Mark Morris, P.E. #126, 1317-M, Summerville, SC 29483

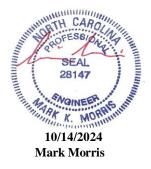
843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

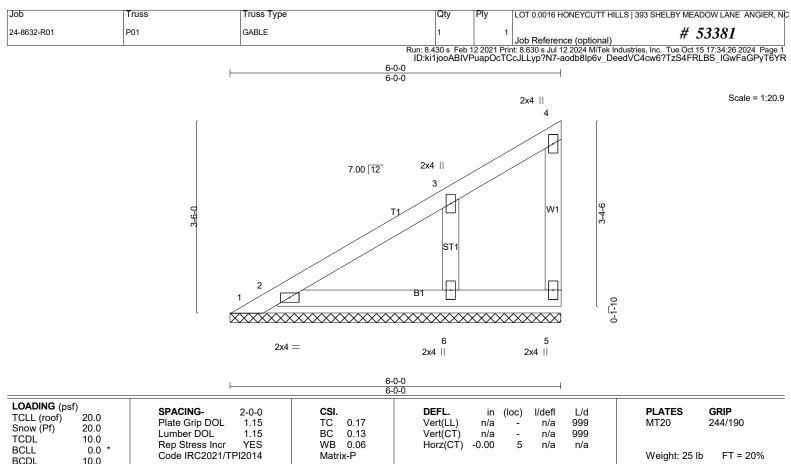
AST #: 53381 JOB: 24-8632-R01 JOB NAME: LOT 0.0016 HONEYCUTT HILLS Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2018 as well as IRC 2021. 25 Truss Design(s)

Trusses:

P01, P02, R01, R02, R03, R04, R05, R06, R07, R08, R10, R11, R12, R15, R16, R17, R18, R19, R20, SP01, SP02, V10, V11, V12, V13



Warning !-- Verify design parameters and read notes before use.



BOBE 10:0		
LUMBER-	BRACING-	
TOP CHORD 2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except
BOT CHORD 2x4 SP No.3		end verticals.
WEBS 2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS 2x4 SP No.3		MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 6-0-0.

(lb) - Max Horz 1=93(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 5, 2, 6 except 1=-112(LC 21)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 2=338(LC 21), 6=339(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WFBS 3-6=-273/121

NOTES-(12)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit HUMBER CAR 9) Bearing at joint(s) 1, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity in CARO of bearing surface.
- ROFESS
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2, 6 except (Feb) 1=112.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard

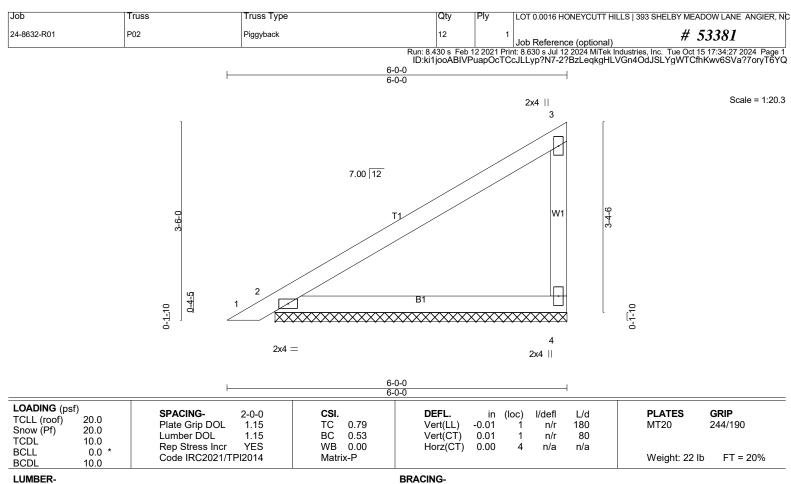
MORADINAL TA/20, and Warning !---Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, guality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

SEAL

28147

ARK K. MORAL VOINEE

10/14/2024



TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3

WFBS 2x4 SP No.3 Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS. (lb/size) 4=198/5-1-13 (min. 0-1-8), 2=236/5-1-13 (min. 0-1-8) Max Horz 2=92(LC 11) Max Uplift4=-39(LC 14), 2=-22(LC 14) Max Grav 4=289(LC 21), 2=326(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

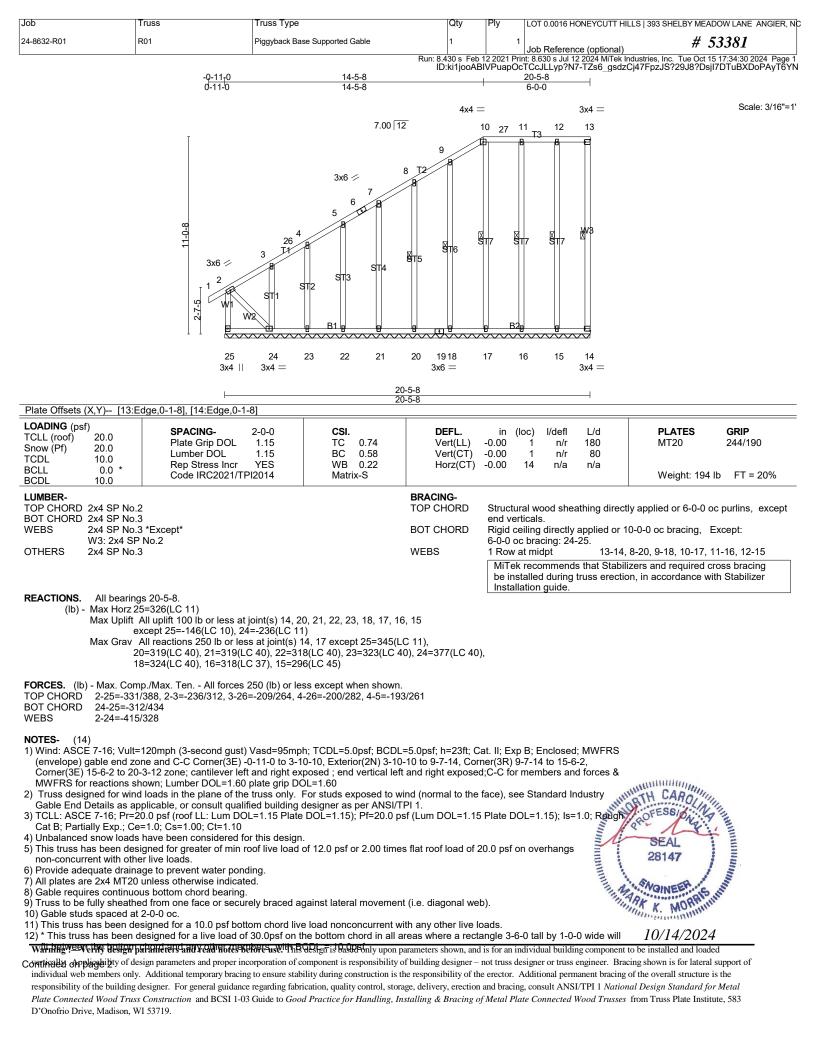
- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads.

- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 7) between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2. 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard





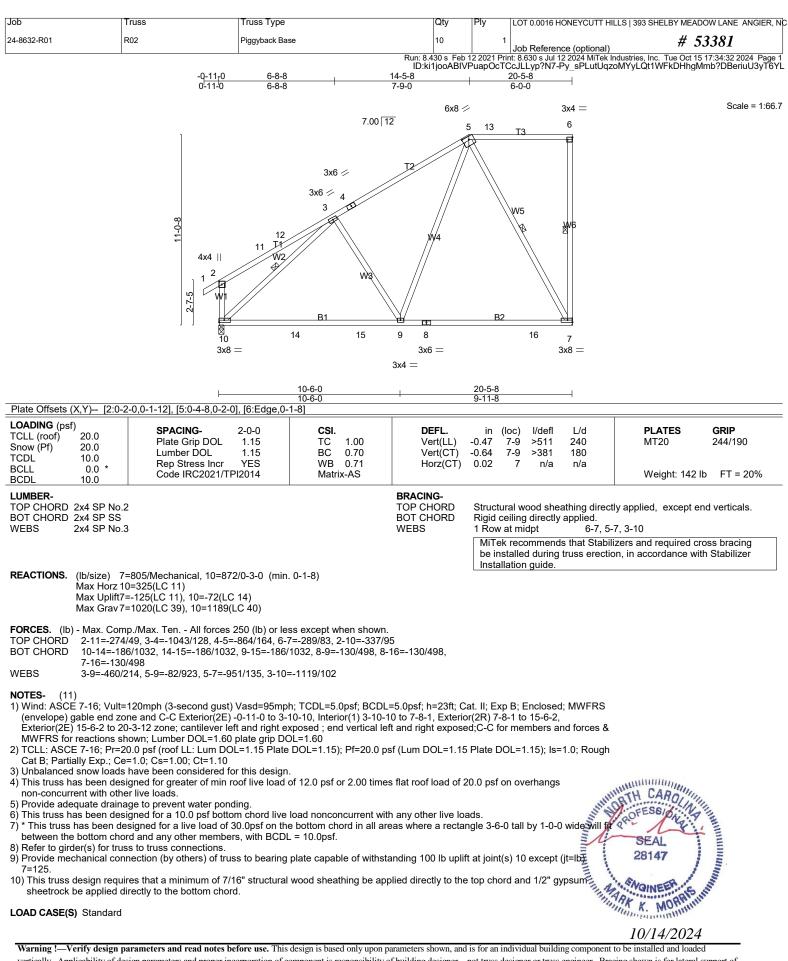
Job	Truss	Truss Type	Qty	Ply	LOT 0.0016 HONEYCUTT HILLS 393 SHELBY MEADOW LANE ANGIER, NC	
24-8632-R01	R01	Piggyback Base Supported Gable	1	1	Job Reference (optional) # 53381	
Run: 8.430 s Feb 12 2021 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 15 17:34:30 2024 Page 2 ID:ki1jooABIVPuapOcTCcJLLyp?N7-TZs6_gsdzCj47FpzJS?29J8?DsjI7DTuBXDoPAyT6YN						

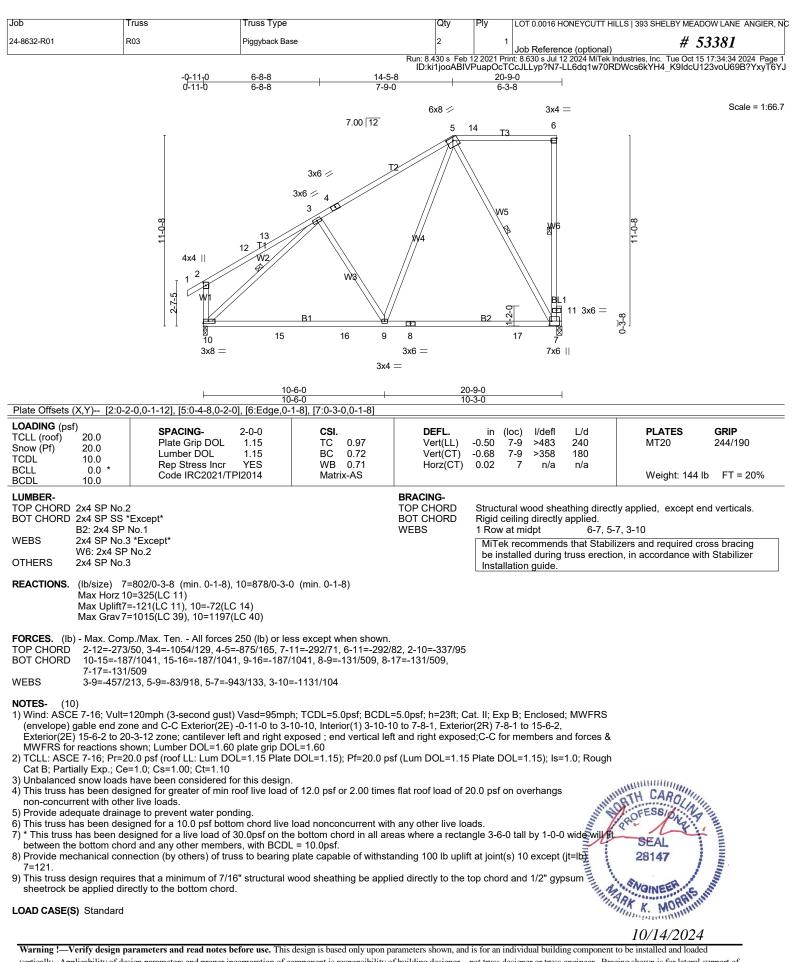
NOTES- (14)

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14, 20, 21, 22, 23, 18, 17, 16, 15 except (jt=lb) 25=146, 24=236.

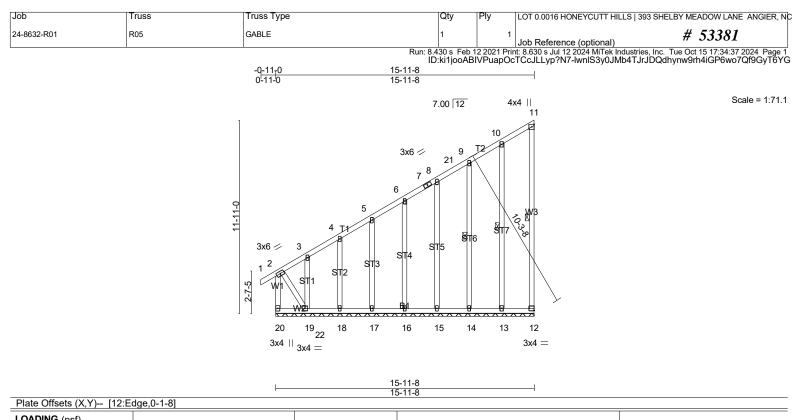
LOAD CASE(S) Standard







	Truss	Truss Type		Qty	Ply	LOT 0.0016 HONEYCUTT	HILLS 393 SHELBY MEADOW LANE ANGIER, NC
24-8632-R01	R04	Monopitch		7 Run: 8.430 s Feb	1 12 2021 Prir	Job Reference (optional t: 8.630 s Jul 12 2024 MiTel	Industries, Inc. Tue Oct 15 17:34:35 2024 Page 1
		-0 <u>-11₋0</u> 0-11-0	<u>7-11-8</u> 7-11-8	15	VPuapOc⊺ <u>∙11-8</u> ∙0-0	「CcJLLyp?N7-pXg?1Nw ─────	InILMD0hw5?bDsMrq0tLAoPQdLpxZ4NyŤ6YI
		0-11-0	7-11-0			4x6 🖉	Scale = 1:65.9
	т			7.00 12	_	5	
	0		2x4	3 11	12 V4	W5	
		5x5 = 1 2 W1 8	9 TH W2	W3 7 11		6 6	
		3x4		4x8 =		3x4 =	
			7-11-8 7-11-8		-11-8 -0-0		
Plate Offsets (X,Y) [2:0-2	2-0,0-1-12], [5:0-2-14,0-2	2-0], [6:Edge,0-1-8					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC 0.86 BC 0.85 WB 0.55	DEFL. Vert(LL) Vert(CT) Horz(CT)		6-7 >641 240 6-7 >493 180	PLATES GRIP MT20 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2021/T		Matrix-AS		-0.01	6 n/a n/a	Weight: 115 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No.: BOT CHORD 2x4 SP No.: WEBS 2x4 SP No.: W5: 2x4 SP	2 3 *Except*			BRACING- TOP CHORD BOT CHORD WEBS	Rigid cei 1 Row a MiTek be insta	iling directly applied. t midpt 5-6, 4 recommends that Stab alled during truss erec	ctly applied, except end verticals. 5-7 ilizers and required cross bracing tion, in accordance with Stabilizer
Max Hórz 8 Max Uplift6	=625/Mechanical, 8=693 =348(LC 11) =-143(LC 14), 8=-39(LC =865(LC 24), 8=718(LC	14)	3)		Installa	tion guide.	
FORCES. (lb) - Max. Com TOP CHORD 2-9=-660/a 2-8=-644/ BOT CHORD 7-8=-329/a WEBS 3-7=-595/a	84, 3-9=-566/104, 3-4=-6 137	\$93/198, 4-10=-59		34, 5-6=-698/188,			
cantilever left and right e Lumber DOL=1.60 plate 2) TCLL: ASCE 7-16; Pr=2	one and C-C Exterior(2E exposed ; end vertical le grip DOL=1.60 20.0 psf (roof LL: Lum DO) -0-11-0 to 3-10-1 ft and right expose	0, Interior(1) 3-10-1 d;C-C for members	0 to 11-0-2, Exteri and forces & MW	or(2E) 11 FRS for r	-0-2 to 15-9-12 zone; eactions shown;	
Cat B; Partially Exp.; Ce 3) Unbalanced snow loads 4) This truss has been des non-concurrent with othe 5) This truss has been des 6) * This truss has been de between the bottom cho 7) Refer to girder(s) for trus 8) Provide mechanical com 9) This truss design require	⇒=1.0; Cs=1.00; Ct=1.10 have been considered 1 igned for greater of min r live loads. igned for a 10.0 psf bott basigned for a live load of rd and any other membe ss to truss connections. nection (by others) of tru	or this design. roof live load of 12 om chord live load 30.0psf on the bo rrs, with BCDL = 1 iss to bearing plat 6" structural wood	2.0 psf or 2.00 times nonconcurrent with ttom chord in all are 0.0psf. e capable of withsta	flat roof load of 2 any other live loa as where a rectar nding 100 lb uplifi	0.0 psf or ds. gle 3-6-0 : at joint(s	overhangs	
						0	K. MORINI
							10/14/2024



	uge,0-1-0j							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.84 BC 0.65 WB 0.16 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.00 1-2 0.00 1-2 -0.00 12	n/r 180 n/r 80	PLATES GRIP MT20 244/190 Weight: 147 lb FT = 20%		
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 *Except* W3: 2x4 SP No.2 BRACING- TOP CHORD OTHERS 2x4 SP No.3 *Except* W3: 2x4 SP No.2 BOT CHORD OTHERS 2x4 SP No.3 REACTIONS. All bearings 15-11-8. (ib) - Max Horz 20=348(LC 11) Max Uplift All uplift 100 ib or less at joint(s) 12, 13, 14, 15, 16, 17, 18 except 20=-230(LC 12), 19=-340(LC 11) WEBS Max Grav All reactions 250 ib or less at joint(s) 12 except 20=463(LC 11), 13=301(LC 5), 14=275(LC 5), 15=251(LC 24), 16=253(LC 24), 17=251(LC 24), 18=256(LC 24), 19=333(LC 24) Minexters								
	, /548, 2-3=-259/343, 3-4=-238/322, 4- 5/460		74, 7-8=-173/251					
 (envelope) gable end zo cantilever left and right e Lumber DOL=1.60 plate 2) Truss designed for wind Gable End Details as ap 3) TCLL: ASCE 7-16; Pr=2 Cat B; Partially Exp.; Ce 4) Unbalanced snow loads 5) This truss has been des non-concurrent with othe 6) All plates are 2x4 MT20 7) Gable requires continuo 8) Truss to be fully sheathe 9) Gable studs spaced at 2 10) This truss has been des 11) * This truss has been des 12) This truss has been des 13) * This truss has been des 14) * This truss has been des 15) * This truss has been des 16) * Centification * This truss has been des 17) * This truss has been des 18) * Centification * This truss has been des 11) * This truss has been des 11) * This truss has been des 12) * This truss has been des 13) * This truss has been des 14) * This truss has been des 15) * This truss has been des 16) * This truss has been des 17) * This truss has been des 18) * Centification * This truss has been des 11) * This truss has been des 12) * This truss has been des 13) * This truss has been des 14) * This truss has been des 15) * This truss has been des 16) * This truss has been des 17) * This truss has been des 18) * This truss has been des 19) * This truss has been des 11) * This truss has been des 12) * This truss has been des 13) * This truss has been des 14) * This truss has been des 15) * This truss has been des 16) * This truss has been des 17) * This truss has been des 18) * This truss has been des <l< td=""><td>unless otherwise indicated. us bottom chord bearing. d from one face or securely braced as 2-0-0 oc. signed for a 10.0 psf bottom chord live lesigned for a live load of 30.0psf on the chord and any other members, with B presented Solution Constitution Solution 30, 19=340. 30, 19=340. 30, 19=340. 30 and 30 a</td><td>1-8, Exterior (2N) 3-11-8 posed;C-C for members For studs exposed to wi designer as per ANSI/TI DOL=1.15); Pf=20.0 ps of 12.0 psf or 2.00 times gainst lateral movement e load nonconcurrent wi he bottom chord in all at CDL = 10.0psf.</td><td>B to 11-0-2, Cornes and forces & MV ind (normal to the PI 1. sf (Lum DOL=1.15 s flat roof load of 2 t (i.e. diagonal we ith any other live I reas where a rect translites 100 km sinc f building designer – he responsibility of th</td><td>er(3E) 11-0-2 f WFRS for read face), see St 5 Plate DOL= 20.0 psf on ov 20.0 psf</td><td>to 15-9-12 zone; ctions shown; tandard Industry 1.15); Is=1.0; Rough verhangs all by 1-0-0 wide will deal bailding tomponeh ner or truss engineer. Bra itional permanent bracing</td><td>10/14/2024 to be installed and loaded acing shown is for lateral support of g of the overall structure is the</td></l<>	unless otherwise indicated. us bottom chord bearing. d from one face or securely braced as 2-0-0 oc. signed for a 10.0 psf bottom chord live lesigned for a live load of 30.0psf on the chord and any other members, with B presented Solution Constitution Solution 30 , 19=340. 30 , 19=340. 30 , 19=340. 30 and 30 a	1-8, Exterior (2N) 3-11-8 posed;C-C for members For studs exposed to wi designer as per ANSI/TI DOL=1.15); Pf=20.0 ps of 12.0 psf or 2.00 times gainst lateral movement e load nonconcurrent wi he bottom chord in all at CDL = 10.0psf.	B to 11-0-2, Cornes and forces & MV ind (normal to the PI 1. sf (Lum DOL=1.15 s flat roof load of 2 t (i.e. diagonal we ith any other live I reas where a rect translites 100 km sinc f building designer – he responsibility of th	er(3E) 11-0-2 f WFRS for read face), see St 5 Plate DOL= 20.0 psf on ov 20.0 psf	to 15-9-12 zone; ctions shown; tandard Industry 1.15); Is=1.0; Rough verhangs all by 1-0-0 wide will deal bailding tomponeh ner or truss engineer. Bra itional permanent bracing	10/14/2024 to be installed and loaded acing shown is for lateral support of g of the overall structure is the		
responsibility of the building	lesigner. For general guidance regarding fabr	ication, quality control, stora	ge, delivery, erection	n and bracing, co	onsult ANSI/TPI 1 Natio	nal Design Standard for Metal		

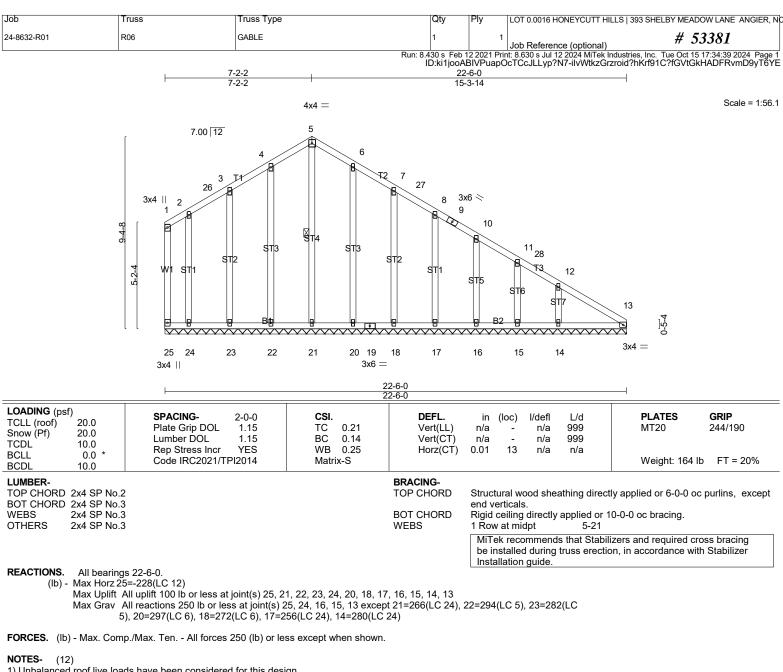
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Job	Truss	Truss Type	Qty	Ply	LOT 0.0016 HONEYCUTT HILLS 393 SH	ELBY MEADOW LANE ANGIER, NC
24-8632-R01	R05	GABLE	1	1	Job Reference (optional)	# 53381
Run: 8 430 s Feb 12 2021 Print: 8 630 s Jul 12 2024 MiTek Industries Inc. Tup Oct 15 17:34:37 2024 Page 2						

Run: 8.430 s Feb 12 2021 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 15 17:34:37 2024 Page 2 ID:ki1jooABIVPuapOcTCcJLLyp?N7-lwnlS3y0JMb4TJrJDQdhynw9rh4iGP6wo7Qf9GyT6YG

LOAD CASE(S) Standard





- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) 0-1-12 to 5-2-2, Corner(3R) 5-2-2 to 11-11-12, Exterior(2N) 11-11-12 to 17-8-6, Corner(3E) 17-8-6 to 22-6-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough
- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 5) Unbalanced snow loads have been considered for this design.
- 6) All plates are 2x4 MT20 unless otherwise indicated. Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will
 - fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 21, 22, 23, 24, 20 Annun ARARX , 18, 17, 16, 15, 14, 13.

LOAD CASE(S) Standard

MORPHS MILLING Warning !---Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

STITUTE TH CAROL

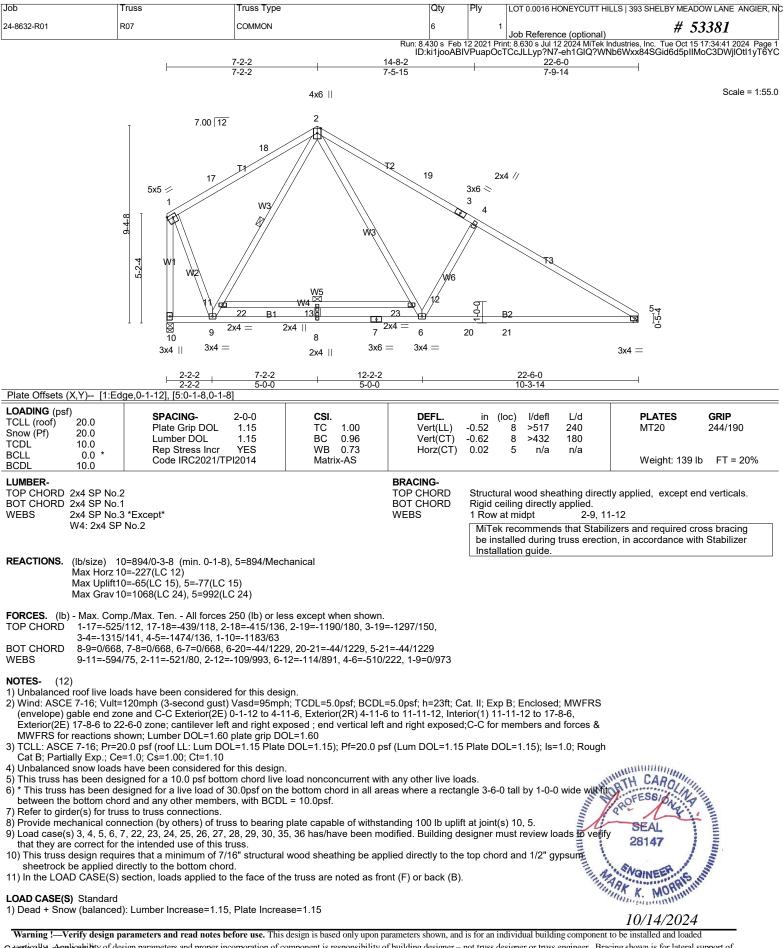
ROFESS

SEAL

28147

NOINEE ARK K. MORRI

10/14/2024



Contrivable on programmeters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0016 HONEYCUTT HILLS 393 SH	HELBY MEADOW LANE ANGIER, N
24-8632-R01	R07	COMMON	6		1 Job Reference (optional)	# 53381
					Print: 8.630 s Jul 12 2024 MiTek Industries, In OcTCcJLLyp?N7-6tbeVm088uENZ5jG0	
			,			
LOAD CASE(S) S Uniform Loads						
	(pii) 2=-60, 2-5=-60, 10-14=-2	20				
		5 Uninhab. Attic Storage: Lumber Increas	se=1.15 Plate Increa	se=1 15		
Uniform Loads		o onininab. Allio olorage. Euriber inoreal		30-1.10		
		20, 20-21=-65, 14-21=-20, 22-23=-45(F)				
		ninhab. Attic Storage: Lumber Increase=1	.15. Plate Increase=	1.15		
Uniform Loads		5	,	-		
		20, 20-21=-65, 14-21=-20, 22-23=-45(F)				
		Uninhab. Attic Storage: Lumber Increase	=1.15, Plate Increase	=1.15		
Uniform Loads	(plf)	2				
		29, 10-20=-20, 20-21=-65, 14-21=-20, 22				
6) Dead + 0.75 Sr	ow (Unbal. Right) + 0.75	5 Uninhab. Attic Storage: Lumber Increas	e=1.15, Plate Increas	se=1.15		
Uniform Loads						
		50, 10-20=-20, 20-21=-65, 14-21=-20, 22				
		rage: Lumber Increase=1.25, Plate Increa	ase=1.25			
Uniform Loads						
	2=-20, 2-5=-20, 10-14=-4		-			
		umber Increase=1.25, Plate Increase=1.2	.5			
Uniform Loads						
		-20, 20-21=-80, 14-21=-20, 22-23=-60(F)			4.00 Dista la successión d.00	
		ab. Attic Storage + 0.75(0.6 MWFRS Win	a (Neg. Int) Leπ): Lur	mber incre	ease=1.60, Plate Increase=1.60	
Uniform Loads		-20, 20-21=-65, 14-21=-20, 22-23=-45(F)				
	-256, 2-544, 10-20- 1-2=8, 2-5=6, 1-10=16	-20, 20-2105, 14-2120, 22-2545(F)				
		ab. Attic Storage + 0.75(0.6 MWFRS Win	d (Neg. Int) Pight): L	umber Inc	rease=1.60. Plate Increase=1.60	
Uniform Loads		ab. Allic Stolage + 0.75(0.0 MWI NS WIII			lease - 1.00, Flate Increase - 1.00	
		-20, 20-21=-65, 14-21=-20, 22-23=-45(F)				
	1-2=-4, 2-5=-8, 1-10=-5	20, 20 21-00, 14-21-20, 22-20-40(1)	1			
		ab. Attic Storage + 0.75(0.6 MWFRS Win	d (Neg. Int) 1st Paral	lel): Lumb	per Increase=1.60. Plate Increase=1	.60
Uniform Loads			(,,	,		
		-20, 20-21=-65, 14-21=-20, 22-23=-45(F))			
	1-2=-16, 2-5=6, 1-10=14					
26) Dead + 0.75 S	now (bal.) + 0.75 Uninha	ab. Attic Storage + 0.75(0.6 MWFRS Win	d (Neg. Int) 2nd Para	allel): Lum	ber Increase=1.60, Plate Increase=	1.60
Uniform Loads						
		-20, 20-21=-65, 14-21=-20, 22-23=-45(F))			
	1-2=-6, 2-5=16, 1-10=-5					
(17) Deed + 0.75 D	loof Live (bol) IO 75 Llm	inhoh Attic Storage + 0.75/0.6 MM/EDS	Wind (Nog Int) off)	Lumber	Increase-1.60 Plate Increase-1.60	

27) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-58, 2-5=-44, 10-20=-20, 20-21=-65, 14-21=-20, 22-23=-45(F) Horz: 1-2=8, 2-5=6, 1-10=16

28) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-44, 2-5=-58, 10-20=-20, 20-21=-65, 14-21=-20, 22-23=-45(F)

Horz: 1-2=-6, 2-5=-8, 1-10=-5

29) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-34, 2-5=-44, 10-20=-20, 20-21=-65, 14-21=-20, 22-23=-45(F)

Vert: 1-2=-34, 2-5=-44, 10-20=-20, 20-21=-65, 14-21=-20, 22-23=-45(F Horz: 1-2=-16, 2-5=6, 1-10=14

30) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-44, 2-5=-34, 10-20=-20, 20-21=-65, 14-21=-20, 22-23=-45(F)

Horz: 1-2=-6, 2-5=16, 1-10=-5

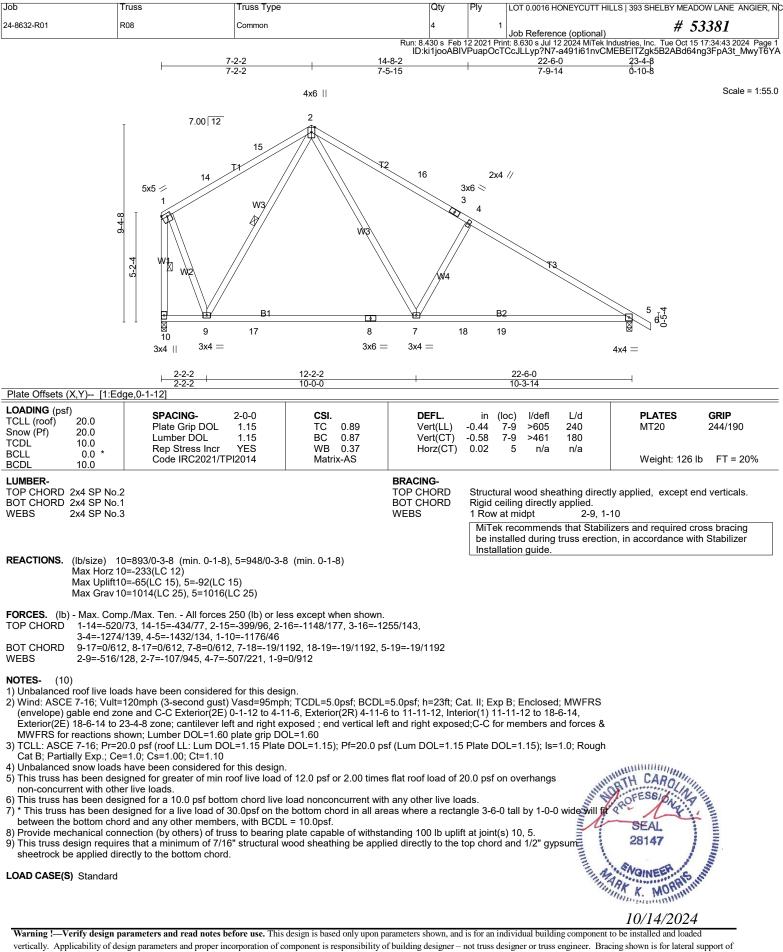
 35) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 Vert: 1-2=-50, 2-5=-20, 10-20=-20, 20-21=-65, 14-21=-20, 22-23=-45(F)

36) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15

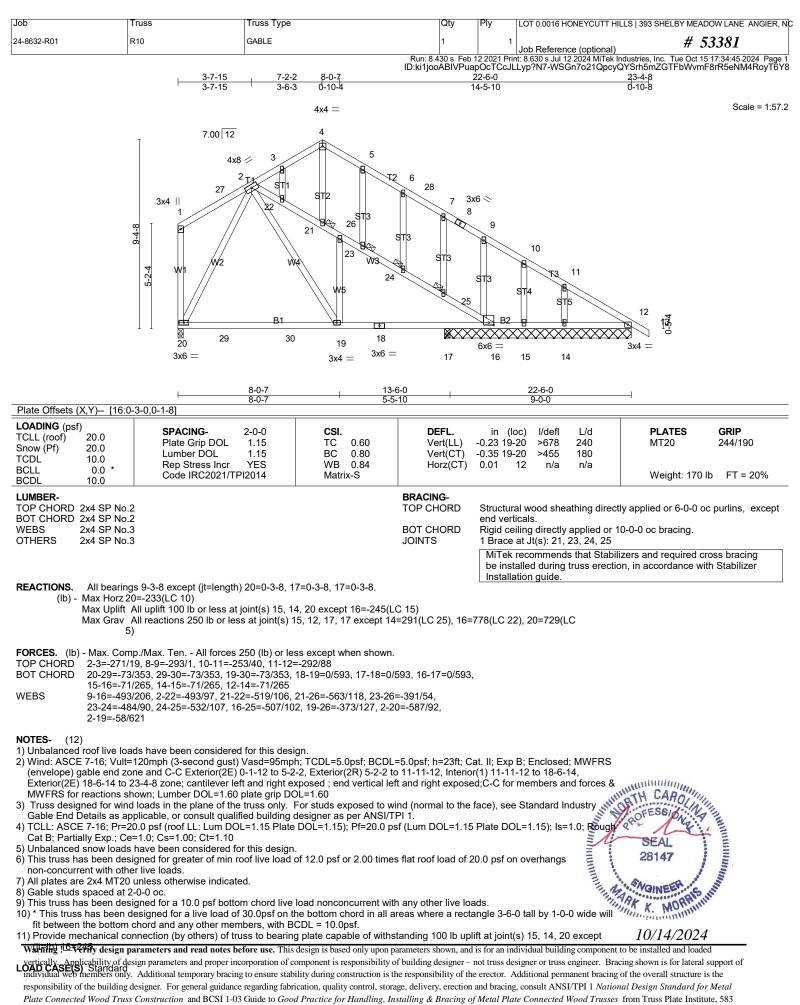
Uniform Loads (plf)

Vert: 1-2=-20, 2-5=-50, 10-20=-20, 20-21=-65, 14-21=-20, 22-23=-45(F)





vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



D'Onofrio Drive, Madison, WI 53719.

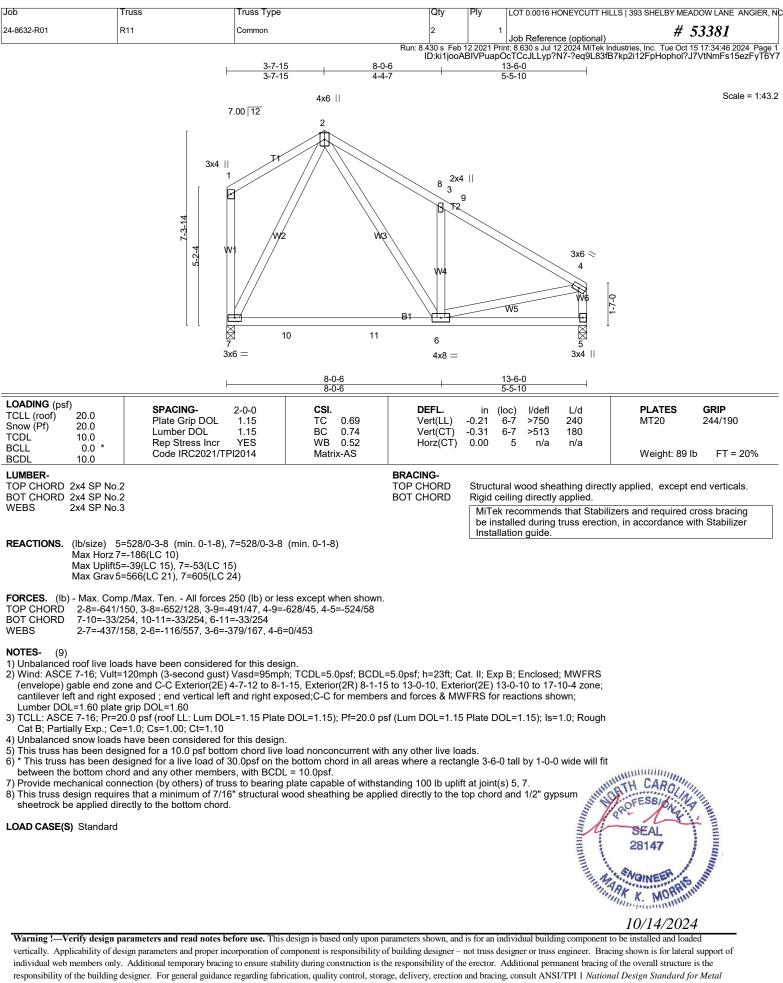
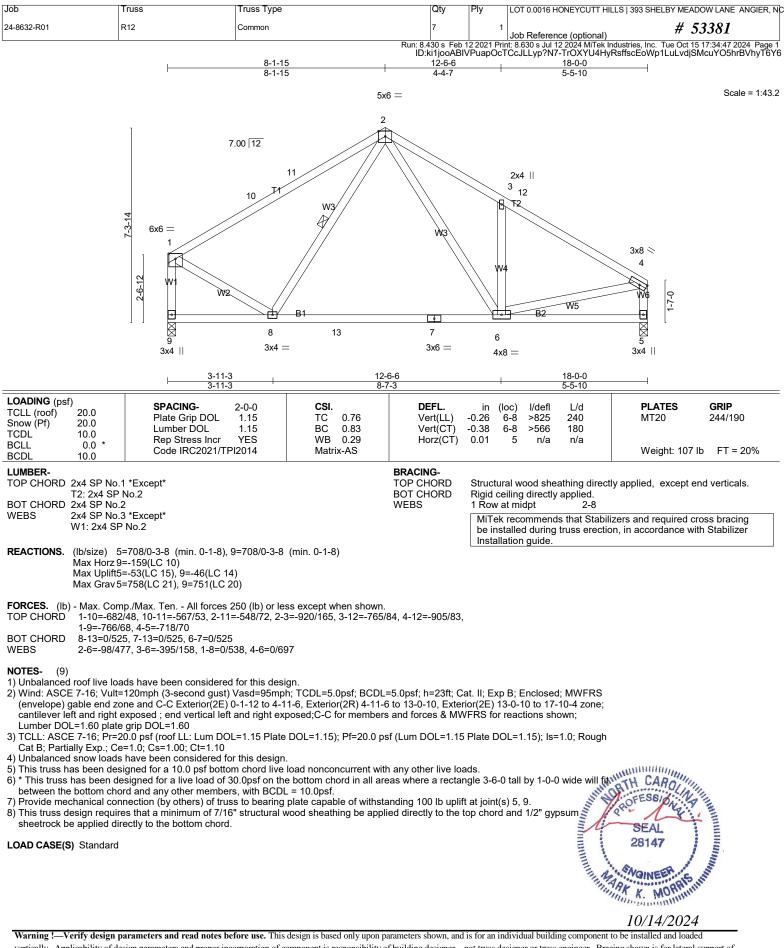
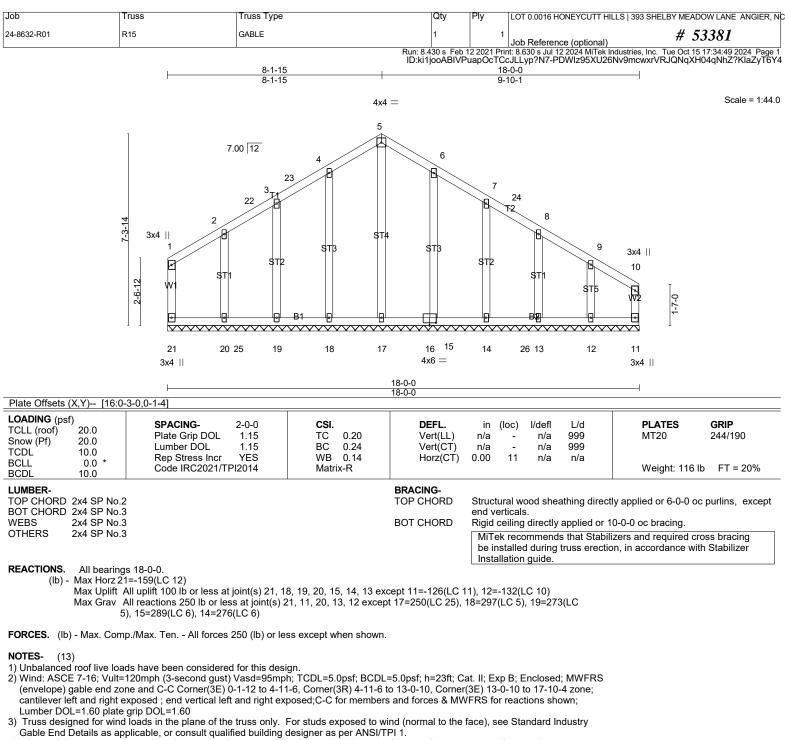


Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 THE CARO

5) Unbalanced snow loads have been considered for this design.

6) All plates are 2x4 MT20 unless otherwise indicated.

Gable requires continuous bottom chord bearing.

8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

9) Gable studs spaced at 2-0-0 oc.

- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide (11)fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 21, 18, 19, 20, 15, 13 except (jt=lb) 11=126, 12=132.

LOAD CASE(S) Standard

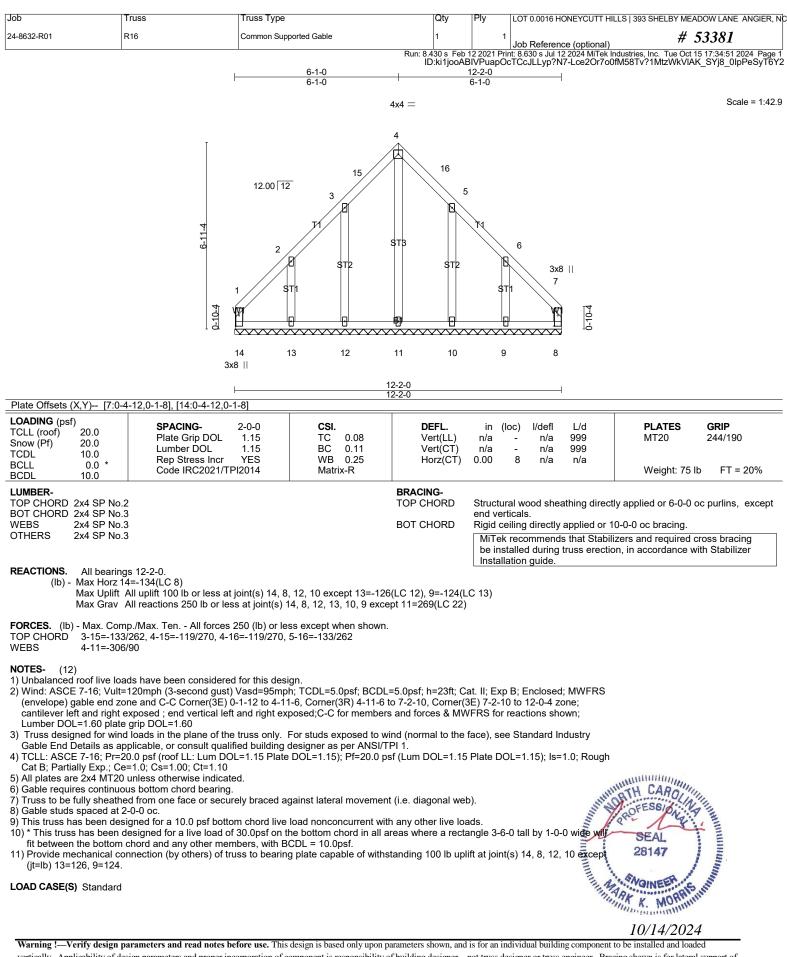
NORRESING NIL4/20 d and Warning !---Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

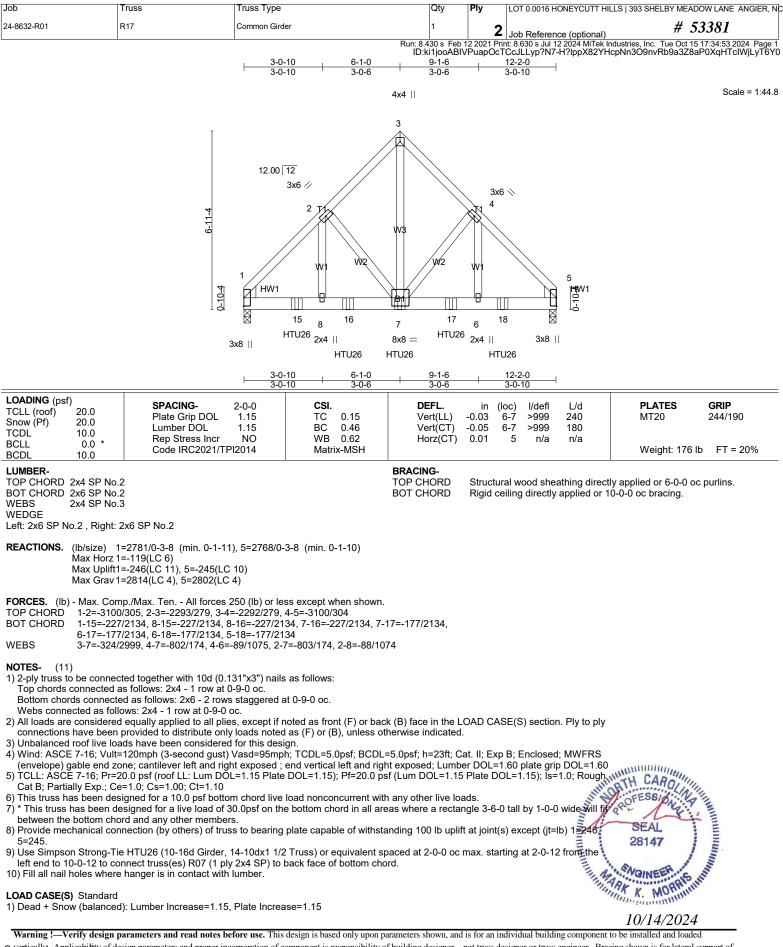
SEAL

28147

NOINEE ARK K. MORAL

10/14/2024





Job	Truss	Truss Type	Qty	Ply	LOT 0.0016 HONEYCUTT HILLS 393 SHE	LBY MEADOW LANE ANGIER, NC	
24-8632-R01	R17	Common Girder	1	2	Job Reference (optional)	# 53381	
Pup: 8 430 c. Epb 12 2021 Print: 8 630 c. Jul 12 2024 MiTok Industrias, Inc. Tuo Oct 15 17:34:54 2024 Page 2							

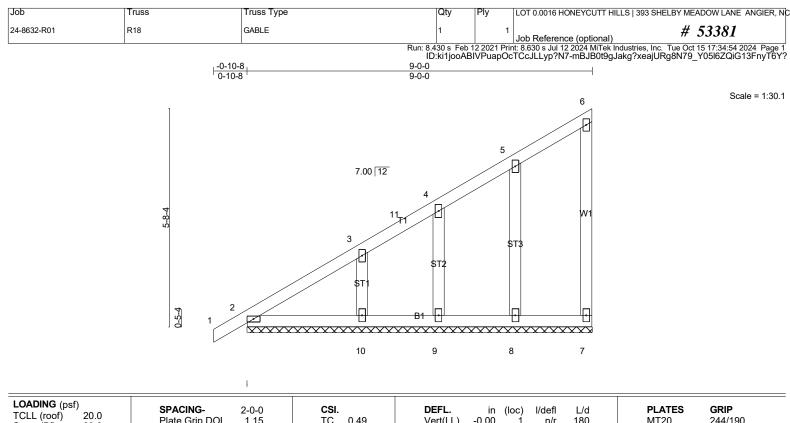
: 8.430 s. Feb 12 2021 Print: 8.630 s.Jul 12 2024 Mi Tek Industries, Inc. Tue Oct 15 17:34:54 2024 Page 2 ID:ki1jooABIVPuapOcTCcJLLyp?N7-mBJB0t9gJakg?xeajURg8N7EJYwel_4QiG13FnyT6Y?

LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 9-12=-20

Concentrated Loads (lb) Vert: 7=-915(B) 15=-915(B) 16=-915(B) 17=-915(B) 18=-915(B)





TCLL (roof) Snow (Pf) TCDL BCLL BCDL	20.0 20.0 10.0 0.0 * 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	TC 0.49 BC 0.12 WB 0.08 Matrix-S	Vert(LL) Vert(CT) Horz(CT)	-0.00 0.00 0.00	1 n/r 1 n/r 7 n/a	180 80 n/a	MT20 Weight: 51 lb	244/190 FT = 20%
LUMBER-				BRACING-	a				
TOP CHORD BOT CHORD				TOP CHORD	end verti		athing diree	ctly applied or 6-0-0 o	c purlins, except
	2x4 SP No.3			BOT CHORD	Rigid cei	iling directly	applied or	10-0-0 oc bracing.	
OTHERS	2x4 SP No.3				be insta			ilizers and required cr tion, in accordance wi	

REACTIONS. All bearings 9-0-0.

(lb) - Max Horz 2=163(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 7, 2, 8, 9, 10 Max Grav All reactions 250 lb or less at joint(s) 7, 2, 9, 10 except 8=268(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.

10) * This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide with CARO fit between the bottom chord and any other members, with BCDL = 10.0psf.
11) Provide mechanical connection (by others) of truss to begin a true.

- 10. The second s

LOAD CASE(S) Standard

Warning !---Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

WANTER HALF

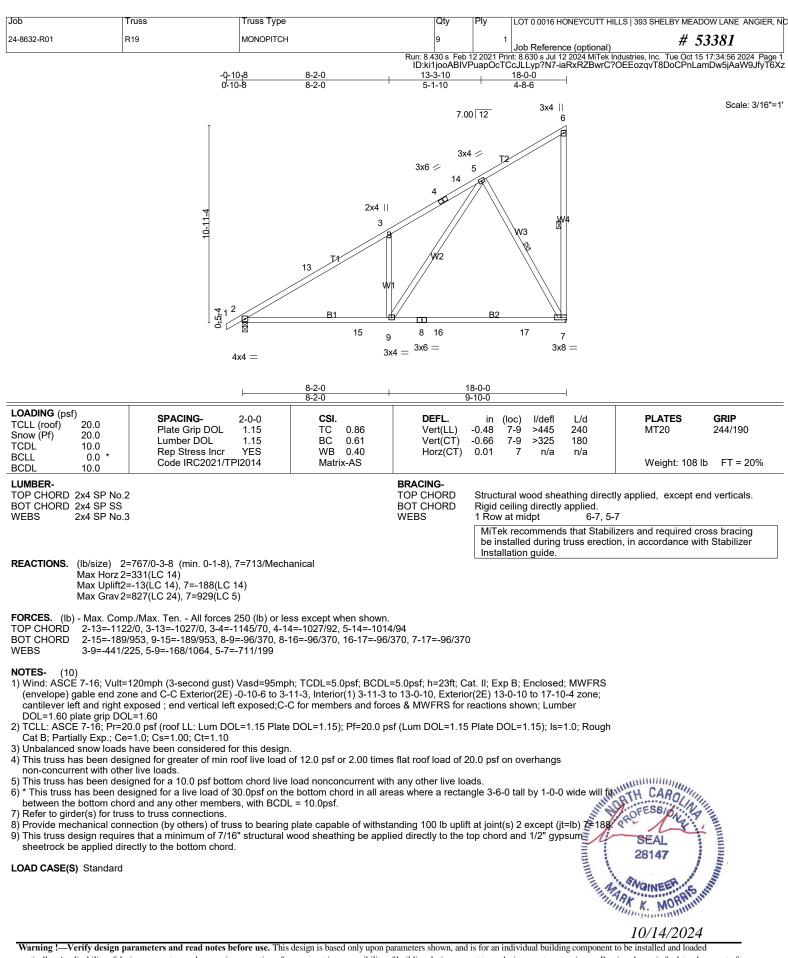
SEAL

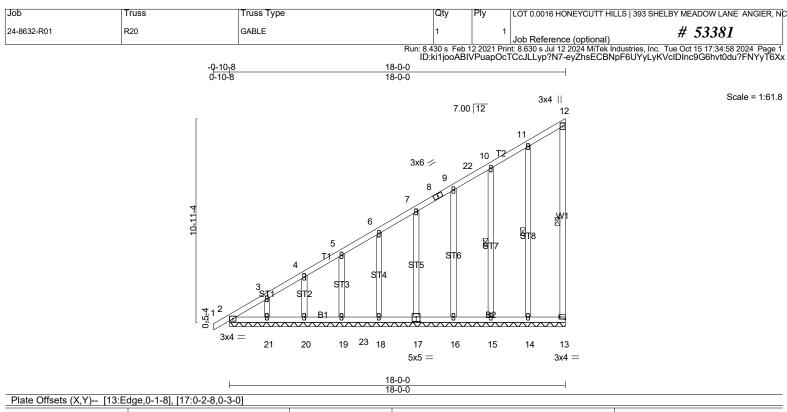
28147

GINE K. MORR

10/14/2024

MORRIS





LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2021/TPI2014	CSI. TC 0.71 BC 0.56 WB 0.12 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/de 0.00 1 n. 0.00 1 n. 0.00 13 n/	/r 180 /r 80	PLATES MT20 Weight: 136 lb	GRIP 244/190 • FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.2 OTHERS 2x4 SP No.3	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing directly applied or 6-0-0 oc purlins, exc end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Row at midpt 12-13, 11-14, 10-15					
0111ENG 244 3F NO.3			WEBS	MiTek recomme	ends that Stabil	izers and required cro on, in accordance with	

Installation guide.

28147

NOINEE ARK K. MORRIS

10/14/2024

REACTIONS. All bearings 18-0-0.

- (lb) Max Horz 2=319(LC 11)
 - Max Uplift All uplift 100 lb or less at joint(s) 13, 2, 14, 15, 16, 17, 18, 19, 20, 21

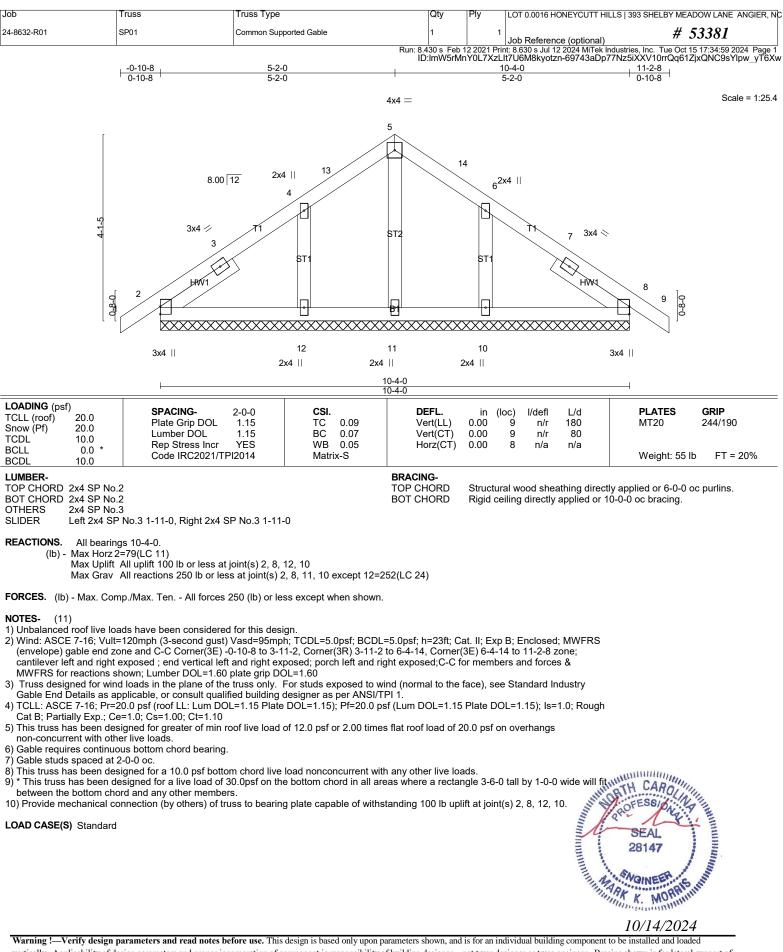
Max Grav All reactions 250 lb or less at joint(s) 13, 2, 19, 20, 21 except 14=301(LC 5), 15=275(LC 5), 16=252(LC 24), 17=251(LC 24), 18=257(LC 24)

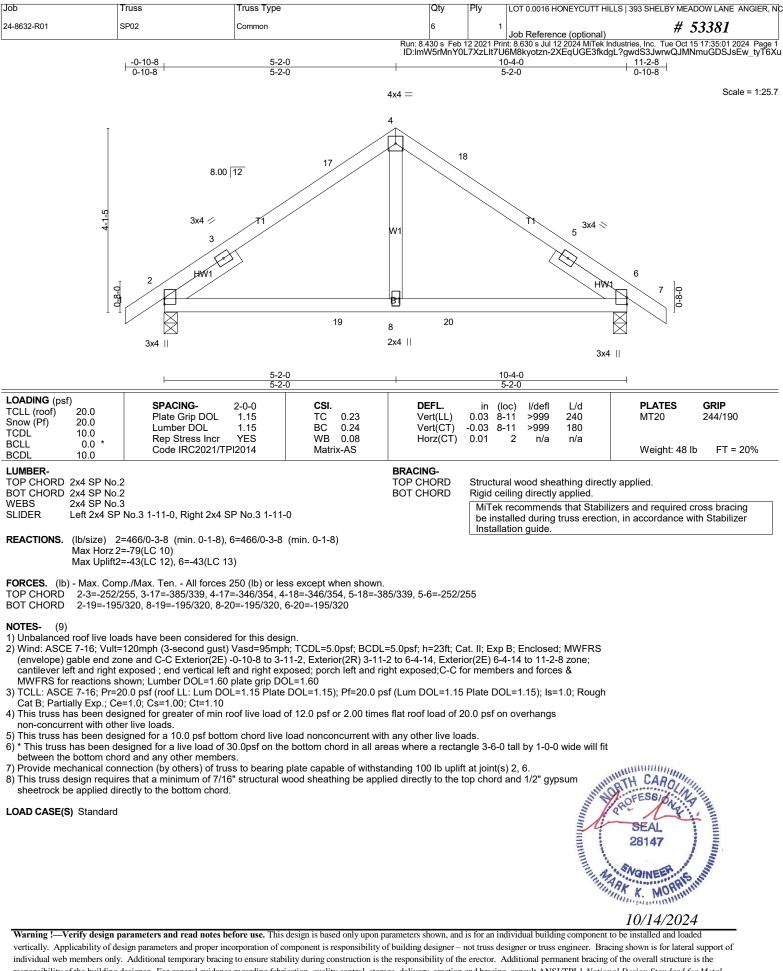
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-287/364, 3-4=-255/327, 4-5=-231/305, 5-6=-208/283, 6-7=-190/260

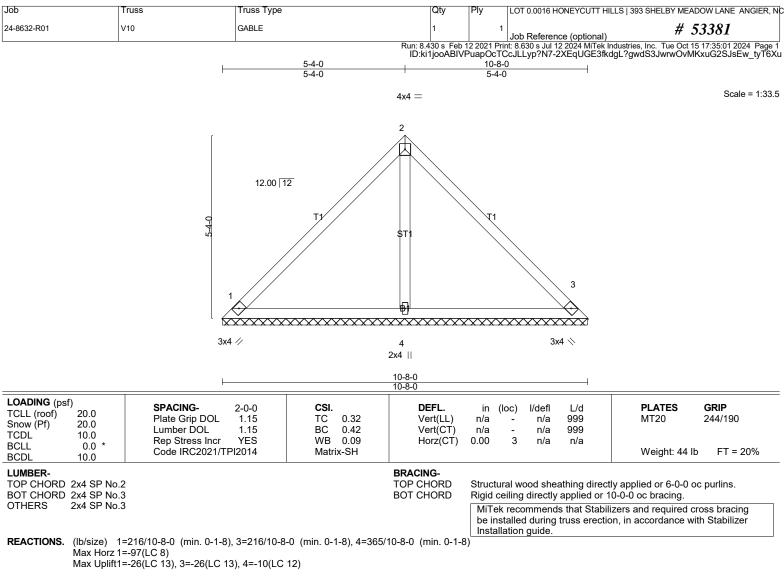
NOTES-(12)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-6 to 4-0-0, Exterior(2N) 4-0-0 to 13-0-10, Corner(3E) 13-0-10 to 17-10-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- PROFESS 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 2-0-0 oc.





responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

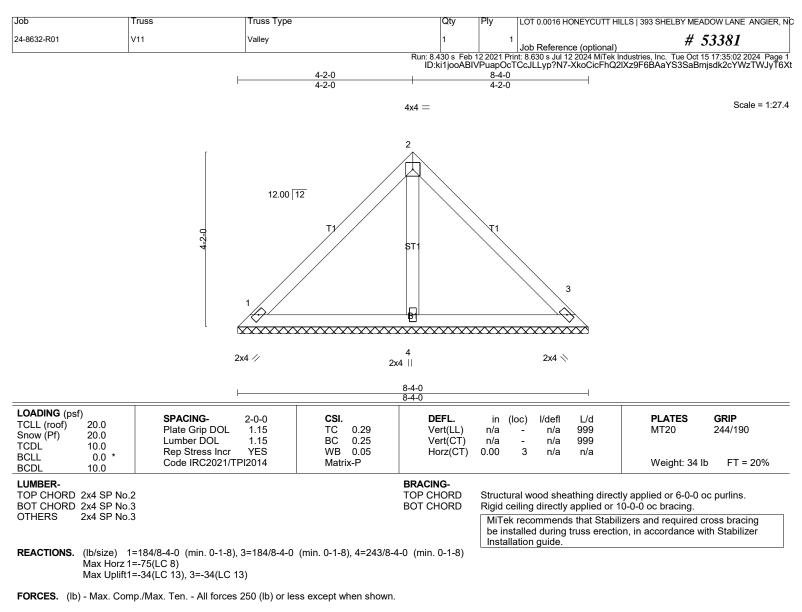
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard





NOTES-(8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed: MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

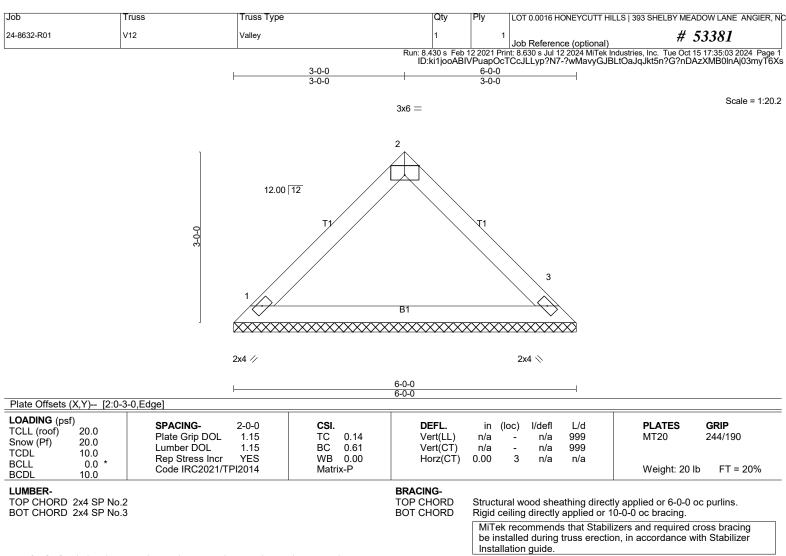
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 1=212/6-0-0 (min. 0-1-8), 3=212/6-0-0 (min. 0-1-8) Max Horz 1=-52(LC 8) Max Uplift1=-10(LC 13), 3=-10(LC 12)

NOTES- (8)

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

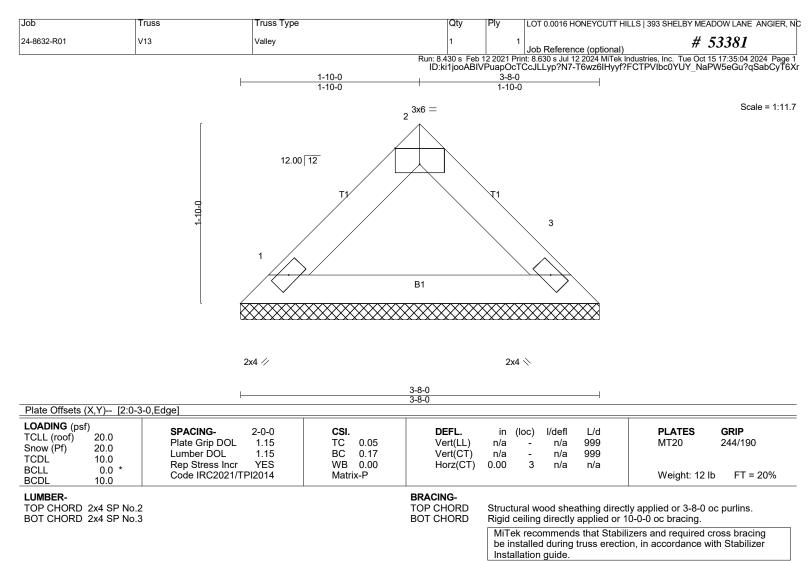
LOAD CASE(S) Standard



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

¹⁾ Unbalanced roof live loads have been considered for this design.

²⁾ Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



REACTIONS. (lb/size) 1=119/3-8-0 (min. 0-1-8), 3=119/3-8-0 (min. 0-1-8) Max Horz 1=-29(LC 8) Max Uplift1=-6(LC 13), 3=-6(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



¹⁾ Unbalanced roof live loads have been considered for this design.