

Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0524-2974 Lot 10 Magnolia Hills

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I68612293 thru I68612314

My license renewal date for the state of North Carolina is December 31, 2024.

North Carolina COA: C-0844



October 3,2024

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required



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2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 17-3-0, Exterior(2) 17-3-0 to 21-7-13, Interior(1) 21-7-13 to 34-5-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



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	11-8-9	11-0-14	11-8-9	—
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. DEFL. in TC 0.33 Vert(LL) -0.16	(loc) l/defl L/d 10-13 >999 360	PLATES GRIP MT20 244/190
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	BC 0.62 Vert(CT) -0.26 WB 0.42 Horz(CT) 0.05 Matrix-S Wind(LL) 0.05	2-13 >999 240 8 n/a n/a 2-13 >999 240	Weight: 280 lb FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x6 SP No.1 *Except
	7 10 3 13 2v4 CD N

WEBS 2x6 SP No.1 *Except* 7-10,3-13: 2x4 SP No.2 REACTIONS. (size) 2=0-5-8, 8=0

 (size) 2=0-5-8, 8=0-5-8 Max Horz 2=-293(LC 10) Max Uplift 2=-90(LC 12), 8=-90(LC 13) Max Grav 2=1662(LC 19), 8=1662(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2274/402, 3-5=-2130/506, 5-7=-2130/506, 7-8=-2274/402

BOT CHORD 2-13=-164/1998, 10-13=0/1296, 8-10=-176/1802

WEBS 5-10=-184/1111, 7-10=-578/335, 5-13=-184/1111, 3-13=-578/336

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 17-3-0, Exterior(2) 17-3-0 to 21-7-13, Interior(1) 21-7-13 to 35-6-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



Structural wood sheathing directly applied or 4-10-10 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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Job		Truss	Truss Type	Qty	Ply	Lot 10 Magnolia Hills	
							168612298
J0524-2974		B2	COMMON GIRDER	1	2		
					_	Job Reference (optional)	
Comtech, Inc,	Fayettevi	lle, NC - 28314,			8.630 s Ji	Il 12 2024 MiTek Industries, Inc. Tue Oct 1 14:28:08 2024	Page 2
			ID:JJp	3_bNirdpel	XA5mDh	25?y7p3U-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7	J4zJC?f

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 11=-1347(B) 12=-1349(B) 13=-1347(B) 14=-1347(B) 15=-1347(B) 16=-1347(B) 17=-1347(B) 18=-1347(B) 19=-1347(B) 20=-1351(B)

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Job	Truss	Truss Type	Qty	Ply	Lot 10 Magnolia Hills	002213831
J0524-2974	C1-GE	ATTIC	1	1		1060 12299
Comtech, Inc, Fayette	 ville, NC - 28314,			8.630 s Ji	Job Reference (optional) JI 12 2024 MiTek Industries	s, Inc. Tue Oct 1 14:28:09 2024 Page 1
	, 1-2-8	ID:J 5-3-4 7-10-3 8-11-12 11-3-0 13-6-5	p3_bNirdpeL	XA5mDh?	22-6-0 23-8-8	NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f
	1-2-8	5-3-4 2-6-15 1-1-8 2-3-4 2-3-4	1-1-8 2-6-15		5-3-4 1-2-8	
		4x6 =				Scale = 1:81.1
	T	6				
		2x4 =	2x4 =			
		5	7			
		2x4		2x4	+	
	2-6-1	4 x6 //		8		
		8-24		$\left \right\rangle$	4x6 ∖\	
		3 11-6-0			¢.	
	2	5-6-0				
	ਸ਼ੂ 1 ਕ		-			²
	⁴ 4 ∞1			 ~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		1-1
	5x8	14 2x6 2x6 2x6	13 2x6	12	5x8 U	
		8x8 =	6x8 = 8	8x8 =		
	F	5-3-4 11-3-0 ^{2x6}	17-2-12	2	22-6-0	
Plate Offsets (X,Y) [6:0)-3-0,Edge]	5-3-4 5-11-12	5-11-12		5-3-4 '	
LOADING (psf)	SPACING- 2-0-0	CSI. DEFL	. in	(loc)	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.11 Vert(L	L) -0.00	11	n/r 120	MT20 244/190
BCLL 0.0 *	Rep Stress Incr YES	WB 0.15 Horz(CT) -0.00 CT) 0.00	10	n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S				Weight: 236 lb FT = 20%
LUMBER-	<u>)</u> 1	BRAC	ING-	Structur	al wood sheathing direct	ly applied or 6-0-0 oc purlins
BOT CHORD 2x10 SP N	lo.1 *Except*	BOT	CHORD	Rigid ce	iling directly applied or 1	0-0-0 oc bracing.
12-14: 2x6 WEBS 2x6 SP No	5 SP No.1 0.1					
WEDGE Left: 2x4 SP No 2 Right: 2	2x4 SP No 2					
(lb) - Max Horz	ngs 22-6-0. 2=295(LC 11)					
Max Uplift	 All uplift 100 lb or less at jo 13) 	int(s) except 14=-117(LC 12), 12=-116(LC				
Max Grav	All reactions 250 lb or less	at joint(s) except 2=576(LC 1), 14=1026(LC				
	20), 12-1023(EC 21), 10-37					
FORCES. (lb) - Max. Con TOP CHORD 2-4=-647	mp./Max. Ten All forces 250 7/29, 4-5=-566/133, 7-8=-565) (lb) or less except when shown. '133, 8-10=-644/24				
BOT CHORD 2-14=0/3 WEBS 4-14=-52	88, 12-14=0/388, 10-12=0/38	38 350/163				
NOTES						
1) Unbalanced roof live loa	ads have been considered for	this design.				
 Wind: ASCE 7-10; Vult= and C-C Corner(3) -1-0- 	=130mph Vasd=103mph; TCI -14 to 3-3-15. Exterior(2) 3-3-	DL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp 15 to 11-3-0. Corner(3) 11-3-0 to 15-7-13. Ex	C; Enclosed terior(2) 15-	; MWFRS 7-13 to 23	S (envelope) 3-6-14	ANNULLI
zone;C-C for members	and forces & MWFRS for rea	ctions shown; Lumber DOL=1.60 plate grip D	OL=1.60			TH CARO
4) This truss has been des	signed for a 10.0 psf bottom c	hord live load nonconcurrent with any other li	ve loads.			NOP EESCIA N.
5) 1 his truss has been de will fit between the botto	esigned for a live load of 30.0 om chord and any other mem	pst on the bottom chord in all areas where a l pers, with BCDL = 10.0psf.	ectangle 3-6	5-0 tall by	2-0-0 wide	al they
 6) Ceiling dead load (10.0 7) Provide mechanical con 	psf) on member(s). 4-5, 7-8,	5-7; Wall dead load (5.0psf) on member(s).4	-14, 8-12 unlift at ioir	t 14 and	116 lb uplift	SEAL E
at joint 12.	(000 deflection		apint at join			036322
o) Allic room checked for L						030322
						A BALL CRIA
						AND GINEE PONT
						A. GILD
						October 3,2024

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Job	Truss	Truss Type	Qty	Ply	Lot 10 Magnolia Hills	169612200
J0524-2974	C2	ATTIC	6	1	lah Dafaranan (antianal	100012300
Comtech, Inc, Fay	etteville, NC - 28314,			8.630 s J	Job Reference (optional ul 12 2024 MiTek Industri	l) es, Inc. Tue Oct 1 14:28:09 2024 Page 1
	1-2-8	5-3-4 7-10-3 8 ₁ 11-12	ID:JJp3_bNirdpe 2 11-3-0 13-6-5 14-7-13 17-2-	LXA5mDh [*]	?5?y7p3U-RfC?PsB70Hq <u>22-6-0 23-8-8</u>	3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f
	1-2-8	5-3-4 ' 2-6-15 1-1-8'	2-3-4 ' 2-3-4 1-1-8' 2-6-	5	5-3-4 '1-2-8'	
			4x6 =			Scale = 1:80.6
			<u>_</u>			
	-	$2x4 = 12.00 \overline{12}$ 5 $2x4 16$ 4	2x4 =	17 2x 8	4	
	¢ 2	4x6 1/ B 3 ¹⁵ 5-6-0	τ α δ 11-6-0		18 4x6 \\ 9 10	
	4 1-3-7 8-1					-2-12
		8 = 14	13	12	8x8 =	
		2x6 2x6 8x8 ==	2x6 2x6 8x8 =	10x10 =		
		5-3-4 . 11-3-0	2x6 112-2-12		22-6-0	
Plate Offsets (X V)-	[2:Edge 0.4.12] [6:0.3.0 Edge	5-3-4 5-11-12 [10:Edge 0-4-12] [12:0-5-0 0-3	5-11-12		5-3-4	
					l/doft l/d	
TCLL 20.0	Plate Grip DOL 1.	5 TC 0.67	Vert(LL) -0.2	9 12-14	>911 360	MT20 244/190
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1. Rep Stress Incr YI	5 BC 0.82 S WB 0.18	Vert(CT) -0.5 Horz(CT) 0.0	1 12-14 1 10	>522 240 n/a n/a	
BCDL 10.0	Code IRC2015/TPI201	4 Matrix-S	Wind(LL) 0.1) 12-14	>999 240	Weight: 236 lb FT = 20%
LUMBER- TOP CHORD 2x6 SF 1-3,9- BOT CHORD 2x10 S 12-14:	2 2400F 2.0E *Except* 1: 2x6 SP No.1 P No.1 *Except* 2x6 SP No.1		BRACING- TOP CHORD BOT CHORD	Structur Rigid ce	al wood sheathing direction direction of the second sheathing directly applied or	ctly applied or 5-4-10 oc purlins. 8-4-3 oc bracing.
WEBS 2x6 SF WEDGE	P No.1					
Left: 2x4 SP No.2 , Rig	ht: 2x4 SP No.2					
REACTIONS. (siz Max H Max C	e) 2=0-3-8, 10=0-3-8 lorz 2=295(LC 11) lrav 2=1518(LC 20), 10=1518(_C 21)				
FORCES. (lb) - Max. TOP CHORD 2-4= BOT CHORD 2-14 WEBS 4-14	Comp./Max. Ten All forces 2 .1928/0, 4-5=-1039/146, 5-6=0 =0/1088, 12-14=0/1088, 10-12 =0/939, 8-12=0/939, 5-7=-1492	50 (lb) or less except when show 385, 6-7=0/386, 7-8=-1038/146, a 0/1088 /196	/n. 8-10=-1927/0			
 NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; \ and C-C Exterior(2) zone;C-C for memb 3) This truss has been will fit between the H 5) Ceiling dead load (1 6) Bottom chord live lo 7) Attic room checked 	e loads have been considered t /ult=130mph Vasd=103mph; T -1-0-14 to 3-3-15, Interior(1) 3 ers and forces & MWFRS for re designed for a 10.0 psf bottom n designed for a live load of 30 bottom chord and any other me 0.0 psf) on member(s). 4-5, 7-i ad (40.0 psf) and additional bo for L/360 deflection.	br this design. CDL=6.0psf; BCDL=6.0psf; h=15f 3-15 to 11-3-0, Exterior(2) 11-3-0 actions shown; Lumber DOL=1.6 chord live load nonconcurrent wi 0psf on the bottom chord in all a nbers. i, 5-7; Wall dead load (5.0psf) or tom chord dead load (10.0 psf) a	ft; Cat. II; Exp C; Enclose) to 15-7-13, Interior(1) 15 60 plate grip DOL=1.60 ith any other live loads. reas where a rectangle 3 n member(s).4-14, 8-12 pplied only to room. 12-1	d; MWFR i-7-13 to 2 -6-0 tall by 4	S (envelope) 3-6-14 2-0-0 wide	TH CAROLINA
					tititi ta	SEAL 036322

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J0524-2974	СЗ	ATTIC	2	2		168612301
Comtech, Inc, Fayett	eville, NC - 28314,			8.630 s J	Job Reference (optional) Jul 12 2024 MiTek Industries, Industries	uc. Tue Oct 1 14:28:10 2024 Page 1
· · · ·			ID:JJp3_bNirdp 14-7-13	eLXA5mDh	?5?y7p3U-RfC?PsB70Hq3NSg	JPqnL8w3ulTXbGKWrCDoi7J4zJC?f
	1-2- 1-2-	8 5-3-4 7-10-3 9-2-3 8 5-3-4 2-6-15 1-4-0	<u>11-3-0 13-3-13</u>	2-6-15	<u>22-6-0</u> 23-8-8 5-3-4 1-2-8	
			6x8 =	2010		Scale = 1:80.6
			6			
		3x6 =		a —		
		12.00 12		5 —		
		, A				
		6x8 ¹⁶ ♥		\$ 17	6v8	
	- -	4 🕈		\$	8	
	12-1	6x8 // # 1	2-4	Ŕ	18 ♥ 6x8 ℕ	
		3 ¹⁵ <u>p</u>	ά		9	
		\$ 5-6-0 \$	11-6-0		×∕\	
	2				\$ 10-	
	1-3-4 1-3-4 1-3-4					7-7-
		⊠ 8x8 = 14	13	12	8x8 =	-
		2x6 2x6	6 2x6 6x8 =	2x6 10x10 =	=	
		10x10 =		10,110		
		5-3-4 5-3-4 5-11-12	2x6 117-2-12 2 5-11-12		<u>22-6-0</u> 5-3-4	
Plate Offsets (X,Y) [2	:Edge,0-4-4], [3:0-4-0,Edge], [4	4:0-8-6,Edge], [6:0-4-0,Edge], [8:0	0-8-6,Edge], [9:0-4-0,I	dge], [10:E	Edge,0-4-12], [12:0-5-0,0-2-8]	, [14:0-5-0,0-3-0]
LOADING (psf)	SPACING- 4-9-0	CSI.	DEFL.	in (loc)	l/defl L/d I	PLATES GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15	5 TC 0.84 5 BC 0.48	Vert(LL) -0. Vert(CT) -0.	26 12-14 17 12-14	>999 360 I >572 240	MT20 244/190
BCLL 0.0 *	Rep Stress Incr NC	WB 0.24	Horz(CT) 0.	01 10	n/a n/a	Weight: 505 lb 57 - 20%
BCDL 10.0		Matrix-S	VVING(LL) U.	13 12-14	>999 240	weight. 505 lb FT = 20%
LUMBER- TOP CHORD 2x8 SP 2	400F 2.0E *Except*		BRACING- TOP CHORD	2-0-0 o	c purlins (5-2-3 max.)	
1-3,9-11:	2x6 SP No.1			(Switch	ed from sheeted: Spacing > 2	2-8-0).
BOT CHORD 2x10 SP 12-14: 2x	2400F 2.0E *Except* 6 SP No.1		BOT CHORD	Rigid c	eiling directly applied or 10-0-	0 oc bracing.
WEBS 2x6 SP N	lo.1					
Left: 2x4 SP No.2 , Right:	2x4 SP No.2					
REACTIONS. (size)	2=0-3-8, 10=0-3-8					
Max Hor Max Gra	z 2=-696(LC 10) v 2=5052(LC 20) 10=4038(L(21)				
	v z=3032(EC 20), 10=4030(EC					
FORCES. (lb) - Max. Co TOP CHORD 2-4=-58	omp./Max. Ten All forces 25 357/98, 4-5=-2882/441, 5-6=-1	0 (lb) or less except when shown. 36/1482, 6-7=-98/1334, 7-8=-303	0/478,			
8-10=-5	5634/60 /3262 12 14-0/3262 10 12-0	13262				
WEBS 4-14=0/	/3443, 8-12=0/2956, 5-7=-491	1/789				
NOTES-						
1) 2-ply truss to be conne	ected together with 10d (0.131	'x3") nails as follows:				
Bottom chords connected	ted as follows: 2x6 - 2 rows stagg	taggered at 0-5-0 oc.	ggered at 0-9-0 oc.			
Webs connected as fo 2) All loads are considered	llows: 2x6 - 2 rows staggered and equally applied to all plies.	at 0-9-0 oc. except if noted as front (F) or back	(B) face in the LOAD	CASE(S)	section. Plv to	
ply connections have b	been provided to distribute only	/ loads noted as (F) or (B), unless	otherwise indicated.	(-)	A	RTHUARO
4) Wind: ASCE 7-10; Vul	t=130mph Vasd=103mph; TCI	DL=6.0psf; BCDL=6.0psf; h=15ft;	Cat. II; Exp C; Enclos	ed; MWFR	S (envelope)	O. ESSION A
and C-C Exterior(2) -1-	-0-14 to 3-3-15, Interior(1) 3-3 and forces & MWERS for rea	-15 to 11-3-0, Exterior(2) 11-3-0 to	o 15-7-13, Interior(1) ⁻	5-7-13 to 2	23-6-14	
5) This truss has been de	signed for a 10.0 psf bottom o	hord live load nonconcurrent with	any other live loads.			SEAL E
will fit between the bot	tom chord and any other mem	pst on the bottom chord in all are bers.	as where a rectangle	3-6-0 tali D <u>i</u>	y 2-0-0 wide	036322
 Ceiling dead load (10.0 Bottom chord live load 	0 psf) on member(s). 4-5, 7-8, (40.0 psf) and additional botto	5-7; Wall dead load (5.0psf) on n	nember(s).4-14, 8-12	14		N Z E
9) Magnitude of user add	ed load(s) on this truss have b	een applied uniformly across all	gravity load cases with	no adjustr	ments.	A AVGINEER A
10) Graphical purlin repre 11) Hanger(s) or other co	esentation does not depict the onnection device(s) shall be pr	size or the orientation of the purili ovided sufficient to support conce	n along the top and/or ntrated load(s) 1882 l	bottom che down and	ord. d 470 lb up at	CABE
5-6-8 on bottom chor 12) Attic room checked fr	d. The design/selection of suc	ch connection device(s) is the resp	ponsibility of others.			A. GIL
						October 3,2024
Comanue Asi E (SigeStanda	rđ					
WARNING - Verify des Design valid for use only a truss system. Before u building design. Bracing	sign parameters and READ NOTES ON y with MiTek® connectors. This design use, the building designer must verify the a indicated is to prevent buckling of ind	THIS AND INCLUDED MITEK REFERENCI is based only upon parameters shown, an e applicability of design parameters and p vidiual trues web and/or chord members o	E PAGE MII-7473 rev. 1/2/20 d is for an individual buildin roperly incorporate this des	23 BEFORE L g component, gn into the ov	JSE. not erall bracing	

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outloase with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Lot 10 Magnolia Hills	
					168	612301
J0524-2974	C3	ATTIC	2	2		
				_	Job Reference (optional)	
Comtech, Inc, F	ayetteville, NC - 28314,			8.630 s Ju	Il 12 2024 MiTek Industries, Inc. Tue Oct 1 14:28:10 2024 Pag	ge 2

8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 1 14:28:10 2024 Page 2 ID:JJp3_bNirdpeLXA5mDh?5?y7p3U-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-4=-143, 4-5=-190, 5-6=-142, 6-7=-142, 7-8=-190, 8-11=-142, 2-14=-47, 12-14=-95, 10-12=-47, 5-7=-47

Drag: 4-14=-24, 8-12=-24

Concentrated Loads (lb)

Vert: 14=-1860(F)

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Vert: 1-4=-60, 4-14=-80, 6-14=-60, 6-9=-60, 2-10=-20



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Plate Offs	ets (X,Y)	[2:0-0-8,0-0-9], [6:0-0-8,0-	0-9]									
LOADING	i (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.12	Vert(LL)	-0.01	10	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.13	Vert(CT)	-0.02	10	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.00	6	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI	2014	Matrix	(-S	Wind(LL)	0.02	8	>999	240	Weight: 52 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x4 SP No.1

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x4 SP No.2

 OTHERS
 2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 6=0-3-0 Max Horz 2=-63(LC 17) Max Uplift 2=-120(LC 9), 6=-120(LC 8)

Max Grav 2=470(LC 1), 6=470(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-498/559, 3-4=-455/613, 4-5=-455/613, 5-6=-498/559
- BOT CHORD 2-10=-395/386, 9-10=-395/386, 8-9=-395/386, 6-8=-395/386

WEBS 4-9=-420/250

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 120 lb uplift at joint 2 and 120 lb uplift at joint 6.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 2 and 91 lb uplift at joint 4.



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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 5, 93 lb uplift at joint 2 and 93 lb uplift at joint 6.

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LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	тс	0.37	Vert(LL)	-0.04	2-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.27	Vert(CT)	-0.09	2-4	>728	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/TP	12014	Matri	ĸ-P	Wind(LL)	0.10	2-4	>655	240	Weight: 22 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1WEBS2x6 SP No.1

WEBS 2x6 SP No.1 REACTIONS. (size) 2=0-3-0, 4=

NS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=56(LC 8) Max Uplift 2=-130(LC 8), 4=-85(LC 8) Max Grav 2=306(LC 1), 4=206(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 5-6-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify
- capacity of bearing surface. 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 130 lb uplift at joint 2 and 85 lb uplift at joint 4.

Structural wood sheathing directly applied or 5-9-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

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LOADING	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.15	TC 0.22	Vert(LL) -0.02 2-4 >999 360 MT20 244/190	
TCDL	10.0	Lumber DOL 1.15	BC 0.17	Vert(CT) -0.04 2-4 >999 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 n/a n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.04 2-4 >999 240 Weight: 18 lb FT = 20%	

BRACING-

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WEBS

2x6 SP No.1 REACTIONS.

2=0-3-0, 4=0-1-8 (size) Max Horz 2=48(LC 8) Max Uplift 2=-117(LC 8), 4=-67(LC 8) Max Grav 2=268(LC 1), 4=164(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 4-6-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify
- capacity of bearing surface. 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 2 and 67 lb uplift at joint 4.

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TOP CHORD Structural wood sheathing directly applied or 5-9-0 oc purlins, except end verticals. BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 5, 85 lb uplift at joint 2 and 65 lb uplift at joint 6.

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Edenton, NC 27932

October 3,2024

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 7-9-9, Exterior(2) 7-9-9 to 12-2-6, Interior(1) 12-2-6 to 15-2-14 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=186, 6=186.

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3) Gable requires continuous bottom chord bearing.

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5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=162.

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A. GIL

REACTIONS. (size) 1=7-6-6, 3=7-6-6, 4=7-6-6 Max Horz 1=-83(LC 8) Max Uplift 1=-30(LC 13), 3=-30(LC 13) Max Grav 1=168(LC 1), 3=168(LC 1), 4=216(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 1 and 30 lb uplift at joint 3.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

					4-10-12				0-0	-6		
LOADING TCLL	G (psf) 20.0	SPACING- 2-0- Plate Grip DOL 1.1	0 CSI. 5 TC	0.07	DEFL. Vert(LL)	in n/a	(loc) -	l/defl n/a	L/d 999	PLATES MT20	GRIP 244/190	
BCLL BCDL	10.0 0.0 * 10.0	Lumber DOL 1.1 Rep Stress Incr YE Code IRC2015/TPI2014	S BC S WB Matrix	0.03 0.01 (-P	Vert(CT) Horz(CT)	n/a 0.00	- 3	n/a n/a	999 n/a	Weight: 19 lb	FT = 20%	

BRACING-TOP CHORD

BOT CHORD

```
LUMBER-
```

2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

TOP CHORD

REACTIONS. (size) 1=4-10-6, 3=4-10-6, 4=4-10-6 Max Horz 1=-51(LC 8) Max Uplift 1=-18(LC 13), 3=-18(LC 13) Max Grav 1=103(LC 1), 3=103(LC 1), 4=132(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 1 and 18 lb uplift at joint 3.

Structural wood sheathing directly applied or 4-11-2 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oclapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Scitcu Information. Storage that the component development of the prevent oclapse with possible for the Storage Trust Plate Institute (www.tpinst.org) and PCB Building Component Scitcu Information. Storage to the component development of the prevent oclapse with possible for the Storage to the prevent oclapse with possible for the Storage to the prevent oclapse to the prevent oclapse to the prevent oclapse to the prevent oclapse to the prevent occap to t and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Edenton, NC 27932

Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0524-2975 Lot 10 Magnolia Hills

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I68612315 thru I68612326

My license renewal date for the state of North Carolina is December 31, 2024.

North Carolina COA: C-0844

October 3,2024

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	Lot 10 Magnolia Hills	1000.100.1
J0524-2975	ET1	GABLE	1	1		I68612315
Comtech, Inc, Fayette	 ville, NC - 28314,			8.630 s Ju	Job Reference (optional) ul 12 2024 MiTek Industries,	Inc. Tue Oct 1 14:28:05 2024 Page 1
0-1-8			ID:JJp3_bNirdpel	_XA5mDh?	?5?y7p3U-RfC?PsB70Hq3N	SgPqnL8w3uITXbGKWrCDoi7J4zJC?f
° H°						° H°
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	4 5 34 33 32 41 3x6 FP = 2 2x6 2x6	5 7 8 6 6 6 6 6 6 8 6 6 8 7 8 8 6 6 8 7 8 8 6 6 8 7 8 8 7 8 8 6 7 8 7 8 8 7 8 8 8 8 <t< th=""><th>9 10 9 10 9 28 27 2x6 2x6 </th><th>11 0 26 2x6 </th><th>3x6 12 13 14 14 19 10 10 10 10 10 10 10 10 10 10 10 10 10</th><th>FP = 15 16 17 18 15 16 17 18 10 10 10 10 10 10 10 10 10 10 10 10 10</th></t<>	9 10 9 10 9 28 27 2x6 2x6	11 0 26 2x6	3x6 12 13 14 14 19 10 10 10 10 10 10 10 10 10 10 10 10 10	FP = 15 16 17 18 15 16 17 18 10 10 10 10 10 10 10 10 10 10 10 10 10
0-6-4 1-10-4 3 0-6-4 1-4-0 1 Plate Offsets (X,Y) [20	-2-4 4-6-4 5-10-4 4-0 1-4-0 1-4-0 :Edge,0-3-0], [37:Edge,0-3-0]	+ 7-2-4 8-6-4 9-10-4 1-4-0 1-4-0 1-4-0	<u>11-2-4</u> <u>12-6-4</u> <u>1-4-0</u> <u>1-4-0</u>	13-1	0-4 15-2-4 16-6-4 -0 1-4-0 1-4-0	17-10-4 19-2-4 19-8-8 1-4-0 1-4-0 0-6-4
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	ı (loc)	l/defl L/d	PLATES GRIP
TCLL 40.0 TCDL 10.0	Plate Grip DOL 1.00 Lumber DOL 1.00	TC 0.07 BC 0.03	Vert(LL) n/a Vert(CT) n/a	-	n/a 999 n/a 999	MT20 244/190
BCLL 0.0 BCDL 5.0	Rep Stress Incr NC Code IRC2015/TPI2014	WB 0.03 Matrix-R	Horz(CT) -0.00	19	n/a n/a	Weight: 109 lb FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No BOT CHORD 2x4 SP No WEBS 2x4 SP No OTHERS 2x4 SP No	0.1(flat) 0.1(flat) 0.3(flat) 0.3(flat)		BRACING- TOP CHORD BOT CHORD	Structur except e Rigid ce	al wood sheathing directly end verticals. illing directly applied or 6-0	applied or 10-0-0 oc purlins, -0 oc bracing.
REACTIONS. All beari (lb) - Max Uplif Max Grav	ngs 19-8-8. All uplift 100 lb or less at jo All reactions 250 lb or less 19	int(s) 37 at joint(s) 28, 29, 30, 31, 32, 34, 3	35, 36, 27, 26, 25, 24, 2	2, 21, 20,		

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 37.
 7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 19-37=-10, 1-18=-100

Concentrated Loads (lb)

Vert: 30=-74 34-74 27-74 24-74 19-78 40-74 41-74 42-74 43-74 44-74

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of frusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **PCB_Building** Component **5**, the fabrication and the fabrication of the prevention. and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Image: Non-sector of the sector of	I OADING (nsf)	SPACING.	2-0-0	<u></u>	DEEL	in (loc)	l/dofl l/d		GRIP
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	<u> 1-4-0</u> 1-4-0	2-8-0 4-0-0 1-4-0 1-4-0	<u>5-4-0</u> 1-4-0	6-8-0 8-0-0 1-4-0 1-4-0	<u>9-4-0</u> 1-4-0	<u>10-8-0 1</u> <u>1-4-0</u>	2-0-0 13-4-0 -4-0 1-4-0	<u>14-8-0 16-0-</u> 1-4-0 1-4-0	0 + 17-0-0 0 + 1-0-0 +
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	3x4 =	3x6 FP=	20 23	27 2	.5 22	21	20 1	5 10	3x4 =
J0524-2975 ET2 GABLE 1	30 29					21	20 10		
J0524-2975 ET2 GABLE 1 1 1 1 Comtech, Inc, Fayetteville, NC - 28314, 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 1 14:28:05 2024 Page 1 ID:JJp3_bNirdpeLXA5mDh?5?y7p3U-RtC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 01f8 01f8 5 6 7 8 9 10 11 12 13 14 15 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15									
J0524-2975 ET2 GABLE 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 2	3 4	4 5	6 7	8	9	10 1 ²	1 <u>12</u> 13	14 15
J0524-2975 ET2 GABLE 1 1 1 1 Comtech, Inc, Fayetteville, NC - 28314, 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 1 14:28:05 2024 Page 1 ID:JJp3_bNirdpeLXA5mDh?5?y7p3U-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 0178 0178								3x6 FP=	
J0524-2975 ET2 GABLE 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1									Scale = 1:28.2
J0524-2975 ET2 GABLE 1 1 Image: Contect, Inc, Inc, Inc, Inc, Inc, Inc, Inc, Inc	0-118								0- <mark>1</mark> -18
J0524-2975 ET2 GABLE 1 1 Job Reference (optional)	Contech, Inc, Fay	ellevine, NC - 26314,			ID:JJp3_	bNirdpeLXA5mDl	n?5?y7p3U-RfC?PsB70l	Hq3NSgPqnL8w3uITXb	GKWrCDoi7J4zJC?f
168612316	J0524-2975	ET2	GABLE		1	1	Job Reference (option	nal)	1:29:05 2024 Dags 1
				-					168612316

TCLL 40.0 Plate Grip DOL 1.00 TC 0.0 TCDL 10.0 Lumber DOL 1.00 BC 0.1 BCLL 0.0 Rep Stress Incr YES WB 0.1	6 Vert(LL) n/a - n/a 999 MT20 244/190 1 Vert(CT) n/a - n/a 999 3 Horz(CT) 0.00 16 n/a n/a
BCDL 5.0 Code IRC2015/TPI2014 Matrix-R	Weight: 71 lb FT = 20%F, 11%

LUMBER-

 TOP CHORD
 2x4 SP No.1(flat)

 BOT CHORD
 2x4 SP No.1(flat)

 WEBS
 2x4 SP No.3(flat)

 OTHERS
 2x4 SP No.3(flat)

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 17-0-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 30, 16, 29, 28, 26, 25, 24, 23, 22, 21, 20, 19, 18, 17

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Lot 10 Magnolia Hills		168612317
J0524-2975	ET3	GABLE	1	1	Job Reference (option	al)	100012317
Comtech, Inc, Fayett	eville, NC - 28314,		ID: Lin2, hNirdna	8.630 s Jul	12 2024 MiTek Indust	ries, Inc. Tue Oct 1 14:	28:06 2024 Page 1
0 _] 18			D.JJpS_binitope	LAASIIIDII?	5?y/p30-RIC?PSB/0n	Iq3N3gPqnLow3u11XbG	0-1-8
							Scale = 1:25.8
1 2	3 4	5 6	7 8		9 10	11	12 13
						•	

26 25	24 23	22 21	20 19		18 17	16	15 14
<u> 1-4-0</u> 1-4-0	<u>2-8-0</u> <u>4-0-0</u> 1-4-0 1-4-0	5-4-0 6-8-0 8-0 1-4-0 1-4-0 1-4	0-0 <u>9-4-0</u> 1-0 1-4-0	10-8-0 1-4-0	12-0-0 1-4-0	<u>13-4-0</u> <u>14-8-0</u> 1-4-0 <u>1-4-0</u>	<u>15-7-0</u> 0-11-0
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-1 Plate Grip DOL 1.0 Lumber DOL 1.0 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-R	DEFL. i Vert(LL) n/: Vert(CT) n/: Horz(CT) 0.00	n (loc) a - a -) 14	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES 0 MT20 2 Weight: 66 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP N BOT CHORD 2x4 SP N WEBS 2x4 SP N OTHERS 2x4 SP N	lo.1(flat) lo.1(flat) lo.3(flat) lo.3(flat)		BRACING- TOP CHORD BOT CHORD	Structura except er Rigid ceil	I wood sheathing dire nd verticals. ling directly applied o	ectly applied or 6-0-0 o r 10-0-0 oc bracing.	c purlins,

REACTIONS. All bearings 15-7-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 26, 14, 25, 24, 23, 22, 21, 20, 19, 18, 17, 16, 15

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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A MiTek Af 818 Soundside Road

Edenton, NC 27932

			F	-	-	
Job		Truss	Truss Type	Qty	Ply	Lot 10 Magnolia Hills
						168612319
J0524-2975		F2	Floor	1	1	
						Job Reference (optional)
Comtech, Inc,	Fayettev	ille, NC - 28314,			8.630 s Ju	Il 12 2024 MiTek Industries, Inc. Tue Oct 1 14:28:07 2024 Page 1
	-		ID:JJp	3_bNirdpeL	_XA5mDh?	25?y7p3U-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f
0-1-8						
_1-3-0		2-4-4				<u>2-2-0</u> <u>1-1-12</u> 0-1-8 Scale = 1:55.1

L	12-11-12					32-5-	0		
	12-11-12	Ι				19-5	4		I
Plate Offsets (X, Y)	[34:0-1-8,Edge], [35:0-1-8,Edge]		1					1	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.74	Vert(LL)	-0.28	25	>824	480	MT20	244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.73	Vert(CT)	-0.38	25	>604	360		
BCLL 0.0	Rep Stress Incr YES	WB 0.69	Horz(CT)	0.05	22	n/a	n/a		
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S						Weight: 163 lb	FT = 20%F, 11%E
LUMBER-			BRACING-						
TOP CHORD 2x4	SP No.1(flat) *Except*		TOP CHORI	D	Structu	ural wood	I sheathing di	rectly applied or 6-0-0	oc purlins,
1-10	: 2x4 SP 2400F 2.0E(flat)				except	end vert	icals.		
BOT CHORD 2x4	SP No.1(flat) *Except*		BOT CHORI	D	Rigid o	ceiling dir	ectly applied	or 6-0-0 oc bracing.	
22-2	9: 2x4 SP 2400F 2.0E(flat)								
WEBS 2x4	SP No.3(flat)								
Max	(Grav 37=608(1 C 3) 31=2131(1 C 1) 22=	=934(I C 4)							
With /	Clav 07-000(E0 0), 01-2101(E0 1), 22-	-304(204)							
FORCES. (lb) - Ma	x. Comp./Max. Ten All forces 250 (lb) o	r less except when showr	1.						
TOP CHORD 2-3	3=-1162/48, 3-4=-1560/543, 4-5=-1560/54	3, 5-6=-1560/543, 6-7=-4	56/1438,						
7-8	3=0/2740, 8-9=0/2740, 9-11=-325/363, 11-	-12=-2211/0, 12-13=-221	1/0, 13-15=-3332/0,						
15	5-16=-3800/0, 16-17=-3800/0, 17-18=-325	9/0, 18-19=-3259/0, 19-20	0=-1967/0						
BOT CHORD 36	-37=-9/746, 35-36=-178/1515, 34-35=-543	3/1560, 33-34=-1066/1064	4, 31-33=-1777/0,						
30	-31=-1239/0, 28-30=-50/1381, 27-28=0/29	915, 26-27=0/3800, 25-26	=0/3800,						
24	-25=0/3625, 23-24=0/2733, 22-23=0/1171								
WEBS 2-3	37=-933/12, 2-36=-50/541, 3-36=-459/170	, 7-31=-1407/0, 7-33=0/9	53, 6-33=-1014/0,						
6-3	34=0/1132, 5-34=-523/0, 3-35=-484/57, 9-	31=-1884/0, 9-30=0/1447	, 11-30=-1420/0,						
11	-28=0/1108, 13-28=-933/0, 13-27=0/647,	15-2/=-86//0, 20-22=-140	56/0, 20-23=0/1036	,					
15	-2339110, 19-24=0/012, 11-24=-461/0,	17-20100/499							
NOTES-									
1) Unbalanced floor	live loads have been considered for this d	esian.							1111
2) All plates are 3x4	MT20 unless otherwise indicated.							11''''' C	10'11
3) Plates checked for	r a plus or minus 1 degree rotation about	its center.						THU	ANO MA
4) Recommend 2x6	strongbacks on edge spaced at 10-0-0	oc and fastened to each to	uss with 3-10d (0.1	31" X	3") naile	-		C Chine	S. J. A.

Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") n.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

A MiTek Affili 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 10 Magnolia Hills
					168612320
J0524-2975	F3	Floor	1	1	
					Job Reference (optional)
Comtech, Inc, Fayet	eville, NC - 28314,			8.630 s Ju	Il 12 2024 MiTek Industries, Inc. Tue Oct 1 14:28:08 2024 Page 1
		ID:JJp3	_bNirdpel	XA5mDh?	5?y7p3U-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

|| <mark>1-3-0</mark>

2-1-4

2-2-0 0-1-8 Scale = 1:55.1

	12-11-12 12-11-12			32-5-0 19-5-4		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.85 BC 0.67 WB 0.75 Matrix-S	DEFL. in (h Vert(LL) -0.32 -0.42 Vert(CT) -0.42 -0.42 Horz(CT) 0.04 -0.44	oc) l/defl L/d 29 >732 480 29 >548 360 25 n/a n/a	PLATES GRIP MT20 244/190 M18AHS 186/179 Weight: 179 lb FT = 20%F, 11%E	
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	LUMBER- TOP CHORD BRACING- 2x4 SP 2400F 2.0E(flat) BOT CHORD 2x4 SP 2400F 2.0E(flat) WEBS 2x4 SP No.3(flat) BOT CHORD BOT CHORD Rescent and the second s					
REACTIONS. (siz Max U Max C	e) 40=0-3-0, 34=0-3-8, 25=0-3-0 Jplift 40=-128(LC 4) Grav 40=473(LC 3), 34=3045(LC 1), 25=	1014(LC 4)				
FORCES. (lb) - Max. TOP CHORD 2-3= 10-1 18-1	Comp./Max. Ten All forces 250 (lb) or -833/382, 3-5=-747/1454, 5-6=-747/142 11=0/4362, 11-14=-503/47, 14-15=-3294 9=-4487/0, 10-20=-4487/0, 20-21=-3640	less except when shown 7, 6-8=-777/1427, 8-9=0/2 /0, 15-16=-3287/0, 16-18 /0, 21-22=-3649/0, 22-23	ı. 2770, 9-10=0/4362, =-4214/0, =-2170/0			
BOT CHORD 39-4 33-3 27-2 WEBS 2-40 8-36 14-3 19-2	Terre 191947/0, 19-204407/0, 20-213049/0, 21-2-23-3049/0, 21-22-23-2170/0 39-40198/564, 38-39650/1032, 37-381427/747, 36-37=-2250/0, 34-36=-3265/0, 33-341862/0, 32-33=0/2683, 30-32=0/3911, 29-30=0/4487, 28-29=0/4487, 27-28=0/4124, 26-27=0/3024, 25-26=0/1278 EBS 2-40=-704/250, 2-39=-239/350, 3-39=-259/349, 9-34=-1644/0, 9-36=0/1161, 8-36=-1206/0, 8-37=0/1567, 6-37=-796/0, 3-38=-1010/0, 11-34=-3136/0, 11-33=0/2566, 14-33=-2761/0, 14-32=0/823, 16-32=-827/0, 16-30=0/482, 18-30=-572/55, 5-38=0/517, 19-28=-327/0, 23-25=-1600/0, 23-26=0/1161, 22-26=-1112/0, 22-27=0/799,					
 NOTES- 1) Unbalanced floor liv 2) All plates are MT200 3) All plates are 3x6 M 4) Plates checked for a 5) Provide mechanical 6) Recommend 2x6 st Strongbacks to be a 7) CAUTION, Do not e 8) Hanger(s) or other of chord. The design/s 9) In the LOAD CASE(s) ELOAD CASE(s) Stan 1) Dead + Floor Live (I) Uniform Loads (plf) Vert: 25-400 Concentrated Loads Vert: 14=-8 	7=-606/0, 20-28=07/40 re loads have been considered for this de plates unless otherwise indicated. IT20 unless otherwise indicated. a plus or minus 1 degree rotation about i connection (by others) of truss to bearir rongbacks, on edge, spaced at 10-0-0 contacted to walls at their outer ends or reservent truss backwards. connection device(s) shall be provided susselection of such connection device(s) is selection, loads applied to the face of t dard balanced): Lumber Increase=1.00, Plate i=-10, 1-24=-100 s (lb) (50(B)	esign. Is center. Ig plate capable of withstance and fastened to each tr strained by other means. ufficient to support concert the responsibility of other he truss are noted as from Increase=1.00	anding 128 lb uplift at joint 40 uss with 3-10d (0.131" X 3") ntrated load(s) 930 lb down a rs. tt (F) or back (B).). nails. Z	SEAL 036322 October 3,2024	

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Job	Truss	Truss Type	Qty	Ply	Lot 10 Magnolia Hills	169610201
J0524-2975	F4	Floor	6	1		100012321
				0.000	Job Reference (optional)	
Comtecn, Inc, Fayette	VIIIe, NC - 28314,		D: Lin2 hNirdnol	8.630 S JU	1 12 2024 MITEK Industries, Inc. Tu	
			ID.JJp3_biviluper	ASINDIT	Siyrpso-RicirsBrondsNogrqiild	6w3u172bGRW1CD01/3423C?1
0-1-8						
⊢		2-1-	0			
						Scale = 1:25.5
4x4 =	3x4 =	3x4 =			3x4 =	4x4 =
1 2	3	4 5	6 ^{3x4} =	7	8	9 10
				0		
19				H		
~~ []	\sim			>		⁴ ٩
					4	-
			0		1 1.	
\bigotimes						\bigotimes
18	17	16 15	14	13	12	
3x6 =	4x4 =	3x6 =		3x6	= 4x4 =	3x6 =

			<u>15-7-0</u> 15-7-0				
Plate Offsets (X,	() [5:0-1-8,Edge], [6:0-1-8,Edge]		13-1-0				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.41 BC 0.71 WB 0.42 Matrix-S	DEFL. Vert(LL) - Vert(CT) - Horz(CT)	in (loc) -0.17 14-15 -0.23 14-15 0.05 11	l/defi L/d >999 480 >799 360 n/a n/a	PLATES MT20 Weight: 79 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD BRACING- TOP CHORD TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) BOT CHORD BOT CHORD Rigid ceiling directly applied or 10-0 oc bracing.							
REACTIONS.	REACTIONS. (size) 18=0-3-0, 11=0-3-8 Max Grav 18=837(LC 1), 11=837(LC 1)						
FORCES. (Ib) - TOP CHORD	Max. Comp./Max. Ten All forces 250 (lb) c 2-3=-1726/0, 3-4=-2770/0, 4-5=-2770/0, 5-6 8-9=-1726/0	r less except when shown. =-3060/0, 6-7=-2770/0, 7-8	3=-2770/0,				
BOT CHORD	17-18=0/1045, 16-17=0/2371, 15-16=0/3060 11-12=0/1045	, 14-15=0/3060, 13-14=0/3	3060, 12-13=0/2371	1,			

- WEBS 2-18=-1308/0, 2-17=0/886, 3-17=-840/0, 3-16=0/510, 9-11=-1308/0, 9-12=0/886, 8-12=-840/0, 8-13=0/510, 6-13=-645/6, 5-16=-645/6
- 0 12 0 10/0, 0 10

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

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I			17-0-0			1
Plate Offsets (X,Y)	[5:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.70 BC 0.95 WB 0.48 Matrix-S	DEFL. ir Vert(LL) -0.27 Vert(CT) -0.37 Horz(CT) 0.06	1 (loc) l/defl L/d / 15-16 >748 480 / 15-16 >543 360 } 13 n/a n/a	PLATES MT20 M18AHS Weight: 87 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied c 2-2-0 oc bracing: 16-17,15-16	ectly applied or 6-0-0 r 10-0-0 oc bracing, i.	oc purlins, Except:

17-0-0

REACTIONS.	(size)	21=0-3-0, 13=0-3-8
	Max Grav	21=915(LC 1), 13=915(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1923/0, 3-4=-3138/0, 4-5=-3138/0, 5-6=-3635/0, 6-7=-3635/0, 7-8=-3168/0, 8-10=-3168/0, 10-11=-1918/0

BOT CHORD	20-21=0/1147, 18-20=0/2659, 17-18=0/3635, 16-17=0/3635, 15-16=0/3508, 14-15=0/2663,
	13-14=0/1146

WEBS 2-21=-1436/0, 2-20=0/1010, 3-20=-958/0, 3-18=0/611, 5-18=-890/0, 11-13=-1435/0, 11-14=0/1006, 10-14=-970/0, 10-15=0/645, 7-15=-434/0, 7-16=-149/525

NOTES-

F

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

 			15-5-0				
Plate Offsets (X,) [1:Edge,0-1-8], [15:0-1-8,Edge]		1000				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.97 BC 0.97 WB 0.43 Matrix-S	DEFL. in Vert(LL) -0.26 Vert(CT) -0.36 Horz(CT) 0.05	(loc) l/defl 13-14 >691 13-14 >509 11 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 79 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD Structural wood sheathing directly applied, except end verticals. BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.				
REACTIONS.	(size) 17=Mechanical, 11=0-3-0 lax Grav 17=834(LC 1), 11=828(LC 1)						
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1678/0, 3-4=-2890/0, 4-5=-2890/0, 6-7=-2743/0, 7-8=-2743/0, 8-9=-1698/0 BOT CHORD 16-17=0/1032, 15-16=0/2343, 14-15=0/2890, 13-14=0/2965, 12-13=0/2345, 11-12=0/1030 WEBS 9-11=-1290/0, 9-12=0/869, 8-12=-842/0, 8-13=0/508, 6-13=-295/0, 6-14=-298/319, 4-15=-437/0, 2-17=-1295/0, 2-16=0/841, 3-16=-866/0, 3-15=0/903							

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

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L			15	-8-8			
			15	5-8-8			l
LOADING TCLL TCDL BCLL	(psf) 40.0 10.0 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.61 BC 0.92 WB 0.46 Matrix S	DEFL. ir Vert(LL) -0.25 Vert(CT) -0.34 Horz(CT) 0.05	n (loc) l/defi L/d 5 13-14 >742 480 5 13-14 >548 360 5 11 n/a n/a	PLATES MT20	GRIP 244/190
BCDL	5.0	Code IRC2015/1P12014	Matrix-S			weight: 81 ib	FI = 20%F, 11%E
LUMBER- TOP CHOR BOT CHOR WEBS	D 2x4 SP D 2x4 SP 2x4 SP S. (size	2400F 2.0E(flat) No.1(flat) No.3(flat) 2) 17=0-3-8, 11=0-3-0	· · · · ·	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o 2-2-0 oc bracing: 13-14.	rectly applied or 6-0-0 or 10-0-0 oc bracing,	oc purlins, Except:
	Max G	rav 17=844(LC 1), 11=844(LC 1)					

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1719/0, 3-4=-3017/0, 4-5=-3017/0, 5-6=-3017/0, 6-7=-2821/0, 7-8=-2821/0, 8-9=-1738/0

BOT CHORD

9-11=-1316/0, 9-12=0/894, 8-12=-865/0, 8-13=0/535, 6-13=-313/0, 6-14=-289/360, 4-15=-434/0, 2-17=-1320/0,

2-16=0/866, 3-16=-874/0, 3-15=0/956

NOTES-

WEBS

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

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			<u>13-1-8</u> 13-1-8			
LOADING(psf)TCLL40.0TCDL10.0BCLL0.0BCDL5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.41 BC 0.49 WB 0.32 Matrix-S	DEFL. in Vert(LL) -0.11 Vert(CT) -0.14 Horz(CT) 0.03	(loc) l/defl L/d 12-13 >999 480 12-13 >999 360 9 n/a n/a	PLATES MT20 Weight: 68 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x BOT CHORD 2x WEBS 2x	4 SP No.1(flat) 4 SP No.1(flat) 4 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	ectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,
REACTIONS.	(size) 14=0-3-0, 9=0-3-8 lax Grav 14=702(LC 1), 9=702(LC 1)					

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1385/0, 3-4=-2140/0, 4-5=-2140/0, 5-6=-2140/0, 6-7=-1385/0

BOT CHORD 13-14=0/869, 12-13=0/1865, 11-12=0/2140, 10-11=0/1865, 9-10=0/869

WEBS 2-14=-1088/0, 2-13=0/671, 3-13=-626/0, 3-12=0/555, 4-12=-257/0, 5-11=-257/0, 7-9=-1088/0, 7-10=0/671,

6-10=-626/0, 6-11=0/555

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

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			J-1-12	
Plate Offsets (X,Y)	[5:0-3-0,Edge], [9:0-1-8,0-0-8]			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO	CSI. TC 0.24 BC 0.30 WB 0.38	DEFL. in (loc) l/defl L/d Vert(LL) -0.01 7 >999 480 Vert(CT) -0.02 7 >999 360 Horz(CT) 0.01 6 n/a n/a	PLATES GRIP MT20 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-P		Weight: 37 lb FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S	: P No.1(flat) P No.1(flat)	II	BRACING- TOP CHORD Structural wood sheathing dir except end verticals.	ectly applied or 5-1-12 oc purlins,

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

WEBS 2x4 SP No.3(flat)

REACTIONS. (size) 8=Mechanical, 6=0-3-8 Max Grav 8=950(LC 1), 6=1050(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1537/0, 3-4=-1537/0

BOT CHORD 7-8=0/1252, 6-7=0/1327

2-8=-1538/0, 4-6=-1622/0, 4-7=0/284, 3-7=-428/0, 2-7=0/385 WEBS

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Refer to girder(s) for truss to truss connections.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

CAUTION, Do not erect truss backwards.

5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 734 lb down at 1-10-4, and 734 lb down at 3-10-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 6-8=-10, 1-5=-100

Concentrated Loads (lb)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall a loss system. Before use, the building designer index very the applications of design had very the applications of design index very the applications of design had very the application of the applicatin of the application and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

	Client:		Date: 1	0/1/2024 Jeal Baggett	Page 2 o
isDesign	Address:		Job Name: L	ot 10 Magnolia Hills	
			Project #:		
BM1 Kerto-S	; LVL 1.750" X [·]	14.000" 2-Ply	- PASSED	el: Level	
• • •	••••	• • •	• • •	• • • •	
• •	• • • •	• • •	• • •	• • •	<u>.</u> 1'2"
•••	• • •	• • •	• • •	• • • •	·⊣¥Ш↓
1 SPF 0-3-8				2 SPF 0-3	i-8 //
<u>/</u>		15'9"			3 1/2"
<u>/</u>		15'9"			
				1.00	
isten all plies using 3	rows of 10d Box nails (.12	(8x3") at 12" o.c Maxi	num end distance not to	o exceed 6".	
ad	228.5 PLF				
eld Limit per Foot	245.6 PLF				
eld Limit per Fastener	81.9 lb.				
v eld Mode	IV				
ge Distance	1 1/2"				
 End Distance 	3"				
ad Combination	D+L				
ration Factor	1.00				

Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. UVL not to be treated with fire retardant or corrosive	Handling & Installation 1. LVL beams must not be cut or drilled 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation	ponding This design is valid until 6/28/2026	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	

	Client:	Date:	10/1/2024	Page 5 of
TisDesign	Address:	Job Nam	e: Lot 10 Magnolia Hills	
		Project #		
GDH Kerto-S	LVL 1.750" X 24.000"	2-Ply - PASSED	Level: Level	
· · · ·	· · · · · ·		•••	П
			5	M
•••		• • • • •	· · / /	2'
			<u></u>	
1 SPF End Grain 0-4-8		2 SPF End Gra	ain 0-4-8 //	
f	17'			3 1/2"
ł	17'			
lulti-Ply Analysis				
asten all nlies using 3	rows of 10d Box nails (128x3") at 12	" o.c. Maximum end distance n	ot to exceed 6"	
ipacity	0.0 %			
ad Id Lineit n en Er et	0.0 PLF			
Id Limit per Foot	245.6 PLF 81.9 lb.			
, N	1			
ld Mode				
ge Distance n. End Distance	3"			
ad Combination				
uration Factor	1.00			
otes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	
alculated Structured Designs is responsibl tructural adequacy of this component ba	e only of the Handling & Installation ased on the 1. IVI beams must not be gut or drilled	ponding	Metsä Wood 301 Merritt 7 Building, 2nd Floor	
sign criteria and loadings shown. sponsibility of the customer and/or the o	It is the 2. Refer to manufacturer's product information contractor to regarding installation requirements. multi-nly	1	Norwalk, CT 06851	
sure the component suitability of the oplication, and to verify the dimensions and	he intended fastening details, beam strength values, and code approvals	2	(800) 622-5850 www.metsawood.com/us	
. Dry service conditions, unless noted other	3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained			
 LVL not to be treated with fire retardant 	or corrosive 5. Provide lateral support at bearing points to avoid lateral displacement and rotation	This design is valid until 6/28/2026		

1	isDesign	Client: Project: Address:			Date: Input by Job Nan Breiot f	10/1/2024 Neal Baggett te: Lot 10 Magnolia Hills	Page 7 of 9
BM2	Kerto-S L	.VL 1.75)" X 9.250'	' 2-Ply	- PASSED	Level: Level	
•	•	•	•	•	•	•	9 1
	• PF End Grain 0-3-0	•	•	•	• 2 SPF End Gra	• • • • • • • • • •	
			6'7" 6'7"			/ /	3 1/2"
Multi-Pl	y Analysis						
Fasten all Capacity	l plies using 2 row	vs of 10d Box nail	s (.128x3") at 12'	' o.c Maximu	m end distance r	not to exceed 6".	
Load Yield Limit p	er Foot	0.0 PLF 163.7 PLF					
Yield Limit p Cm Vield Mode	er Fastener	81.9 lb. 1 IV					
Edge Distan Min, End Dis	ce	1 1/2" 3"					
Load Combi	nation	1.00					
Notes		chemicals		6. For flat roofs provi	de proper drainage to prevent	Manufacturer Info	
Calculated Struct structural adequidesign criterio	ctured Designs is responsible only uacy of this component based of and loadings shown it is	of the Handling & Instal	lation be cut or drilled	ponding	·	Metsä Wood 301 Merritt 7 Building, 2nd Floor	
responsibility of ensure the co application and	the customer and/or the contract opponent suitability of the inter- to verify the dimensions and loads	ended fastening details, be	cturer's product information ion requirements, multi-ply am strength values, and code			Norwalk, CT 06851 (800) 622-5850	
Lumber 1. Dry service of	conditions, unless noted otherwise	 Damaged Beams mu Design assumes top Provide lateral sum 	edge is laterally restrained			www.mctsawoou.com/us	
2. LVL not to b	e treated with fire retardant or cor	rosive lateral displacement	and rotation	This design is y	alid uptil 6/29/2026		

1	isDesign		Client: Project: Address:			Date: Input by: Job Name Project #:	10/1/2024 Neal Baggett : Lot 10 Magnolia Hills	Page 9 of 9
FB1	SP #2	2.000"	X 10.000)" 2-Ply -	PASSED	L	evel: Level	
•	•	•	•	• •	•	•	• •	9 1/
•	•	•		• •	•		• • •	
	SPF 0-3-8			7'			2 SPF 0-3-8	3"
<i>∤</i> −−−−				7'				
Multi-Pl	ly Analysis							
Fasten al Capacity	ll plies using 3 ı	rows of 10d I 75.0 %	Box nails (.128	x3") at 12" o.c M	laximum end di	stance no	t to exceed 6".	
Load Yield Limit p	per Foot	262.0 PLF 349.5 PLF	:					
rield Limit p Cm (ield Mode	per Fastener	116.5 lb. 1 IV						
Edge Distar	nce	1 1/2"						
Load Comb	bination	S D+S						
Duration Fa	actor	1.15						
						Г	Manufacturer Info	
						F		1
				This d	esian is valid until 6/28	/2026		