 11) Max Uplift1=-5(LC 12), 3=-5(LC 13)

#### NOTES- (8)

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

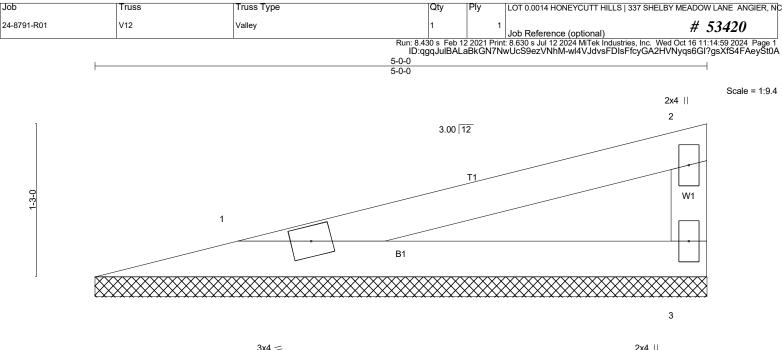
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



2x4 ||

LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2021/TPI2014	<b>CSI.</b> TC 0.33 BC 0.27 WB 0.00 Matrix-P	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	l/defl n/a n/a n/a	L/d 999 999 n/a	<b>PLATES</b> MT20 Weight: 14 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD	end verticals	s.		tly applied or 5-0-0 o 10-0-0 oc bracing.	c purlins, exce
					d during		ilizers and required ci ion, in accordance wi	

REACTIONS. (lb/size) 1=147/5-0-0 (min. 0-1-8), 3=147/5-0-0 (min. 0-1-8) Max Horz 1=29(LC 11) Max Uplift1=-18(LC 10), 3=-21(LC 14) Max Grav 1=184(LC 20), 3=184(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard

