

All Walls Shown Are Considered Load Bearing
Roof Area = 3092.61 sq.ft.
Ridge Line = 115.32 ft.
Hip Line = 0 ft.
Horiz. OH = 183.78 ft.
Raked OH = 218.43 ft.
Decking = 106 sheets

Dimension Notes 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise

2. All interior wall dimensions are to face of

3. All exterior wall to truss dimensions are to face of stud unless noted otherwise

stud unless noted otherwise

Hatch Legend 5' 6-3/4" Walls 5' 11-1/8" Walls Second Floor Walls Vaulted Ceiling



	Conne	Nail Info	ormation			
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS26	USP	35	NA	16d/3-1/2"	16d/3-1/2"
	THD26-2	USP	2	NA	16d/3-1/2"	10d/3"

▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)

LOAD CH	ART FOR JAC	CK STUDS	BUILDER	New Home, Inc.	CITY / CO.	Lillington / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design shorts for each trus design identified on the algoment drawing. The building designer	
NOWBER OF J	HEADER/GIRDER	DS FOR HOLE	JOB NAME	Lot 152 Duncan's Creek	ADDRESS	528 Duncan Creek Road	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
END REAC (UP TI (UP TI (UP TI (Z) PLY HI	END REAC (UP Tr (UP Tr (UP Tr (3) PLY H	END REAC (UP T (UP T) REQ'D STL	PLAN	The Guilford - Georgian "A", 2GRF	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 1 6800 2 10200 3	SEAL DATE	04/01/23	DATE REV.	09/23/24	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BEAMS
6800 4 8500 5 10200 6	10200 4 12750 5 15300 6	13600 4 17000 5	QUOTE #		DRAWN BY	Jonathan Landry	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.	Fayetteville, N.C. 28309 Phone: (910) 864-8787
11900 7 13600 8 15300 9			JOB #	J0924-5146	SALES REP.	Paul Hawkins	SignatureJonathan Landry	Fax: (910) 864-4444



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0924-5146 Lot 152 Duncan's Creek

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I68422658 thru I68422685

My license renewal date for the state of North Carolina is December 31, 2024.

North Carolina COA: C-0844



September 25,2024

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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A MiTek / 818 Soundside Road Edenton, NC 27932



TOP CHORD 1-2=-1632/542, 2-3=-921/393, 3-4=-865/349, 4-6=-881/370

BOT CHORD 1-9=-455/1430 7-9=-456/1439

WEBS 2-7=-1145/435, 3-7=-128/563, 4-7=-75/542, 2-9=-130/662

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 3-7-13 to 8-0-10, Interior(1) 8-0-10 to 15-0-8, Exterior(2) 15-0-8 to 19-5-5, Interior(1) 19-5-5 to 22-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10 except (jt=lb) 6=131.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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Job	Truss	Truss Type	Qty	Ply	Lot 152 Duncan's Creek
					168422663
J0924-5146	A3	ROOF SPECIAL	14	1	
					Job Reference (optional)
Comtech, Inc, Fayettev	ville, NC - 28314,		8	.630 s Jul	12 2024 MiTek Industries, Inc. Mon Sep 23 16:30:24 2024 Page 1
		ID:w_NkY	aQd4RDp	mohcDk_i	DbzuUsG-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f
		7-8-4	15-8-0		
		7-8-4	7-11-12		
					Scale = 1:68.2
		1 2			
		\$x4 8.00 <u> 12</u>			



-0 ₁ 3-8	7-8-4	15-8-0
0-3-8	7-8-4	7-11-12

LOADING	G (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (lo	oc) l/de	efl L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.22	Vert(LL)	-0.04 7	7-8 >99	9 360	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.23	Vert(CT)	-0.09 7	7-8 >99	9 240		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.40	Horz(CT)	0.04	6 n	/a n/a		
BCDL	10.0	Code IRC2015/TPI2014	Matrix-AS	Wind(LL)	0.03	7-8 >99	9 240	Weight: 146 lb	FT = 20%

LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2 *Except*
	2-8,5-6: 2x6 SP No.1

Plate Offsets (X,Y)-- [7:0-2-8,0-3-8]

BRACING-TOP CHORD BOT CHORD

WEBS

Structural wood sheathing directly applied, except end verticals.
 Rigid ceiling directly applied.
 1 Row at midpt 2-8, 3-8

REACTIONS. (size) 9=0-3-8, 6=Mechanical Max Horz 9=-441(LC 13) Max Uplift 9=-341(LC 13)

Max Grav 9=708(LC 20), 6=618(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-8=-295/235, 3-5=-887/0, 5-6=-591/64
- BOT CHORD 8-9=-761/665, 7-8=0/708

WEBS 3-8=-938/286, 3-7=0/567, 5-7=0/511

NOTES-

 Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 15-3-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

5) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=341.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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Job		Truss	Truss Type	Qty	Ply	Lot 152 Duncan's Creek	
						1684226	364
J0924-5146		A3GE	GABLE	2	1		
						Job Reference (optional)	
Comtech, Inc,	Fayettev	/ille, NC - 28314,		8	.630 s Jul	12 2024 MiTek Industries, Inc. Mon Sep 23 16:30:24 2024 Page 1	





7-8-4 Plate Offsets (X,Y)-- [6:0-4-0,0-4-8], [12:0-3-0,0-4-4], [16:0-1-8,0-0-2]

LOADING (TCLL 2 TCDL 7 BCLL	(psf) 20.0 10.0 0.0 *	SPACING- 2-0 Plate Grip DOL 1.7 Lumber DOL 1.7 Rep Stress Incr YE	-0 CSI. 15 TC 0. 15 BC 0. 15 WB 0.	0.07 DEFL. 0.06 Vert(LL) 0.21 Horz(CT)	in (-0.00 -0.00 0.01	(loc) l/defl 1 n/r 1 n/r 11 n/a	L/d 120 120 n/a	PLATES MT20	GRIP 244/190
BCDL '	10.0	Code IRC2015/TPI2014	4 Matrix-S	S				Weight: 168 lb	FT = 20%
LUMBER-				BRACING-					
TOP CHOR	D 2x6 SP	' No.1		TOP CHOR	D S	tructural wood s	heathing dire	ectly applied or 6-0-0 o	c purlins,
BOT CHOR	D 2x6 SP	' No.1			e	xcept end vertic	als.		
WEBS	2x6 SP	No.1 *Except*		BOT CHOR	D R	igid ceiling dired	ctly applied o	r 10-0-0 oc bracing, E	xcept:
	10-12:	2x4 SP No.2			6-	-0-0 oc bracing:	17-18,16-17	,15-16.	
OTHERS	2x4 SP	' No.2		WEBS	1	Row at midpt	2-	20, 3-19	

15-8-0

7-11-12

REACTIONS. All bearings 15-11-8.

Max Horz 21=-634(LC 13) (lb) -Max Uplift All uplift 100 lb or less at joint(s) 20 except 21=-224(LC 13), 16=-136(LC 11), 11=-341(LC 11), 19=-109(LC 13), 18=-141(LC 13), 17=-126(LC 13), 15=-127(LC 13), 14=-106(LC 13), 13=-150(LC 13), 12=-959(LC 13) Max Grav All reactions 250 lb or less at joint(s) 20, 21, 19, 18, 17, 15, 14, 13 except 16=396(LC 13), 11=1026(LC 13), 12=428(LC 11)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. 4-5=-253/205, 5-6=-354/285, 6-7=-453/360, 7-8=-538/427, 8-9=-649/517,

TOP CHORD

- 9-10=-727/578, 10-11=-1000/772
- 20-21=-531/674, 19-20=-532/671, 18-19=-533/671, 17-18=-534/671, 16-17=-532/667, BOT CHORD
- 15-16=-526/658, 14-15=-524/662, 13-14=-523/662, 12-13=-524/663
- WEBS 10-12=-683/866

NOTES-

- 1) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20 except (jt=lb) 21=224, 16=136, 11=341, 19=109, 18=141, 17=126, 15=127, 14=106, 13=150, 12=959.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 20, 16, 11, 19, 18, 17, 15, 14, 13, 12.



Scale = 1:71.9



818 Soundside Road

Edenton, NC 27932

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7-8-4	15-8-0
7-8-4	7-11-12

LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.23	Vert(LL)	-0.03	7-8	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.17	Vert(CT)	-0.06	7-8	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.38	Horz(CT)	0.03	6	n/a	n/a		
BCDL	10.0	Code IRC2015/TP	12014	Matri	x-AS	Wind(LL)	0.00	7	>999	240	Weight: 145 lb	FT = 20%

LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2 *Except*
	2-8.5-6: 2x6 SP No.1

Plate Offsets (X,Y)-- [7:0-2-8,0-3-8]

BRACING-TOP CHORD BOT CHORD

WEBS

D Structural wood sheathing directly applied, except end verticals.
 D Rigid ceiling directly applied.
 1 Row at midpt 2-8, 3-8

REACTIONS. (size) 8=Mechanical, 6=0-3-8 Max Horz 8=-441(LC 13) Max Uplift 8=-353(LC 13)

Max Grav 8=720(LC 20), 6=598(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-8=-298/237, 3-5=-836/0, 5-6=-566/52
- BOT CHORD 7-8=0/654

WEBS 3-8=-892/266, 3-7=0/528, 5-7=-14/484

NOTES-

 Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 15-3-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=353.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.





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BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

TOP CHORD 2x10 SP No.1 BOT CHORD 2x6 SP No.1

REACTIONS. (size) 1=0-3-8, 5=0-3-8 Max Horz 1=-163(LC 8) Max Uplift 1=-61(LC 12), 5=-65(LC 13) Max Grav 1=517(LC 1), 5=511(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-375/204, 2-3=-489/227, 3-4=-492/225, 4-5=-370/207

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 6-3-5, Exterior(2) 6-3-5 to 10-8-2, Interior(1) 10-8-2 to 12-4-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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Job	Truss	Truss Type	Qty	Ply	Lot 152 Duncan's Creek	
						168422667
J0924-5146	B1-GR	ROOF SPECIAL	1	2		
				-	Job Reference (optional)	
Comtech, Inc, Fayet	eville, NC - 28314,		8	.630 s Jul	12 2024 MiTek Industries, Inc. Mon Sep 23 16:30:26 2024	Page 2

ID:w_NkYaQd4RDpmohcDk_iObzuUsG-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-8=-86, 3-8=-60, 3-4=-60, 4-5=-86, 7-10=-20

Concentrated Loads (lb) Vert: 7=-1310(B) 12=-1308(B) 13=-865(B) 14=-865(B)

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5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=149, 5=153.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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and C-C Exterior(2) 0-2-12 to 4-7-9, Interior(1) 4-7-9 to 5-11-8, Exterior(2) 5-11-8 to 10-4-5, Interior(1) 10-4-5 to 12-8-1 zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=123, 5=130.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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TRENGINEERING BY A MITCH A HILIDA

September 25,2024



			5-2-8		5-2-8	
LOADING	i (psf)	SPACING- 2-0-0	CSI.	DEFL. in	n (loc) l/defl	L/d PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.09	Vert(LL) -0.00) 5-6 >999	360 MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.06	Vert(CT) -0.01	1 5-6 >999	240
BCLL	0.0 *	Rep Stress Incr YES	WB 0.04	Horz(CT) 0.00) 5 n/a	n/a
BCDL	10.0	Code IRC2015/TPI2014	Matrix-AS	Wind(LL) 0.00	0 6 >999	240 Weight: 85 lb FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2 *Excep
	2-7.4-5: 2x6 SP No.1

REACTIONS. (size) 7=0-3-8, 5=0-3-8 Max Horz 7=120(LC 9) Max Uplift 7=-78(LC 12), 5=-60(LC 13) Max Grav 7=460(LC 1), 5=395(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-377/200, 3-4=-372/195, 2-7=-462/290, 4-5=-403/218

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-1 to 3-7-12, Interior(1) 3-7-12 to 5-2-8, Exterior(2) 5-2-8 to 9-7-5, Interior(1) 9-7-5 to 10-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 5.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

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			020	020
LOADING	(psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.09	Vert(LL) -0.00 6-7 >999 360 MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.06	Vert(CT) -0.01 6-7 >999 240
BCLL	0.0 *	Rep Stress Incr YES	WB 0.04	Horz(CT) 0.00 6 n/a n/a
BCDL	10.0	Code IRC2015/TPI2014	Matrix-AS	Wind(LL) 0.00 7 >999 240 Weight: 102 lb FT = 20%

BRACING-

TOP CHORD BOT CHORD

LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2 *Except
	2-8,4-6: 2x6 SP No.1
OTHERS	2x4 SP No.2

2-8,4-6: 2x6 SP No.1 OTHERS 2x4 SP No.2 REACTIONS. (size) 8=0-3-8, 6=0-3-8

Max Horz 8=-159(LC 10) Max Uplift 8=-166(LC 12), 6=-166(LC 13) Max Grav 8=457(LC 1), 6=457(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-373/208, 3-4=-373/208, 2-8=-459/298, 4-6=-459/297

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=166, 6=166.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

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REACTIONS. (size) 8=0-3-0, 6=0-3-0

Max Horz 8=-104(LC 10) Max Uplift 8=-85(LC 8), 6=-85(LC 9)

Max Grav 8=293(LC 1), 6=293(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-163/273, 3-4=-163/273, 2-8=-274/369, 4-6=-274/369

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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IONS. (size) 8=0-3-0, 6=0-3-0 Max Horz 8=-130(LC 10) Max Uplift 8=-106(LC 13), 6=-106(LC 12) Max Grav 8=293(LC 1), 6=293(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-163/273, 3-4=-163/273, 2-8=-274/369, 4-6=-274/369

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=106, 6=106.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=112. 4=112.
- 6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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Job	Truss	Truss Type		Qty	Ply	Lot 152 Duncan's Cree	k
J0924-5146	M1	MONOPITCH		3	1		168422679
						Job Reference (optiona	l)
Comtech, Inc, Fayettev	ville, NC - 28314,			8	630 s Jul	12 2024 MiTek Industrie	s, Inc. Mon Sep 23 16:30:31 2024 Page 1
		-1-4-0	6-3-8	адаакор	moncDk_i 6-	·8-8	JSINSGPGNE8WSULLXDGKWICD017J4ZJC?I
		1-4-0	6-3-8		d-	-5-0	
							Scale = 1:42.1
				3	x4	4	
		I		-	2 /	1 .	
		8.0	0 12	/			
				//			
			9				
			8				
		4x8 1/2					
		7-9-2	. /			2-15	
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		04	\sim				
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		Ŕ			6 -		
		7 3v/ 11			- 5 6x6 —		
		374 []			0.00 —		
		<u> </u>	6-3-8				
Plate Offsets (X Y) [6:0	-1-8 0-3-0]		6-3-8				
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	ir	n (loc)	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.24	Vert(LL)	-0.01	6-7	>999 360	MT20 244/190
BCLL 0.0 *	Rep Stress Incr YES	WB 0.24	Horz(CT) -0.00	6	n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-AS	Wind(LL	ý 0.00	7	**** 240	Weight: 66 lb FT = 20%
			BRACIN	~			
TOP CHORD 2x6 SP No	1		TOP CH	G- ORD	Structur	al wood sheathing dire	ctly applied except end verticals
BOT CHORD 2x6 SP No	.1		BOT CH	ORD	Rigid ce	iling directly applied.	
WEBS 2x6 SP No	.1 *Except*				-		
2-6: 2x4 SI	P No.2						
REACTIONS. (size)	7=0-3-8, 6=Mechanical						
Max Horz	7=236(LC 9)						
Max Uplift	6=-225(LC 12)						
Max Grav	7=319(LC 1), 6=357(LC 19)						
FORCES. (Ib) - Max. Cor	np./Max. Ten All forces 250	0 (lb) or less except when showr	٦.				
TOP CHORD 3-6=-331	/300, 2-7=-262/23						
BOT CHORD 6-7=-404	/345						
VVEDS 2-0=-381	/44/						

NOTES-

 Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-1 to 3-2-12, Interior(1) 3-2-12 to 6-8-8 zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 This product force of 0 and forces for the plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=225.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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Job	Truss	Truss Type			Qtv	Plv	Lot 152 Duncan's Cree	k	
10004 5440	MAGE								168422680
J0924-5146	MIGE	GABLE			1	1	Job Reference (optiona	l)	
Comtech, Inc, Fayette	ille, NC - 28314,		-1-4-0 1-4-0	ID:w_NkYa <u>6-3-8</u> 6-3-8	8 aQd4RDp	.630 s Jul mohcDk_i 6-8 0-5	12 2024 MiTek Industrie ObzuUsG-RfC?PsB70Ho 3-8 5-0	s, Inc. Mon Sep 23 1 J3NSgPqnL8w3uITXI	6:30:31 2024 Page 1 oGKWrCDoi7J4zJC?f
									Scale = 1:43.2
		7-6-5 0-4-0 3-0-10	8.00 12 4x8 = 1 3x6	2 2x4 3x6	33 2x4 		4		
			⊠ 7 2x4	2x4		6 ₅			
			3x4			6x6 =			
				6-3-8 6-3-8					
Plate Offsets (X,Y) [6:0	-1-8,0-3-0]								
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014		CSI. TC 0.24 BC 0.13 WB 0.24 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT Wind(LL	in -0.01 -0.03) -0.00) 0.00	(loc) 6-7 6 6-7 6 6 7	l/defl L/d >999 360 >999 240 n/a n/a **** 240	PLATES MT20 Weight: 84 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP No BOT CHORD 2x6 SP No WEBS 2x6 SP No 2-6: 2x4 SP OTHERS 2x4 SP No	.1 .1 *Except* P No.2 .2			BRACIN TOP CH BOT CH	G- ORD ORD	Structur Rigid ce	al wood sheathing dire illing directly applied.	ctly applied, except	end verticals.
REACTIONS. (size) Max Horz Max Uplift Max Grav	7=0-3-8, 6=Mechanical 7=277(LC 9) 6=-362(LC 12) 7=319(LC 1), 6=377(LC 19)								

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 3-6=-331/329, 2-7=-262/12

 BOT CHORD
 6-7=-427/345

WEBS 2-6=-381/472

NOTES-

 Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=362.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



ENGINEERING BY AMITEK Affiliat

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Lot 152 Duncan's Cre	ek
10004 5440	10					I68422681
JU924-5146	MZ	MONOPITCH	4	1	Job Reference (option	al)
Comtech, Inc, Fayettev	ille, NC - 28314,		8	8.630 s Jul	12 2024 MiTek Industri	es, Inc. Mon Sep 23 16:30:32 2024 Page 1
		ID:w_N 6-3-8	lkYaQd4RDp	omohcDk_i 6-8-8	ObzuUsG-RfC?PsB70F	q3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f
		6-3-8		0-5-0		
						Scolo - 1:42 1
			0.1			Scale = 1:42.1
		I	3X4		-	
		8.00 12	2	2		
			8	A		
			//			
		7				
		419 (
		4x8			-15	
					2-2	
		110				
		3-0	\sim			
			\sim			
					l	
		Ŕ		5.		
		6 3x4	6	×6 =		
			0	X0 —		
		6-3-8				
Plate Offsets (X,Y) [5:0	-1-8,0-3-0]	0-3-8				
	004.0110	001 0551			1/-141 1/-1	
TCLL 20.0	Plate Grip DOI 1 15	TC 0.22 Vert	"	n (loc) 1 5-6	I/defi L/d	PLATES GRIP MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.13 Vert	CT) -0.03	3 5-6	>999 240	W120 244/100
BCLL 0.0 *	Rep Stress Incr YES	WB 0.21 Horz	CT) -0.00	5 5	n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-AS Wind	(LL) 0.00	0 6	**** 240	Weight: 63 lb FT = 20%
LUMBER-		BRAG	CING-			
TOP CHORD 2x6 SP No.	1	TOP	CHORD	Structur	al wood sheathing dire	ectly applied, except end verticals.
BOT CHORD 2x6 SP No.	1 1 *Eveent*	BOT	CHORD	Rigid ce	eiling directly applied.	
1-5: 2x4 SF	P No.2					
	. –					
REACTIONS. (size)	6=0-3-8, 5=Mechanical					
Max Horz Max Uplift	5=-216(LC - 9) 5=-216(LC - 12)					
Max Grav	6=225(LC 1), 5=364(LC 19)					
TOP CHORD 2-5346	ip./iviax. Ten All forces 250	ס (סו) or less except when shown.				
BOT CHORD 5-6=-358/	/303					
WEBS 1-5=-336	/397					

NOTES-

Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-2-12 to 4-7-9, Interior(1) 4-7-9 to 6-8-8 zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord any other members.

will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=216.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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TOP CHORD BOT CHORD 6-7=-413/356

WEBS 2-6=-372/432

NOTES-

1) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-1 to 3-2-12, Interior(1) 3-2-12 to 7-8-8 zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=221.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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 Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=362.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS

2x4 SP No.2

REACTIONS. 1=8-2-5, 3=8-2-5, 4=8-2-5 (size) Max Horz 1=78(LC 11) Max Uplift 1=-48(LC 12), 3=-55(LC 13), 4=-4(LC 12) Max Grav 1=159(LC 1), 3=161(LC 20), 4=266(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4. 6) Non Standard bearing condition. Review required.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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LUMBER-



BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD

2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. 1=6-8-5, 3=6-8-5, 4=6-8-5 (size) Max Horz 1=62(LC 11) Max Uplift 1=-38(LC 12), 3=-44(LC 13), 4=-3(LC 12) Max Grav 1=126(LC 1), 3=128(LC 20), 4=212(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4. 6) Non Standard bearing condition. Review required.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)

LOAD CHART FOR JACK STUDS (BASED ON TABLES R502.5(1) & (b))		BUILDER	New Home, Inc.	CITY / CO.	Lillington / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design checks for each twice design identified on the placement during. The building designer		
NOWBER OF 3	HEADER/GIRDER	NOTE (0 NOT SC RATION	JOB NAME Lot 152 Duncan's Creek		ADDRESS	528 Duncan Creek Road	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
END REAC (UP Tr (UP Tr (2) PLY HI	END REAC (UP TC (UP TC (3) PLY HI	END REAC (UP T REQ'D STU (4) PLY H	PLAN	The Guilford - Georgian "A", 2GRF	MODEL	Floor	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 1 6800 2 10200 3	SEAL DATE	04/01/23	DATE REV.	09/23/24	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BEAMS
6800 4 8500 5 10200 6	10200 4 12750 5 15300 6	13600 4 17000 5	QUOTE #		DRAWN BY	Jonathan Landry	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.	Fayetteville, N.C. 28309 Phone: (910) 864-8787
11900 7 13600 8 15300 9			JOB #	J0924-5147	SALES REP.	Paul Hawkins	SignatureJonathan Landry	Fax: (910) 864-4444



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0924-5147 Lot 152 Duncan's Creek

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I68394785 thru I68394794

My license renewal date for the state of North Carolina is December 31, 2024.

North Carolina COA: C-0844



September 24,2024

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



LUMBER-

 TOP CHORD
 2x4 SP No.1(flat)

 BOT CHORD
 2x4 SP No.1(flat)

 WEBS
 2x4 SP No.3(flat)

 OTHERS
 2x4 SP No.3(flat)

 BRACING

 TOP CHORD
 Structural wood sheathing directly applied or 3-8-8 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 3-8-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.



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Edenton, NC 27932



			7-3-8							
Plate Offsets (X,Y)	[7:Edge,0-1-8], [8:0-1-8,Edge], [9:0-1-8,	Edge], [11:0-1-8,0-1-8]	7-3-8							
LOADING (psf) TCLL 40.0 TCDL 10.0 BCU 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO	CSI. TC 0.23 BC 0.30 WB 0.23	DEFL. in Vert(LL) -0.03 Vert(CT) -0.05 Horz(CT) 0.01	(loc) l/defl 9-10 >999 9-10 >999 7 p/a	L/d 480 360	PLATES MT20	GRIP 244/190			
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S	1012(01) 0.01	7 11/a	11/a	Weight: 39 lb	FT = 20%F, 11%E			
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP	2400F 2.0E(flat) No.1(flat) No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural woo except end ver Rigid ceiling di	d sheathing dir ticals. rectly applied c	ectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,			
Max Gi	rav 10=563(LC 1), 7=445(LC 1)									
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-837/0, 3-4=-837/0, 4-5=-837/0 BOT CHORD 9-10=0/677, 8-9=0/837, 7-8=0/497 WEBS 2-10=-847/0, 2-9=0/299, 5-7=-623/0, 5-8=0/475										
NOTES- 1) Unbalanced floor live 2) Plates checked for a 3) Refer to girder(s) for 4) Recommend 2x6 strr Strongbacks to be at 5) CAUTION, Do not er 6) Hanger(s) or other co chord. The design/s 7) In the LOAD CASE(s	e loads have been considered for this de plus or minus 1 degree rotation about i truss to truss connections. ongbacks, on edge, spaced at 10-0-0 o ttached to walls at their outer ends or re rect truss backwards. onnection device(s) shall be provided so election of such connection device(s) is S) section, loads applied to the face of the	esign. ts center. c and fastened to each tru strained by other means. Ifficient to support concen the responsibility of others ne truss are noted as front	iss with 3-10d (0.131" X trated load(s) 240 lb dow s. : (F) or back (B).	3") nails. /n at 1-10-0 on	top					
LOAD CASE(S) Stand 1) Dead + Floor Live (b Uniform Loads (plf) Vert: 7-10=- Concentrated Loads	dard alanced): Lumber Increase=1.00, Plate 10, 1-6=-100 (lb)	Increase=1.00			Ċ	L'UNRTH C	AROLIN			

Vert: 12=-240(F)



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						7-3-8						
Diata Offacta (X		7.Edgo 0 1 9] [9:0 1 9]		Edge] [11:0 1	0 0 1 01	7-3-8						1
Plate Olisets (A	(, t)	[7.Euge,0-1-6], [6.0-1-6,	zugej, [9.0-1-6,	Eugej, [11.0-1	-0,0-1-0]							
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	F) D D D	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.00 1.00 NO	CSI. TC (BC (WB (0.70 0.78 0.71	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.08 -0.11 0.02	(loc) 7-8 7-8 7	l/defl >999 >762 n/a	L/d 480 360 n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	0	Code IRC2015/T	PI2014	Matrix-	S						Weight: 39 lb	FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP 2400F 2.0E(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) REACTIONS. (size) 10=0-3-8, 7=Mechanical Max Grav Max Grav 10=924(LC 1), 7=1599(LC 1)					BRACING- TOP CHOP BOT CHOP	RD RD	Structu except Rigid c	ral wood end verti eiling dire	sheathing dire cals. ectly applied o	ectly applied or 6-0-0 r 10-0-0 oc bracing.	oc purlins,	
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 6-7=-267/0, 2-3=-2258/0, 3-4=-2258/0, 4-5=-2258/0 BOT CHORD 9-10=0/1117, 8-9=0/2258, 7-8=0/1710 WEBS 5-7=-2146/0, 2-10=-1395/0, 5-8=0/891, 2-9=0/1496, 3-9=-797/0, 4-8=-539/0												

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 7-10=-10, 1-3=-100, 3-6=-503(F=-403)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



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818 Soundside Road Edenton, NC 27932

September 24,2024



6-3-8 6-3-8											I	
Plate Offsets (X,	Y)	1:Edge,0-1-8], [3:0-3-0,I	Edge], [4:0-1-8,I	Edge], [6:Ed	ge,0-1-8], [8:0	-1-8,Edge], [10:0	-1-8,0-1	1-8]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0		SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/T	2-0-0 1.00 1.00 NO PI2014	CSI. TC BC WB Matrix	0.66 0.68 0.73 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.07 -0.09 0.02	(loc) 8-9 8-9 6	l/defl >999 >803 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 41 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER-TOP CHORD2x4 SP No.1(flat)BOT CHORD2x4 SP No.1(flat)WEBS2x4 SP No.3(flat)					BRACING- TOP CHOR BOT CHOR	D D	Structu except Rigid c	ral wood end verti eiling dire	sheathing dire cals. ectly applied o	ectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,	
REACTIONS.	REACTIONS. (size) 9=0-3-8, 6=Mechanical Max Grav 9=1198(LC 1), 6=1989(LC 1)											
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 5-6=-357/155, 2-3=-2564/0, 3-4=-2564/0 BOT CHORD 8-9=0/1603, 7-8=0/2564, 6-7=0/2564 WEBS 2-9=-1966/0, 2-8=0/1285, 3-8=-750/0, 4-6=-3102/0												

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 6-9=-10, 1-11=-100, 5-11=-707(F=-607)



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A MiTek Affil 818 Soundside Road

Edenton, NC 27932

Job	Truss		Truss Type		Qty	Ply	Lot 152 Dun	can's Creek			1683047	790
J0924-5147	F3		FLOOR		8	1					1003347	30
					_		Job Reference	e (optional)				
Comtech, Inc,	Fayetteville, NC - 2831	4,				8.630 s Jul	12 2024 MiTel	< Industries, Inc	. Mon Sep 23 0	7:17:31 20	24 Page 1	
				ID:w	/_NKYaQd4RL	pmohcDk_i	ObzuUsG-RfC	?PsB70Hq3NS	gPqnL8w3u11Xb	GKWrCD	or/J4zJC?t	
0-1-8												
HI <mark>1-3-0</mark>		1-8-12	4				1-	5-4			0-1-8	
			1								Scale = 1:6	67.9
1.5x3			1.5x3			1.5x3					1.5x3	
1.5x3 =	1.5x3		3x6 FP = 3x6	= 4x6 $=$	4x6 = 4x	4 =	3x6 FP =		1.5x3		1.5x3 =	
1 2	3 4 5	6 7	7 891011	12 13	14 1	15 16 1	7 18 19	20	21 22 23	24	25	
9 47		A 1			*		× = /f			×	10	ļ
											₩ *°	1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-
☆				☆							緻	
46	45 44	43 42 4	41 40 39 38	37 36	35	34 33	32 31	30 29	28	27	26	
3x6 =	3x6 =	1.5x3	3x6 FI	P = 3x10 =	= 4x6 =	4x6 =		1.5x3	3x6 =		3x6 =	

3x6 FP = 1.5x3 ||

1.5x3 ||

 $4x6 \equiv 4x6 \equiv$

	39-11-0						
Plate Offsets (X,Y)	[6:0-1-8,Edge], [7:0-1-8,Edge], [19:0-1-	8,Edge], [20:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 1-7-3 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.77 BC 0.66 WB 0.63 Matrix-S	DEFL. Vert(LL) -0.2 Vert(CT) -0.3 Horz(CT) 0.0	in (loc) l/defl 27 42-43 >881 35 42-43 >677 04 26 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 201 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	BRACING- TOP CHORD BOT CHORD	CING- ' CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. ' CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.					
REACTIONS. (size Max G	e) 46=0-3-8, 26=0-3-8, 36=0-3-8 Grav 46=749(LC 3), 26=737(LC 4), 36=2	133(LC 1)					
FORCES. (lb) - Max. TOP CHORD 2-3=- 8-10= 14-11 20-2 ⁻ BOT CHORD 45-44 40-4 ⁻ 34-35 28-25 WEBS 2-46= 11-33 6-43=	Comp./Max. Ten All forces 250 (lb) or -1580/0, 3-4=-2600/0, 4-5=-2600/0, 5-6= =-1472/826, 10-11=-1472/826, 11-12=0/ 5=0/1573, 15-16=-1460/879, 16-17=-144 1=-2947/0, 21-22=-2544/0, 22-23=-2544 6=0/940, 44-45=0/2192, 43-44=0/2958, 1=-18/3001, 39-40=-573/2092, 37-39=-1 5=-1202/745, 32-34=-621/2067, 31-32=- 9=0/2885, 27-28=0/2149, 26-27=0/924 =-1177/0, 2-45=0/834, 3-45=-796/0, 3-4 7=-1301/0, 11-39=0/1052, 8-39=-883/0, =-73/484, 6-42=-334/0, 7-41=0/357, 24-	less except when shown. -3033/0, 6-7=-3001/18, 7- 1502, 12-13=0/3656, 13-1 50/879, 17-19=-2454/384, /0, 23-24=-1551/0 42-43=-18/3001, 41-42=-1 148/738, 36-37=-2330/0, 78/2917, 30-31=-78/2917 4=0/521, 12-36=-1663/0, 1 8-40=0/688, 7-40=-960/0, 26=-1158/0, 24-27=0/816.	8=-2500/336, 4=0/3656, 19-20=-2917/78, 8/3001, 35-36=-2343/0, , 29-30=-78/2917, 12-37=0/1317, 5-44=-457/3, 23-27=-779/0.				
23-28 15-34 19-3	6-43=-73/464, 6-42=-534/0, 7-41=0/537, 24-20=-1150/0, 24-27=0/616, 25-27=-779/0, 23-28=-7/504, 21-28=-435/20, 14-36=-1647/0, 14-35=0/1302, 15-35=-1285/0, 15-34=0/1034, 17-34=-876/0, 17-32=0/668, 19-32=-905/0, 20-29=-60/477, 20-30=-325/0, 19-31=0/344						
NOTES- 1) Unbalanced floor livv 2) All plates are 3x4 M 3) Plates checked for a 4) Recommend 2x6 str Strongbacks to be a 5) CAUTION, Do not e	e loads have been considered for this de T20 unless otherwise indicated. a plus or minus 1 degree rotation about i rongbacks, on edge, spaced at 10-0-0 c tttached to walls at their outer ends or re rect truss backwards.	esign. ts center. c and fastened to each tru strained by other means.	uss with 3-10d (0.131"	X 3") nails.		SE 458 SE 458 SE SE SE SE SE SE SE SE SE SE SE SE SE	AROK AL B44 NEFERSOTIUM

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September 24,2024



18

1.5x3 ||

17

16

3x6 =

15

4x4 =

14

3x6 =

	10.110								
Plate Of	fsets (X,Y)	[6:0-1-8,Edge], [7:0-1-8,Edge]		19-11-0					
LOADIN TCLL TCDL BCLL BCDL	IG (psf) 40.0 10.0 0.0 5.0	SPACING- 1-7-3 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.48 BC 0.85 WB 0.48 Matrix-S	DEFL. i Vert(LL) -0.3 Vert(CT) -0.4 Horz(CT) 0.0	in (loc) 5 18-19 8 18-19 8 14	l/defl L/d >677 480 >492 360 n/a n/a	PLATES MT20 M18AHS Weight: 102 lb	GRIP 244/190 186/179 FT = 20%F, 11%E	
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	3RACING- "OP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. 3OT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.					
REACTIONS. (size) 24=0-3-8, 14=Mechanical Max Grav 24=862(LC 1), 14=867(LC 1)									
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1862/0, 3-4=-3166/0, 4-5=-3166/0, 5-6=-3867/0, 6-7=-4083/0, 7-8=-3867/0, 8. 0. 2456/0, 40.11 2456/0, 41.12 9. 0. 2456/0, 40.11 24.552/0									
BOT CH	BOT CHORD 23-24=0/1087, 22-23=0/2609, 20-22=0/3640, 19-20=0/4083, 18-19=0/4083, 17-18=0/4083, 16-17=0/3640, 15-16=0/2609, 14-15=0/1088								

10-11-9

WEBS 2-24=-1362/0, 2-23=0/1008, 3-23=-973/0, 3-22=0/711, 5-22=-605/0, 5-20=0/410, 6-20=-506/75, 12-14=-1365/0, 12-15=0/1008, 11-15=-972/0, 11-16=0/711, 8-16=-605/0, 8-17=0/410, 7-17=-506/75

22 21

3x6 =

3x10 M18AHS FP =

20

19

1.5x3 ||

NOTES-

1) Unbalanced floor live loads have been considered for this design.

23

3x6 =

4x4 =

2) All plates are MT20 plates unless otherwise indicated.

3) All plates are 3x4 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.

5) Refer to girder(s) for truss to truss connections.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.



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Job	Truss	Truss Type	Qty	Ply	Lot 152 Duncan's Cre	ek
J0924-5147	F4A	Floor Girder	1	1		168394792
Comtech, Inc, Fayett				3.630 s Ju	Job Reference (option 1 12 2024 MiTek Industrie	al) es, Inc. Mon Sep 23 07:17:32 2024 Page 1
· · · ·			ID:w_NkYaQd4RDp	omohcDk_	iObzuUsG-RfC?PsB70H	lq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f
0-1-8	109	1.4	10 100 1	20	100 100	
H ⊨ 1-3-0	1-9-0			-2-0	<u></u>	1-2-0 1-2-0 1-4-12 0-1-0 Scale = 1:32.9
3x4 3x6	2x6 3x6	4x6 3x4	3x4 =		3x6 FP=	3x4 =
	3 33 4	34 5 6 7	8 9	10	11 12 13	14 15 16
3						
×	20 28	27 26				
3x6 =	29 20 3x4 =	3x4 = 3x6 FP =	24 23	22	21 20	3x6 =
		3x6 =				
	8-6-8	8-8-0			10-11-8	
	8-6-8	0-1-8			11-3-8	
	:Eage,0-1-8], [3:0-3-0,Eage], [i	3:0-1-8,Eagej, [15:0-1-8,Eagej, [25	9:0-1-8,Eage]			
LOADING (psf) TCLL 40.0	SPACING- 2-0-0 Plate Grip DOL 1.00	CSI. TC 0.27	DEFL. i Vert(LL) -0.02	n (loc) 2 28	l/defl L/d >999 480	PLATES GRIP MT20 244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.27	Vert(CT) -0.03	3 28	>999 360	
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S	Horz(CT) 0.0	1 25	n/a n/a	Weight: 106 lb FT = 20%F, 11%E
LUMBER-			BRACING-			
TOP CHORD 2x4 SP N	lo.1(flat)		TOP CHORD	Structu	ral wood sheathing dire	ectly applied or 6-0-0 oc purlins,
WEBS 2x4 SP N	lo.3(flat)		BOT CHORD	Rigid c	eiling directly applied o	r 6-0-0 oc bracing.
REACTIONS. All bear	ings 11-5-0 except (it=length)	30=0-3-8.				
(lb) - Max Upli Max Gray	ft All uplift 100 lb or less at jo	int(s) 17 except 24=-318(LC 4)	$x_{0} = \frac{1}{20} = \frac{502(1 - 1)}{20}$	5-1221/1	C 4)	
Wax Gra	25=1321(LC 1), 18=255(LC	4)	xcept 30=392(EC 1), 2	J=1321(L	0 4),	
FORCES. (lb) - Max. Co	omp./Max. Ten All forces 250) (Ib) or less except when shown.				
TOP CHORD 2-3=-11	48/0, 3-4=-1148/0, 4-5=-653/0	, 5-7=0/818, 7-8=0/818				
WEBS 2-30=-8	379/0, 2-29=0/541, 5-25=-1162	//0, 5-27=0/639, 4-27=-618/0, 3-29	9=-285/0,			
8-25=-8	349/0, 8-24=0/323					
NOTES-	ands have been considered fo	r this docian				
2) All plates are 1.5x3 MT	20 unless otherwise indicated					
3) Plates checked for a pl4) Provide mechanical co	lus or minus 1 degree rotation innection (by others) of truss to	about its center. b bearing plate capable of withstar	nding 100 lb uplift at joi	nt(s) 17 e	except (jt=lb)	
24=318. 5) Recommend 2x6 stron	indhacks on edge spaced at 1	0-0-0 oc and fastened to each true	ss with 3-10d (0 131")	(3") naile		
Strongbacks to be atta	ched to walls at their outer end	ds or restrained by other means.	33 Will 5 100 (0.101 7	() naiis		MANINIT
 6) CAUTION, Do not erec 7) Hanger(s) or other con 	ct truss backwards. inection device(s) shall be prov	vided sufficient to support concent	rated load(s) 186 lb do	wn at 1-8	8-15, 125 lb	TH CARO
down at 3-4-2, and 15	5 lb down at 4-11-5, and 186	Ib down at 6-6-8 on top chord. Th	he design/selection of	such conr	nection	OF FESSION N
8) In the LOAD CASE(S)	section, loads applied to the fa	ace of the truss are noted as front	(F) or back (B).		Ų	mingun
LOAD CASE(S) Standar	rd					SFAL
1) Dead + Floor Live (bala	anced): Lumber Increase=1.00), Plate Increase=1.00				45844
Vert: 17-30=-1	10, 1-16=-100					
Concentrated Loads (II Vert: 2=-106(E	р) 3) 4=-106(В) 33=-106(В) 34=-1	106(B)				TA SNOTER S
,						DALGINE
						WIEW JOH
						September 24.2024

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A MITEK ATHI

Job	Truss	Truss Type	Qty	Ply	Lot 152 Duncan's Creek		10000 (700
10924-5147	F5	FLOOR	6	1			168394793
00024-0147	15		U		Job Reference (optional)		
Comtech, Inc, Fayettev	ille, NC - 28314,		8	.630 s Jul	12 2024 MiTek Industries, Inc.	Mon Sep 23 07:17:33 2	024 Page 1
		ID:w_	_NkYaQd4RDp	mohcDk_i0	DbzuUsG-RfC?PsB70Hq3NSgP	qnL8w3uITXbGKWrCE	oi7J4zJC?f
1-3-0		<u> 1-0-0 1-5-0</u>					0- <u>1</u> -8
							Scale = 1:32.8
					1.5x3		1.5x3
4x4 =	1.5x3	II			3x6 FP =	4x4 =	1.5x3 =
1 2	3 4	5 6 7		8	9 10 11	12	13
							25
24	23 22 21	20 19 18	3 17		16	15	14
3x6 =	3x8 M18AHS FP = 3x6 = 4x4 =	= 1.5x3 1.5x	x3		3x6 =	4x4 =	3x6 =

+			<u>19-8-0</u>						
Plate Offsets (X,Y) [1:Edge,0-1-8], [6:0-1-8,Edge], [7:0-1-8,Edge]									
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 1-7-3 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.47 BC 0.83 WB 0.47 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.33 -0.46 0.07	(loc) 18 18 14	l/defl >704 >512 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 101 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x BOT CHORD 2x WEBS 2x REACTIONS.	BRACING-TOP CHORDStructural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.BOT CHORDRigid ceiling directly applied or 10-0-0 oc bracing.					c purlins,			
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1830/0, 3-4=-3105/0, 4-5=-3105/0, 5-6=-3778/0, 6-7=-3962/0, 7-8=-3772/0, 8-10=-3102/0, 10-11=-3102/0, 11-12=-1830/0 BOT CHORD 23-24=0/1071, 21-23=0/2561, 20-21=0/3552, 19-20=0/3962, 18-19=0/3962, 17-18=0/3962, 16-17=0/3562, 15-16=0/2562, 14-15=0/1071 WEBS 2-24=-1344/0, 2-23=0/987, 3-23=-952/0, 3-21=0/694, 12-14=-1341/0, 12-15=0/988,									

11-15=-953/0, 11-16=0/690, 8-16=-588/0, 8-17=0/389, 7-17=-470/86, 5-21=-571/0, 5-20=0/415, 6-20=-480/83

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.3) All plates are 3x4 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.

5) Refer to girder(s) for truss to truss connections.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancement description (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



			4-0-0			
			4-0-0		I	
Plate Offsets (X,Y)	[1:Edge,0-1-8], [2:0-1-8,Edge], [3:0-1-8,	Edge], [9:0-1-8,0-1-8]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.09 BC 0.06 WB 0.06 Matrix-S	DEFL. in Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) 0.00	(loc) l/defl L/d 7 >999 480 7 >999 360 5 n/a n/a	PLATES MT20 Weight: 24 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	ectly applied or 4-0-0 r 10-0-0 oc bracing.	oc purlins,	

REACTIONS. (size) 8=Mechanical, 5=0-3-8 Max Grav 8=206(LC 1), 5=200(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



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A MiTek Affi 818 Soundside Road Edenton, NC 27932

