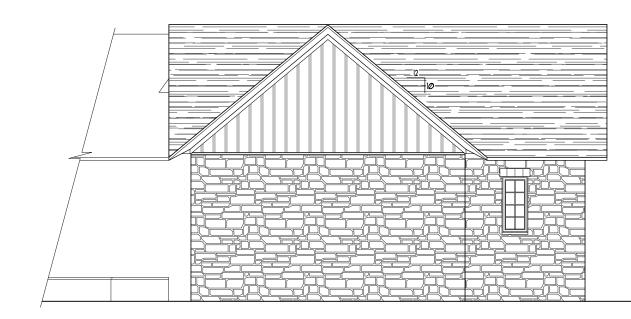


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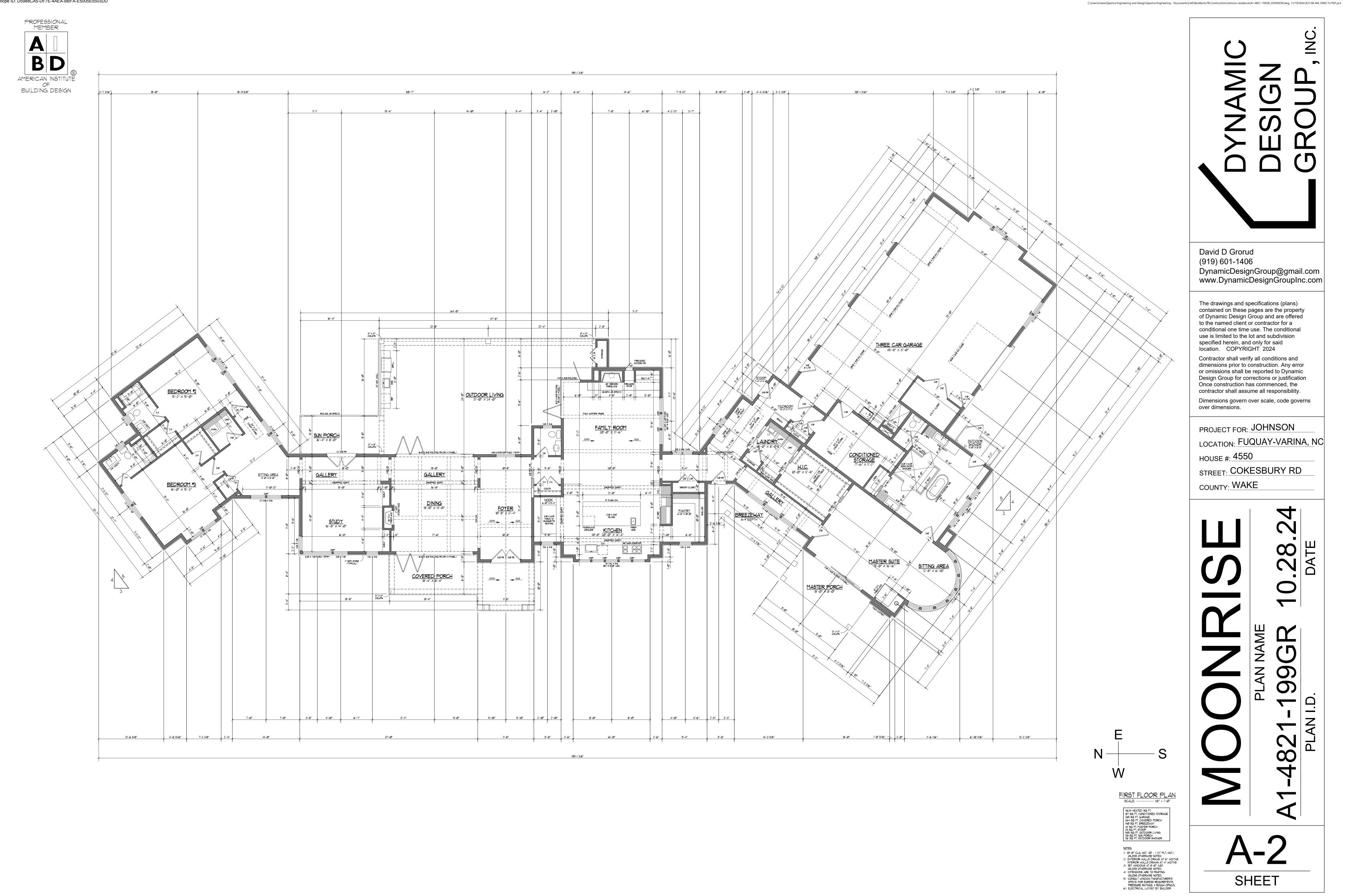
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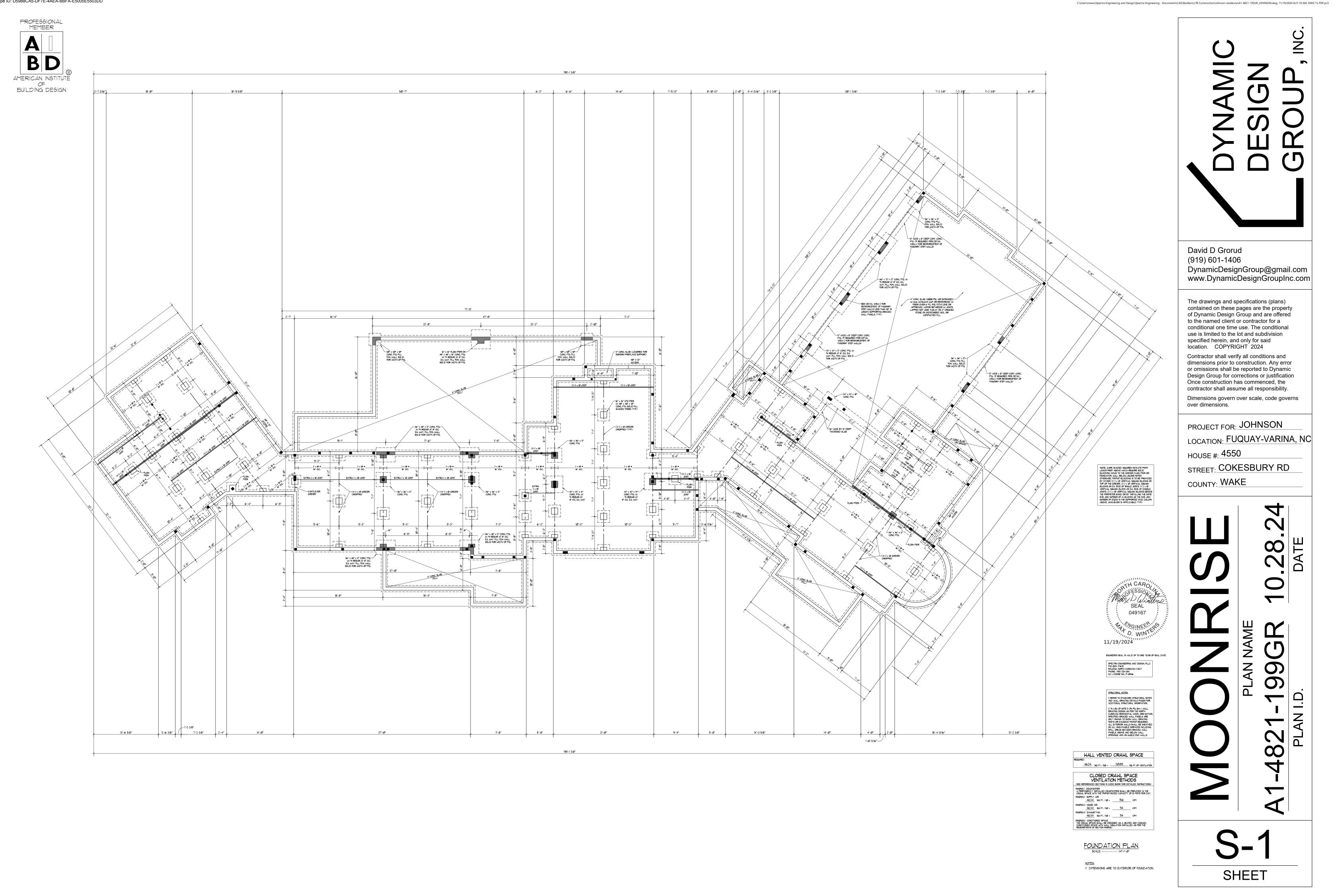


THIS PLAN CONFORMS TO THE 2018 EDITION OF THE I.R.C. / NORTH CAROLINA RESIDENTIAL CODE.

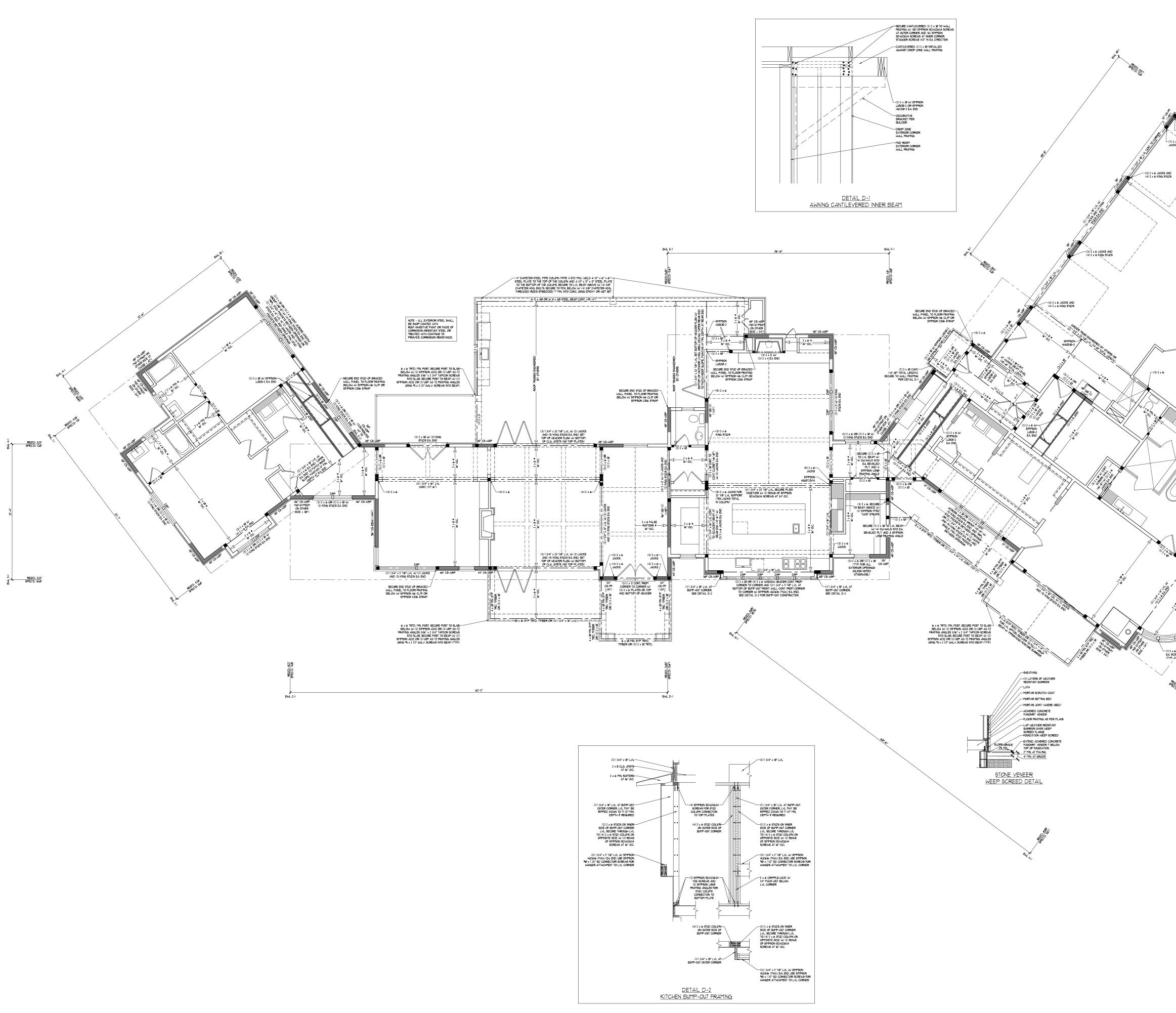
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AS SPEC'D A BATTEN AS SPEC'D	
ЭЕ.	David D Grorud (919) 601-1406 DynamicDesignGroup@gmail.com www.DynamicDesignGroupInc.com
	The drawings and specifications (plans) contained on these pages are the property of Dynamic Design Group and are offered to the named client or contractor for a conditional one time use. The conditional use is limited to the lot and subdivision specified herein, and only for said location. COPYRIGHT 2024 Contractor shall verify all conditions and dimensions prior to construction. Any error or omissions shall be reported to Dynamic Design Group for corrections or justification Once construction has commenced, the contractor shall assume all responsibility. Dimensions govern over scale, code governs over dimensions.
	PROJECT FOR: JOHNSON LOCATION: FUQUAY-VARINA, NC HOUSE #: 4550 STREET: COKESBURY RD
	COUNTY: WAKE
	A1-4821-199GR PLAN ID PLAN ID PLAN ID
	A-1

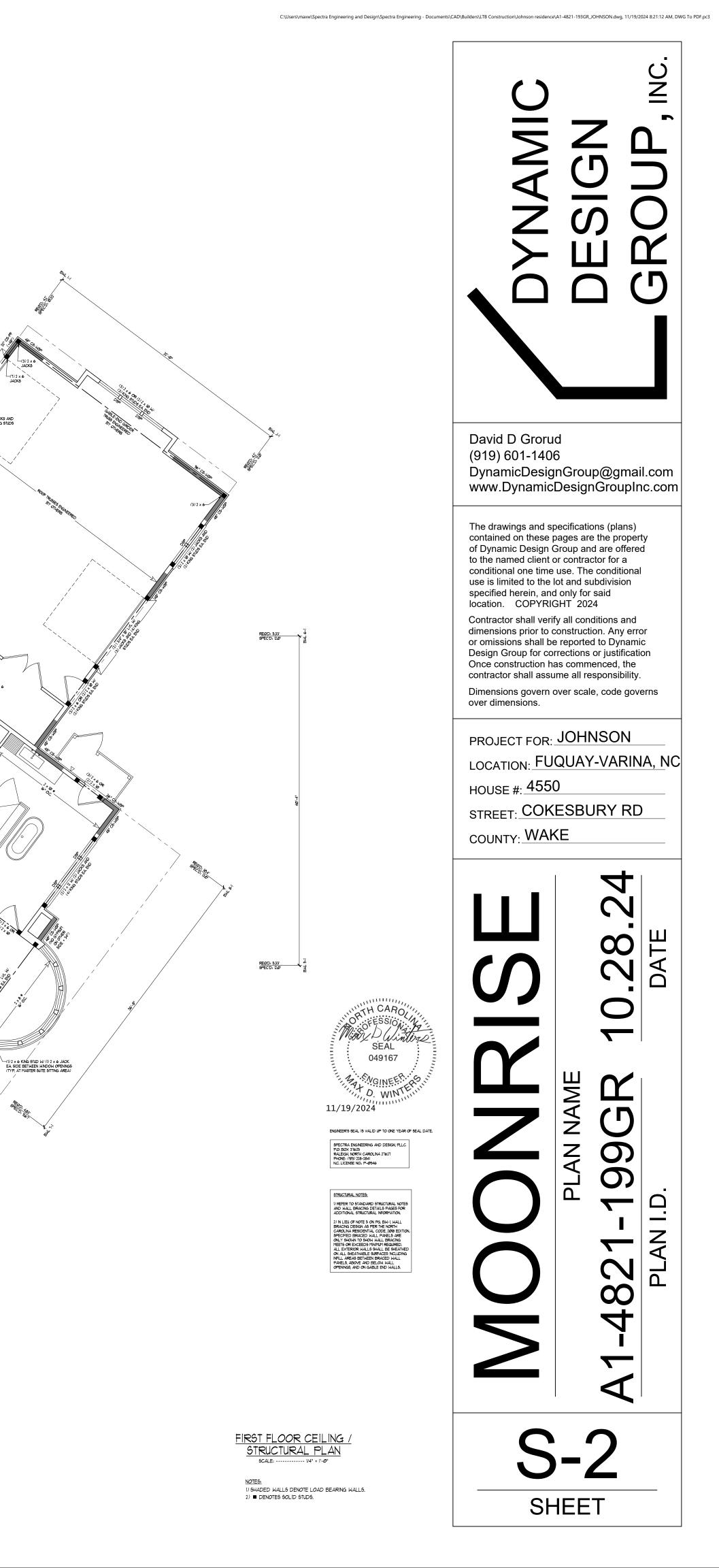
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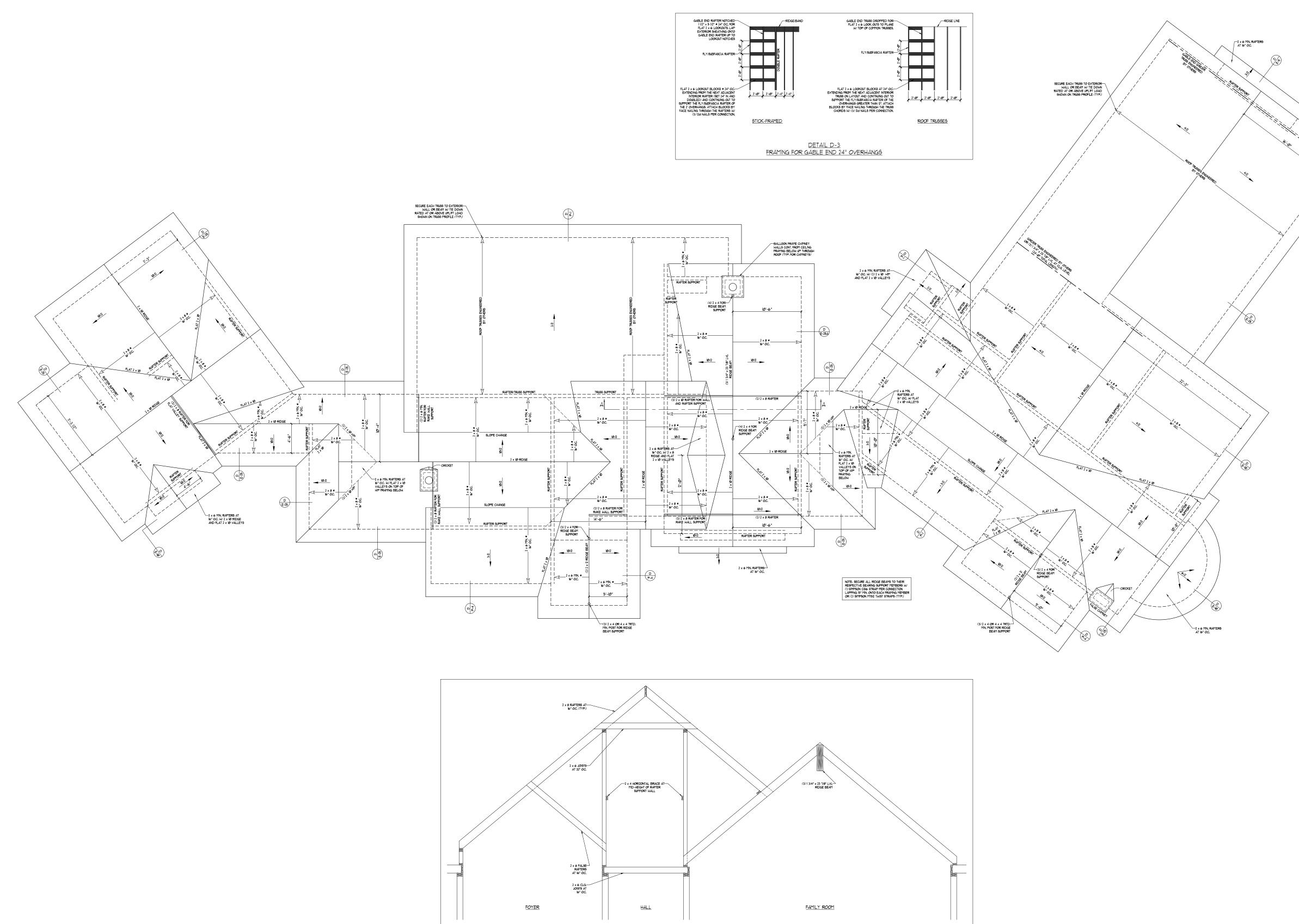




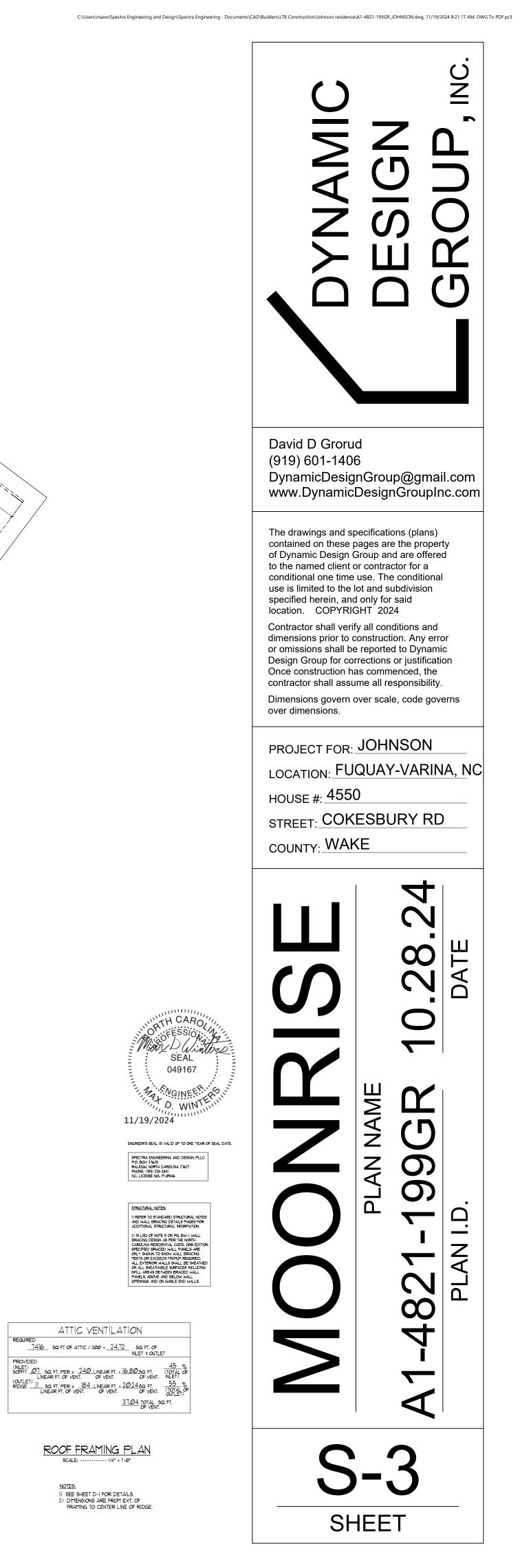




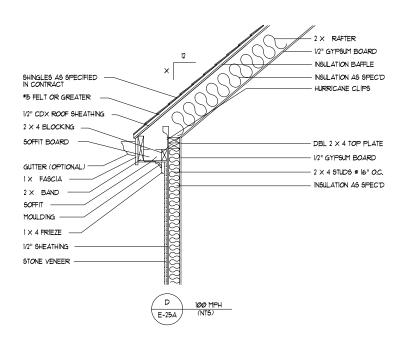


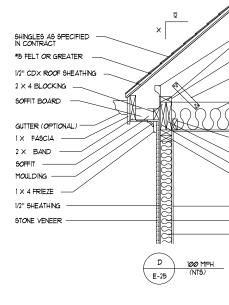


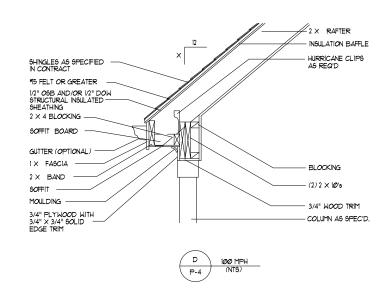
SECTION A-A

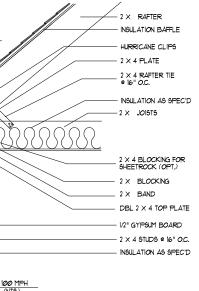


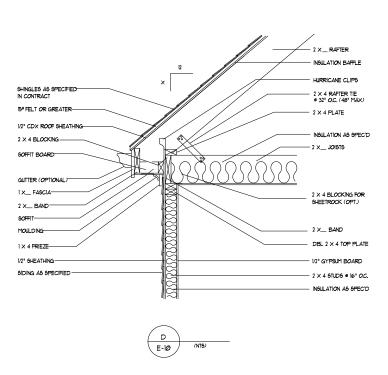


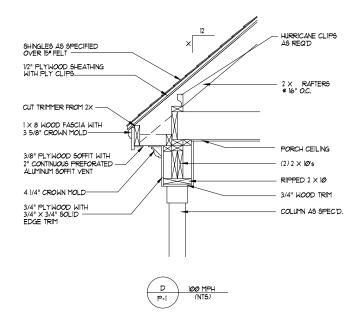


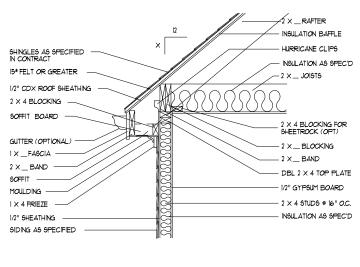




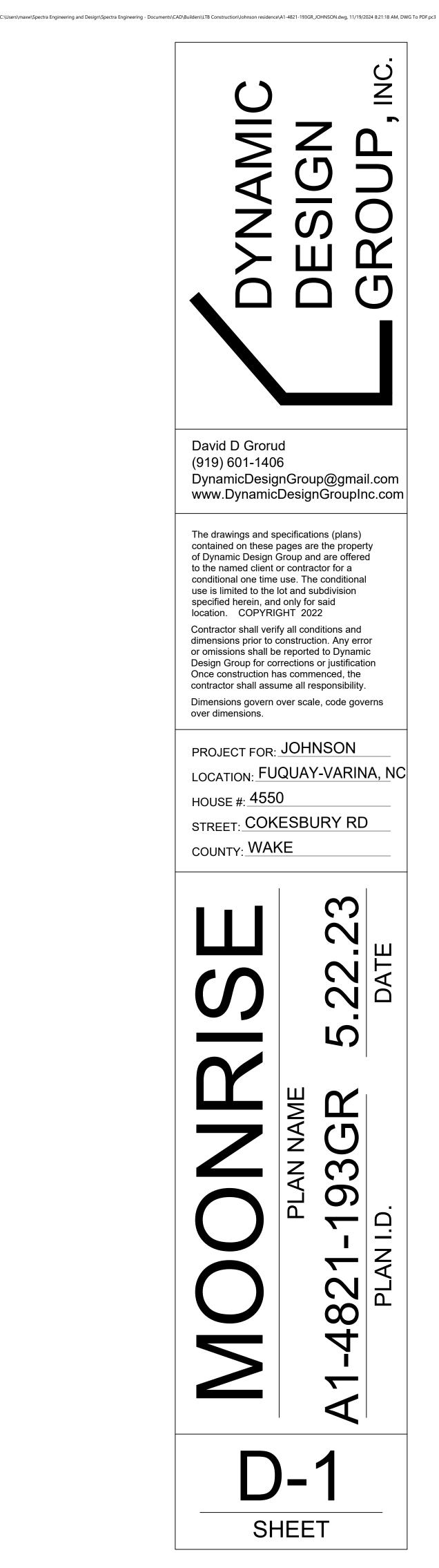


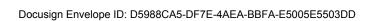


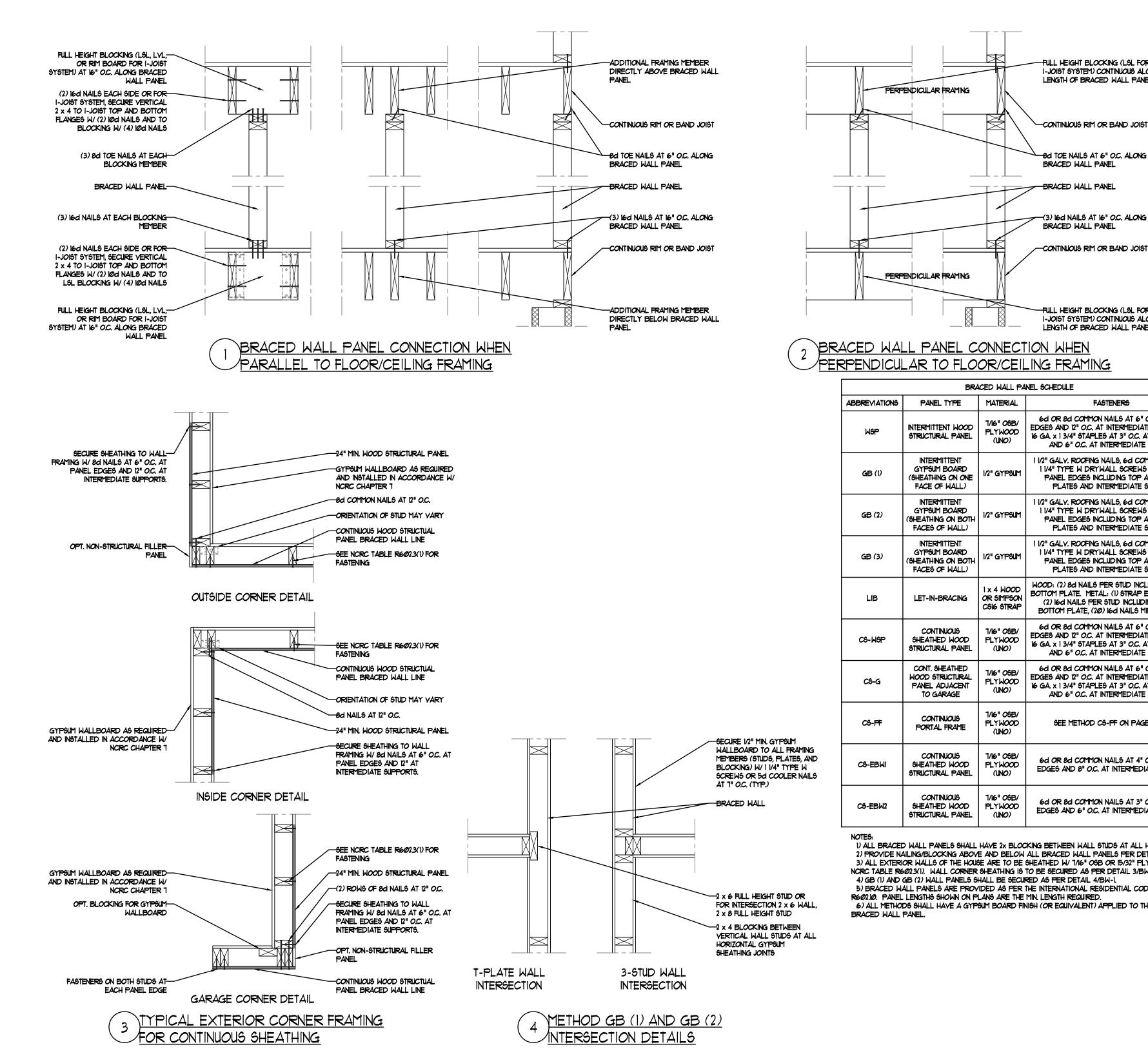




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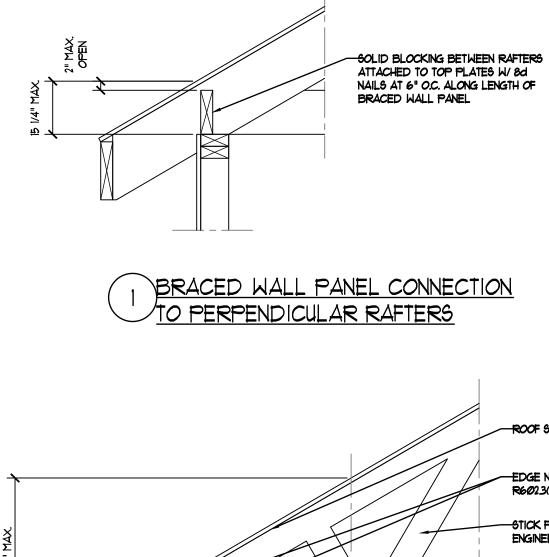


	BRACED WALL PANEL SCHEDULE					
ABBREVIATIONS	PANEL TYPE	MATERIAL	FA			
WSP	INTERMITTENT WOOD STRUCTURAL PANEL	1/16" 08B/ PLYWOOD (UNO)	6d OR 8d Common Edges and 12" O.C. At 16 G.A. x 1 3/4" Staple And 6" O.C. At 1			
GB (1)	INTERMITTENT GYPSUM BOARD (SHEATHING ON ONE FACE OF WALL)	1/2" GYPSUM	1 1/2" GALV. ROOFING 1 1/4" TYPE W DRY1 PANEL EDGES INC PLATES AND IN			
GB (2)	INTERMITTENT GYPSUM BOARD (SHEATHING ON BOTH FACES OF WALL)	1/2" GYPSUM	1 1/2" GALV. ROOFING 1 1/4" TYPE W DRY1 PANEL EDGES INC PLATES AND IN			
GB (3)	INTERMITTENT GYPSUM BOARD (SHEATHING ON BOTH FACES OF WALL)	1/2" GY <b>PSUM</b>	1 1/2" GALV. ROOFING 1 1/4" TYPE W DRY1 PANEL EDGES INC PLATES AND IN			
LIB	LET-IN-BRACING	I x 4 WOOD OR SIMPSON CSIG STRAP	WOOD: (2) 8d NAILS F BOTTOM PLATE. META (2) 16d NAILS PER BOTTOM PLATE, (20			
C9-W <del>9P</del>	CONTINUOUS SHEATHED WOOD STRUCTURAL PANEL	1/16" 08B/ PLYWOOD (UNO)	6d OR 8d Common EDGES AND 12" O.C. AT 16 G.A. x 1 3/4" STAPLE AND 6" O.C. AT 1			
୯୫-ଜ	CONT. SHEATHED WOOD STRUCTURAL PANEL ADJACENT TO GARAGE	1/16" 08B/ PLYWOOD (UNO)	6d OR 8d Common Edges and 12" O.C. At 16 G.A. X   3/4" Staple And 6" O.C. At I			
C3- <del>PT</del>	CONTINUOUS PORTAL FRAME	1/16" OSB/ PLYWOOD (UNO)	See Method C			
СЭ-ЕВЫІ	CONTINUOUS SHEATHED WOOD STRUCTURAL PANEL	1/16" 098 PLYWOOD (UNO)	6d OR 8d Common EDGES AND 8" O.C.			
C <del>S</del> -EBW2	CONTINUOUS SHEATHED WOOD STRUCTURAL PANEL	1/16" 08B/ PLYWOOD (UNO)	6d OR 8d COMMON EDGES AND 6" O.C.			

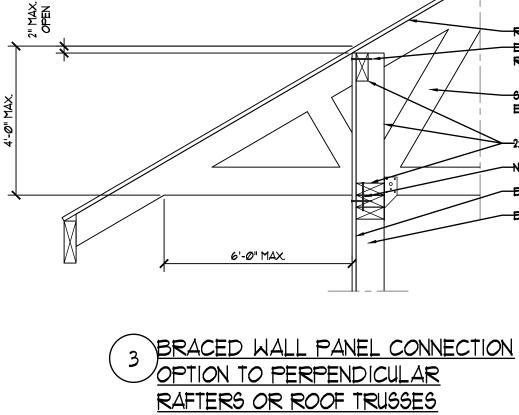
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G STERES STERES STATE OF AD EXPORTS OR SS AT 3 CO. AT PARE EXPERIENCE STERES SATE OF AD EXTON HALS, AGE COPTON NULL, OR AULL SCREED AT DEPORTS NULLS, OF AD EXTON HALS, AT 9' OC, AT PARE AT INTERVENTS EXPORTS NULLS AT 4' OC, AT PARE AT INTERVENTS NULLS AT 4' OC, AT PARE AT INTERVENTS HALF AT INTERVENTS HALF AT INTERVENTS HALF AT INTERVENTS HALF AT AN AT A' OC, AT PARE AT INTERVENTS HALF A' OC,	ig" O.C. Along Anel	NC BO NC BO NC 19 SATH SATH SATH SATH SATH SATH SATH SATH
BETENERS         WITENERS         INNELS AT 6' OC. AT PAREL ENTRETEDATE SUPPORTS OR 55 AT 7' OC. AT PAREL EXECUTION NULL, OR AULL SOCIES AT 0' OC. AT SUDRE TO A DECTORI HEREDATE SUPPORTS         NULLS AC COTTON NULL, OR AULL SOCIES AT 0' OC. AT SUDRE TO A DECTORI HEREDATE SUPPORTS         NULLS AC COTTON NULL, OR AULL SOCIES AT 0' OC. AT PAREL AUL SOCIES AT 0' OC. AT PAREL AULL SOCIES AT 0' OC. AT PAREL AUL SOCIES AT 0' OC. AT PAREL SOLIAL SOCIES AT 0' OC. AT PAREL SOLIAL SOCIES AT 0' OC. AT PAREL SOLIAL SOCIES AT PAREL AT INTERPEDUATE SUPPORTS OR 55 AT 7' OC. AT PAREL AT INTERPEDUATE SUPPORTS ON 55 AT 7	OR BAND JOIGT	
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NALLS, 6d COMPON NALLS, OR         VALL SCREMS AT TY OC. AT         VILL SCREMS AT YOUR ALLS, OR         VILL SCREMS AT YOUR ALLS, OR YOUR	NAILS, 6d COMMON NAILS, OR NALL SCREWS AT 1" O.C. AT CLUDING TOP AND BOTTOM	
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ANAILS AT 4" O.C. AT PANEL AT INTERMEDIATE SUPPORTS INAILS AT 3" O.C. AT PANEL AT INTERMEDIATE SUPPORTS STUDS AT ALL HORIZONTAL SHEET EDGES. WELS PER DETAIL 1/5W-1 AND 2/5W-1. 3 OR 15/32" PLYWOOD SECURED PER	T INTERMEDIATE SUPPORTS OR ES AT 3" O.C. AT PANEL EDGES	
TUDS AT ALL HORIZONTAL SHEET EDGES. NELS PER DETAIL I/BW-1 AND 2/BW-1. S OR 15/32" PLYWOOD SECURED PER	S-PF ON PAGE BW-3	
AT INTERMEDIATE SUPPORTS TUDS AT ALL HORIZONTAL SHEET EDGES. NELS PER DETAIL 1/BW-1 AND 2/BW-1. S OR 15/32" PLYWOOD SECURED PER		MALL MAL
ANELS PER DETAIL 1/BW-1 AND 2/BW-1. 3 OR 15/32" PLYWOOD SECURED PER	NAILS AT 3" O.C. AT PANEL AT INTERMEDIATE SUPPORTS	
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SHEET  $\times$  OF  $\times$ 

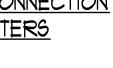


6'-0" MAX.



PROVIDE VENTING PER NORC SECTION R806 (NOT SHOWN)





BRACED WALL PANEL CONNECTION

OPTION TO PERPENDICULAR

RAFTERS OR ROOF TRUSSES

PROVIDE VENTING PER NORC SECTION R806 (NOT SHOWN)

-ROOF SHEATHING

R602.3(1) (TYP.)

2x BLOCKING

R602.3(1) (TYP.)

-2x BLOCKING

-BRACED WALL LINE

-ROOF SHEATHING

→2x BLOCKING

BRACED WALL PANEL

BRACED WALL LINE

EDGE NAILING PER NORC TABLE R602.3(1) (TYP.)

NAILING PER NCRC TABLE R602.3(1)

EDGE NAILING PER NORC TABLE

ENGINEERED BY OTHERS

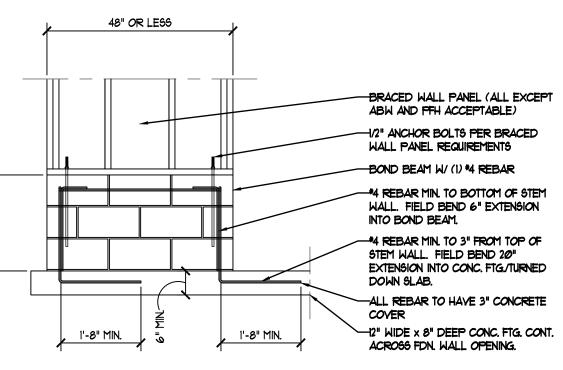
-BRACED WALL PANEL

-STICK FRAMING OR ROOF TRUSSES

-NAILING PER NORC TABLE R602.3(1)

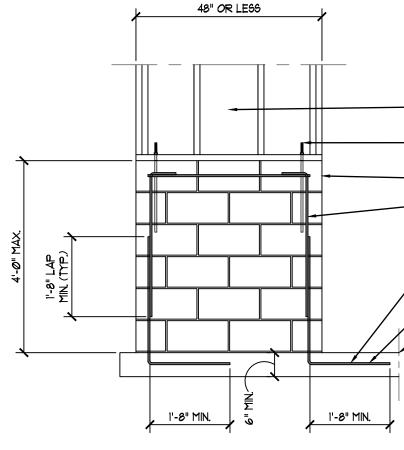
-EDGE NAILING PER NORC TABLE

ATTACHED TO TOP PLATES W/ 8d NAILS AT 6" O.C. ALONG LENGTH OF BRACED WALL PANEL



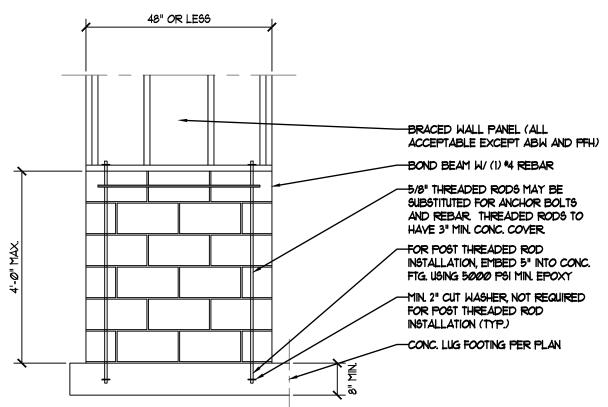
-BRACED WALL PANEL (ALL EXCEPT ABW AND PFH ACCEPTABLE) -1/2" ANCHOR BOLTS PER BRACED WALL PANEL REQUIREMENTS -BOND BEAM W/ (1) \*4 REBAR - \*4 REBAR MIN. TO BOTTOM OF STEM WALL. FIELD BEND 6" EXTENSION INTO BOND BEAM. 4 REBAR MIN. TO 3" FROM TOP OF STEM WALL. FIELD BEND 20" EXTENSION INTO CONC. FTG./TURNED DOWN SLAB. -ALL REBAR TO HAVE 3" CONCRETE COVER





-BRACED WALL PANEL (ALL EXCEPT ABW AND PFH ACCEPTABLE) -BOND BEAM W/ (1) \*4 REBAR --\*4 REBAR MIN. FIELD BEND 6" EXTENSION INTO BOND BEAM. DOWN SLAB. -ALL REBAR TO HAVE 3" CONCRETE COVER -12" WIDE X 8" DEEP CONC. FTG. CONT. ACROSS FDN. WALL OPENING.



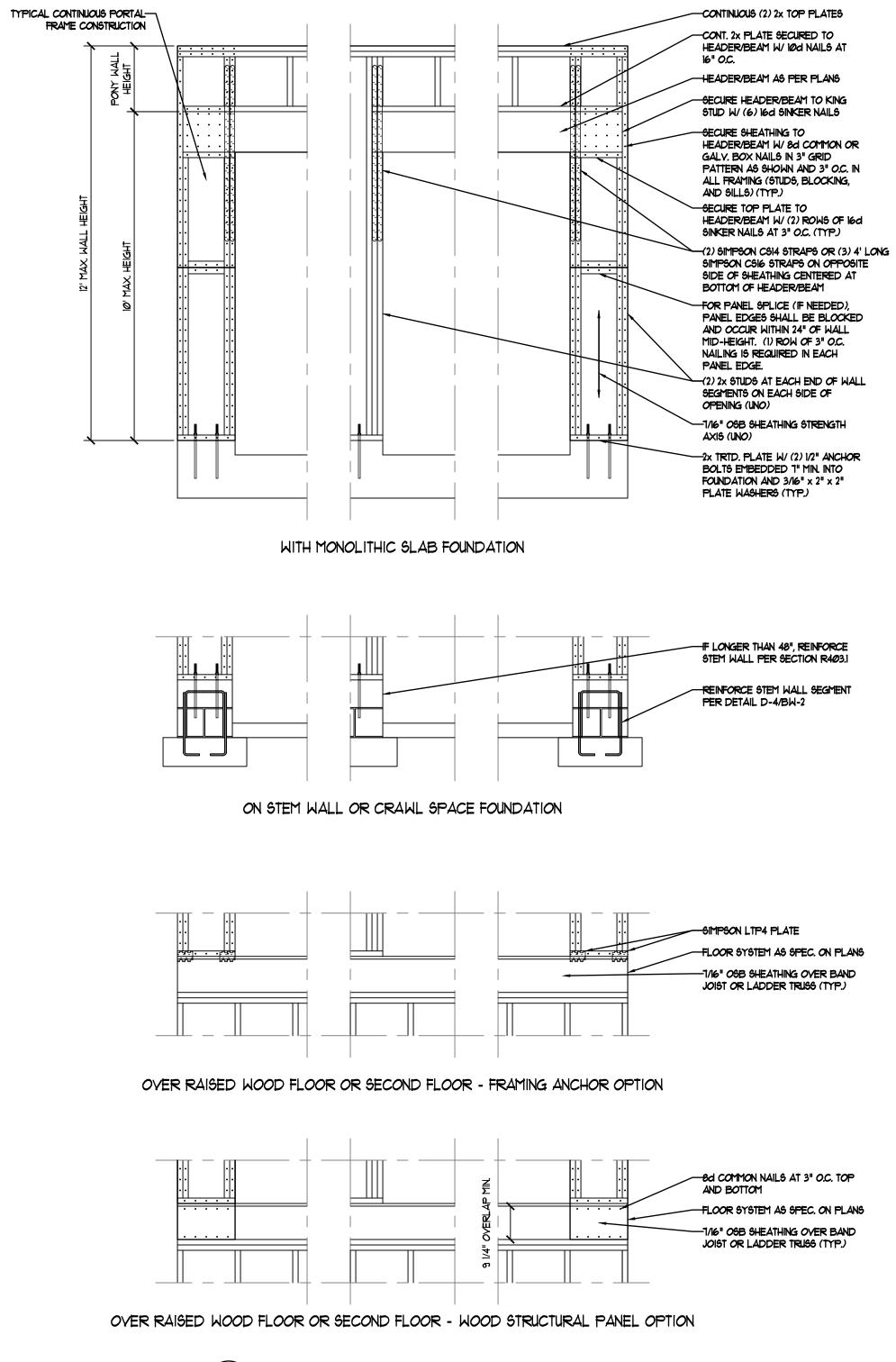


OPT. STEM WALL REINFORCEMENT CONFIGURATION

4 MASONRY STEM WALLS SUPPORTING BRACED WALL PANELS

LLC DATE: JUNE 23, 2001 SCALE: NTS DRAMN BY: TSZ ENGINEERED BY: TSZ REVIEWED BY: TSZ
SPECTRA ENGINEERING AND DESIGN, PLLC P.O. BOX 37625 RALEIGH, NORTH CAROLINA 27621 TEL.: (919) 228-2841 LICENSE NO. NC: P-0946 VA: 000462
SPECTR2 SPECTR2
ALS
MALL BRACING DETAILS
SEAL 049167 11/19/2024 DETAILS
BH-2

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DATE: JUNE 23, 2021	SCALE: NTS	DRAMN BY: 152	ENGINEERED BY: TSZ	REVIEMED BY: 162				
SPECTRA ENGINEERING AND DESIGN, PLLC     DATE: JUNE : DATE: JUNE : P.O. BOX 31625       P.O. BOX 31625     SCALE: NT5       P.O. BOX 31625     SCALE: NT5       RALEIGH, NORTH CAROLINA 21621     DRAWN BY: DRAWN BY: TEL.: (919) 228-2841       LICENGE NO. NC: P-0946 VA: 000462     ENGINEERED       REVIEWED B								
		WALL BRACING DETAILS						
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DISCLAIMER - ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF THE NORTH CAROLINA RESIDENTIAL CODE (NCRC), 2018 EDITION, PLUS ALL LOCAL CODES AND REGULATIONS. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR, AND WILL NOT HAVE CONTROL OF, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, OR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE CONSTRUCTION WORK. NOR WILL THE ENGINEER BE RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO CARRY OUT THE CONSTRUCTION WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. ENGINEER'S SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS INCLUDING: ROOF RAFTERS, HIPS, VALLEYS, RIDGES, FLOORS, WALLS, BEAMS, HEADERS, COLUMNS, CANTILEVERS, OFFSET LOAD BEARING WALLS, PIERS, GIRDER SYSTEM AND FOOTING. ENGINEER'S SEAL DOES NOT CERTIFY DIMENSIONAL ACCURACY OR ARCHITECTURAL LAYOUT INCLUDING ROOF. ENGINEER'S SEAL DOES NOT APPLY TO I-JOIST OR FLOOR/ROOF TRUSS LAYOUT DESIGN AND ACCURACY.

STRUCTURAL DESIGN - STRUCTURAL DESIGN AS PER NCRC, INCLUDING CHAPTER 45 FOR CONSTRUCTION IN 130, 140, AND 150 MPH WIND ZONES. DESIGN LOADS ARE AS FOLLOWS: LIVE LOAD DEFLECTION

	(PSF)	(LL)
ATTIC WITH LIMITED STORAGE	2Ø	L/24Ø
ATTIC WITHOUT STORAGE	10	L/36Ø
DECKS	40	L/36Ø
EXTERIOR BALCONIES	60	L/36Ø
FIRE ESCAPES	40	L/36Ø
GUARDRAILS AND HANDRAILS	200	L/36Ø
PASSENGER VEHICLE GARAGES	50	L/36Ø
ROOMS OTHER THAN SLEEPING ROOMS	40	L/36Ø
SLEEPING ROOMS	30	L/36Ø
STAIRS	40	L/36Ø
SNOW	2Ø	L/36Ø
WIND LOAD (BASED ON "WALL	AND ROOF	CLADDING DESIGN LOADS

TABLE, WIND ZONE, MEAN ROOF HEIGHT AND EXPOSURE)

- STICK FRAMED SYSTEMS ARE DESIGNED WITH 10 PSF DEAD LOAD.

- I-JOIST SYSTEMS ARE DESIGNED WITH 12 PSF DEAD LOAD.

- FLOOR TRUSS SYSTEMS ARE DESIGNED WITH 15 PSF DEAD LOAD.

HIGH WIND ZONES - CONSTRUCTION IN 130, 140, AND 150 MPH WIND ZONES SHALL BE IN ACCORDANCE WITH CHAPTER 45 OF THE NCRC. CONSTRUCTION IN THE COASTAL AND FLOOD PLAINS SHALL BE IN ACCORDANCE WITH CHAPTER 46 OF THE NCRC.

CONCRETE FOOTING AND SLAB PREPARATION - FOR ALL CONCRETE SLABS AND FOOTINGS, THE AREA WITHIN THE PERIMETER OF THE BUILDING ENVELOPE SHALL HAVE ALL VEGETATION, TOP SOIL AND FOREIGN MATERIAL REMOVED. FILL MATERIAL SHALL BE FREE OF VEGETATION AND FOREIGN MATERIAL. THE FILL SHALL BE COMPACTED TO ASSURE UNIFORM SUPPORT OF THE SLAB, AND EXCEPT WHERE APPROVED, THE FILL DEPTHS SHALL NOT EXCEED 24" FOR CLEAN SAND OR GRAVEL AND 8" FOR EARTH. A 4" THICK BASE COURSE CONSISTING OF CLEAN GRADED SAND, GRAVEL, OR CRUGHED BLAST-FURNACE SLAG PASSING A 2" SIEVE SHALL BE PLACED ON THE PREPARED SUBGRADE WHEN THE SLAB IS BELOW GRADE. A BASE COURSE IS NOT REQUIRED WHEN A CONCRETE SLAB IS INSTALLED ON WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS CLASSIFIED AS GROUP I ACCORDING TO THE UNITED SOIL CLASSIFICATION SYSTEM IN ACCORDANCE WITH TABLE R405.1 OF THE NCRC. PROPERLY DEWATER EXCAVATION PRIOR TO POURING CONCRETE WHEN BOTTOM OF CONCRETE SLAB IS AT OR BELOW WATER TABLE.

SOIL BEARING CAPACITY - THE ALLOWABLE MINIMUM BEARING CAPACITY FOR SOIL IS ASSUMED TO BE 2000 PSF. CONTACT GEOTECHNICAL ENGINEER IF BEARING CAPACITY IS NOT ACHIEVED.

CONCRETE - CONCRETE SHALL CONFORM TO SECTION R4022 OF THE NCRC. CONCRETE REINFORCING STEEL TO BE ASTM A615 GRADE 60. WELDED WIRE FABRIC TO BE ASTM A185. MAINTAIN A MINIMUM CONCRETE COVER AROUND REINFORCING STEEL OF 3" IN FOOTINGS AND 1 1/2" IN SLABS. FOR POURED SECURE ALL BEAMS SUPPORTING ROOF TRUSSES OR RAFTERS TO THEIR RESPECTIVE BEARING. CONCRETE WALLS, CONCRETE COVER FOR REINFORCING STEEL MEASURED FROM THE INSIDE FACE OF SUPPORT MEMBERS WITH (1) SIMPSON CSIG STRAP PER CONNECTION LAPPING 14" MIN. ONTO EACH THE WALL SHALL NOT BE LESS THAN 3/4". CONCRETE COVER FOR REINFORCING STEEL MEASURED FROM THE OUTSIDE FACE OF THE WALL SHALL NOT BE LESS THAN 1 1/2" FOR #5 BARS OR SMALLER, AND NOT LESS THAN 2" FOR \*6 BARS OR LARGER.

CONCRETE CONTROL JOINTS - IF APPLICABLE, CONTROL JOINTS ARE TO BE SAWED TO A DEPTH OF 25% OF SLAB THICKNESS WITHIN 4 TO 12 HOURS OF CONCRETE FINISHING. CONTROL JOINTS SHOULD BE SPACED NO MORE THAN 12'-O" APART AND SECTIONS SHOULD BE RECTANGULAR WITH SIDE RATIOS NO GREATER THAN 15 LONG TO I WIDE.

MAGONRY - MAGONRY UNITS TO CONFORM TO ACE 530/AGCE 5/TMS 402. MORTAR SHALL CONFORM TO ASTM C270. REINFORCING STEEL TO BE ASTM A615 GRADE 60.

REBAR LAP SPLICES - REINFORCEMENT SHALL BE THE LONGEST LENGTHS PRACTICAL OR BE LAP SPLICED 30" MINIMUM FOR #4 REBAR, 38" MINIMUM FOR #5 REBAR, 45" MINIMUM FOR #6 REBAR, OR THE FULL DEPTH SOLID BLOCKED WITH LUMBER NOT LESS THAN 2" SPACED NOT MORE THAN 4'-0" O.C. MINIMUM REQUIRED LAP SPLICE LENGTH OF THE SMALLER BAR AS PER FIGURE R608.5.4(1) OF THE NCRC.

CONCRETE AND MAGONRY FOUNDATION WALLS - ALL CONCRETE AND MAGONRY FOUNDATION WALLS ARE TO BE CONSTRUCTED IN ACCORDANCE WITH THE PROVISIONS OF SECTION R404 OF THE NCRC OR IN ACCORDANCE WITH ACI 318, ACI 332, NCMA TR68-A OR ACE 530/AGCE 5/TMS 402, MAGONRY FOUNDATION WALLS ARE TO BE REINFORCED PER TABLE R404.1.(1) THROUGH R404.1.(4) OF THE NCRC. SUPPORT. FOR ALL HEADERS 8'-0" AND GREATER IN LENGTH, BOLT A 6" x 4" x 5/16" STEEL ANGLE CONCRETE FOUNDATION WALLS ARE TO BE REINFORCED PER TABLE R404.1.2(1) THROUGH R404.1.2(5) OF THE NCRC. PRECAST CONCRETE FOUNDATION WALLS ARE TO CONFORM TO SECTION R404.5 OF THE NCRC. STEP CONCRETE FOUNDATION WALLS TO 2 × 6 FRAMED WALLS AT 16" O.C. WHERE GRADE PERMITS (UNO).

PIERS - THE UNSUPPORTED HEIGHT OF MASONRY PIERS SHALL NOT EXCEED 10 TIMES THEIR LEAST DIMENSION. WHEN STRUCTURAL CLAY TILE HOLLOW CONCRETE MASONRY UNITS ARE USED FOR ISOLATED PIERS TO SUPPORT BEAMS AND GIRDERS, THE CELLULAR SPACES SHALL BE FILLED SOLIDLY WITH CONCRETE OR TYPE M OR 5 MORTAR, EXCEPT UNFILLED HOLLOW PIERS MAY BE USED IF THEIR UNSUPPORTED HEIGHT IS NOT MORE THAN FOUR TIMES THEIR LEAST DIMENSION. HOLLOW PIERS SHALL BE CAPPED WITH 4" OF SOLID MASONRY OR CONCRETE FOR ONE STORY AND 8" OF SOLID MASONRY OR CONCRETE FOR TWO STORY AND TWO AND ONE-HALF STORY OR SHALL HAVE CAVITIES OF THE TOP COURSE FILLED WITH CONCRETE OR GROUT OR OTHER APPROVED METHODS. SHADED OR NOTED PIERS ARE TO BE FILLED SOLID WITH CONCRETE OR GROUT OR OTHER APPROVED METHOD.

PIER/GIRDER LOCATION - THE CENTER OF EACH PIER SHALL BEAR IN THE MIDDLE THIRD OF ITS RESPECTIVE FOOTING. EACH GIRDER SHALL BEAR IN THE MIDDLE THIRD OF EACH PIER.

FOUNDATION ANCHORAGE - FOR 115, 120, AND 130 MPH WIND ZONES, THE WOOD SOLE PLATE AT EXTERIOR WALLS ON MONOLITHIC SLABS, WOOD SOLE PLATES OF BRACED WALL PANELS AT BUILDING INTERIORS ON MONOLITHIC SLAB, AND ALL WOOD SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WITH ANCHOR BOLTS SPACED A MAXIMUM OF 6'-O" O.C. (4'-O" O.C. FOR 130 MPH WIND ZONE) AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF TWO BOLTS PER PLATE SECTION. BOLTS SHALL BE AT LEAST 1/2" IN DIAMETER AND SHALL EXTEND A MINIMUM OF 1" INTO MASONRY OR CONCRETE (15" INTO MAGONRY FOR 130 MPH WIND ZONE). BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. INTERIOR BEARING WALL SOLE PLATES ON MONOLITHIC SLAB FOUNDATIONS NOT PART OF A BRACED WALL PANEL SHALL BE POSITIVELY ANCHORED WITH APPROVED FASTENERS. FOR 140 MPH AND 150 MPH WIND ZONES, FOUNDATION ANCHORAGE IS TO COMPLY WITH SECTION 4504 OF THE NCRC.

FRAMING LUMBER - ALL FRAMING LUMBER SHALL BE #2 SYP MINIMUM (Fb = 150 PSI, Fv = 175 PSI, E = 1400000 PSI) UNLESS NOTED OTHERWISE (UNO). ALL TREATED LUMBER SHALL BE #2 SYP MINIMUM (Fb = 150 PSI, Fv = 115 PSI, E = 1400000 PSI) UNLESS NOTED OTHERWISE (UNO).

STEEL BEAMS - ALL STRUCTURAL STEEL SHALL BE ASTM A36. STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3 1/2" AND FULL FLANGE WIDTH (UNO). PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO LAG SCREWS (1/2" DIAMETER X 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOIGTS ARE TOE NAILED TO THE 2x NAILER ON TOP OF THE STEEL BEAM, AND THE 2x NAILER IS SECURED TO THE BEAM FLANGE OR THE TOP OF THE STEEL BEAM IS INSTALLED WITHIN 1 1/2" OF THE TOP OF THE JOISTS.

POINT LOADS - SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. SHADED SQUARES DENOTE POINT LOADS FROM ABOVE WHICH REQUIRE SOLID BLOCKING TO SUPPORTING MEMBER BELOW.

LOAD BEARING HEADERS - ALL LOAD BEARING HEADERS ARE TO CONFORM TO TABLES R602.1(1), R602.7(2) AND R602.7(3) OR BE (2) 2 x 10 WITH (1) JACK AND (1) KING STUD EACH END (UNO), WHICHEVER IS GREATER ALL HEADERS ARE TO BE SECURED TO EACH JACK STUD WITH (4) 8d NAILS. ALL BEAMS ARE TO BE SUPPORTED WITH (2) STUDS AT EACH BEARING POINT (UNO).

BEAM BEARING - ALL BEAMS, HEADERS, OR GIRDER TRUSSES PARALLEL TO BEARING WALL ARE TO BEAR FULLY ON (1) JACK OR (2) STUDS MINIMUM OR THE NUMBER OF JACKS OR STUDS NOTED. ALL BEAMS OR GIRDER TRUSSES PERPENDICULAR TO WALL AND SUPPORTED BY (3) STUDS OR LESS ARE C TO HAVE I 1/2" MINIMUM BEARING (UNO). ALL BEAMS OR GIRDER TRUSSES PERPENDICULAR TO WALL AND SUPPORTED BY MORE THAN (3) STUDS OR OTHER NOTED COLUMN ARE TO BEAR FULLY ON SUPPORT COLUMN FOR ENTIRE WALL DEPTH (UNO). BEAM ENDS THAT BUTT INTO ONE ANOTHER ARE TO EACH BEAR EQUAL LENGTHS (UNO).

STEEL FLITCH PLATE BEAM - STEEL FLITCH PLATE BEAMS SHALL BE BOLTED TOGETHER USING 1/2" DIAMETER BOLTS (ASTM A307) WITH WASHERS PLACED AT THREADED END OF BOLT. BOLTS SHALL BE SPACED AT 24" CENTERS (MAXIMUM), AND STAGGERED AT TOP AND BOTTOM OF BEAM (2" EDGE DISTANCE), WITH (2) BOLTS LOCATED 6" FROM EACH END (UNO).

I-JOIST/TRUSS LAYOUTS - ALL I-JOIST OR TRUSS LAYOUTS ARE TO BE IN COMPLIANCE WITH THE OVERALL DESIGN SPECIFIED ON THE PLANS. ALL DEVIATIONS ARE TO BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD PRIOR TO INSTALLATION.

WALL BRACING - BRACED WALL PANELS SHALL BE CONSTRUCTED ACCORDING TO SECTION R602.10 OF THE INTERNATIONAL RESIDENTIAL CODE, 2015 EDITION. THE LENGTH OF BRACING IN EACH BRACED WALL LINE SHALL COMPLY WITH TABLE R602.10.3(1) OR R602.10.3(3) OF THE INTERNATIONAL RESIDENTIAL CODE, 2015 EDITION, WHICHEVER IS GREATER. REFER TO WALL BRACING DETAILS WHEN  $H_{
m c}$ 

PROVIDED.

UPLIFT CONNECTIONS - SECURE ALL RAFTERS TO EXTERIOR WALL OR SUPPORTING BEAM WITH SIMPSON H2.5A HURRICANE TIE, EQUIVALENT CONNECTOR OR ALTERNATE CONNECTION CONFORMING TO H THE NCRC. SECURE EACH ROOF TRUGG TO EXTERIOR WALL OR SUPPORTING BEAM WITH UPLIFT CONNECTOR RATED AT OR ABOVE UPLIFT LOAD SHOWN ON TRUSS PROFILE. INSTALL ALL RAFTER/ROOF TRUSS-TO-WALL CONNECTORS DIRECTLY TO WALL FRAMING THROUGH EXTERIOR SHEATHING, WHERE CONNECTORS ARE INSTALLED TO INSIDE FACE OF TOP PLATES, INSTALL UPLIFT CONNECTOR SECURING RAFTER/ROOF TRUSS DIRECTLY TO WALL STUD BELOW OR INSTALL ADDITIONAL EQUIVALENT CONNECTOR SECURING THE TOP PLATE TO THE WALL STUD.

FRAMING MEMBER OR (2) SIMPSON MTSI2 TWIST STRAPS (TYP. UNLESS NOTED OTHERWISE.)

BRACED WALL PANELS LOCATED AT EXTERIOR WALLS SUPPORTING RAFTERS OR ROOF TRUSSES, INCLUDING STORIES BELOW TOP STORY, SHALL BE CONSTRUCTED TO RESIST UPLIFT FORCES CONTINUOUS FROM ROOF TO FOUNDATION. EXTERIOR SHEATHING SHALL SECURE STORY ABOVE AND BELOW FLOOR BAND BY LAPPING ONTO OR ACROSS BAND. WHERE EXTERIOR SHEATHING IS INSTALLED WITH HORIZONTAL JOINT SPLICE AT THE TOP AND/OR BOTTOM OF THE FLOOR BANDS, SECURE EXTERIOR SHEATHING AND/OR BAND ACROSS SPLICE AT THE BRACED WALL PANELS WITH SIMPSON LTP4 FRAMING PLATES AT 24" O.C. MAX. OR SIMPSON CSIG COIL STRAPS AT 48" O.C. MAX. (TWO STRAPS MIN. PER BRACED WALL PANEL) LAPPING THE WALL FRAMING 14" MIN.

WALLS PARALLEL TO JOISTS - PROVIDE DOUBLE JOIST UNDER ALL WALLS PARALLEL TO FLOOR JOISTS. DOUBLE JOISTS SEPARATED TO PERMIT THE INSTALLATION OF PIPING OR VENTS SHALL BE PROVIDE SUPPORT UNDER ALL WALLS PARALLEL TO FLOOR TRUGGES OR I-JOISTS PER MANUFACTURER'S SPECIFICATIONS. INSTALL BLOCKING BETWEEN JOISTS OR TRUSSES FOR POINT LOAD SUPPORT FOR ALL POINT LOADS ALONG OFFSET LOAD LINES.

BRICK SUPPORT - FOR ALL HEADERS SUPPORTING BRICK VENEER THAT ARE LESS THAN 8'-0" IN LENGTH, REST A 6" x 4" x 5/16" STEEL ANGLE WITH 4" MINIMUM EMBEDMENT AT SIDES FOR BRICK TO HEADER WITH 1/2" LAG SCREWS AT 12" O.C. STAGGERED FOR BRICK SUPPORT. FOR ALL BRICK SUPPORT AT ROOF LINES, BOLT A 6" x 4" x 5/16" STEEL ANGLE TO 2 x 10 BLOCKING INSTALLED BETWEEN WALL STUDS WITH 1/2" LAG SCREWS AT 12" O.C. STAGGERED AND IN ACCORDANCE WITH SECTION RTØ3.8.2.2 OF THE 2018 NCRC.

ROOF MEMBER SUPPORT - FOR STICK FRAMED ROOFS: CIRCLES DENOTE (3) 2 x 4 POSTS FOR ROOF MEMBER SUPPORT.

HIP SPLICES - HIP SPLICES ARE TO BE SPACED A MINIMUM OF 8'-O". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS AT 16" O.C.

DECKS - ALL DECK FRAMING, LATERAL BRACING, GUARDRAIL CONSTRUCTION, ATTACHMENT TO THE HOUSE STRUCTURE AND THE CONNECTIONS WITHIN THE DECK FRAMING ARE TO COMPLY WITH APPENDIX M OF THE NORC.

ENERGY EFFICIENCY - ENERGY EFFICIENCY COMPLIANCE TO BE IN ACCORDANCE WITH CHAPTER II OF THE NCRC. THE BUILDING THERMAL ENVELOPE SHALL MEET THE REQUIREMENTS OF TABLE NII02.12 BASED ON THE CLIMATE ZONE SPECIFIED.

ENGINEERED LUMBER - LAMINATED VENEER LUMBER (LVL) SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES: Fb = 2600 PSI, Fv = 285 PSI, E = 1900000 PSI. LAMINATED STRAND LUMBER (LSL) SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES: Fb = 2325 PSI, Fv = 525 PSI, E = 1550000 PSI. PARALLEL STRAND LUMBER (PSL) UP TO 1" DEPTH SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES: Fc = 2500 PSI, E = 1800000 PSI, PARALLEL STRAND LUMBER (PSL) MORE THAN 7' DEPTH SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES: FC = 2900 PSI, E = 2000000 PSI. INSTALL ALL CONNECTIONS PER MANUFACTURER'S SPECIFICATIONS.

DORMER FRAMING - FRAME DORMER WALLS ON TOP OF DOUBLE OR TRIPLE RAFTERS AS SHOWN (UNO). FRAME DORMER WALLS ON TOP OF 2 x 4 LADDER FRAMING AT 24" O.C. BETWEEN ADJACENT ROOF TRUSSES. STICK FRAME OVER-FRAMED ROOF SECTIONS WITH 2 x 8 RIDGES, 2 x 6 RAFTERS AT 16" O.C. AND FLAT 2 × 10 VALLEYS (UNO).

## WIND ZONE AND CLIMATE ZONE BY COUNTY

<u>///</u>			
<u>COUNTY</u>	<u>WIND ZONE (MPH)</u> CLIMATE ZONE	<u>COUNTY</u>	<u>WIND ZONE (MPH)/</u> CLIMATE ZONE
	115 / 4	JOHNSTON	120/3
	115 / 4	JONES	140 / 3
	SMR / 5	LEE	115 / 4
	115/3	LENOIR	130/3
ASHE	SMR / 5	LINCOLN	115 / 4
AVERY	SMR / 5	MACON	115 / 4
BEAUFORT	130 / 3	MADISON	SMR/4
BERTIE <sup>a</sup>	120/130 / 4	MARTIN <sup>9</sup>	120/130 / 3
BLADEN <sup>B</sup>	130/140 / 3	MCDOWELL	115 / 4
BRUNSWICK <sup>C</sup>	140/150 / 3-WHC	MECKLENBURG	115 / 3
BUNCOMBE	SMR / 4	MITCHELL	SMR / 5
BURKE	115 / 4	MONTGOMERY	115/3
CABARRUS	115/3	MOORE	115/3
CALDWELL	115 / 4	NASH	115 / 4
	130 / 3	NEW HANOVER <sup>h</sup>	
	150 / 3-WHC	NORTHAMPTON	115 / 4
CASWELL	115 / 4	ONSLOW	130/140/150 / 3-WHC
	115 / 4	ORANGE	115 / 4
CHATHAM		PAMLICO	140 / 3
CHEROKEE	115 / 4 115 / 4	PASQUOTANK	130 / 3
CHOWAN	130/3	PENDER	130/140/150 / 3-WHC
	115 / 4	PERQUIMANS	130 / 3
	115 / 4	PERSON	115 / 4
Columbus	140 / 3-WHC	PITT	130 / 3
CRAVEN .	140 / 3	POLK	115 / 4
CUMBERLAND <sup>d</sup>	120/130 / 3	RANDOLPH	115 / 3
CURRITUCK	130 / 3	RICHMOND	120/3
DARE	130/140 / 3	ROBESON	130 / 3
• •	115/3	ROBESON ROCKINGHAM	115 / 4
	115 / 4		
		ROWAN	115/3
	130 / 3	RUTHERFORD	115 / 4
DURHAM	115 / 4	SAMPSON	130 / 3
	115 / 3	SCOTLAND	120 / 3
ORSYTH	115 / 4	STANLY	115/3
RANKLIN	115 / 4	STOKES	115 / 4
ASTON	115/3	SURRY	115 / 4
ATES	120 / 4	SWAIN	SMR / 4
RAHAM	SMR/4	TRANGYLVANIA	115 / 4
RANVILLE	115 / 4	TYRRELL	130 / 3
REENE	130 / 3	UNION	115/3
JUILFORD	115 / 4	VANCE	115 / 4
1ALIFAX	115 / 4	WAKE	115 / 4
ARNETT	115 / 4	WARREN	115 / 4
ATWOOD	SMR / 4	WASHINGTON	130 / 3
ENDERSON	115 / 4	WATAUGA	SMR / 5
ERTFORD	115 / 4	WAYNE	130 / 3
IOKE ,	120/3	WILKES	115 / 4
IYDE <sup>f</sup>	130/140 / 3	WILSON	120/3
REDELL	115 / 4	YADKIN	115 / 4
IACKSON	SMR/4	YANCEY	SMR / 5

-SMR DESIGNATES "SPECIAL MOUNTAIN REGION"

-WHC DESIGNATES "WARM-HUMID COUNTY"

a. 120 MPH ZONE WEST OF HWY 17, 130 MPH ZONE EAST OF HWY 17.

6. 130 MPH ZONE WEST OF HWY 701, 130 MPH ZONE EAST OF HWY 701. c. 140 MPH ZONE WEST OF HWY 17, 150 MPH ZONE EAST OF HWY 17, 150

MPH ZONE ON BALD HEAD ISLAND.

d. 120 MPH ZONE WEST OF 1-95, 130 MPH ZONE EAST OF 1-95. e. 130 MPH ZONE WEST OF US ROUTE 264, 140 MPH ZONE EAST OF US

ROUTE 264. f. 130 MPH ZONE WEST OF US ROUTE 264, 140 MPH ZONE EAST OF US

ROUTE 264. a. 120 MPH ZONE WEST OF HWY 17, 130 MPH ZONE EAST OF HWY 17,

h 140 MPH ZONE WEST OF HWY 17, 150 MPH ZONE EAST OF HWY 17. 1. 130 MPH ZONE WEST OF HWY 17, 140 MPH ZONE EAST OF HWY 17 TO THE INTRACOASTAL WATERWAY, 150 MPH ZONE EAST OF THE INTRACOASTAL WATERWAY.

j. 140 MPH ZONE IN THE TOWNSHIP OF TOPSAIL WEST OF THE INTRACOASTAL WATERWAY, 150 MPH ZONE EAST OF THE INTRACOASTAL WATERWAY, 130 MPH ZONE IN THE REMAINDER OF THE COUNTY.

			INSULATION A		<b>TABLE NII02.12</b> RATION REQUIREN	1ENTS BY CO	OMPONENT <sup>a</sup>
CLIMATE ZONE	FENESTRATION U-FACTOR <sup>6, J</sup>	SKYLIGHT <sup>b</sup> U-FACTOR	GLAZED FENESTRATION SHGC <sup>D, K</sup>	CEILING R-VALUE <sup>®</sup>	WOOD FRAME WALL R-VALUE <sup>®</sup>	MASS WALL R-VALUE <sup>1</sup>	FL <i>OO</i> R R-VALUE
3	Ø.35	Ø.55	0.30	38 OR 30 CI	15 OR 13+2.5 <sup>h</sup>	5/13 OR 5/10 CI	19
4	Ø.35	Ø.55	0.30	38 OR 30 CI	15 OR 13+2.5 <sup>h</sup>	5/13 OR 5/10 CI	19
5	Ø.35	Ø.55	NR	38 OR 30 CI	19, 13+5 <sup>n</sup> OR 15+3 <sup>h</sup>	13/17 OR 13/12.5 CI	30 <sup>9</sup>

a. R-VALUES ARE MINIMUMS. U-FACTORS AND SHGC ARE MAXIMUMS. WHEN INGULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS OF THE INSULATION, THE INSTALLED R-VALUE OF THE INGULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE.

b. THE FENESTRATION U-FACTOR COLUMN EXCLUDES SKYLIGHTS. THE SHGC COLUMN APPLIES TO ALL GLAZED FENESTRATION.

c. "10/15" MEANS R-10 CONTINUOUS INSULATED SHEATHING ON THE INTERIOR OR EXTERIOR OF THE HOME OR R-15 CAVITY INSULATION AT THE INTERIOR OF THAN 0.10 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE THE BASEMENT WALL OR CRAWL SPACE WALL.

d. R-5 SHALL BE ADDED TO THE REQUIRED SLAB EDGE R-VALUES FOR HEATED SLABS. FOR MONOLITHIC SLABS, INSULATION SHALL BE APPLIED FROM THE INSPECTION GAP DOWNWARD TO THE BOTTOM OF THE FOOTING OR A MAXIMUM OF 24" BELOW GRADE, WHICHEVER IS LESS. FOR FLOATING SLABS, INSULATION SHALL EXTEND TO THE BOTTOM OF THE FOUNDATION WALL OR 24", WHICHEVER IS LESS. (SEE APPENDIX O)

e. DELETED F. BASEMENT WALL INSULATION IS NOT REQUIRED IN WARM-HUMID LOCATIONS AS DEFINED BY FIGURE NIIOI.1 AND TABLE NIIOI.1.

WITH INSULATED SHEATHING OF AT LEAST R-2.

q. OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY, R-19 MINIMUM. h. THE FIRST VALUE IS CAVITY INSULATION, THE SECOND VALUE IS CONTINUOUS INSULATION, SO "13+5" MEANS R-13 CAVITY INSULATION PLUS R-5 CONTINUOUS INSULATION. IF STRUCTURAL SHEATHING COVERS 25% OR LESS OF THE EXTERIOR, INSULATING SHEATHING IS NOT REQUIRED WHERE STRUCTURAL SHEATHING IS USED. IF STRUCTURAL SHEATHING COVERS MORE THAN 25% OF EXTERIOR, STRUCTURAL SHEATHING SHALL BE SUPPLEMENTED

K IN ADDITION TO THE EXEMPTION IN SECTION NII02.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A SHGC NO GREATER COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY. I. R-30 SHALL BE DEEMED TO SATISFY THE CEILING INSULATION REQUIREMENT WHEREVER THE FULL HEIGHT OF UNCOMPRESSED R-30 INSULATION EXTENDS OVER THE WALL TOP PLATE AT THE EAVES. OTHERWIGE R-38 INSULATION IS REQUIRED WHERE ADEQUATE CLEARANCE EXISTS OR INSULATION MUST EXTEND TO EITHER THE INSULATION BAFFLE OR MITH LIMI SPA

THIN I" OF THE ATTIC ROOF DECK.
. TABLE VALUE REQUIRED EXCEPT FOR ROOF EDGE WHERE THE SPACE IS
11TED BY THE PITCH OF THE ROOF, THERE THE INGULATION MUST FILL THE
ACE UP TO THE AIR BAFFLE.
R-19 FIBERGLASS BATTS COMPRESSED AND INSTALLED IN A NOMINAL
5 FRAMING CAVITY IS DEEMED TO COMPLY. FIBERGLASS BATTS RATED

SLAB<sup>d</sup>

R-VALUE

AND DEPTH

CRAWL

R-VALUE

5/13

1Ø/13

10/19

SPACE<sup>C</sup> WALL

BASEMENT

R-VALUE

5/13<sup>f</sup>

10/13

10/13

I. THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION

. IN ADDITION TO THE EXEMPTION IN SECTION NII02.3.3, A MAXIMUM OF TWO

GREATER THAN 0.55 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMUN

CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.

GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A U-FACTOR NO

WALL<sup>C, O</sup>

R-19 OR HIGHER COMPRESSED AND INSTALLED IN A 2x4 WALL IS NOT DEEMED TO COMPLY.

O. BASEMENT WALL MEETING THE MINIMUM MASS WALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM REQUIREMENT.

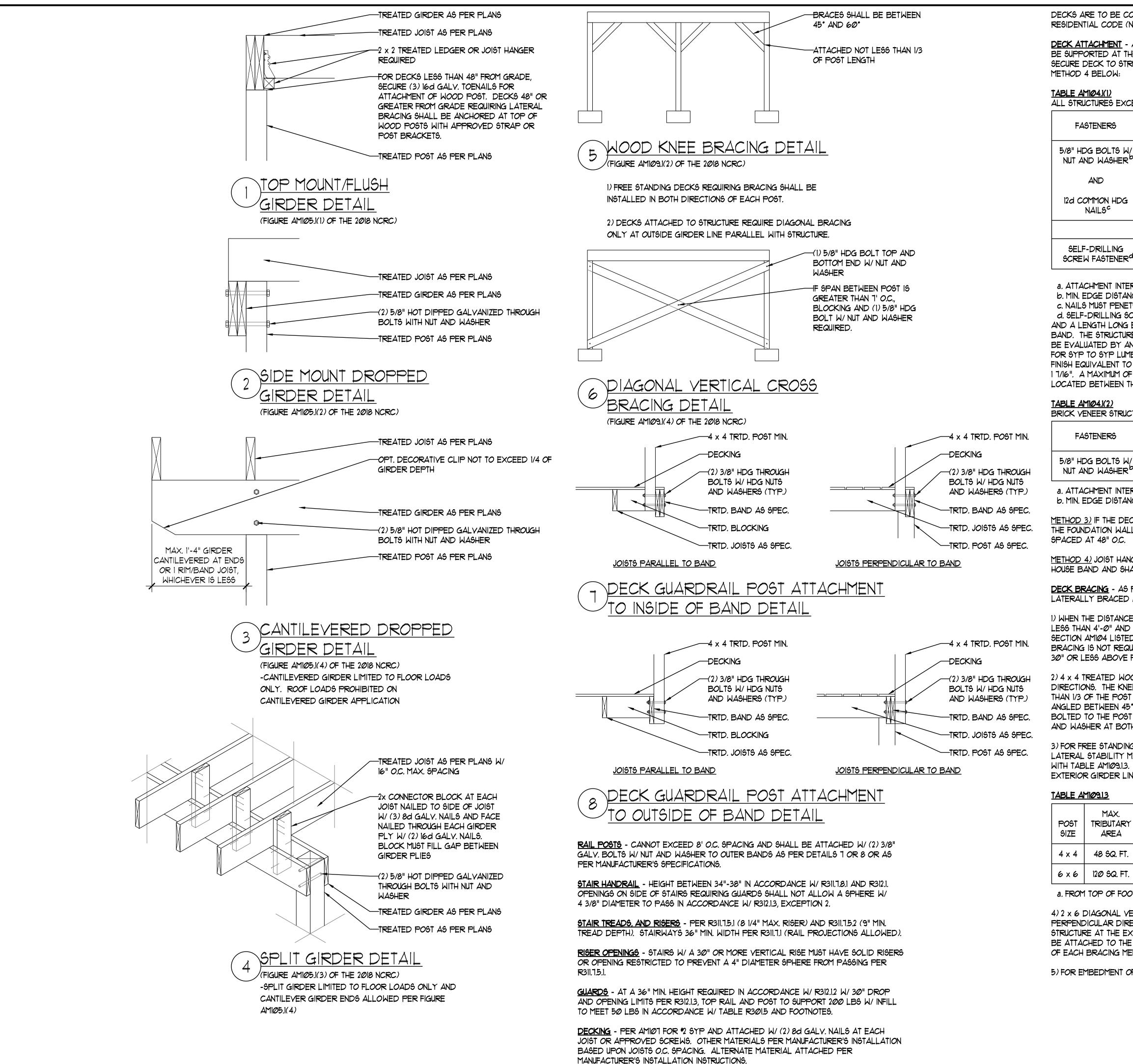
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	SPECTRA ENGINEERING AND DESIGN, PLLC SCALE	RALEIGH, NORTH CAROLINA 27627	TEL.: (919) 228-2841 LICENSE NO. NC: P-Ø946 VA: ØØØ462	
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11/1 STF	9/202 STA		RD NOT	ES -

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	(Positive and negative por)						
WIND ZONE	MEAN ROOF		ROOF CLADDING (PSF) BY ROOF PITCH				
(MPH)	HEIGHT (FT)	Ø < X < 2.5	2.5 < X < T	T < X < 12	CLADDING (PSF)		
	< 3Ø	10.0, -36.0	10.0, -33.0	13.1, -16.0	14.3, -19.0		
115	3Ø < h < 35	10.5, -37.8	10.5, -34.7	13.8, -16.8	15.0, -20.0		
611	35 < h < 40	10.9, -39.2	10.9, -36.0	14.3, -17.4	15.6, -20.7		
	40 < h < 45	11.2, -40.3	11.2, -37.Ø	14.7, -17.9	16.Ø, -21.3		
	< 3Ø	10.0, -39.0	10.0, -36.0	14.2, -18 <i>.</i> 0	15.5, -20.0		
120	3Ø < h < 35	10.5, -41.0	10.5, -36.5	14.9, -18.9	16.3, -21.0		
120	35 < h < 40	10.9, -42.5	10.9, -37.9	15.5, -19.6	16.9, -21.8		
	40 < h < 45	11.2, -43.7	11.2, -39.Ø	15.9, -20.2	17.4, -22.4		
	< 3Ø	10.0, -46.0	10.5, -43.0	16.7, -21.Ø	18.2, -24 <i>.</i> Ø		
120	3Ø < h < 35	10.5, -48.3	11.Ø, -45.2	17.5, -22.1	19.1, -25.2		
130	35 < h < 40	10.9, -50.1	11.4, -46.9	18.2, -22.9	19.8, -26.2		
	40 < h < 45	11.2, -51.5	11.8, -48.2	18.7, -23.5	20.4, -26.9		
	< 3Ø	10.0, 53.0	12.2, -49.0	19.4, -24.0	21.2, -28.Ø		
140	3Ø < h < 35	10.5, -55.7	12.8, -51.5	2Ø.4, -25.2	22.3, -29.4		
1462	35 < h < 40	10.9, -57.8	13.3, -53.4	21.1, -26.2	23.I, -3Ø.5		
	4Ø < h < 45	11.2, -59.4	13.7, -54.9	21.7, -26.9	23.7, -31.4		
	< 3Ø	9.9, -61.Ø	14.0, -57.0	22.2, <b>-</b> 28.Ø	24.3, <b>-</b> 32.Ø		
15.0	3Ø < h < 35	10.4, -64.1	14.7, -59.9	23.3, -29.4	25.5, -33.6		
150	35 < h < 40	10.8, -66.5	15.3, <b>-6</b> 2.1	24.2, -3Ø.5	26.5, -34.9		
	40 < h < 45	11.1, -68.3	15.7, -63.8	24.9, -31.4	27.2, -35.8		

WALL AND ROOF CLADDING DESIGN LOADS

IS ON THE INTERIOR OF THE MASS WALL



DECKS ARE TO BE CONSTRUCTED AS PER APPENDIX M OF THE 2018 NORTH CAROLINA RESIDENTIAL CODE (NCRC) DECK ATTACHMENT - AS PER SECTION AMIØ4 OF THE 2018 NCRC, WHEN A DECK SHALL BE SUPPORTED AT THE STRUCTURE BY ATTACHING THE DECK TO THE STRUCTURE. SECURE DECK TO STRUCTURE AS PER TABLE AMIØ4.1(1), TABLE AMIØ4.1(2), METHOD 3 OR ALL STRUCTURES EXCEPT BRICK VENEER STRUCTURES 8' MAX. JOIST 16' MAX. JOIST SPAN<sup>a</sup> SPAN<sup>a</sup> @ 3'-6" O.C. 1 @ |'-8" O.C. AND AND 2 @ 8" O.C. 3 @ 6" O.C. OR 12" O.C. 6" O.C. STAGGERED STAGGERED a. ATTACHMENT INTERPOLATION BETWEEN 8' AND 16' JOISTS SPAN IS ALLOWED. b. MIN. EDGE DISTANCE FOR BOLTS IS 2 1/2". C. NAILS MUST PENETRATE THE SUPPORTING STRUCTURE BAND A MIN. OF 1 1/2". d. SELF-DRILLING SCREW FASTENER HAVING A MINIMUM SHANK DIAMETER OF Ø.195 AND A LENGTH LONG ENOUGH TO PENETRATE THROUGH THE SUPPORTING STRUCTURE BAND. THE STRUCTURE BAND SHALL HAVE A MINIMUM DEPTH OF 11/8". SCREW SHALL BE EVALUATED BY AN APPROVED TESTING AGENCY FOR ALLOWABLE SHEAR LOAD FOR SYP TO SYP LUMBER OF 250 LBS. AND SHALL HAVE A CORROSION-RESISTANT FINISH EQUIVALENT TO HOT DIP GALVANIZED. MINIMUM EDGE DISTANCE FOR SCREWS IS 1 1/16". A MAXIMUM OF 1/2" THICK WOOD STRUCTURAL PANEL IS PERMITTED TO BE LOCATED BETWEEN THE DECK LEDGER AND THE STRUCTURE BAND. BRICK VENEER STRUCTURES 8' MAX. JOIST 16' MAX. JOIST SPAN<sup>a</sup> SPAN<sup>a</sup> 1 @ 2'-4" O.C. | @ |'-4" O.C. a. ATTACHMENT INTERPOLATION BETWEEN 8' AND 16' JOISTS SPAN IS ALLOWED b. MIN. EDGE DISTANCE FOR BOLTS IS 2 1/2" METHOD 3) IF THE DECK BAND IS SUPPORTED BY A MIN. OF 1/2" MASONRY LEDGE ALONG THE FOUNDATION WALL, SECURE DECK TO STRUCTURE W/ 5/8" HDG BOLTS W/ WASHERS METHOD 4) JOIST HANGERS OR OTHER MEANS OF ATTACHMENT MAY BE CONNECTED TO HOUSE BAND AND SHALL BE PROPERLY FLASHED. DECK BRACING - AS PER SECTION AMION OF THE 2018 NCRC, THE DECK SHALL BE LATERALLY BRACED AS PER ONE OF THE FOLLOWING: 1) WHEN THE DISTANCE FROM THE TOP OF THE DECK FLOOR TO THE FINISHED GRADE IS LEGS THAN 4'-O" AND THE DECK IS ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION AMIØ4 LISTED ABOVE, LATERAL BRACING IS NOT REQUIRED. LATERAL BRACING IS NOT REQUIRED FOR FREE STANDING DECKS WITH A DECK FLOOR HEIGHT OF 30" OR LESS ABOVE FINISHED GRADE. 2) 4 x 4 TREATED WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 45° AND 60° FROM THE HORIZONTAL. KNEE BRACES SHALL BE BOLTED TO THE POST AND THE GIRDER/DOUBLE BAND W/ (1) 5/8" HDG BOLT WITH NUT AND WASHER AT BOTH ENDS OF THE BRACE PER DETAIL 5.

3) FOR FREE STANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POST IN ACCORDANCE WITH TABLE AMIO9.1.3. DECKS ATTACHED TO STRUCTURE CAN ALGO BE BRACED ON EXTERIOR GIRDER LINE W/ EMBEDMENT OPTION.

MAX. BUTARY AREA	MAX. POST HEIGHT <sup>a</sup>	EMBEDMENT DEPTH	CONCRETE DIAMETER
5 Q. FT.	4'-Ø"	2'-6"	'-Ø"
9 SQ. FT.	6'-Ø"	3'-6"	l'-8"

a. FROM TOP OF FOOTING TO TOP OF DECKING

4) 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO PERPENDICULAR DIRECTIONS FOR FREE STANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE 2 x 6'S SHALL BE ATTACHED TO THE POSTS W/ (1) 5/8" HDG BOLT W/ NUT AND WASHER AT EACH END OF EACH BRACING MEMBER PER DETAIL 6.

5) FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE CHAPTER 46.

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SPECTRA ENGINEERING AND DESIGN, PLLC     Date:       P.O. BOX 31625     SCALE:       P.O. BOX 31625     SCALE:       P.O. BOX 31625     SCALE:       RALEIGH, NORTH CAROLINA 21621     DRAMN       TEL.: (919) 228-2841     DRAMN       LICENSE NO. NC: P-0946 VA: 000462     ENGINEE					
STANDARD STRUCTURAL NOTES					
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