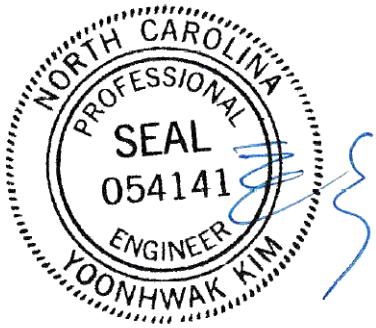


Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

10/04/2024
ABCD Engineering, PLLC NC COA 0838

Site Information:	**Page 1:**
Customer: Structural Building Solutions LLC	*Job Number:* Q2409-242
Job Description: The Farm at Neills Creek	
Address:	

Job Engineering Criteria:	
Design Code: IRC 2021	*IntelliVIEW Version:* 23.02.04A *JRef #:* 1Y3T98360006
Wind Standard: ASCE 7-16 *Wind Speed (mph):* 120 *Building Type:* Closed	*Design Loading (psf):* 40.00

This package contains general notes pages, 38 truss drawing(s) and 7 detail(s).

Item	Drawing Number	Truss
1	278.24.0921.01777	A3G
3	278.24.0921.00821	A3A
5	278.24.0921.01151	A3T
7	278.24.0921.01715	PB2
9	278.24.0921.02012	V5
11	278.24.0921.02061	P1
13	278.24.0921.00743	V1
15	278.24.0921.01464	V3
17	278.24.0921.02028	VA2
19	278.24.0921.01385	VG1
21	278.24.0921.00837	VG3
23	278.24.0921.00774	VG4
25	278.24.0921.01417	T1G
27	278.24.0921.01307	PB1G
29	278.24.0921.01463	PB1
31	278.24.1027.30803	A1G
33	278.24.1026.16317	M1
35	278.24.1027.36327	A2T
37	278.24.1026.21000	M2
39	A12015ENC160118	
41	BRCLBSUB0119	
43	GBLLETIN0118	
45	VALTN160118	

Item	Drawing Number	Truss
2	278.24.0921.01182	PB4G
4	278.24.0921.00868	PB4
6	278.24.1027.59090	A3
8	278.24.0921.01135	PB2G
10	278.24.0921.01716	V2
12	278.24.0921.01761	V4
14	278.24.0921.02075	P1G
16	278.24.0921.01073	P1D
18	278.24.0921.01746	VA1
20	278.24.0921.01811	VG2
22	278.24.0921.01483	VG5
24	278.24.0921.01448	T1
26	278.24.0921.02060	G1G
28	278.24.0921.01166	G1
30	278.24.0921.00775	G2
32	278.24.1026.18863	M1G
34	278.24.1025.40810	A1
36	278.24.1026.23337	M2A
38	278.24.1027.34137	A2
40	A12030ENC160118	
42	GABRST160118	
44	PB160160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:
The bearing area factor, Cb, is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Coated Lumber:
Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
C = Coated lumber.
C-AT = AtTEK coated lumber.
C-FX = FX Lumber Guard coated lumber.
C -TE = TechWood 4400 coated lumber.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-BF = Boraflame Fire Retardant Treated lumber
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-ON = OnWood Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
FRT-PR = ProWood Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

General Notes (continued)

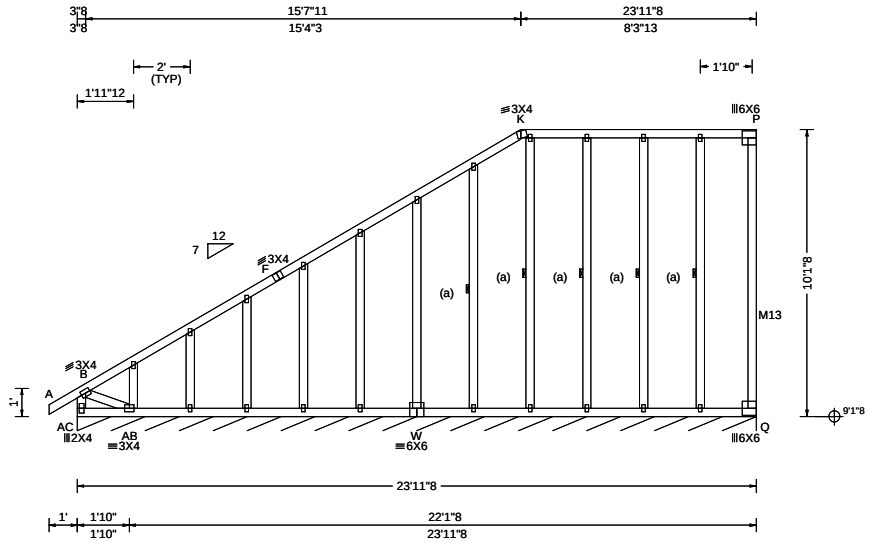
Key to Terms (continued):
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com

SEQN: 4276 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** A3G	Cust: R 9836 JRef:1Y3T98360006 T4 / DrwNo: 278.24.0921.01777 / YK 10/04/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 16.82 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 1.00
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	-0.002 K	999	240
VERT(CL):	0.003 K	999	240
HORZ(LL):	-0.012 P	-	-
HORZ(TL):	0.016 P	-	-

Creep Factor: 2.0
Max TC CSI: 0.236
Max BC CSI: 0.240
Max Web CSI: 0.767

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Q*	86	/-	/-	/50	/-	/19

Wind reactions based on MWFRS
Q Brg Wid = 287 Min Req = -
Bearing AC is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - F	198	- 494	F - K	308	- 389

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
AB- W	493	- 153

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
B -AB	414	- 145

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.
AC- B	130	- 413

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; M13 2x4 SP SS;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

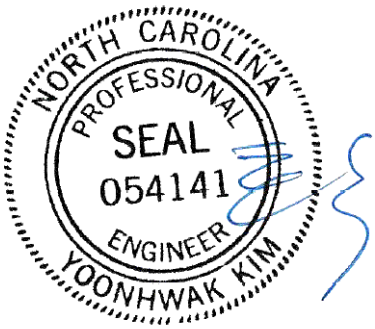
Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



10/04/2024
ABCD Engineering, PLLC NC COA 0838

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

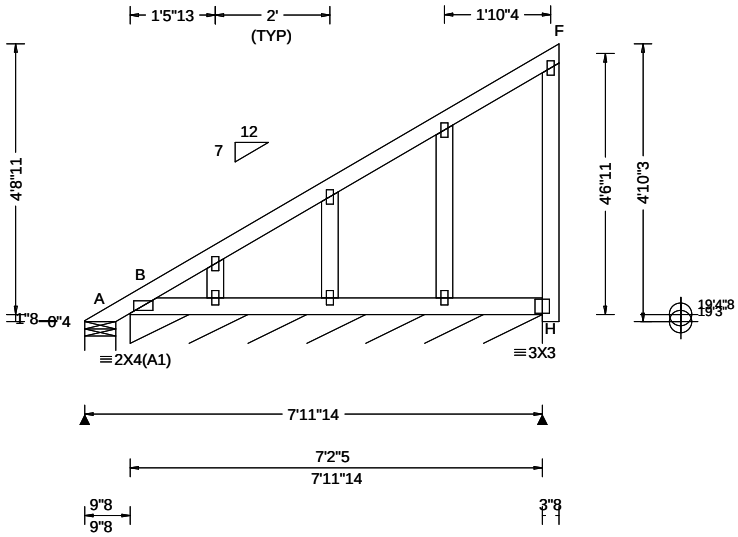
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4518 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** PB4G	Cust: R 9836 JRef:1Y3T98360006 T29 / DrwNo: 278.24.0921.01182 / YK 10/04/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.82 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 E 999 240 VERT(CL): 0.001 E 999 240 HORZ(LL): -0.001 E - - HORZ(TL): 0.001 E - - Creep Factor: 2.0 Max TC CSI: 0.052 Max BC CSI: 0.014 Max Web CSI: 0.038 VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	14	/-	/-	/57	/17	/103
B*	84	/-	/-	/59	/15	/-

Wind reactions based on MWFRS
A Brg Wid = 6.5 Min Req = 1.5 (Truss)
B Brg Wid = 86.3 Min Req = -
Bearings A & B are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

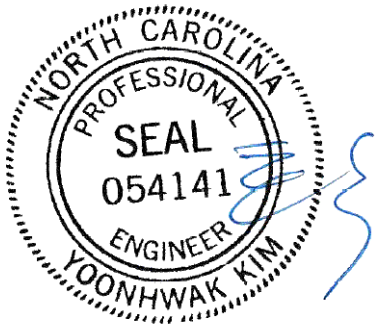
Right end vertical exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



10/04/2024
ABCD Engineering, PLLC NC COA 0838

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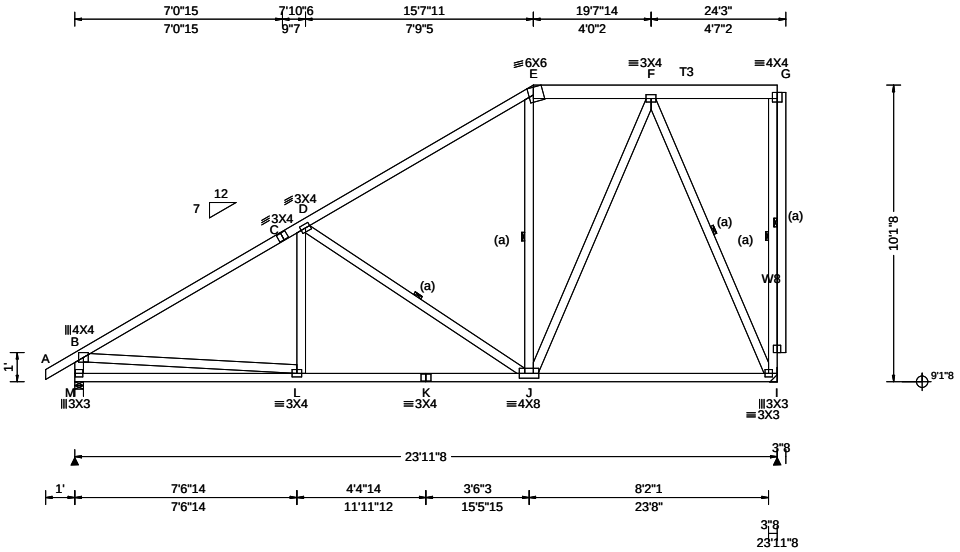
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4290 / MONO FROM:	Ply: 1 Qty: 4	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** A3A	Cust: R 9836 JRef:1Y3T98360006 T31 / DrwNo: 278.24.0921.00821 / YK 10/04/2024



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 16.82 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 1.00
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.035	L	999	240
VERT(CL): 0.072	L	999	240
HORZ(LL): 0.010	I	-	-
HORZ(TL): 0.021	I	-	-

Creep Factor: 2.0
Max TC CSI: 0.862
Max BC CSI: 0.702
Max Web CSI: 0.517

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
M	1065	/-	/-	/626	/-	/226
I	995	/-	/-	/571	/-	/-

Wind reactions based on MWFRS
M Brg Wid = 3.5 Min Req = 1.5 (Truss)
I Brg Wid = - Min Req = -
Bearing M is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	0	-1367	D - E	0	-823
C - D	0	-1142	E - F	31	-597

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
L - K	1087	-148	K - J	1087	-148

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - M	41	-999	J - F	605	-91
B - L	960	0	F - I	0	-893
D - J	142	-593			

Lumber

Top chord: 2x4 SP #2; T3 2x6 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W8 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

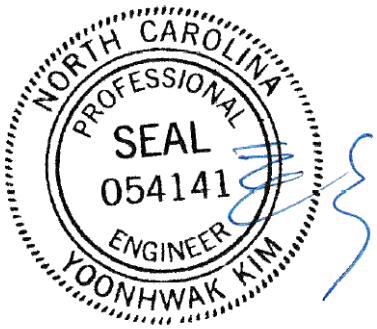
Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical exposed to wind pressure. Deflection meets L/180.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



10/04/2024
ABCD Engineering, PLLC NC COA 0838

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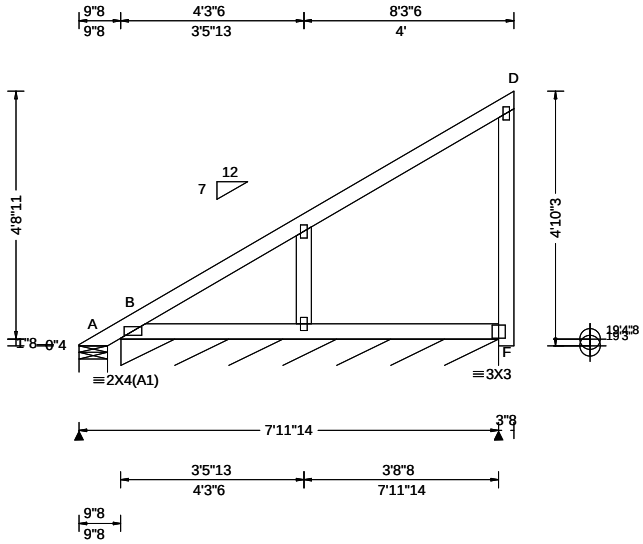
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4516 / GABL FROM:	Ply: 1 Qty: 10	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** PB4	Cust: R 9836 JRef:1Y3T98360006 T30 / DrwNo: 278.24.0921.00868 / YK 10/04/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: 20.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 120 mph	Pf: 15.4 Ce: 1.0	VERT(LL): 0.001 B 999 240
BCLL: 0.00	Enclosure: Closed	Lu: - Cs: 1.00	VERT(CL): 0.002 B 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): -0.001 D - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.001 B - -
NCBCLL: 10.00	Mean Height: 16.82 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	IRC 2021	Max TC CSI: 0.263
Load Duration: 1.15	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.068
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.059
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.04A.0207.13
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-30	/-	/65	/42	/103
B*	91	/-	/-	/63	/16	/-

Wind reactions based on MWFRS
A Brg Wid = 6.5 Min Req = 1.5 (Truss)
B Brg Wid = 86.3 Min Req = -
Bearings A & B are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

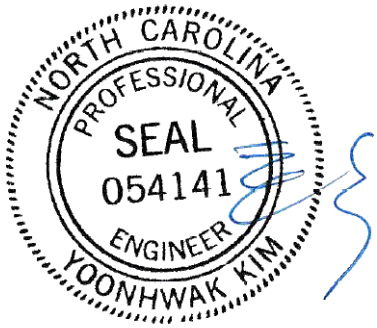
Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



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ABCD Engineering, PLLC NC COA 0838

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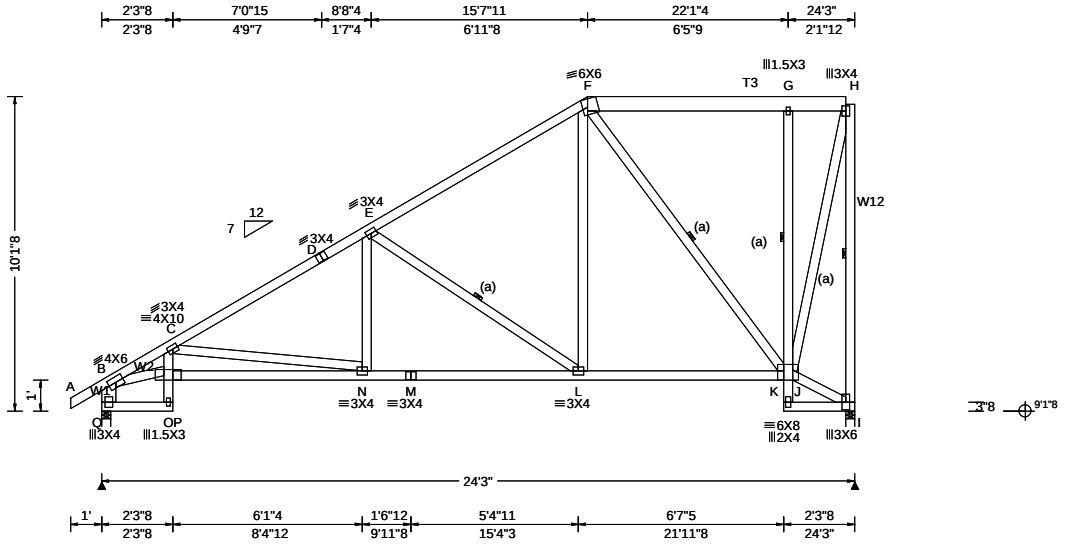
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4296 / MONO FROM:	Ply: 1 Qty: 2	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** A3T	Cust: R 9836 JRef:1Y3T98360006 T32 / DrwNo: 278.24.0921.01151 / YK 10/04/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: 20.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 120 mph	Pf: 15.4 Ce: 1.0	VERT(LL): 0.075 N 999 240
BCLL: 0.00	Enclosure: Closed	Lu: - Cs: 1.00	VERT(CL): 0.155 N 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): 0.062 I - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.128 I - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	IRC 2021	Max TC CSI: 0.641
Load Duration: 1.15	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.728
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.817
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	VIEW Ver: 23.02.04A.0207.13
	GCpi: 0.18	WAVE	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Q	1077	/-	/-	/616	/-	/264
I	989	/-	/-	/563	/18	/-

Wind reactions based on MWFRS
Q Brg Wid = 3.5 Min Req = 1.5 (Truss)
I Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings Q & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	332	- 2590	D - E	175	- 1400
C - D	166	- 1555	E - F	204	- 909

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
O - N	2271	- 921	M - L	1257	- 427
N - M	1257	- 427	L - J	671	- 298

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - Q	128	- 1040	E - L	159	- 695
B - O	2172	- 516	L - F	585	- 8
O - C	460	- 106	F - J	189	- 708
C - N	500	- 1013	J - H	977	- 353
N - E	379	0	H - I	332	- 965

Lumber

Top chord: 2x4 SP #2; T3 2x6 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W1 2x6 SP #2; W2 2x4 SP #2;
W12 2x4 SP SS;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

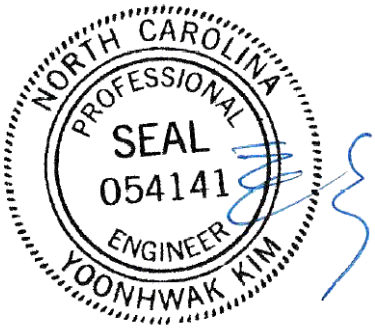
Truss designed for unbalanced snow loads.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.



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ABCD Engineering, PLLC NC COA 0838

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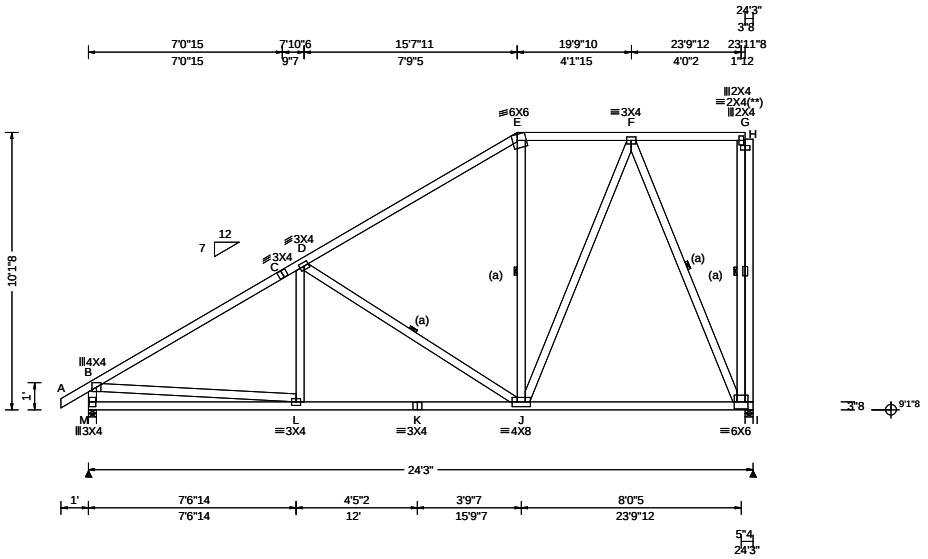
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4428 MONO FROM:	Ply: 1 Qty: 4	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** A3	Cust: R 9836 JRef:1Y3T98360006 T27 DrwNo: 278.24.1027.59090 / YK 10/04/2024



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: 20.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 120 mph	Pf: 15.4 Ce: 1.0	VERT(LL): 0.041 D 999 240
BCLL:	0.00	Enclosure: Closed	Lu: - Cs: 1.00	VERT(CL): 0.084 D 999 240
BCDL:	10.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): 0.011 D - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL): 0.023 D - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	IRC 2021	Max TC CSI: 0.877
Load Duration:	1.15	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.746
Spacing:	24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.515
		C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: Any	Plate Type(s):	
		GCpi: 0.18	WAVE	VIEW Ver: 23.02.04A.0207.13
		Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
M	1077	/-	/-	/616	/-	/264
I	989	/-	/-	/563	/19	/-

Wind reactions based on MWFRS
M Brg Wid = 3.5 Min Req = 1.5 (Truss)
I Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings M & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	161	- 1390	D - E	207	- 829
C - D	165	- 1165	E - F	213	- 596

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
M - L	220	- 475	K - J	1107	- 369
L - K	1107	- 369			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - M	156	- 1012	J - F	634	- 119
B - L	983	- 35	F - I	276	- 875
D - J	129	- 611	H - I	1151	- 1206

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss designed for unbalanced snow loads.

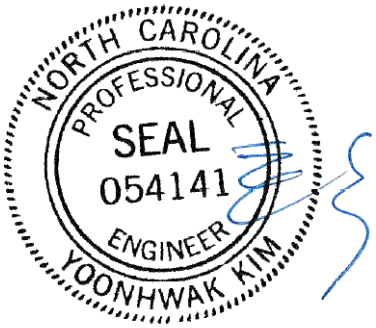
Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



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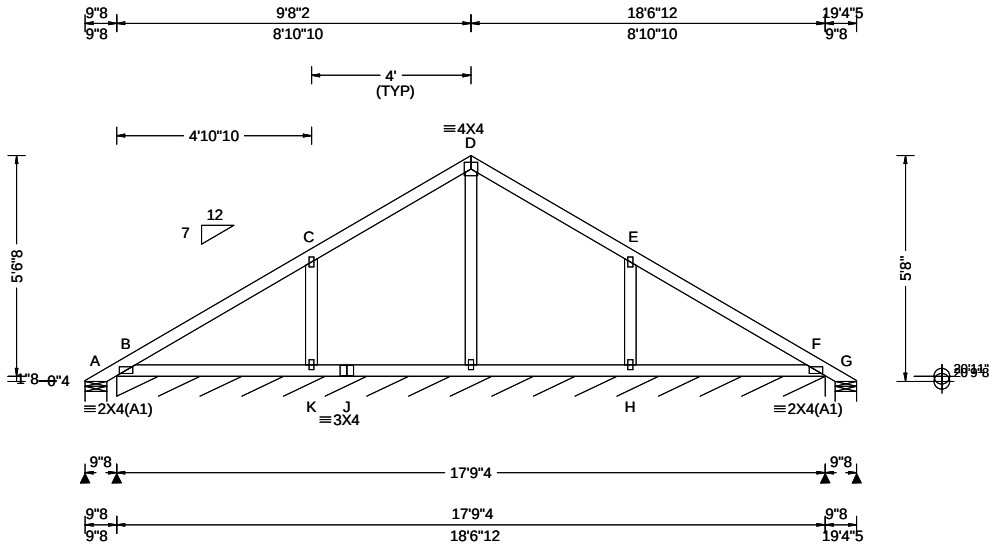
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4451 / GABL FROM:	Ply: 1 Qty: 9	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** PB2	Cust: R 9836 JRef:1Y3T98360006 T28 / DrwNo: 278.24.0921.01715 / YK 10/04/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 18.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.69 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 1.00
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.002	F	999	240
VERT(CL): 0.004	F	999	240
HORZ(LL): 0.001	B	-	-
HORZ(TL): 0.002	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.340
Max BC CSI: 0.101
Max Web CSI: 0.105

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-88	/-	/58	/110	/92
B*	92	/-	/-	/55	/9	/-
G	-	/-88	/-	/22	/65	/-

Wind reactions based on MWFRS
A Brg Wid = 6.5 Min Req = 1.5 (Truss)
B Brg Wid = 213 Min Req = -
G Brg Wid = 6.5 Min Req = 1.5 (Truss)
Bearings A, B, & G are a rigid surface.
Members not listed have forces less than 375#

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.	Gables	Tens.	Comp.
C - K	119	-376	H - E	119	-422

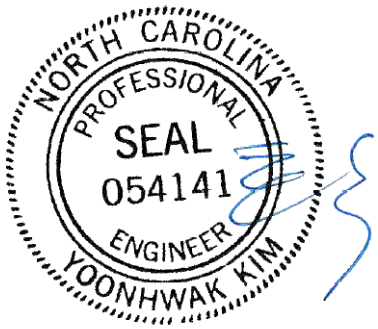
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes
All plates are 1.5X3 except as noted.

Loading
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.
Bottom chord checked for 10.00 psf non-concurrent live load.
Truss designed for unbalanced snow loads.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.
Refer to DWG PB160160118 for piggyback details.



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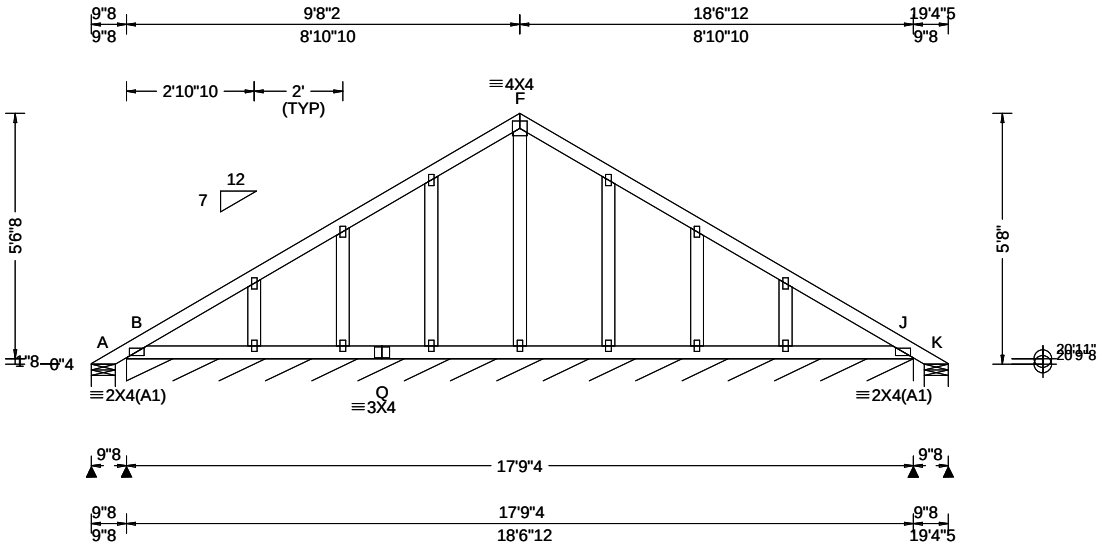
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4453 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** PB2G	Cust: R 9836 JRef:1Y3T98360006 T18 / DrwNo: 278.24.0921.01135 / YK 10/04/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 18.29 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.69 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 1.00
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	G	999	240
VERT(CL): 0.001	G	999	240
HORZ(LL): 0.001	I	-	-
HORZ(TL): 0.001	I	-	-

Creep Factor: 2.0
Max TC CSI: 0.079
Max BC CSI: 0.038
Max Web CSI: 0.071

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-7	/-	/50	/51	/90
B*	83	/-	/-	/49	/7	/-
K	-	/-8	/-	/6	/7	/-

Wind reactions based on MWFRS
A Brg Wid = 6.5 Min Req = 1.5 (Truss)
B Brg Wid = 213 Min Req = -
K Brg Wid = 6.5 Min Req = 1.5 (Truss)
Bearings A, B, & K are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss designed for unbalanced snow loads.

Wind

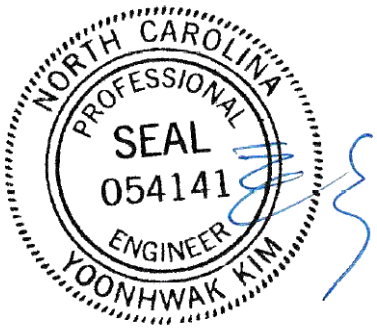
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



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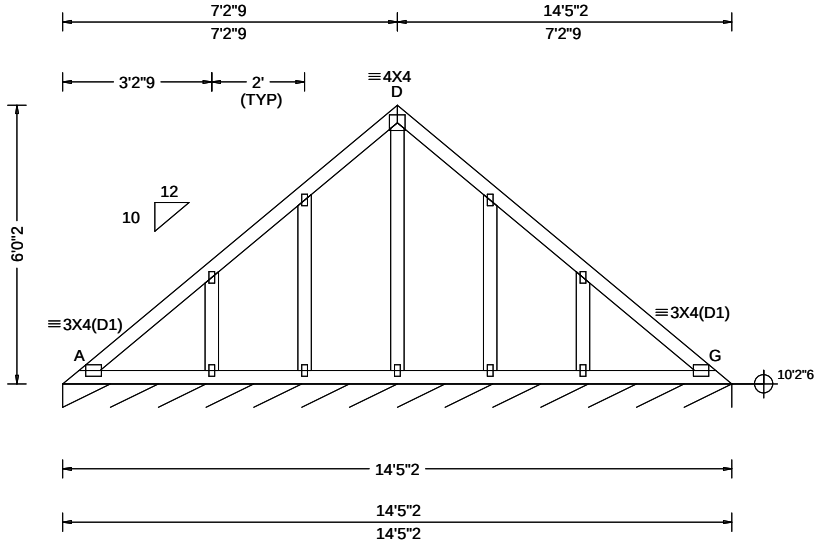
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4305 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** V5	Cust: R 9836 JRef:1Y3T98360006 T13 / DrwNo: 278.24.0921.02012 / YK 10/04/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 0.93 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 A 999 240 VERT(CL): 0.006 A 999 240 HORZ(LL): -0.002 G - - HORZ(TL): 0.003 G - - Creep Factor: 2.0 Max TC CSI: 0.098 Max BC CSI: 0.077 Max Web CSI: 0.096 VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
G*	86	/-	/-	/46	/-	/7

Wind reactions based on MWFRS
G Brg Wid = 173 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

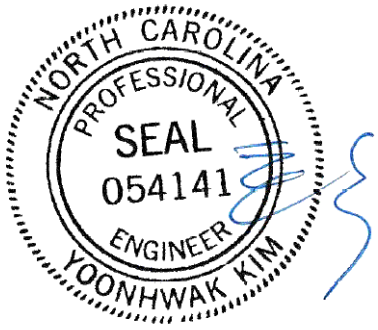
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



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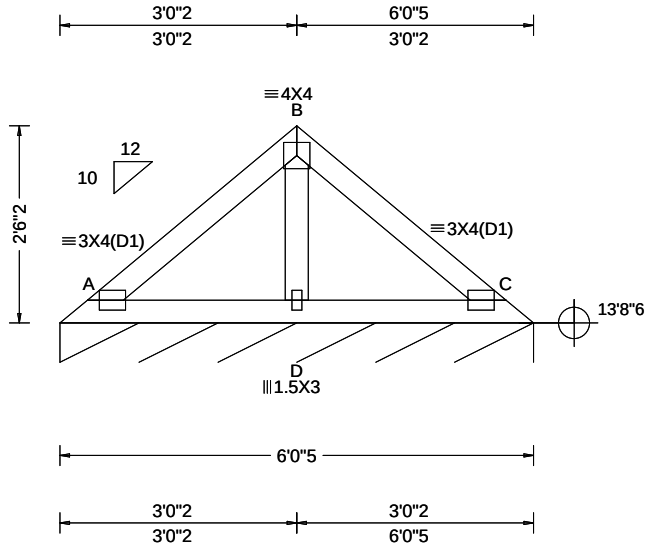
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4405 / VAL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** V2	Cust: R 9836 JRef:1Y3T98360006 T14 / DrwNo: 278.24.0921.01716 / YK 10/04/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: 20.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 120 mph	Pf: 15.4 Ce: 1.0	VERT(LL): 0.002 A 999 240
BCLL: 0.00	Enclosure: Closed	Lu: - Cs: 0.93	VERT(CL): 0.005 A 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): -0.001 C - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.003 C - -
NCBCLL: 10.00	Mean Height: 15.10 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	IRC 2021	Max TC CSI: 0.120
Load Duration: 1.15	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.094
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.055
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.04A.0207.13
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	85	/-	/-	/44	/-	/6

Wind reactions based on MWFRS
C Brg Wid = 72.3 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

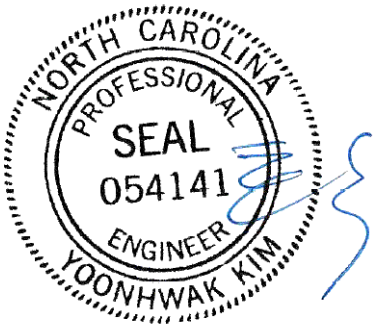
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



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ABCD Engineering, PLLC NC COA 0838

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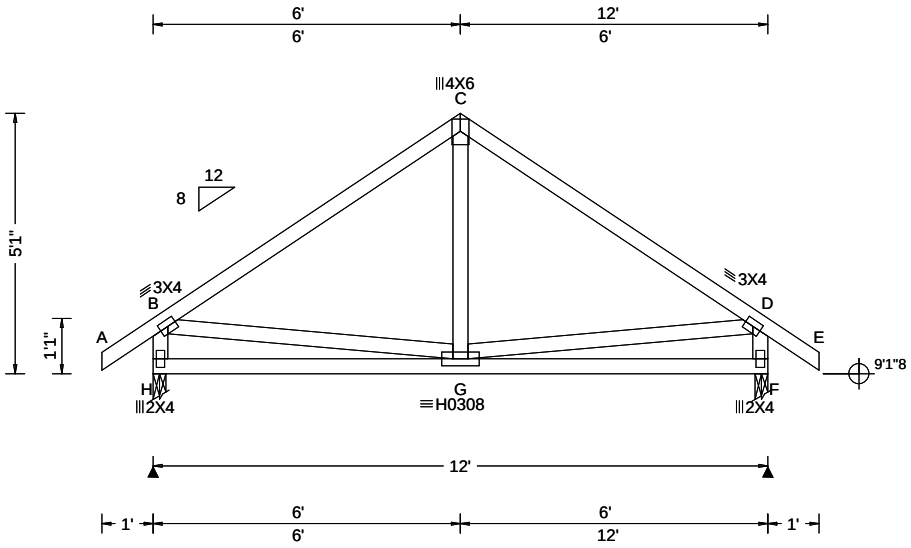
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4428 / COMN FROM:	Ply: 1 Qty: 4	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** P1	Cust: R 9836 JRef:1Y3T98360006 T1 DrwNo: 278.24.0921.02061 / YK 10/04/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: 20.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 120 mph	Pf: 15.4 Ce: 1.0	VERT(LL): 0.006 G 999 240
BCLL: 0.00	Enclosure: Closed	Lu: - Cs: 1.00	VERT(CL): 0.013 G 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): 0.001 D - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.002 D - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	IRC 2021	Max TC CSI: 0.476
Load Duration: 1.15	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.323
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.134
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE, HS	VIEW Ver: 23.02.04A.0207.13
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
H	573	/-	/-	/322	/-	/110
F	573	/-	/-	/322	/-	/-

Wind reactions based on MWFRS
H Brg Wid = 3.0 Min Req = 1.5 (Truss)
F Brg Wid = 3.0 Min Req = 1.5 (Truss)
Bearings H & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	116	-541	C - D	116	-541

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - H	161	-525	D - F	161	-525

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

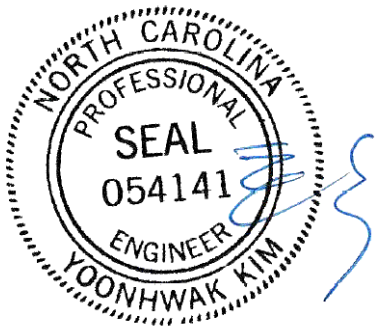
Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

Lanai/Porch Loading : 14.7 PLF wind pressure applied to the bottom chord of the truss from 0.00 ft to 12.00 ft,



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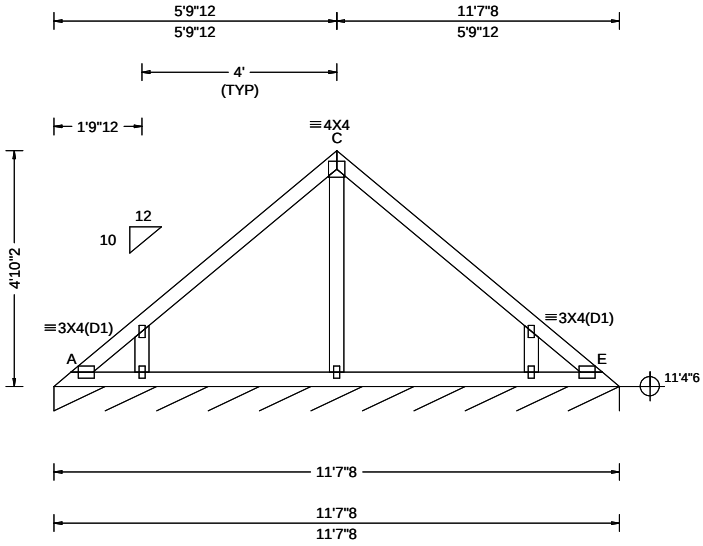
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4409 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** V4	Cust: R 9836 JRef:1Y3T98360006 T15 / DrwNo: 278.24.0921.01761 / YK 10/04/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 0.93
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.001 C	999	240
VERT(CL):	0.001 C	999	240
HORZ(LL):	-0.001 B	-	-
HORZ(TL):	0.001 A	-	-

Creep Factor: 2.0
Max TC CSI: 0.239
Max BC CSI: 0.119
Max Web CSI: 0.065

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E*	86	/-	/-	/46	/-	/7

Wind reactions based on MWFRS
E Brg Wid = 139 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

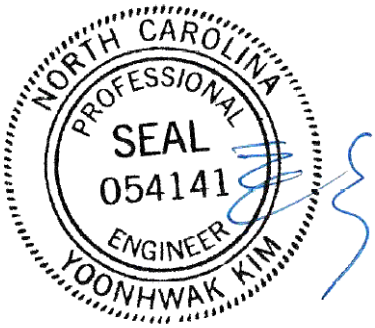
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



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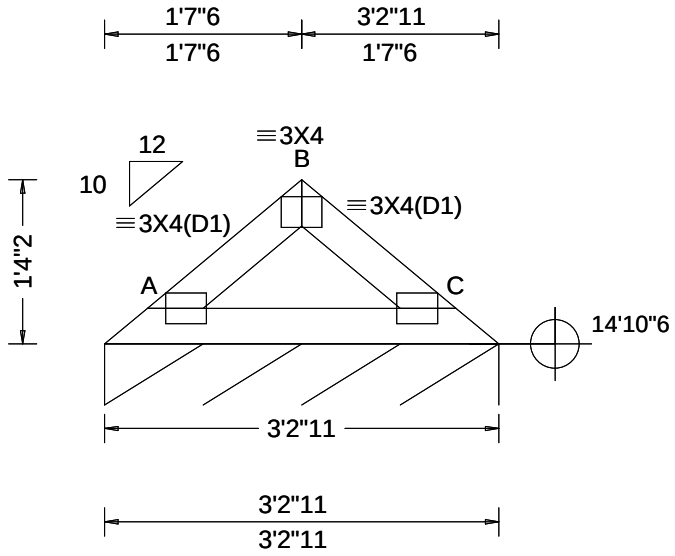
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4389 / VAL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** V1	Cust: R 9836 JRef:1Y3T98360006 T21 / DrwNo: 278.24.0921.00743 / YK 10/04/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.68 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 0.93
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.002 A	999	240
VERT(CL):	0.003 A	999	240
HORZ(LL):	-0.001 C	-	-
HORZ(TL):	0.002 C	-	-

Creep Factor: 2.0
Max TC CSI: 0.049
Max BC CSI: 0.068
Max Web CSI: 0.000

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	85	/-	/-	/41	/-	/5

Wind reactions based on MWFRS
C Brg Wid = 38.7 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

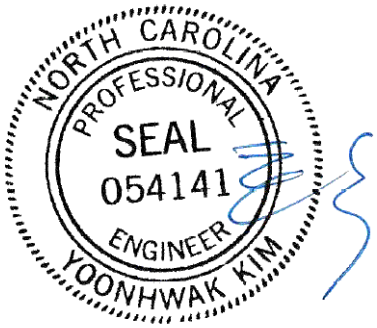
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



10/04/2024
ABCD Engineering, PLLC NC COA 0838

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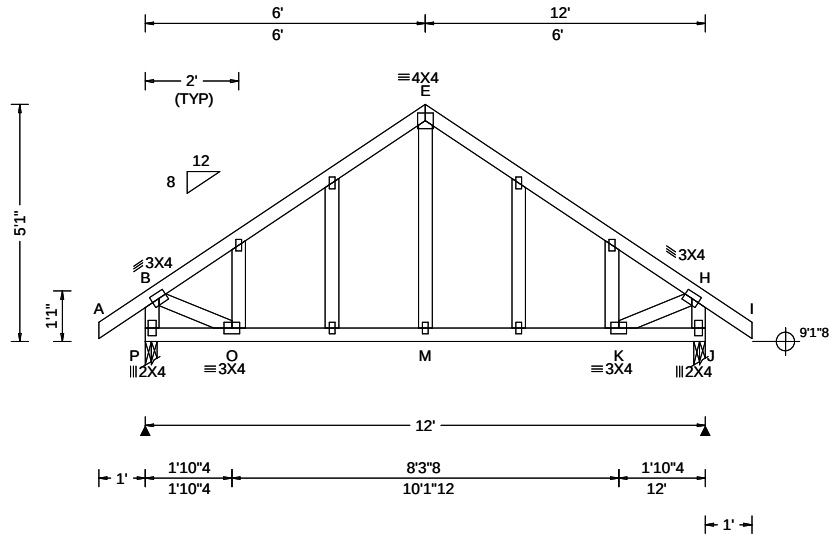
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4311 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** P1G	Cust: R 9836 JRef:1Y3T98360006 T2 / DrwNo: 278.24.0921.02075 / YK 10/04/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 1.00
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.016 G	999	240
VERT(CL):	0.033 G	999	240
HORZ(LL):	0.015 C	-	-
HORZ(TL):	0.030 C	-	-

Creep Factor: 2.0
Max TC CSI: 0.251
Max BC CSI: 0.357
Max Web CSI: 0.158

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
P	573	/-	/-	/322	/-	/110
J	573	/-	/-	/322	/-	/-

Wind reactions based on MWFRS
P Brg Wid = 3.0 Min Req = 1.5 (Truss)
J Brg Wid = 3.0 Min Req = 1.5 (Truss)
Bearings P & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - E	161	-476	E - H	161	-476

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - P	121	-538	K - H	380	0
B - O	380	0	H - J	121	-538

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

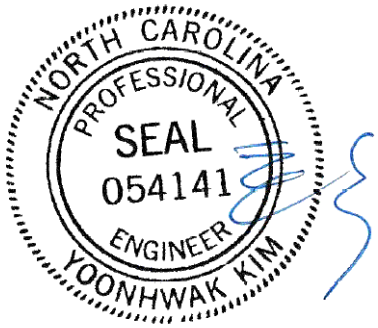
Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



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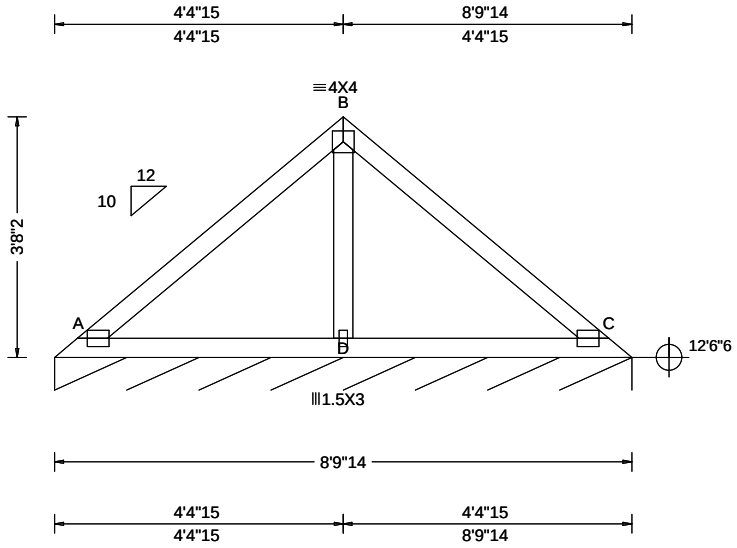
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4407 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** V3	Cust: R 9836 JRef:1Y3T98360006 T12 / DrwNo: 278.24.0921.01464 / YK 10/04/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 0.93 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.007 A 999 240 VERT(CL): 0.016 A 999 240 HORZ(LL): -0.004 C - - HORZ(TL): 0.010 C - - Creep Factor: 2.0 Max TC CSI: 0.296 Max BC CSI: 0.228 Max Web CSI: 0.127 VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	86	/-	/-	/45	/-	/6

Wind reactions based on MWFRS
C Brg Wid = 105 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.Comp.
B - D	208 - 466

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 3X4(D1) except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

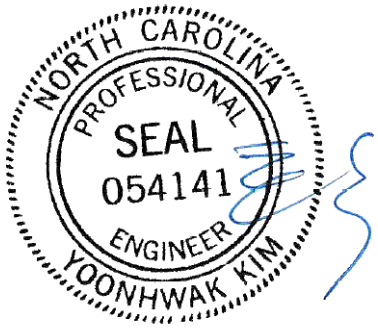
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



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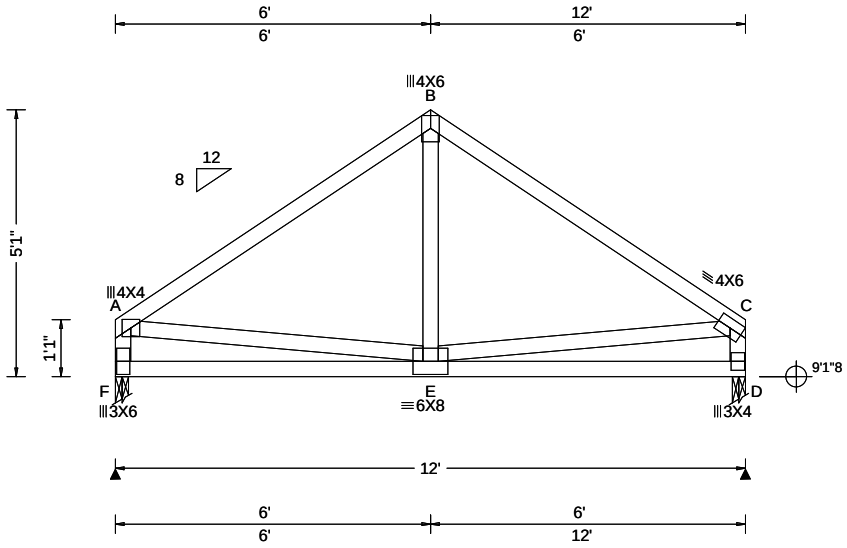
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4584 / COMN FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** P1D	Cust: R 9836 JRef:1Y3T98360006 T3 / DrwNo: 278.24.0921.01073 / YK 10/04/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 1.00
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.013 E	999	240
VERT(CL):	0.012 E	999	240
HORZ(LL):	0.065 B	-	-
HORZ(TL):	0.070 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.605
Max BC CSI: 0.638
Max Web CSI: 0.757

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
F	449	/-	/-	/277	/686	/6000
D	449	/-	/-	/1695	/-	/-

Wind reactions based on MWFRS
F Brg Wid = 3.0 Min Req = 1.5 (Truss)
D Brg Wid = 3.0 Min Req = 2.0 (Truss)
Bearings F & D are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	1254	-486	B - C	120	-2354

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
F - E	5987	-86	E - D	1571	-37

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - F	729	-401	E - C	1828	0
A - E	291	-1111	C - D	121	-1647

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP SS;
Webs: 2x4 SP #3;

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

Loading

Truss transfers a maximum horizontal load of 3000 # (250.00 plf) along top chord, from either direction, to supports where indicated. Diaphragm and connections are to be designed by Engineer of Record.

Drag Loads: Force(#) (PLF) Mbr Start End
Case 1: 3000 250.00 TC 0.00 12.00
3000 250.00 BC 0.00 12.00

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

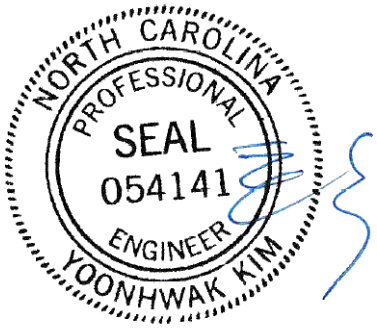
End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

Negative reaction(s) of -686# MAX. Requires uplift connection. See Maximum Reactions.

Lanai/Porch Loading : 14.7 PLF wind pressure applied to the bottom chord of the truss from 0.00 ft to 12.00 ft,



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ABCD Engineering, PLLC NC COA 0838

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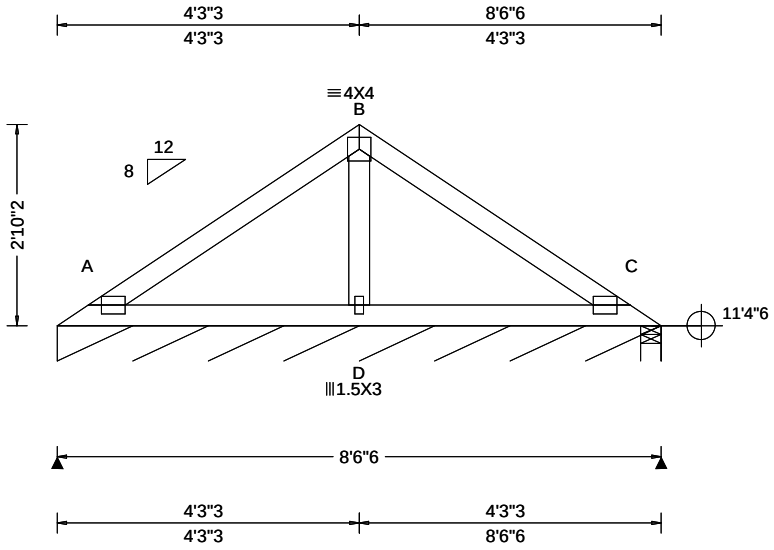
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4524 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** VA2	Cust: R 9836 JRef:1Y3T98360006 T9 / DrwNo: 278.24.0921.02028 / YK 10/04/2024



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 1.00
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.007	A	999	240
VERT(CL): 0.016	A	999	240
HORZ(LL): -0.004	C	-	-
HORZ(TL): 0.008	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.259
Max BC CSI: 0.201
Max Web CSI: 0.098

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A*	77	/-	/-	/40	/-	/-
C	61	/-	/-	/25	/-	/40

Wind reactions based on MWFRS
A Brg Wid = 102 Min Req = -
C Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings A & C Fcperp = 565psi.
Members not listed have forces less than 375#

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.
B - D	182	-438

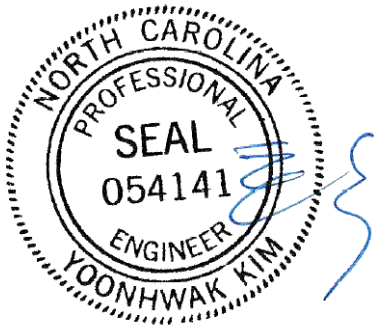
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes
All plates are 3X4(D1) except as noted.

Loading
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



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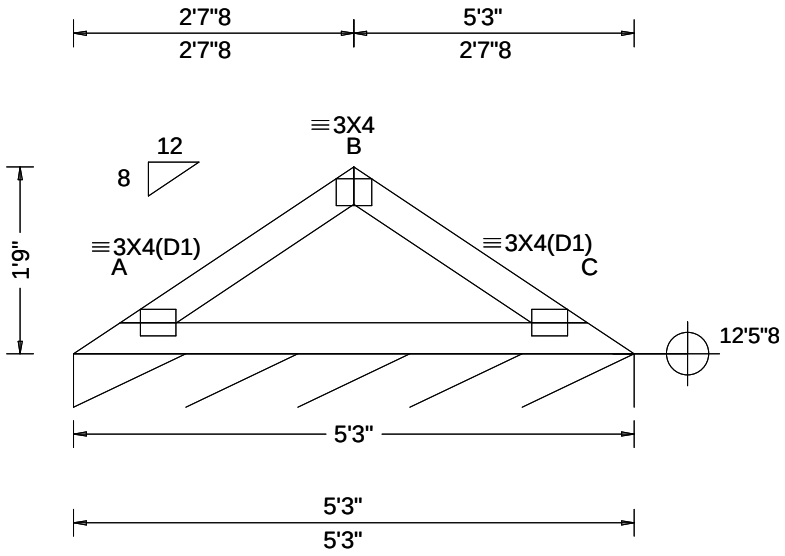
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4523 / VAL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** VA1	Cust: R 9836 JRef:1Y3T98360006 T36 / DrwNo: 278.24.0921.01746 / YK 10/04/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.007 A 999 240 VERT(CL): 0.013 A 999 240 HORZ(LL): -0.003 C - - HORZ(TL): 0.006 C - - Creep Factor: 2.0 Max TC CSI: 0.135 Max BC CSI: 0.166 Max Web CSI: 0.000 VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	83	/-	/-	/41	/-	/4

Wind reactions based on MWFRS
C Brg Wid = 63.0 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

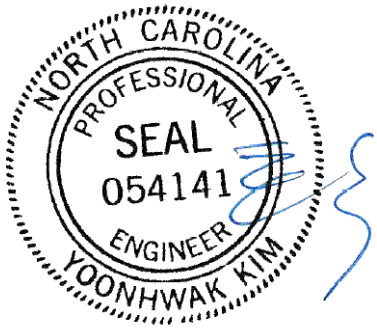
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



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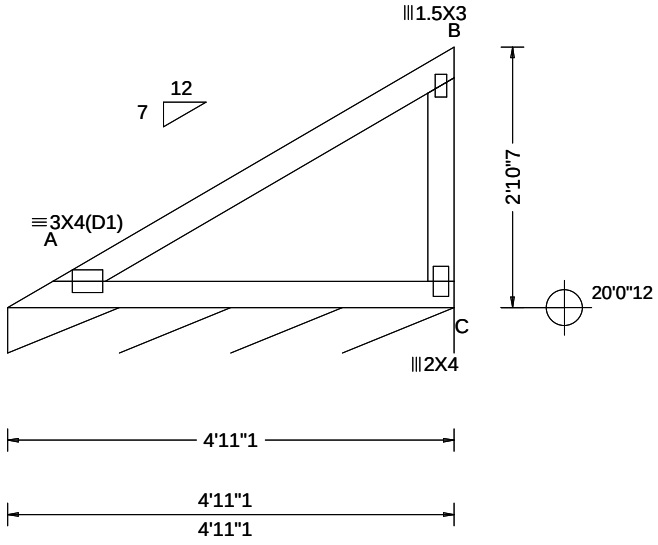
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4532 / VAL FROM:	Ply: 1 Qty: 2	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** VG1	Cust: R 9836 JRef:1Y3T98360006 T35 / DrwNo: 278.24.0921.01385 / YK 10/04/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.64 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.006 A - - HORZ(TL): 0.012 A - - Creep Factor: 2.0 Max TC CSI: 0.323 Max BC CSI: 0.294 Max Web CSI: 0.126 VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	83	/-	/-	/48	/3	/15

Wind reactions based on MWFRS
C Brg Wid = 59.1 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

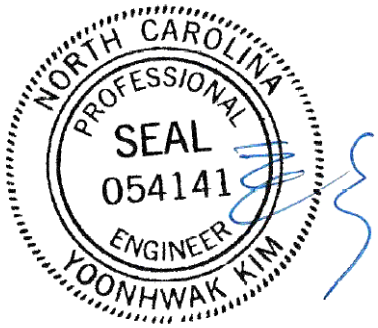
Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



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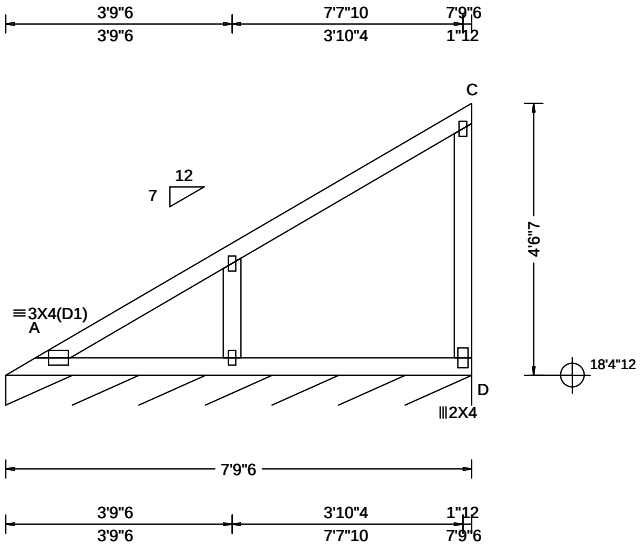
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4541 / GABL FROM:	Ply: 1 Qty: 2	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** VG2	Cust: R 9836 JRef:1Y3T98360006 T34 / DrwNo: 278.24.0921.01811 / YK 10/04/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 20.81 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 1.00
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.004 A	999	240
VERT(CL):	0.008 A	999	240
HORZ(LL):	-0.001 C	-	-
HORZ(TL):	0.002 A	-	-

Creep Factor: 2.0
Max TC CSI: 0.136
Max BC CSI: 0.165
Max Web CSI: 0.252

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D*	83	/-	/-	/49	/3	/16

Wind reactions based on MWFRS
D Brg Wid = 93.4 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP SS;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

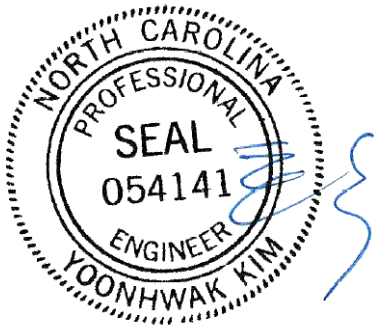
Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



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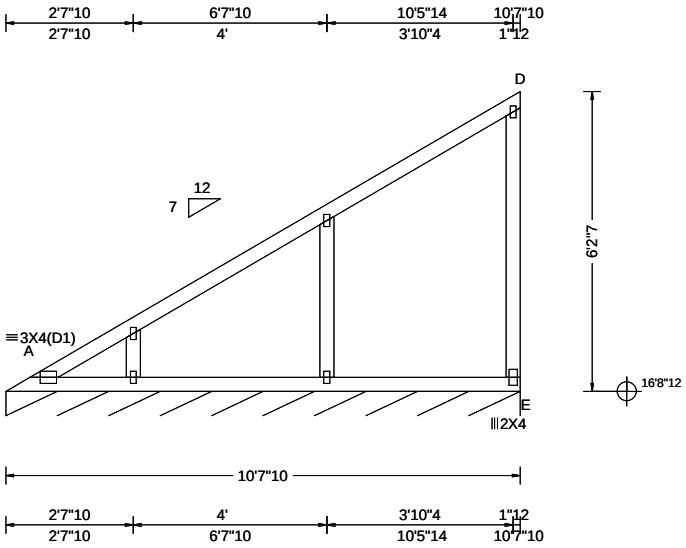
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4543 / GABL FROM:	Ply: 1 Qty: 2	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** VG3	Cust: R 9836 JRef:1Y3T98360006 T17 / DrwNo: 278.24.0921.00837 / YK 10/04/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: 20.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 120 mph	Pf: 15.4 Ce: 1.0	VERT(LL): 0.001 A 999 240
BCLL: 0.00	Enclosure: Closed	Lu: - Cs: 1.00	VERT(CL): 0.002 A 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): -0.003 D - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.004 D - -
NCBCLL: 10.00	Mean Height: 19.98 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	IRC 2021	Max TC CSI: 0.226
Load Duration: 1.15	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.130
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.478
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.04A.0207.13
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E*	83	/-	/-	/50	/2	/16

Wind reactions based on MWFRS
E Brg Wid = 127 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

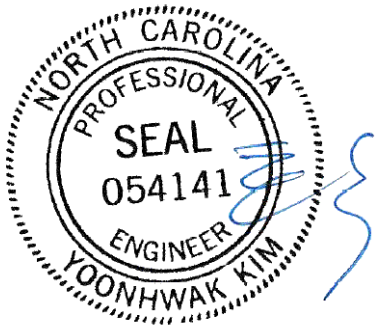
Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



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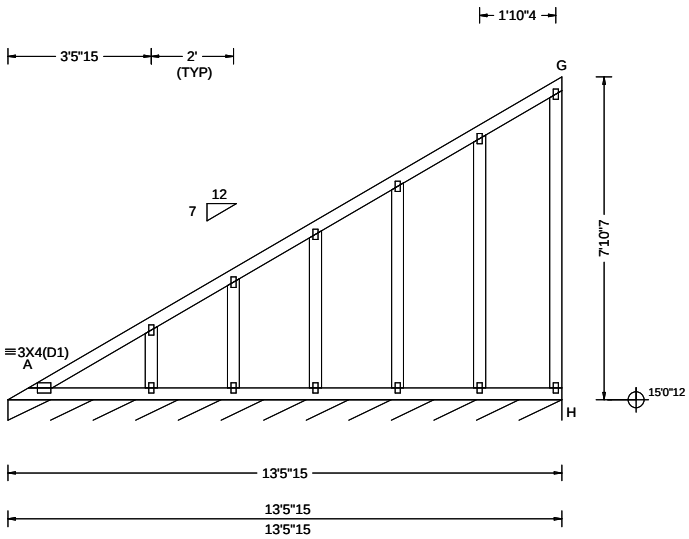
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4549 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** VG5	Cust: R 9836 JRef:1Y3T98360006 T10 / DrwNo: 278.24.0921.01483 / YK 10/04/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.14 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.004 A 999 240 VERT(CL): 0.008 A 999 240 HORZ(LL): -0.004 G - - HORZ(TL): 0.006 G - - Creep Factor: 2.0 Max TC CSI: 0.062 Max BC CSI: 0.084 Max Web CSI: 0.777 VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
H*	83	/-	/-	/50	/2	/16

Wind reactions based on MWFRS
H Brg Wid = 161 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP SS;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

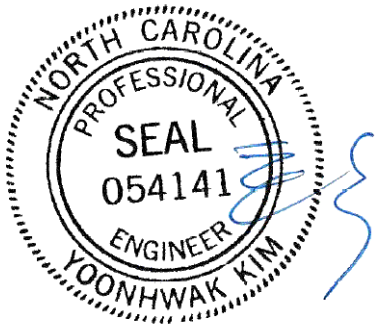
Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



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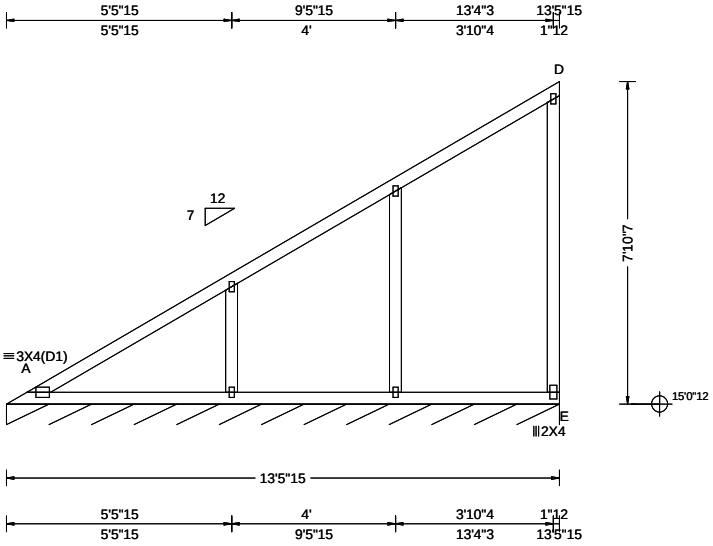
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4545 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** VG4	Cust: R 9836 JRef:1Y3T98360006 T6 DrwNo: 278.24.0921.00774 / YK 10/04/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: 20.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 120 mph	Pf: 15.4 Ce: 1.0	VERT(LL): 0.016 A 999 240
BCLL: 0.00	Enclosure: Closed	Lu: - Cs: 1.00	VERT(CL): 0.034 A 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): 0.005 A - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.010 A - -
NCBCLL: 10.00	Mean Height: 19.14 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	IRC 2021	Max TC CSI: 0.173
Load Duration: 1.15	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.237
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.730
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.04A.0207.13
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E*	83	/-	/-	/50	/2	/16

Wind reactions based on MWFRS
E Brg Wid = 161 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP SS;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

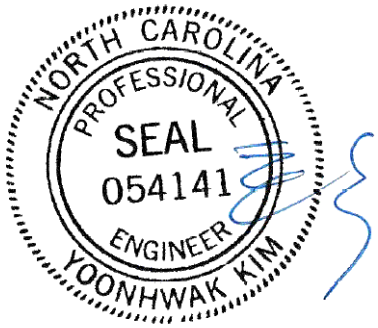
Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



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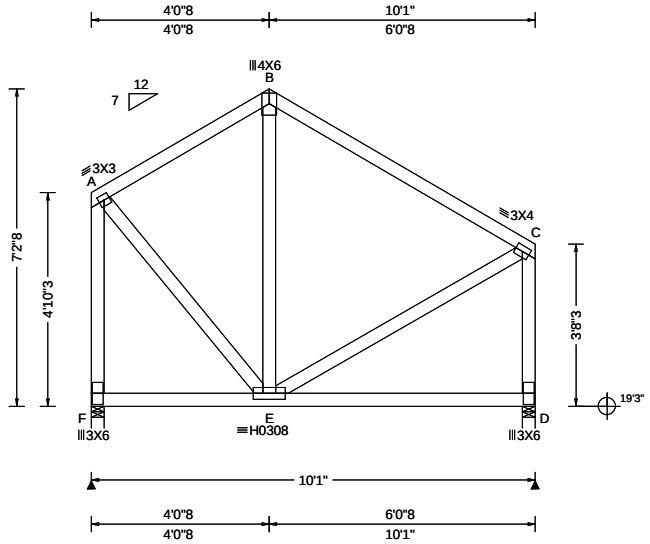
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4538 / COMN FROM:	Ply: 1 Qty: 10	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** T1	Cust: R 9836 JRef:1Y3T98360006 T19 / DrwNo: 278.24.0921.01448 / YK 10/04/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 24.70 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 1.00
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.004 B	999	240
VERT(CL):	0.008 B	999	240
HORZ(LL):	0.001 B	-	-
HORZ(TL):	0.002 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.693
Max BC CSI: 0.302
Max Web CSI: 0.386

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
F	435	/-	/-	/254	/40	/170
D	438	/-	/-	/244	/31	/-

Wind reactions based on MWFRS
F Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings F & D are a rigid surface.
Members not listed have forces less than 375#

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - F	204	-410	C - D	187	-386

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

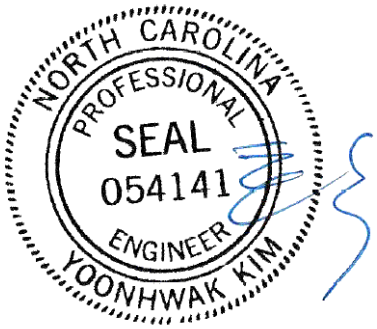
Truss designed for unbalanced snow loads.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.



10/04/2024
ABCD Engineering, PLLC NC COA 0838

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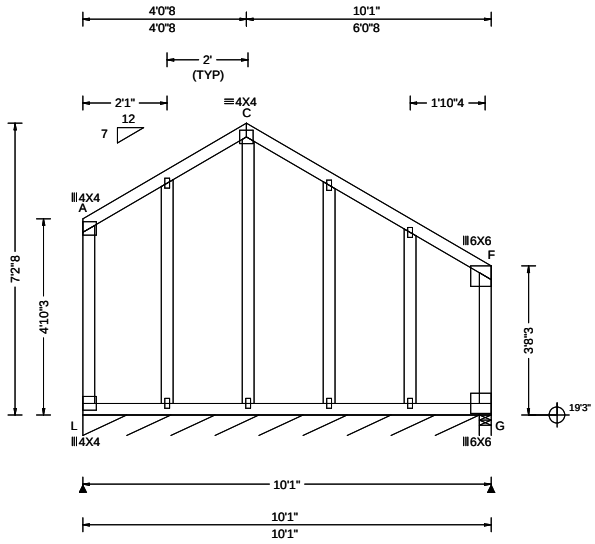
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4369 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** T1G	Cust: R 9836 JRef:1Y3T98360006 T33 / DrwNo: 278.24.0921.01417 / YK 10/04/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 24.70 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.008 E 999 240 VERT(CL): 0.015 E 999 240 HORZ(LL): 0.515 A - - HORZ(TL): 0.755 A - - Creep Factor: 2.0 Max TC CSI: 0.241 Max BC CSI: 0.255 Max Web CSI: 0.563 VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
L*	79	/-	/-	/58	/8	/17
G	69	/-	/-	/216	/142	/-

Wind reactions based on MWFRS
L Brg Wid = 117 Min Req = -
G Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings L & G are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

Fasten rated sheathing to one face of this frame.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss designed for unbalanced snow loads.

Wind

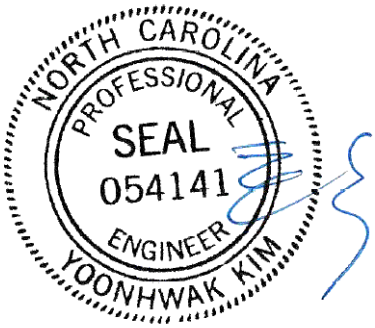
Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



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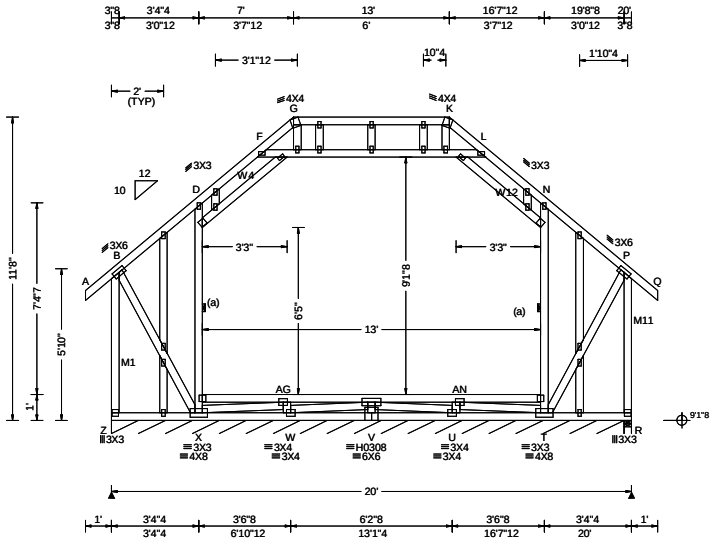
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4379 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** G1G	Cust: R 9836 JRef:1Y3T98360006 T5 / DrwNo: 278.24.0921.02060 / YK 10/04/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.15
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 16.71 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: varies
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.056 I	999	240
VERT(CL):	0.145 I	999	240
HORZ(LL):	0.023 O	-	-
HORZ(TL):	0.059 O	-	-

Creep Factor: 2.0
Max TC CSI: 0.320
Max BC CSI: 0.217
Max Web CSI: 0.405

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

		Gravity		Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Z*	109	/-	/-	/39	/7	/14
R	1387	/-	/-	/602	/-	/-
X		/-300				
T		/-300				

Wind reactions based on MWFRS
Z Brg Wid = 236 Min Req = -
R Brg Wid = 3.5 Min Req = 1.6 (Truss)
Bearings Z & R are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - D	0	-691	K - L	0	-672
D - F	37	-692	L - N	40	-697
F - G	0	-672	N - P	0	-692
G - K	0	-566			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - X	924	-110	U -AN	9	-410
AG- W	11	-410	T - P	924	-105

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.	Gables	Tens.	Comp.
Z - B	0	-1367	P - R	0	-1368

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W4,W12,M1,M11 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Live loads applied in combination per ASCE 7 sec. 2.4.1 use 0.75 factor for multiple live loads.

Attic room loading from 3-6-0 to 16-6-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Truss designed for unbalanced snow loads.

Purlins

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

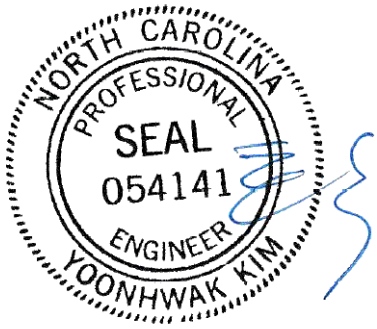
Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



10/04/2024
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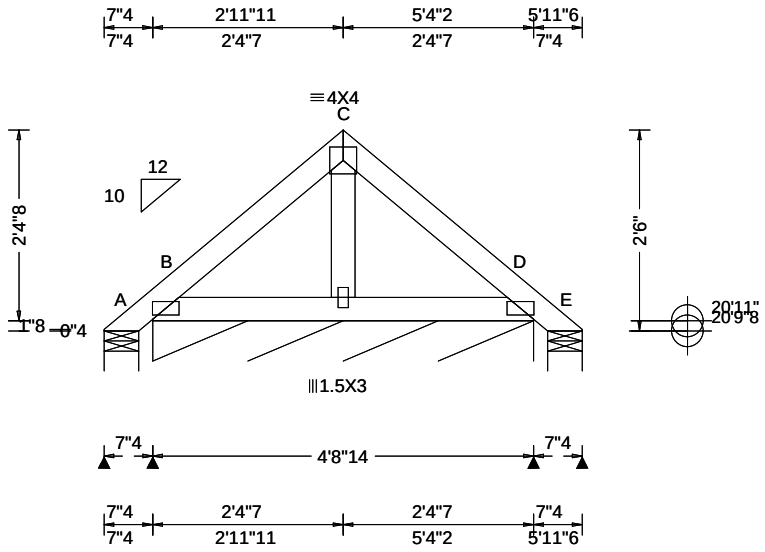
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4623 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** PB1G	Cust: R 9836 JRef:1Y3T98360006 T20 / DrwNo: 278.24.0921.01307 / YK 10/04/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 16.71 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 0.93
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.000 B	999	240
VERT(CL):	0.000 B	999	240
HORZ(LL):	0.000 B	-	-
HORZ(TL):	0.001 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.064
Max BC CSI: 0.039
Max Web CSI: 0.012

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-14	/-	/36	/39	/41
B*	106	/-	/-	/63	/3	/-
E	-	/-14	/-	/11	/12	/-

Wind reactions based on MWFRS
A Brg Wid = 5.2 Min Req = 1.5 (Truss)
B Brg Wid = 56.9 Min Req = -
E Brg Wid = 5.2 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

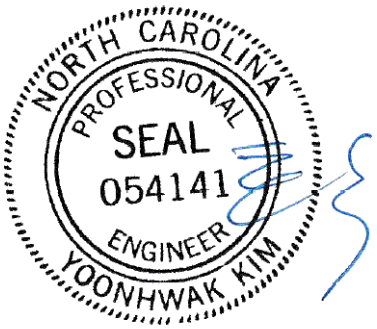
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



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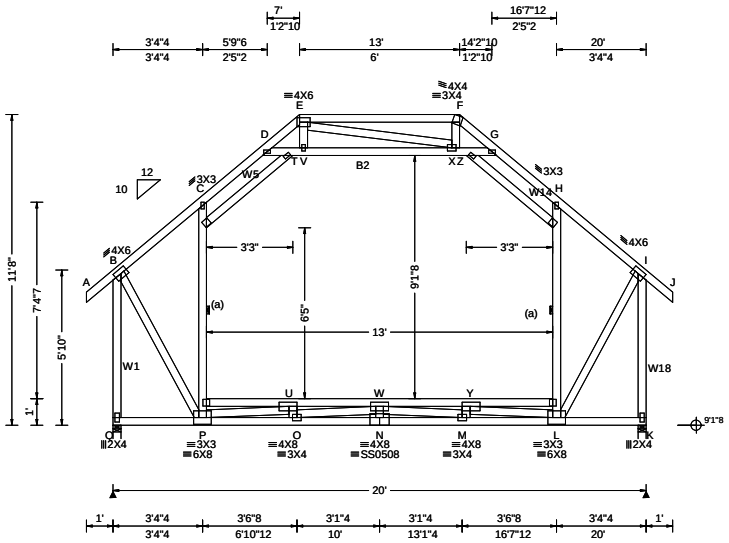
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4377 / COMN FROM:	Ply: 1 Qty: 3	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** G1	Cust: R 9836 JRef:1Y3T98360006 T23 / DrwNo: 278.24.0921.01166 / YK 10/04/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 16.71 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: varies
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, 18SS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.168	N	999	240
VERT(CL): 0.469	N	511	240
HORZ(LL): 0.133	H	-	-
HORZ(TL): 0.217	H	-	-

Creep Factor: 2.0
Max TC CSI: 0.658
Max BC CSI: 0.591
Max Web CSI: 0.949

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Q	1646	/-	/-	/516	/-	/274
K	1646	/-	/-	/516	/-	/-

Wind reactions based on MWFRS
Q Brg Wid = 3.5 Min Req = 1.9 (Truss)
K Brg Wid = 3.5 Min Req = 1.9 (Truss)
Bearings Q & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	113	- 856	F - G	44	- 608
C - D	263	- 751	G - H	262	- 748
D - E	54	- 623	H - I	111	- 856
E - F	53	- 524			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
P - O	2915	- 105	N - M	3890	0
O - N	3890	0	M - L	2916	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - Q	78	- 1662	W - M	141	- 921
B - P	1152	0	W - Y	49	- 2445
P - U	0	- 2517	X - Z	305	- 490
D - T	360	- 462	Y - L	0	- 2518
T - V	300	- 468	Z - G	369	- 480
U - W	27	- 2443	L - I	1152	0
O - W	146	- 923	I - K	80	- 1661
V - X	302	- 452			

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP SS; B2 2x4 SP #2;
Webs: 2x4 SP #3; W1,W5,W14,W18 2x4 SP #2;

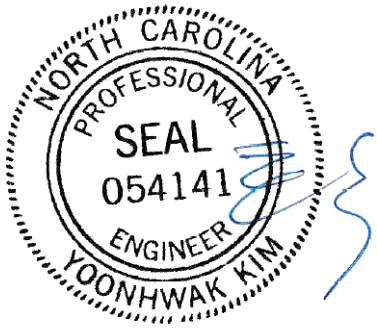
Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 1.5X3 except as noted.

Loading
Bottom chord checked for 10.00 psf non-concurrent live load.
Live loads applied in combination per ASCE 7 sec. 2.4.1 use 0.75 factor for multiple live loads.
Attic room loading from 3-6-0 to 16-6-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF
Truss designed for unbalanced snow loads.

Purlins
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/180.
Wind loading based on both gable and hip roof types.



10/04/2024
ABCD Engineering, PLLC NC COA 0838

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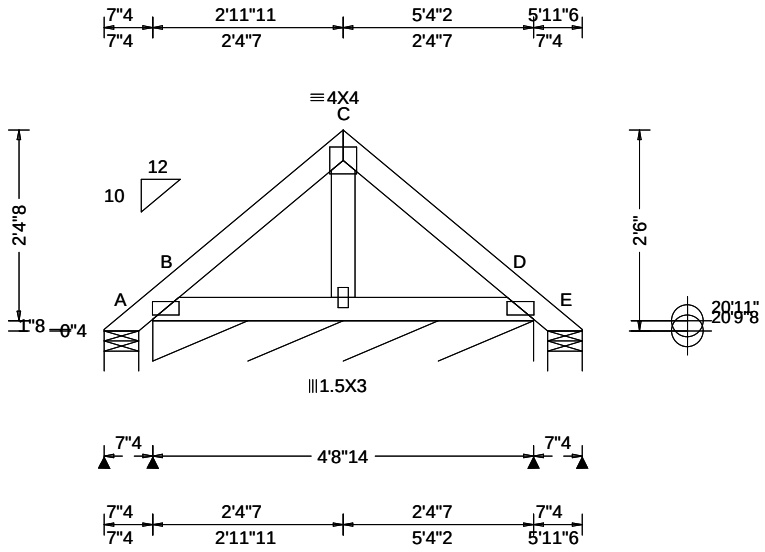
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4624 / GABL FROM:	Ply: 1 Qty: 11	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** PB1	Cust: R 9836 JRef:1Y3T98360006 T22 / DrwNo: 278.24.0921.01463 / YK 10/04/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 16.71 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 0.93
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.000 B	999	240
VERT(CL):	0.000 B	999	240
HORZ(LL):	0.000 B	-	-
HORZ(TL):	0.001 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.064
Max BC CSI: 0.039
Max Web CSI: 0.012

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-14	/-	/36	/39	/41
B*	106	/-	/-	/63	/3	/-
E	-	/-14	/-	/11	/12	/-

Wind reactions based on MWFRS
A Brg Wid = 5.2 Min Req = 1.5 (Truss)
B Brg Wid = 56.9 Min Req = -
E Brg Wid = 5.2 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

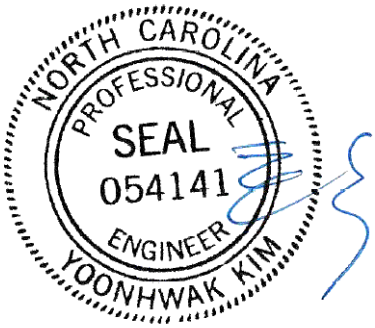
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



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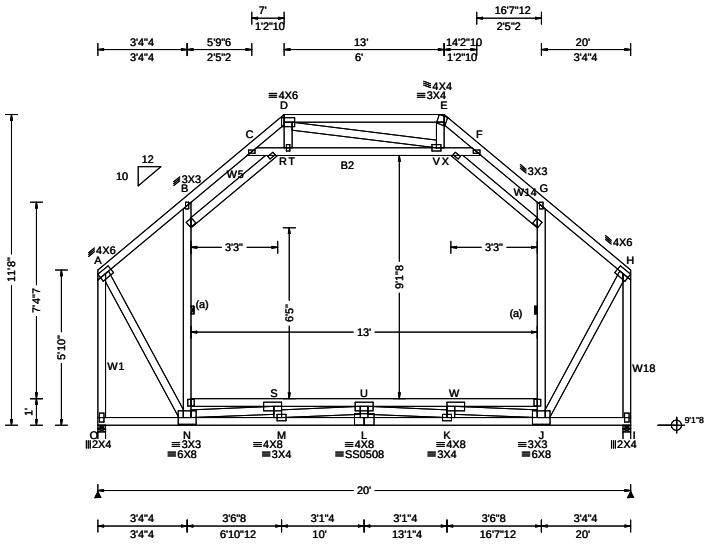
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4381 / COMN FROM:	Ply: 1 Qty: 8	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** G2	Cust: R 9836 JRef:1Y3T98360006 T7 / DrwNo: 278.24.0921.00775 / YK 10/04/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 16.71 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: varies
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, 18SS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.173 L	999	240
VERT(CL):	0.470 L	510	240
HORZ(LL):	0.119 G	-	-
HORZ(TL):	0.197 G	-	-

Creep Factor: 2.0
Max TC CSI: 0.667
Max BC CSI: 0.592
Max Web CSI: 0.914

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
O	1585	/-	/-	/493	/-	/240
I	1585	/-	/-	/493	/-	/-

Wind reactions based on MWFRS
O Brg Wid = 3.5 Min Req = 1.9 (Truss)
I Brg Wid = 3.5 Min Req = 1.9 (Truss)
Bearings O & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	83	- 855	E - F	34	- 606
B - C	241	- 757	F - G	241	- 754
C - D	44	- 621	G - H	82	- 854
D - E	44	- 520			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
N - M	2918	- 67	L - K	3894	0
M - L	3894	0	K - J	2920	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - O	46	- 1601	U - K	122	- 920
A - N	1160	- 20	U - W	29	- 2448
N - S	0	- 2518	V - X	281	- 494
C - R	332	- 467	W - J	0	- 2519
R - T	276	- 472	X - F	340	- 485
S - U	9	- 2446	J - H	1159	- 21
M - U	126	- 923	H - I	48	- 1600
T - V	278	- 456			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP SS; B2 2x4 SP #2;
Webs: 2x4 SP #3; W1,W5,W14,W18 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Live loads applied in combination per ASCE 7 sec. 2.4.1 use 0.75 factor for multiple live loads.

Attic room loading from 3-6-0 to 16-6-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Truss designed for unbalanced snow loads.

Purlins

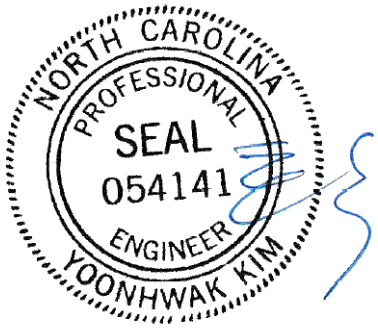
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.



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ABCD Engineering, PLLC NC COA 0838

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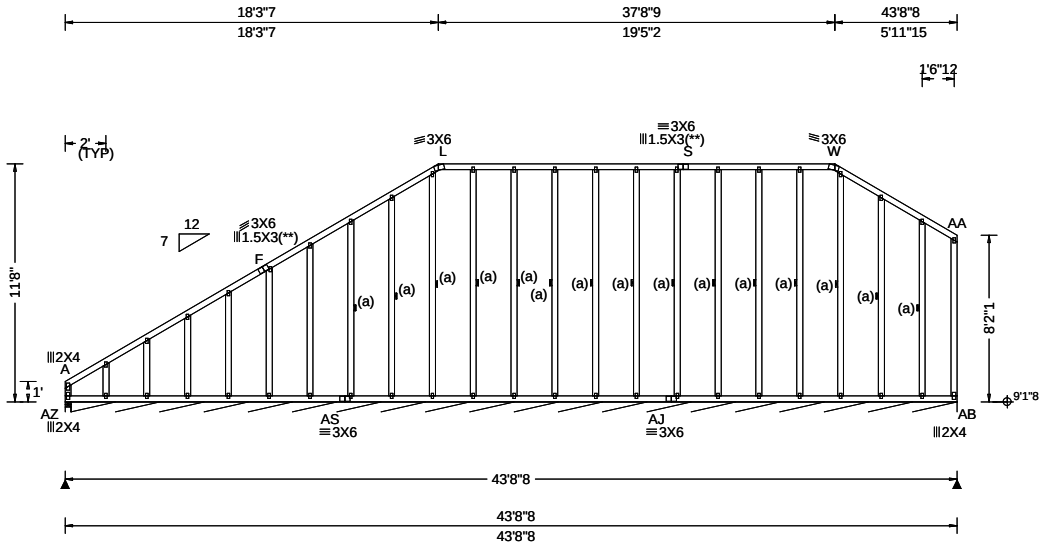
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4432 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** A1G	Cust: R 9836 JRef:1Y3T98360006 T8 DrwNo: 278.24.1027.30803 / YK 10/04/2024	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: 20.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 120 mph	Pf: 15.4 Ce: 1.0	VERT(LL): 0.002 L 999 240
BCLL: 0.00	Enclosure: Closed	Lu: - Cs: 1.00	VERT(CL): 0.004 R 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): -0.049 AA - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.088 AA - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	IRC 2021	Max TC CSI: 0.144
Load Duration: 1.15	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.144
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.809
	C&C Dist a: 4.37 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.04A.0207.13
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AZ	80	/-	/-	/237	/115	/269
AB*	82	/-	/-	/48	/-	/-

Wind reactions based on MWFRS
AZ Brg Wid = 3.5 Min Req = 1.5 (Truss)
AB Brg Wid = 521 Min Req = -
Bearings AZ & AZ are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Fasten rated sheathing to one face of this frame.

Plating Notes

All plates are 1.5X3 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss designed for unbalanced snow loads.

Wind

Wind loads based on MWFRS with additional C&C member design.

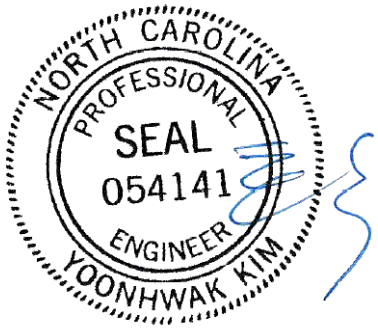
End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



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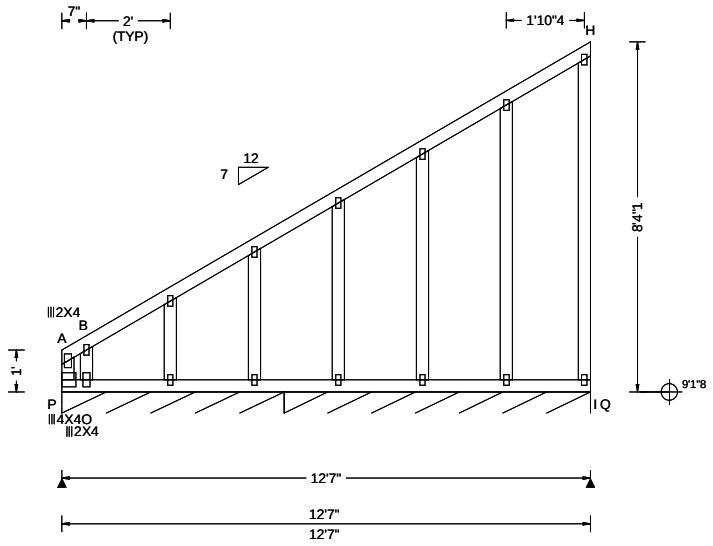
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4803 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** M1G		Cust: R 9836 JRef:1Y3T98360006 T44 DrwNo: 278.24.1026.18863 / YK 10/04/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: 20.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 120 mph	Pf: 15.4 Ce: 1.0	VERT(LL): 0.003 B 999 240
BCLL: 0.00	Enclosure: Closed	Lu: - Cs: 1.00	VERT(CL): 0.005 B 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): -0.043 H - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.061 H - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	IRC 2021	Max TC CSI: 0.171
Load Duration: 1.15	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.160
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.611
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	VIEW Ver: 23.02.04A.0207.13
	GCpi: 0.18	WAVE	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
P*	87	/-	/-	/48	/-	/40
Q*	80	/-	/-	/53	/2	/-

Wind reactions based on MWFRS
P Brg Wid = 63.5 Min Req = -
Q Brg Wid = 87.5 Min Req = -
Bearings P & M are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
A - B	121	-552

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.	Gables	Tens.	Comp.
A - P	125	-560	B - O	457	-138

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

Fasten rated sheathing to one face of this frame.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

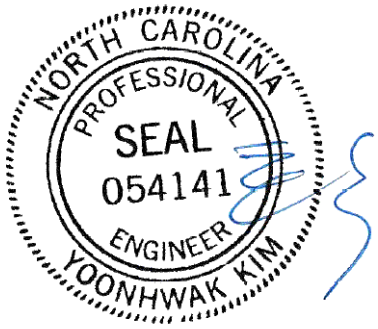
Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



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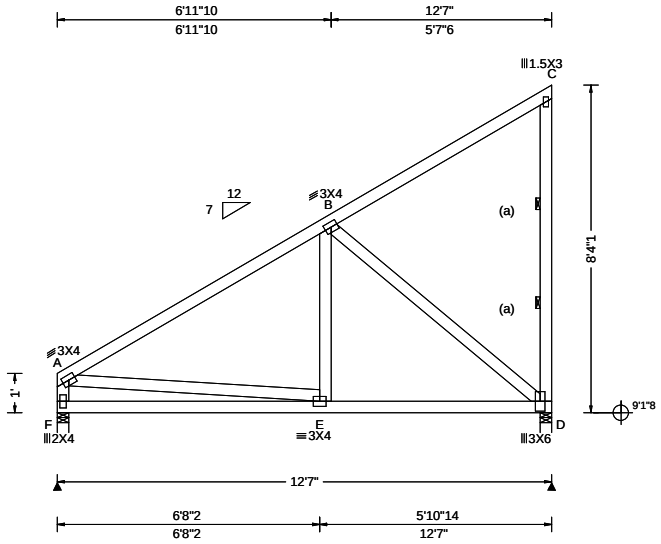
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4436 MONO FROM:	Ply: 1 Qty: 6	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** M1	Cust: R 9836 JRef:1Y3T98360006 T38 DrwNo: 278.24.1026.16317 / YK 10/04/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 1.00
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.008	E	999	240
VERT(CL): 0.018	E	999	240
HORZ(LL): -0.004	C	-	-
HORZ(TL): 0.008	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.816
Max BC CSI: 0.529
Max Web CSI: 0.999

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
F	523	/-	/-	/286	/-	/209
D	523	/-	/-	/353	/25	/-

Wind reactions based on MWFRS
F Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings F & D are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
A - B	103	- 591

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
F - E	164	- 464	E - D	426	- 245

Maximum Web Forces Per Ply (lbs)

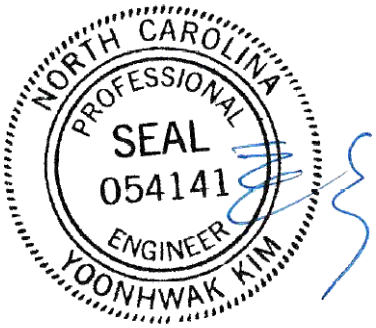
Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - F	118	- 468	B - D	214	- 553

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Loading
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/180.
Wind loading based on both gable and hip roof types.



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ABCD Engineering, PLLC NC COA 0838

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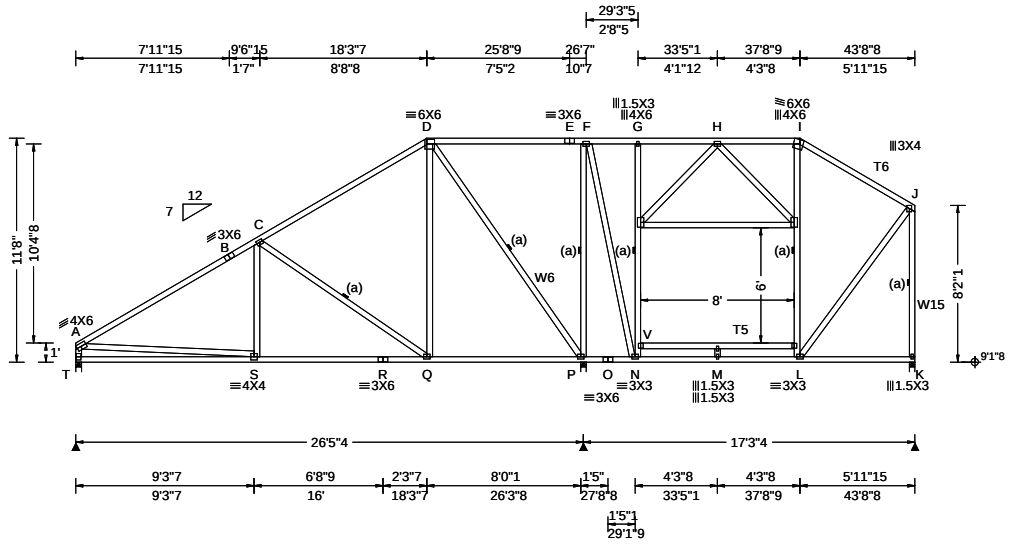
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4444 SPEC FROM:	Ply: 1 Qty: 6	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** A1	Cust: R 9836 JRef:1Y3T98360006 T24 DrwNo: 278.24.1025.40810 / YK 10/04/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 4.37 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 1.00
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.199 W	999	240
VERT(CL):	0.389 W	537	240
HORZ(LL):	-0.023 U	-	-
HORZ(TL):	0.045 U	-	-

Creep Factor: 2.0
Max TC CSI: 0.913
Max BC CSI: 0.864
Max Web CSI: 0.748

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
T	1204	/-	/-	/673	/-	/269
P	1766	/-	/-	/926	/-	/-
K	1061	/-	/-	/503	/-	/-

Wind reactions based on MWFRS
T Brg Wid = 3.5 Min Req = 1.5 (Truss)
P Brg Wid = 3.5 Min Req = 1.7 (Truss)
K Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings T, P, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	81	-1699	G - H	137	-450
B - C	89	-1446	H - I	137	-466
C - D	124	-1020	I - J	116	-616
F - G	137	-455			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
T - S	303	-389	Q - P	729	-104
S - R	1357	-249	N - M	449	-69
R - Q	1357	-249	M - L	449	-69

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - T	84	-1124	P - F	0	-1115
A - S	1157	0	F - N	685	0
C - Q	178	-769	N - V	0	-449
D - Q	664	-18	L - J	762	0
D - P	46	-748	J - K	0	-1036

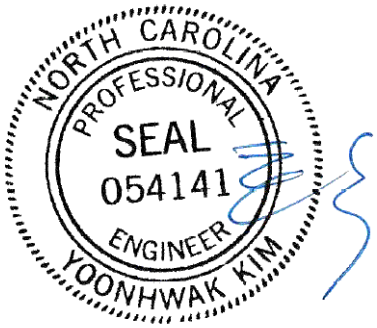
Lumber
Top chord: 2x4 SP SS; T5,T6 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W6,W15 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 3X4 except as noted.

Loading
Bottom chord checked for 10.00 psf non-concurrent live load.
Truss designed for unbalanced snow loads.
Truss supports 250# mech unit; unit centered at 33-6-13; supported by TC; unit width 4-0-0; supported by 6 trusses.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/180.
Wind loading based on both gable and hip roof types.
It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



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ABCD Engineering, PLLC NC COA 0838

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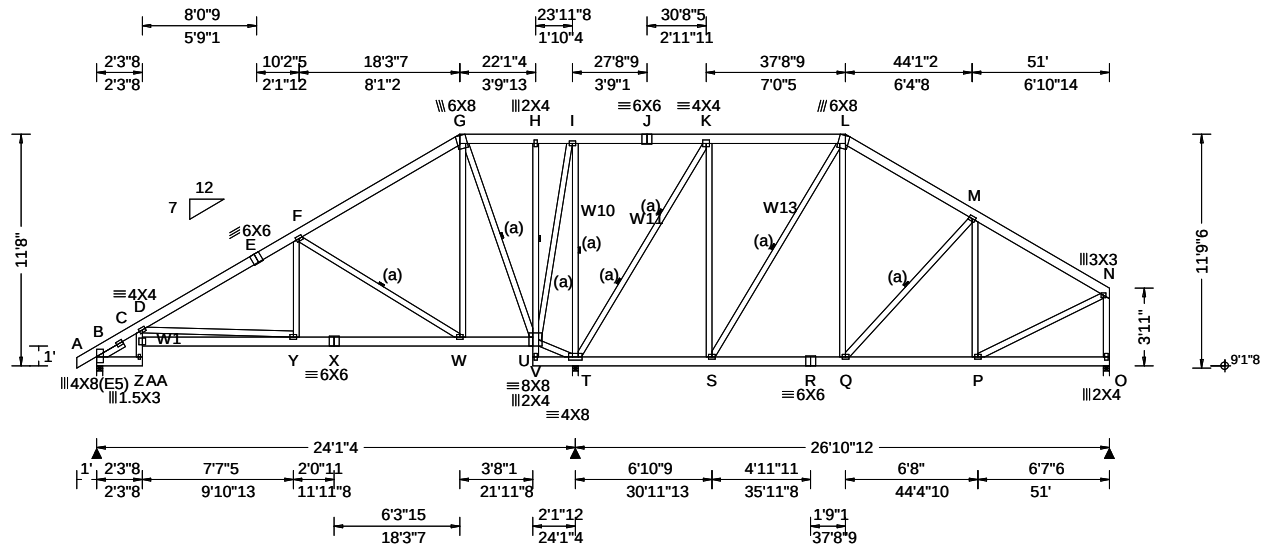
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4476 SPEC FROM:	Ply: 1 Qty: 2	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** A2T	Cust: R 9836 JRef:1Y3T98360006 T26 DrwNo: 278.24.1027.36327 / YK 10/04/2024	



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: 20.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 120 mph	Pf: 15.4 Ce: 1.0	VERT(LL): 0.064 D 999 240
BCLL:	0.00	Enclosure: Closed	Lu: - Cs: 1.00	VERT(CL): 0.133 D 999 240
BCDL:	10.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): 0.048 O - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL): 0.099 O - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	IRC 2021	Max TC CSI: 0.548
Load Duration:	1.15	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.419
Spacing:	24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.849
		C&C Dist a: 5.10 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: Any	Plate Type(s):	
		GCpi: 0.18	WAVE	VIEW Ver: 23.02.04A.0207.13
		Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	578	/-	/-	/324	/-	/228
T	3053	/-	/-	/1797	/-	/-
O	758	/-	/-	/521	/6	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
T Brg Wid = 3.5 Min Req = 3.2 (Truss)
O Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, T, & O are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	95	-647	H - I	901	-46
C - D	98	-603	I - J	1027	-37
D - E	59	-423	J - K	1027	-37
F - G	610	-107	L - M	151	-486
G - H	900	-46	M - N	77	-660

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B -AA	478	-89	W - U	216	-430
Z - Y	980	-166	Q - P	507	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - Y	179	-699	I - T	0	-993
Y - F	427	0	T - K	0	-1480
F - W	152	-842	K - S	816	-25
G - W	623	-17	S - L	108	-822
G - U	116	-1327	L - Q	461	-48
U - I	626	0	P - N	561	0
U - T	227	-1176	N - O	33	-703

Lumber

Top chord: 2x6 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #3; W1 2x4 SP SS; W10,W11, W13 2x4 SP #2;
Lt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss designed for unbalanced snow loads.

Wind

Wind loads based on MWFRS with additional C&C member design.

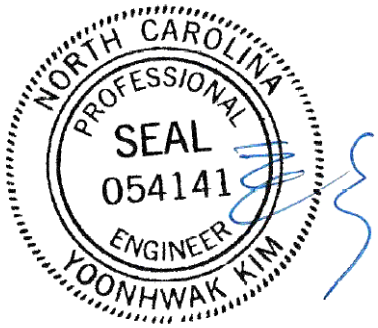
Right end vertical exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



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ABCD Engineering, PLLC NC COA 0838

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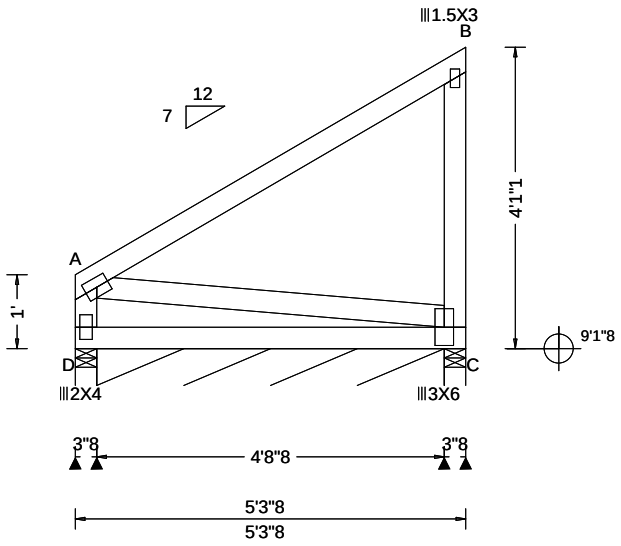
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4797 MONO FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** M2A	Cust: R 9836 JRef:1Y3T98360006 T46 DrwNo: 278.24.1026.23337 / YK 10/04/2024	



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 1.00
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	B	999	240
VERT(CL): 0.001	B	999	240
HORZ(LL): -0.001	B	-	-
HORZ(TL): 0.002	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.499
Max BC CSI: 0.066
Max Web CSI: 0.231

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D	188	/-	/-	/84	/6	/97
D*	27	/-	/-	/14	/-	/-
C	188	/-	/-	/121	/34	/-

Wind reactions based on MWFRS
D Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 56.5 Min Req = -
C Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings D, D, & C are a rigid surface.
Members not listed have forces less than 375#

Maximum Bot Chord Forces Per Ply (lbs)

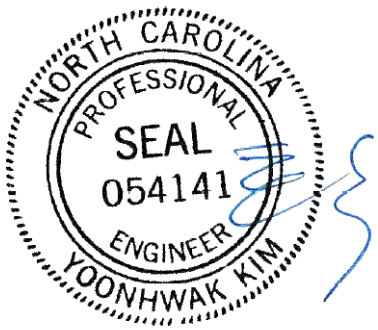
Chords	Tens.	Comp.
D - C	160	-587

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes
All plates are 3X4 except as noted.

Loading
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/180.
Wind loading based on both gable and hip roof types.



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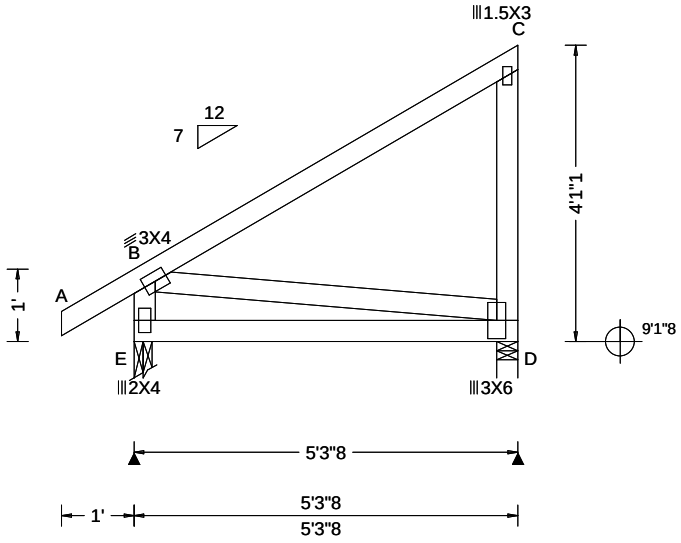
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4807 MONO FROM:	Ply: 1 Qty: 3	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** M2	Cust: R 9836 JRef:1Y3T98360006 T47 DrwNo: 278.24.1026.21000 / YK 10/04/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: 20.0 Ct: 1.1 CAT: II
Pf: 15.4 Ce: 1.0
Lu: - Cs: 1.00
Snow Duration: 1.15

Building Code:
IRC 2021
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.001 C	999	240
VERT(CL):	0.001 C	999	240
HORZ(LL):	-0.001 C	-	-
HORZ(TL):	0.002 C	-	-

Creep Factor: 2.0
Max TC CSI: 0.466
Max BC CSI: 0.299
Max Web CSI: 0.235

VIEW Ver: 23.02.04A.0207.13

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	294	/-	/-	/160	/-	/105
D	214	/-	/-	/150	/18	/-

Wind reactions based on MWFRS
E Brg Wid = 3.0 Min Req = 1.5 (Truss)
D Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings E & D are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

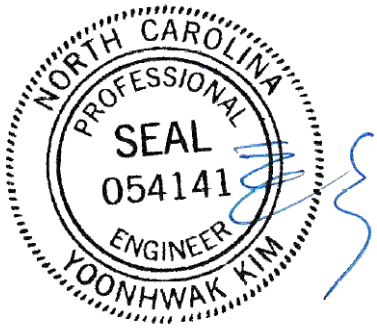
Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

Lanai/Porch Loading : 14.7 PLF wind pressure applied to the bottom chord of the truss from 0.00 ft to 5.29 ft,



10/04/2024
ABCD Engineering, PLLC NC COA 0838

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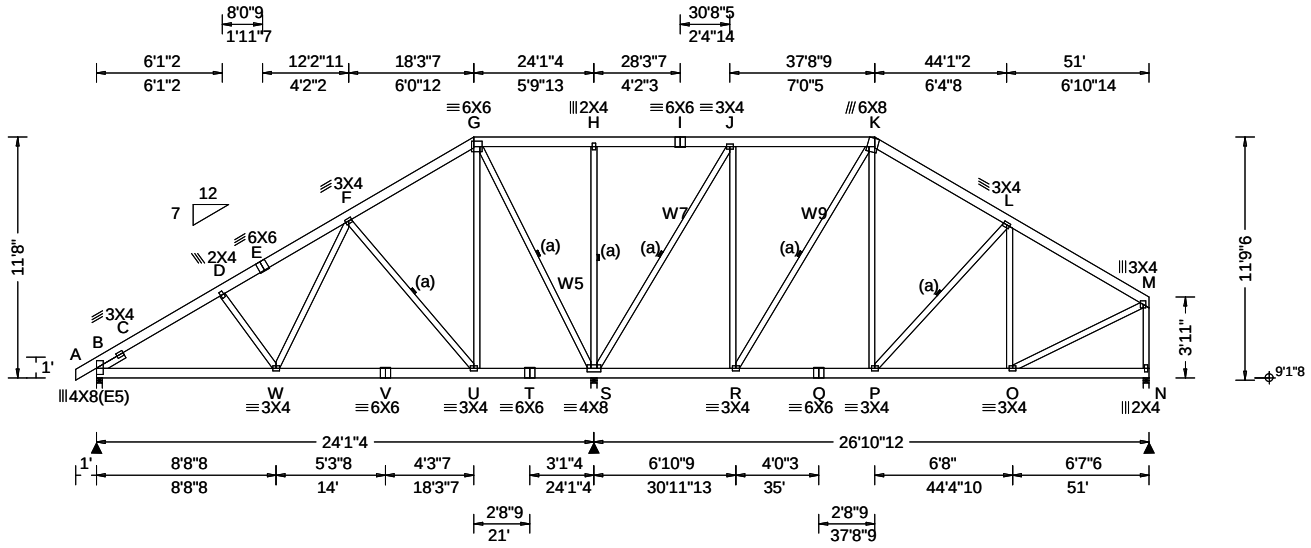
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 4480 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** Q2409-242 The Farm at Neills Creek **Truss Label:** A2	Cust: R 9836 JRef:1Y3T98360006 T25 DrwNo: 278.24.1027.34137 / YK 10/04/2024	



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: 20.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 120 mph	Pf: 15.4 Ce: 1.0	VERT(LL): 0.027 W 999 240
BCLL:	0.00	Enclosure: Closed	Lu: - Cs: 1.00	VERT(CL): 0.055 W 999 240
BCDL:	10.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): 0.011 N - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL): 0.022 N - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	IRC 2021	Max TC CSI: 0.308
Load Duration:	1.15	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.395
Spacing:	24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.886
		C&C Dist a: 5.10 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: Any	Plate Type(s):	VIEW Ver: 23.02.04A.0207.13
		GCpi: 0.18	WAVE	
		Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	883	/-	/-	/526	/-	/228
S	2475	/-	/-	/1371	/-	/-
N	948	/-	/-	/599	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
S Brg Wid = 3.5 Min Req = 2.5 (Truss)
N Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, S, & N are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	203	- 1253	E - F	125	- 857
C - D	96	- 1098	K - L	66	- 710
D - E	101	- 923	L - M	45	- 846

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - W	894	- 117	R - Q	533	0
W - V	516	- 51	Q - P	533	0
V - U	516	- 51	P - O	659	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
W - F	532	- 9	J - R	587	0
F - U	152	- 666	R - K	61	- 508
G - U	656	- 40	K - P	396	- 49
G - S	33	- 1010	O - M	729	0
H - S	0	- 507	M - N	0	- 893
S - J	0	- 1171			

Lumber

Top chord: 2x6 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #3; W5,W7,W9 2x4 SP #2;
Lt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss designed for unbalanced snow loads.

Wind

Wind loads based on MWFRS with additional C&C member design.

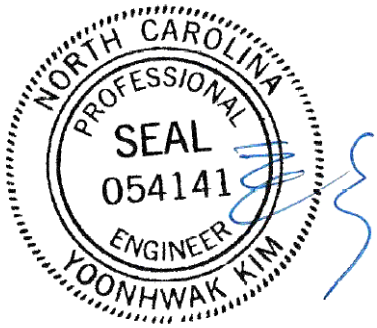
Right end vertical exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Gable Stud Reinforcement Detail

ASCE 7-16: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	7' 8"	8' 2"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	6' 7"	7' 0"	8' 10"	9' 5"	11' 5"	11' 10"	13' 10"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 0"	8' 4"	8' 7"	9' 10"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 8"	7' 0"	7' 5"	9' 3"	9' 11"	11' 5"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 8"	7' 0"	7' 5"	9' 3"	9' 11"	11' 5"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	6' 2"	6' 7"	8' 2"	8' 9"	11' 1"	11' 10"	12' 10"	13' 9"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	8' 1"	8' 7"	10' 10"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 9"	9' 6"	9' 10"	11' 3"	11' 8"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 5"	8' 6"	9' 1"	11' 0"	11' 5"	13' 1"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 5"	8' 6"	9' 1"	11' 0"	11' 5"	13' 1"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	7' 6"	8' 0"	10' 0"	10' 9"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	9' 4"	9' 11"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	6' 4"	10' 6"	10' 10"	12' 4"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 11"	9' 10"	10' 6"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 11"	9' 10"	10' 6"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	8' 8"	9' 3"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

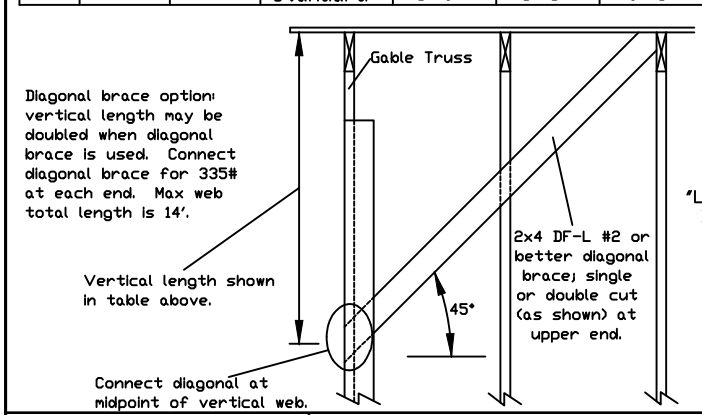
Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

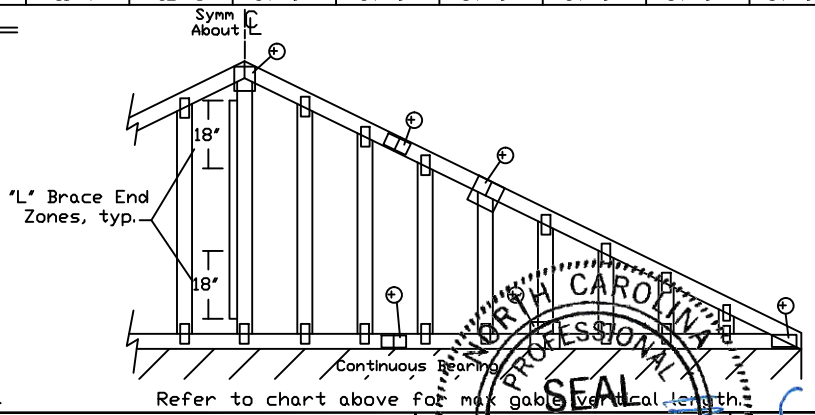
Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) 'L' braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.





Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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For more information see this job's general notes page and these web sites:
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NORTH CAROLINA PROFESSIONAL SEAL 054141 ENGINEER YOONHWAK KIM

ABCD Engineering, PLLC
10/04/2024

MAX. TOT. LD. 60 PSF
NC COA 0838
MAX. SPACING 24.0"

REF ASCE7-16-GAB12015
DATE 01/26/2018
DRWG A12015ENC160118

Gable Stud Reinforcement Detail

ASCE 7-16: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 Mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 100 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 4"	7' 2"	7' 7"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 4"	6' 2"	6' 7"	8' 2"	8' 9"	10' 10"	11' 4"	12' 10"	13' 9"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 11"	8' 2"	9' 4"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	6' 6"	6' 11"	8' 7"	9' 2"	10' 11"	11' 4"	13' 6"	14' 0"	14' 0"	14' 0"
		Stud	4' 6"	6' 6"	6' 11"	8' 7"	9' 2"	10' 11"	11' 4"	13' 6"	14' 0"	14' 0"	14' 0"
		Standard	4' 4"	5' 9"	6' 1"	7' 7"	8' 2"	10' 4"	11' 1"	11' 11"	12' 10"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 10"	9' 3"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	8' 9"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	7' 6"	8' 0"	10' 1"	10' 9"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 6"	9' 1"	9' 5"	10' 8"	11' 1"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	7' 11"	8' 5"	10' 6"	10' 11"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	7' 11"	8' 5"	10' 6"	10' 11"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	7' 0"	7' 5"	9' 4"	10' 0"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	8' 8"	9' 3"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	6' 0"	10' 0"	10' 4"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 8"	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 8"	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	8' 1"	8' 7"	10' 9"	11' 6"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 70 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

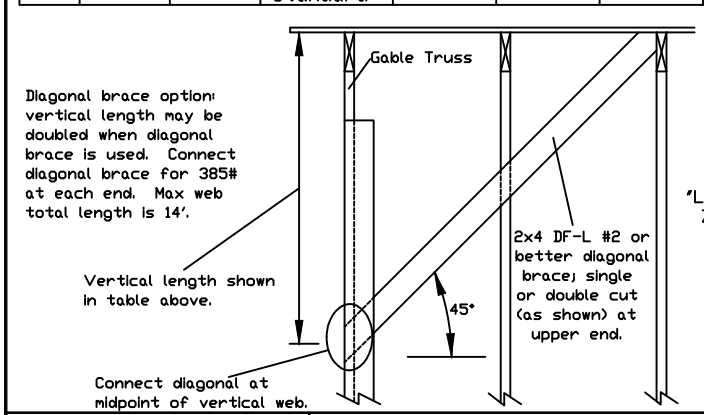
** For (2) 'L' braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

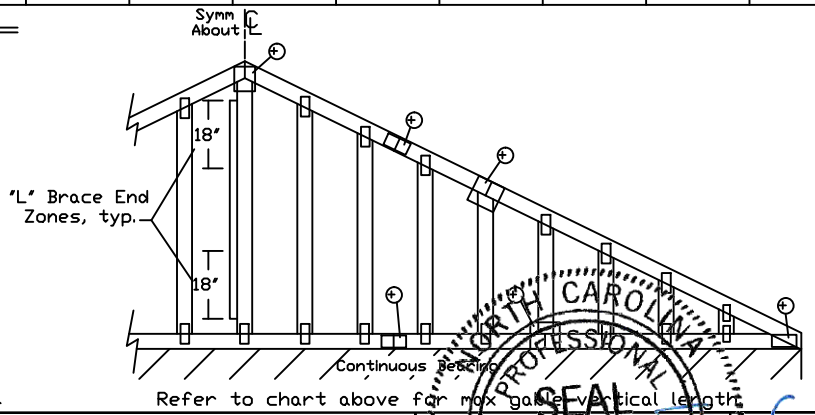
'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





Refer to chart above for max gable vertical length.

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NORTH CAROLINA PROFESSIONAL SEAL 054141 ENGINEER YOONHWAK KIM

ABCD Engineering, PLLC NC COA 0838

10/04/2024

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-16-GAB12030
DATE 01/26/2018
DRWG A12030ENC160118

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

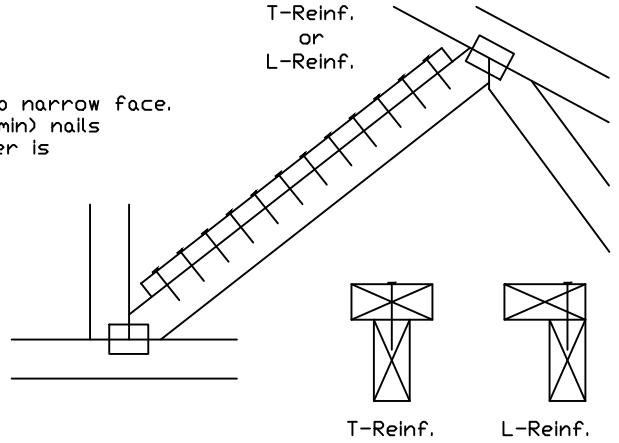
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Alternative Reinforecement Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

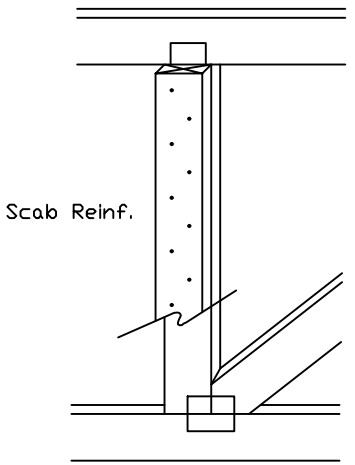
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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ABCD Engineering, PLLC NC COA 0838

10/04/2024

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TC LL		PSF
TC DL		PSF
BC DL		PSF
BC LL		PSF
TOT. LD.		PSF
DUR. FAC.		
SPACING		

REF	CLR Subst.
DATE	01/02/19
DRWG	BRCLBSUB0119

ASCE 7-16: 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

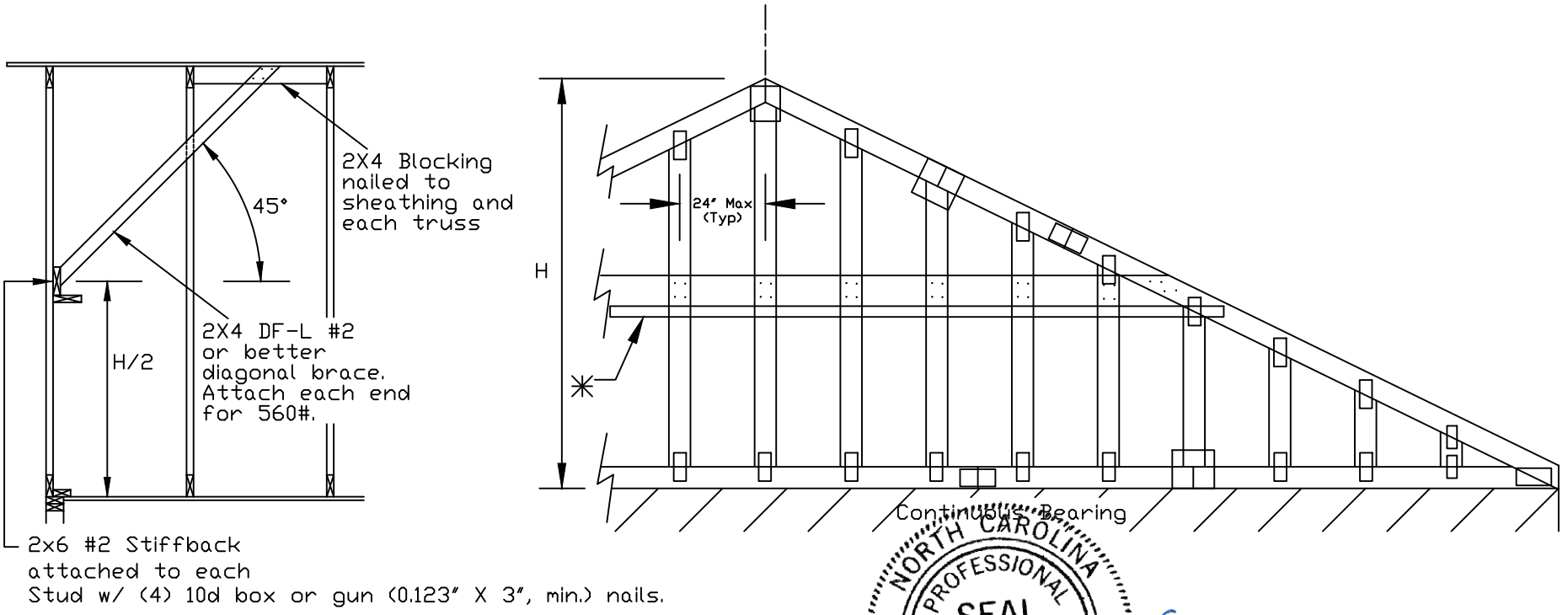
Nails: 10d box or gun (0.128"x3",min) nails.

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC160118).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC160118).

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.



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10/04/2024

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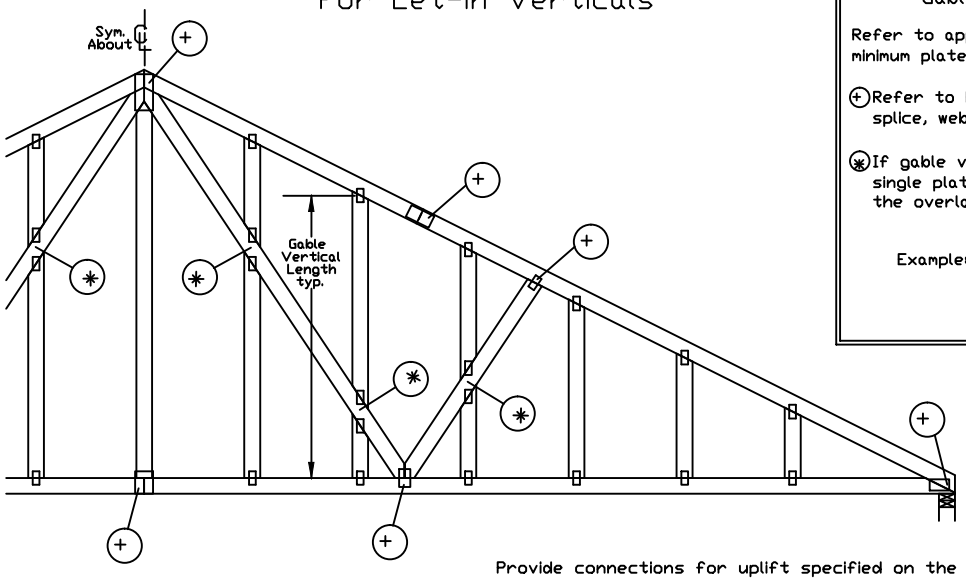
MAX. TOT. LD. 60 PSF

MAX. SPACING

REF	GE WHALER
DATE	01/02/2018
DRWG	GABRST160118

Gable Detail
For Let-in Verticals



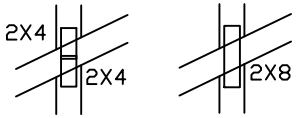
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

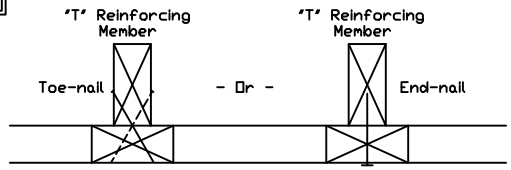
⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊛ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



"T" Reinforcement Attachment Detail



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
10d Common (0.148"x 3.",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

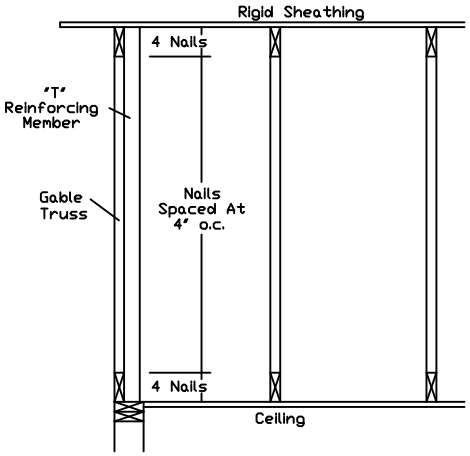
Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118, A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118, S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118, S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

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NORTH CAROLINA PROFESSIONAL SEAL 054141 ENGINEER YOONHWAK KIM

ABCD Engineering, PLLC NC COA 10638

10/04/2024

MAX. TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX. SPACING	24.0"

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

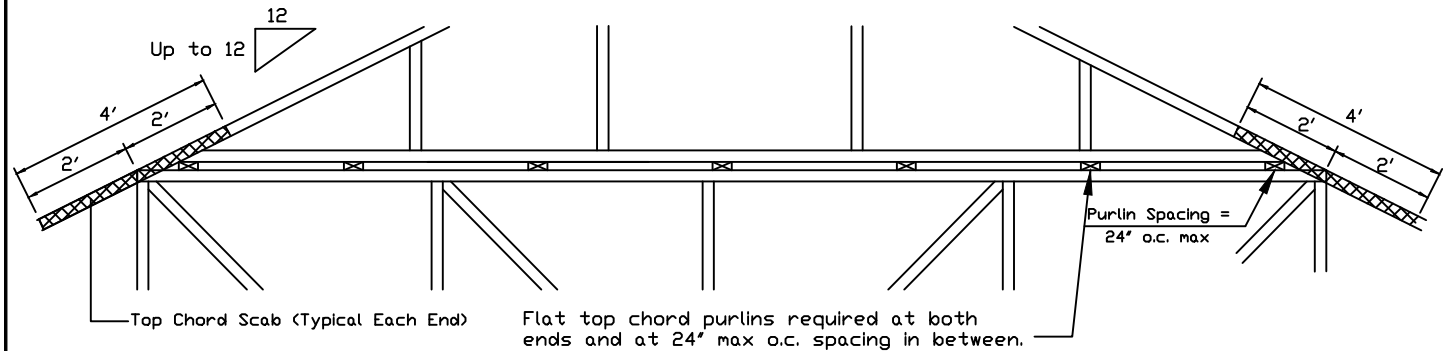
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

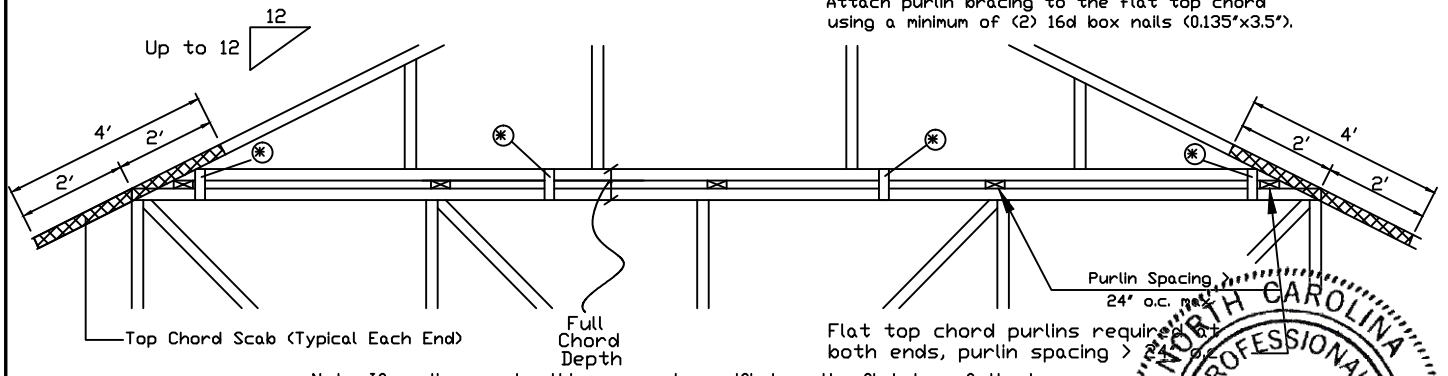
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24" o.c. max. and use Detail A.

* In addition, provide connection with one of the following methods:

Trulox
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate
One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

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NORTH CAROLINA PROFESSIONAL SEAL 054141 ENGINEER YOONHWAK KIM

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10/04/2024

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118
SPACING	24.0"

Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
(2) 16d box (0.135" x 3.5") nails toe-nailed for
ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C,
Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
supporting truss material at connection location:
170 mph for SP (G = 0.55, min.),
155 mph for DF-L (G = 0.50, min.), or
120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

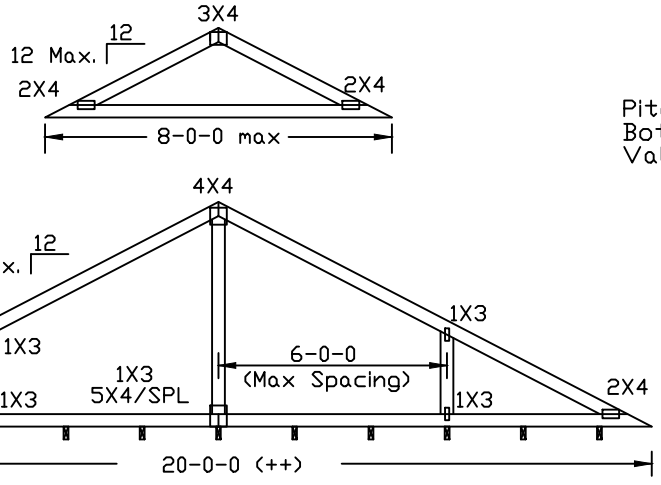
All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

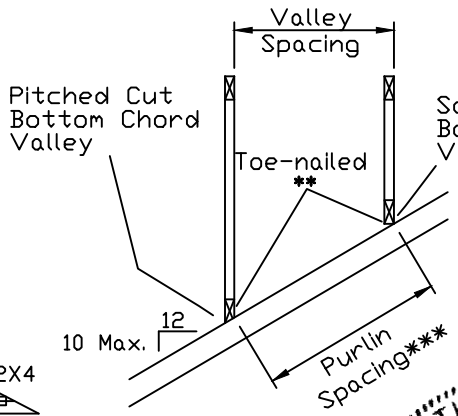
Top chord of truss beneath valley set must be braced with:
properly attached, rated sheathing applied prior to valley truss installation.
Or
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
Or
By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

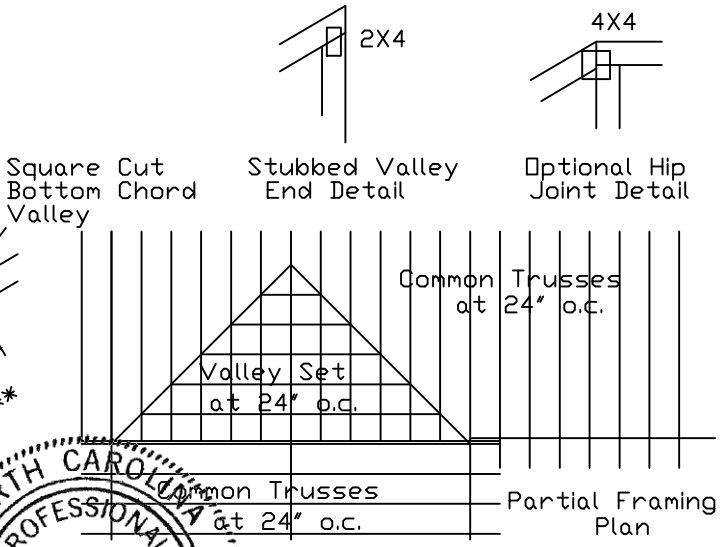
*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



Supporting trusses at 24" o.c. maximum spacing.





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IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

ABCD Engineering, PLLC NC C-4035

10/04/2024

TC LL	30	30	40	PSF
TC DL	20	15	7	PSF
BC DL	10	10	10	PSF
BC LL	0	0	0	PSF
TOT. LD.	60	55	57	PSF
DURATION	1.25	1.33	1.15	1.15
SPACING			24.0"	

REF	VALLEY DETAIL
DATE	01/26/2018
DRWG	VALTN160118