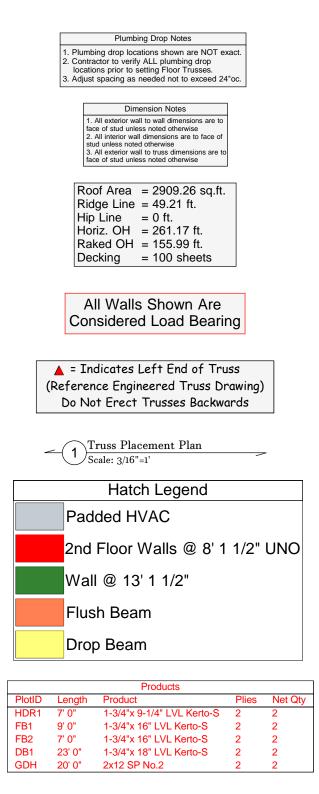


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ilding ding signer. design Iding em and uss walls, ding acing, the	JOB #	J0724-4080	SALESMAN Neil Baggett	Neil Baggett	0 0 0 0 0 1 1 1 1 1 1 1 6 0 0 1	de the tive num 3000# esign ds ined to	∕IS ⁺k



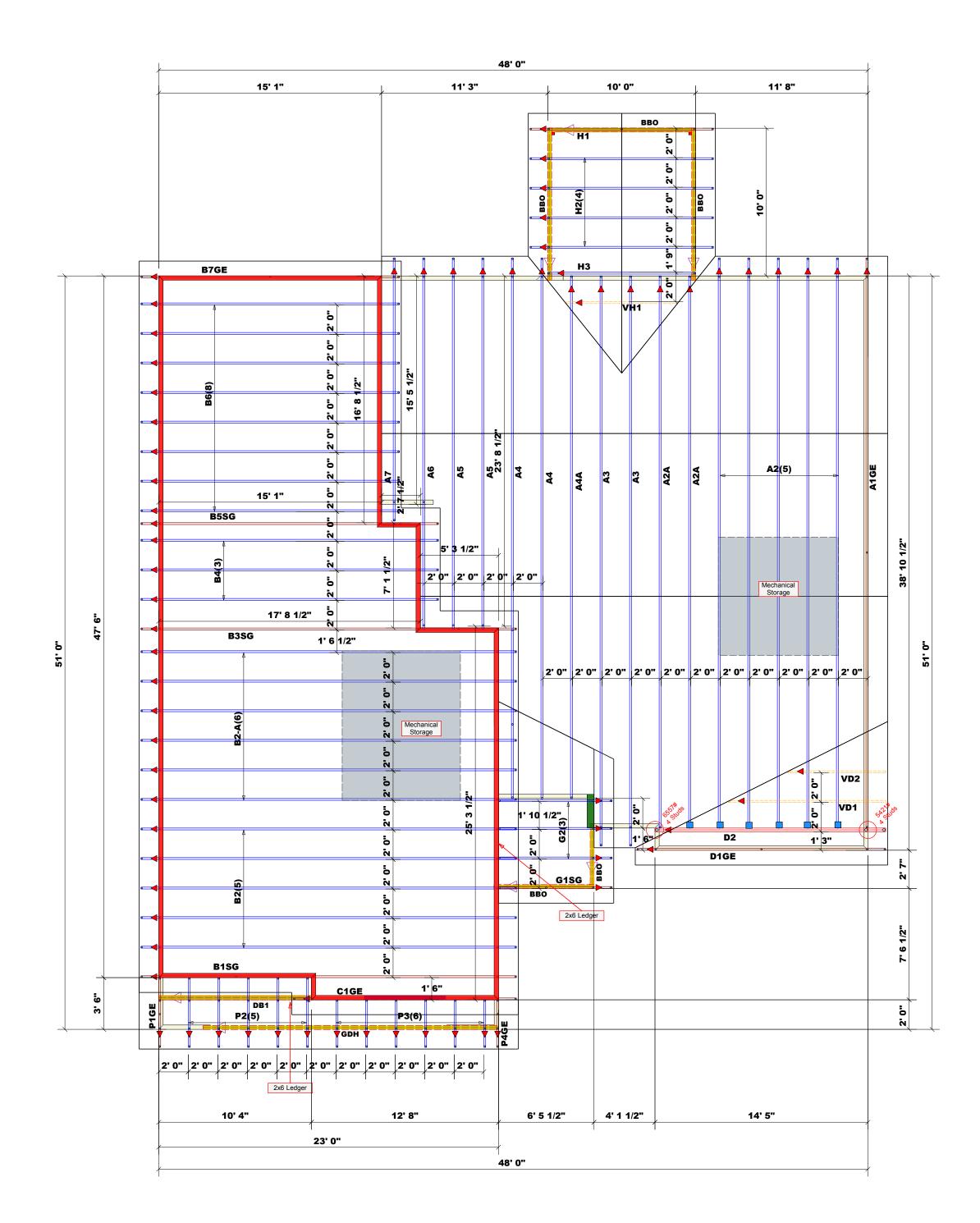
All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise. -- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

	Conne	ctor Info	rmati	on	Nail Info	ormation
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
\bigcirc	HUS410	USP	8	Varies	16d/3-1/2"	16d/3-1/2"
	HUS26	USP	7	Varies	16d/3-1/2"	16d/3-1/2"

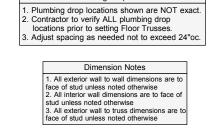
9.61/2" 2.7" 38' 101/2"

6

51.



These to comport design is See ind identified designed support and coll designed consult	BUILDER	Precision Custom Homes	COUNTY	Harnett	NUN	deeme require attache Code r founda require but no profes suppoi those registe design exceed	
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e designe e incorpo acification sign shee placemen onsible fo ng of the includin he respor neral guid and BCS	PLAN	Hazlitt w/CP	MODEL	Roof	ON TABLE	oly with the contra (derived nts) to d and numi port react than 1500 all be reta for any rr in the atta n profess ort system	SES
ed as indi prated into a of the bi- ets for ea the drawing r tempora roof and he design g headers nsibility of lance reg I-B3 prov	SEAL DATE 9/11/2024	9/11/2024	DATE REV.	9/12/2024	ES R502.5(1) REQUIREC /GIRDER UD_SCOLS Q DOJ NAId (E) 1 2 3 0 4 0 5	he prescr ctor shall from the etermine ber of wo ions grea 0#. A reg ined to d eaction th ached Tal sional sha	& FL & B dustr , N.C.
GRAM ON vidual bu o the buil uilding de ch truss (g. The bui ary and floor syst n of the tr s, beams, f the buil arding br; ided with sbcindus	QUOTE #	N/A	DRAWN BY	Neil Baggett	CK STU (4 (b)) (2) CA (b)) (2) CA END (1) CA (4 (b)) (2) CA (4 (b))	iter than 3 istered de esign the nat exceed bles. A all be reta reactions	
ilding ding signer. design lding em and uss walls, ding acing, the	JOB #	J0724-4079	SALESMAN Neil Baggett	Neil Baggett	0 0 0 0 0 8 N 1 REQ'D STUDS FOR 0 (4) PLV HEADER (4) PLV HEADER 0 0 0	de the tive num 3000# esign ds ined to	∕IS ⁺k



Plumbing Drop Notes

Roof Area	= 2909.26 sq.ft.
Ridge Line	= 49.21 ft.
Hip Line	= 0 ft.
Horiz. OH	= 261.17 ft.
Raked OH	= 155.99 ft.
Deckina	= 100 sheets

All Walls Shown Are Considered Load Bearing

Indicates Left End of Truss (Reference Engineered Truss Drawing) Do Not Erect Trusses Backwards

Truss Placement Plan Scale: 3/16"=1'

Hatch Legend Padded HVAC 2nd Floor Walls @ 8' 1 1/2" UNO Wall @ 13' 1 1/2" Flush Beam Drop Beam

		Products		
PlotID	Length	Product	Plies	Net Qty
HDR1	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
FB1	9' 0"	1-3/4"x 16" LVL Kerto-S	2	2
FB2	7' 0"	1-3/4"x 16" LVL Kerto-S	2	2
DB1	23' 0"	1-3/4"x 18" LVL Kerto-S	2	2
GDH	20' 0"	2x12 SP No.2	2	2
-				

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise. -- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

	Conne	ctor Info	rmati	ion	Nail Info	ormation
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
\bigcirc	HUS410	USP	8	Varies	16d/3-1/2"	16d/3-1/2"
	HUS26	USP	7	Varies	16d/3-1/2"	16d/3-1/2"



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0724-4079 Lot 8 Magnolia Hills

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I68187592 thru I68187623

My license renewal date for the state of North Carolina is December 31, 2024.

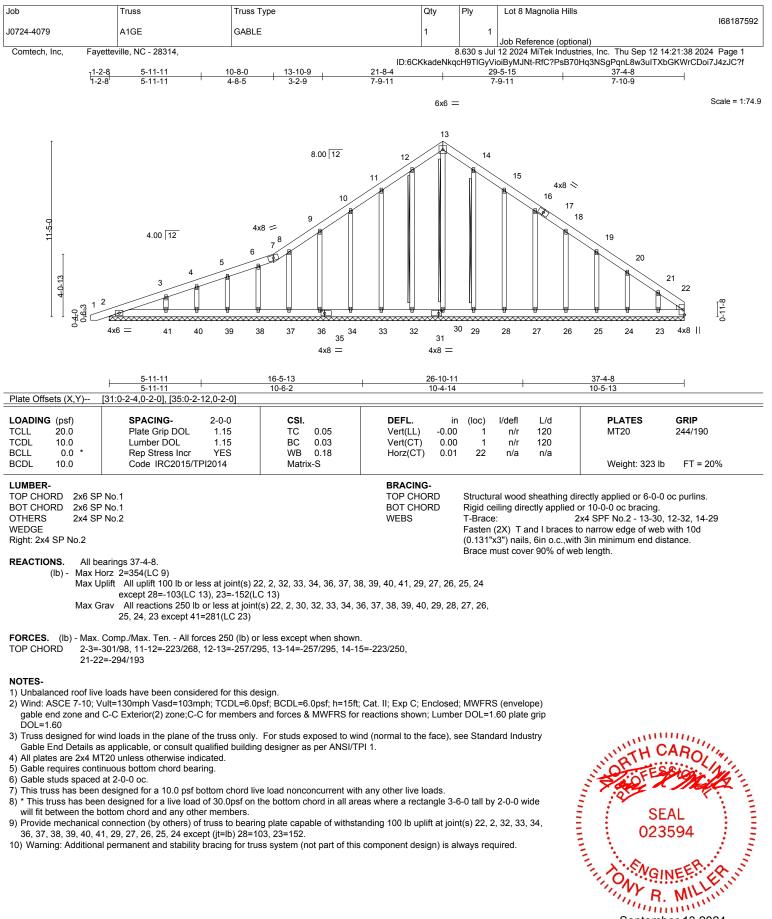
North Carolina COA: C-0844



September 13,2024

Tony Miller

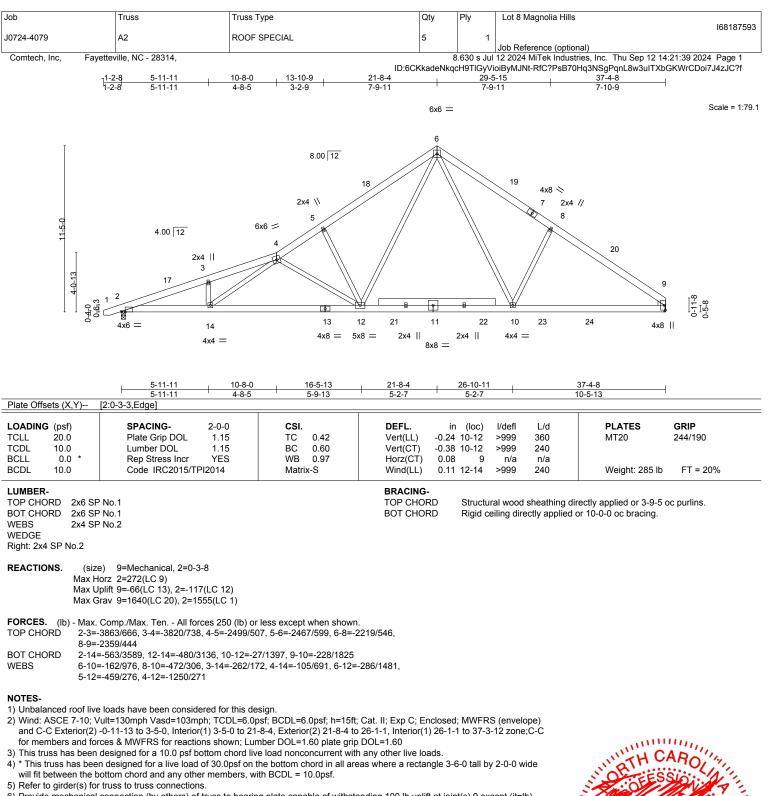
IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



September 13,2024



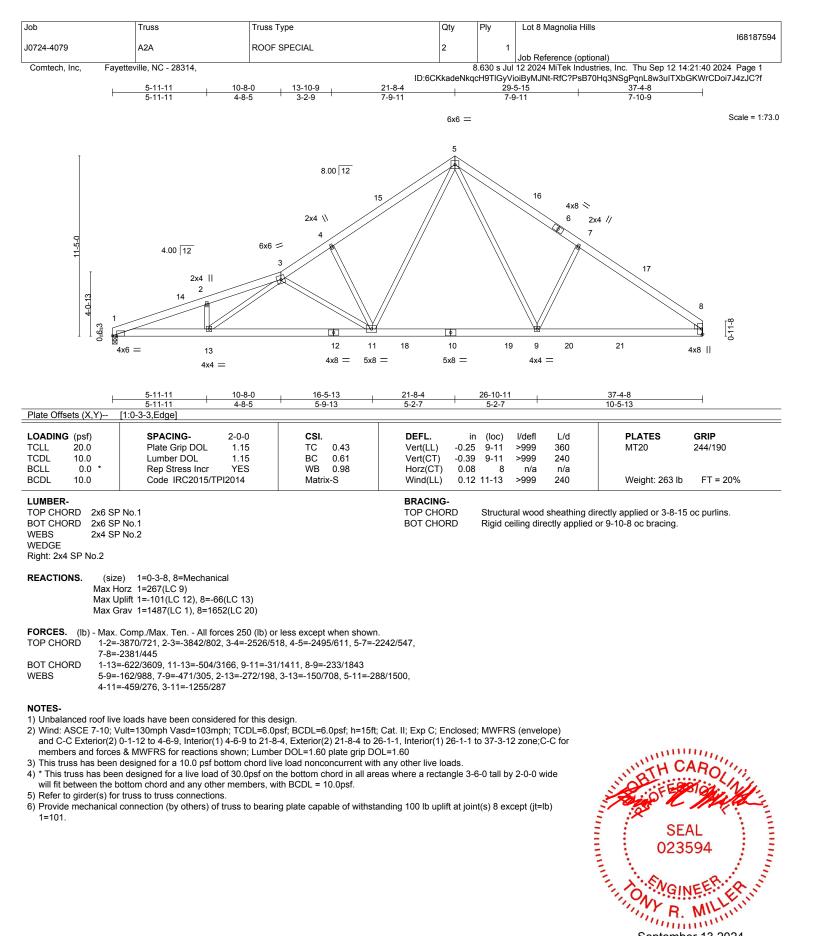
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 2=117.



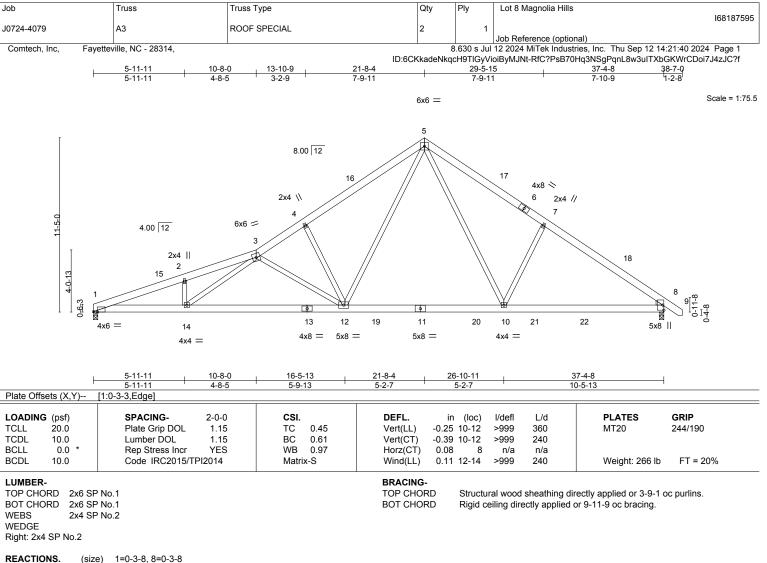
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



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ENGINEERING BY RENCO

September 13,2024



) .	(size)	1=0-3-8, 8=0-3-8
	Max Horz	1=269(LC 11)
	Max Uplift	1=-101(LC 12), 8=-82(LC 13)
	Max Grav	1=1482(LC 1), 8=1719(LC 20)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 1-2=-3857/719, 2-3=-3829/799, 3-4=-2514/515, 4-5=-2483/608, 5-7=-2218/529, 7-8=-2359/430

 BOT CHORD
 1-14=-612/3597, 12-14=-493/3157, 10-12=-22/1406, 8-10=-218/1814

 WEBS
 5-10=-157/965, 7-10=-458/297, 2-14=-272/198, 3-14=-150/708, 5-12=-285/1498, 4-12=-459/275, 3-12=-1252/286

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 21-8-4, Exterior(2) 21-8-4 to 26-1-1, Interior(1) 26-1-1 to 38-5-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

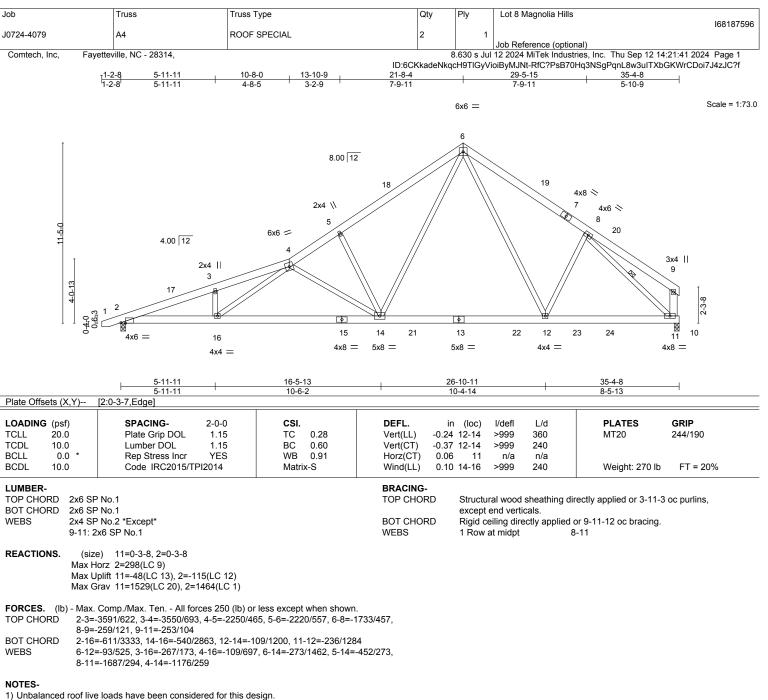
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 1=101.



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2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-13 to 3-5-0, Interior(1) 3-5-0 to 21-8-4, Exterior(2) 21-8-4 to 26-1-1, Interior(1) 26-1-1 to 35-0-4 zone; end vertical right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

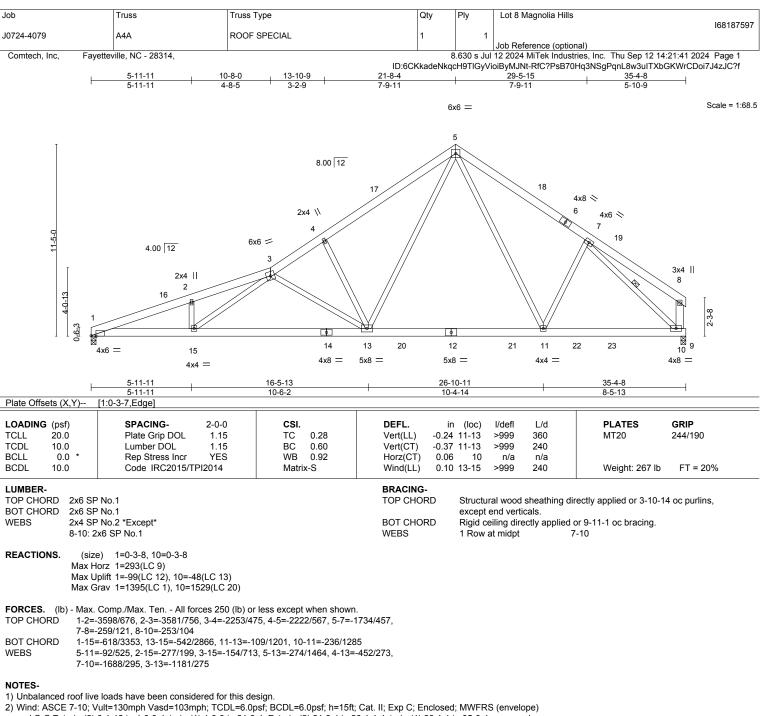
This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11 except (jt=lb) 2=115



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and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 21-8-4, Exterior(2) 21-8-4 to 26-1-1, Interior(1) 26-1-1 to 35-0-4 zone; end

vertical right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

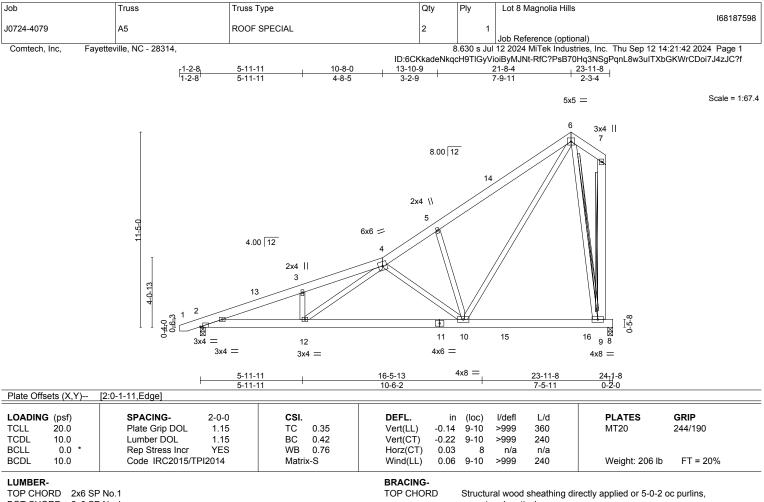
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 10.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





 BOT CHORD
 2x6 SP No.1

 WEBS
 2x4 SP No.2 *Except*

 7-9: 2x6 SP No.1

TOP CHORD BOT CHORD WEBS

except end verticals. Rigid celling directly applied or 10-0 oc bracing. T-Brace: 2x4 SPF No.2 - 6-9, 7-9 Fasten (2X) T and I braces to narrow edge of web with 10d (0.4011/20) pails (0.5 - 9, with 0.9 mails are not distance)

(0.131"x3") nails, 6in o.c.,with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. (size) 8=0-3-8, 2=0-3-8 Max Horz 2=338(LC 12) Max Uplift 8=-142(LC 12), 2=-46(LC 12) Max Grav 8=1050(LC 19), 2=1022(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2278/165, 3-4=-2258/240, 4-5=-1093/63, 5-6=-1111/199

WEBS 6-9=-884/325, 3-12=-296/184, 4-12=-139/720, 6-10=-276/1321, 5-10=-471/284, 4-10=-780/126

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-13 to 3-5-0, Interior(1) 3-5-0 to 21-8-4, Exterior(2) 21-8-4 to 23-5-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 8=142.

6) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

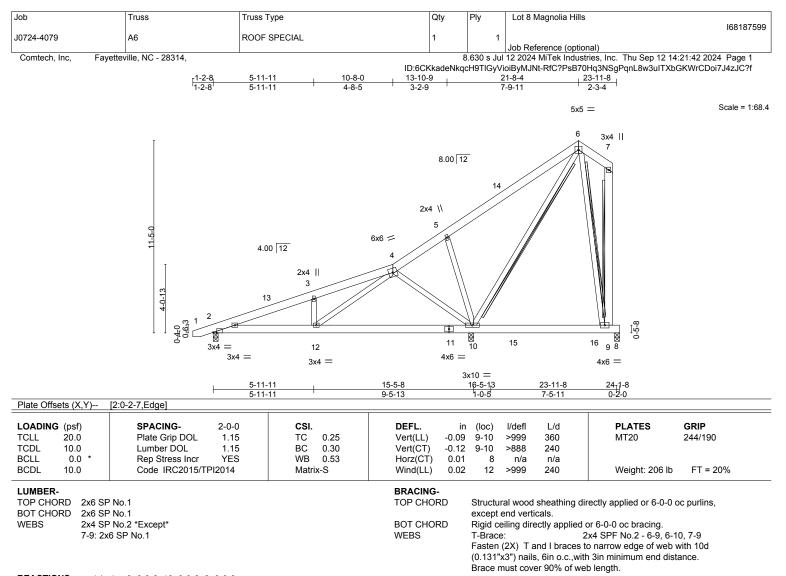


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818 Soundside Road

BOT CHORD 2-12=-460/2096, 10-12=-356/1526



REACTIONS. (size) 8=0-3-8, 10=0-3-8, 2=0-3-8 Max Horz 2=338(LC 12) Max Uplift 8=-53(LC 12), 10=-139(LC 12), 2=-91(LC 8) Max Grav 8=195(LC 19), 10=1343(LC 2), 2=551(LC 23)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-899/70, 3-4=-891/133, 4-5=-203/378, 5-6=-95/455

BOT CHORD 2-12=-227/797

WEBS 3-12=-334/191, 4-12=-155/811, 6-10=-601/64, 5-10=-475/285, 4-10=-398/91

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-13 to 3-5-0, Interior(1) 3-5-0 to 21-8-4, Exterior(2) 21-8-4 to 23-5-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 2 except (jt=lb) 10=139.

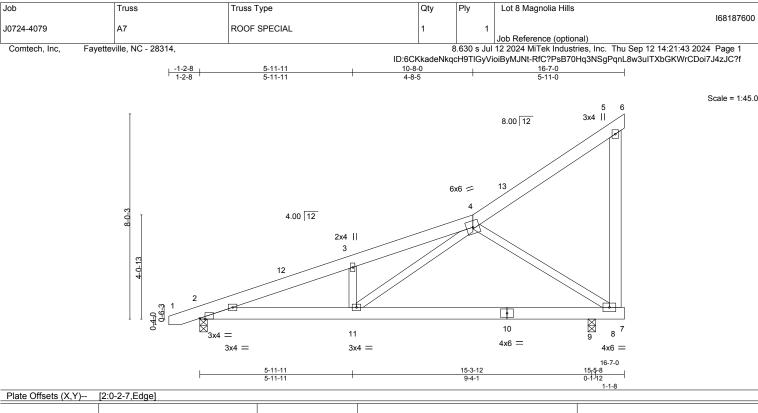
6) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcaccomponents.com)

ERENCO A MiTek Affilia

818 Soundside Road



TOP CHORD

BOT CHORD

LUN	IBER-
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TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2 *Except*
	5-8: 2x6 SP No.1

REACTIONS. (size) 2=0-3-8, 9=0-3-8 Max Horz 2=251(LC 12) Max Uplift 2=-74(LC 8), 9=-133(LC 12) Max Grav 2=673(LC 1), 9=710(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1212/13, 3-4=-1196/77

BOT CHORD 2-11=-188/1092, 9-11=-146/547, 8-9=-146/547

WEBS 3-11=-307/180, 4-11=-71/687, 4-8=-665/187

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-13 to 3-5-0, Interior(1) 3-5-0 to 16-7-0 zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 9=133.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

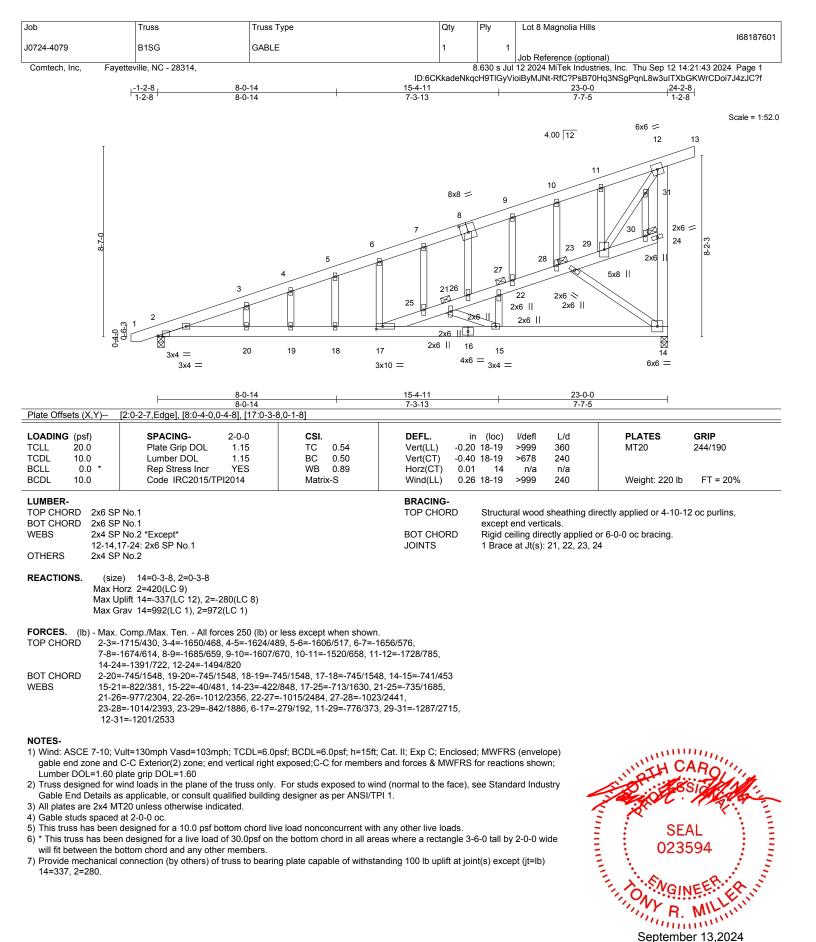
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

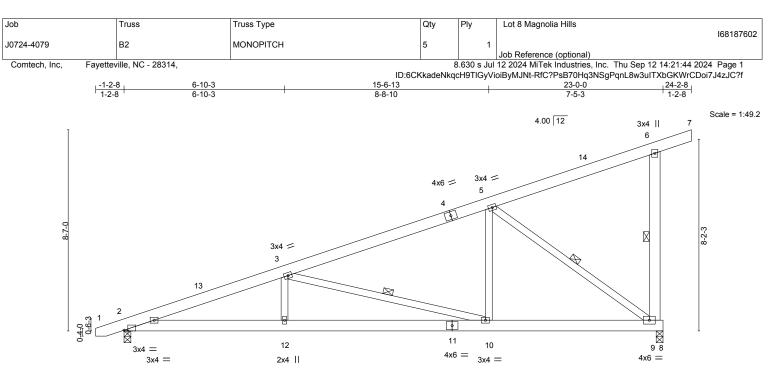
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of frusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **PCB Building Component Scitut Information**. Building from the Structure Building Component Advance interpretention. and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



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	6-10-3	15-6-13	23-0-0	
	6-10-3	8-8-10	7-5-3	
Plate Offsets (X,Y) [2:0-1	-15 Edgel			

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.26	Vert(LL) -0.06 10-12 >999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.31	Vert(CT) -0.14 10-12 >999 240	
BCLL 0.0	Rep Stress Incr YES	WB 0.39	Horz(CT) 0.03 9 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.05 10-12 >999 240	Weight: 168 lb FT = 20%

BRACING-TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2 *Except*
	6-9: 2x6 SP No.1

REACTIONS. (size) 9=0-3-8, 2=0-3-8 Max Horz 2=318(LC 9) Max Upliff 9=-138(LC 12), 2=-118(LC 8) Max Grav 9=1002(LC 1), 2=966(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2096/317, 3-5=-1046/177, 6-9=-268/202

BOT CHORD 2-12=-505/1927, 10-12=-505/1927, 9-10=-286/918

WEBS 3-12=0/320, 3-10=-1044/248, 5-10=0/556, 5-9=-1125/246

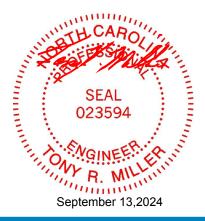
NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-13 to 3-5-0, Interior(1) 3-5-0 to 24-2-8 zone; end vertical right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=138, 2=118.



Structural wood sheathing directly applied or 5-3-2 oc purlins,

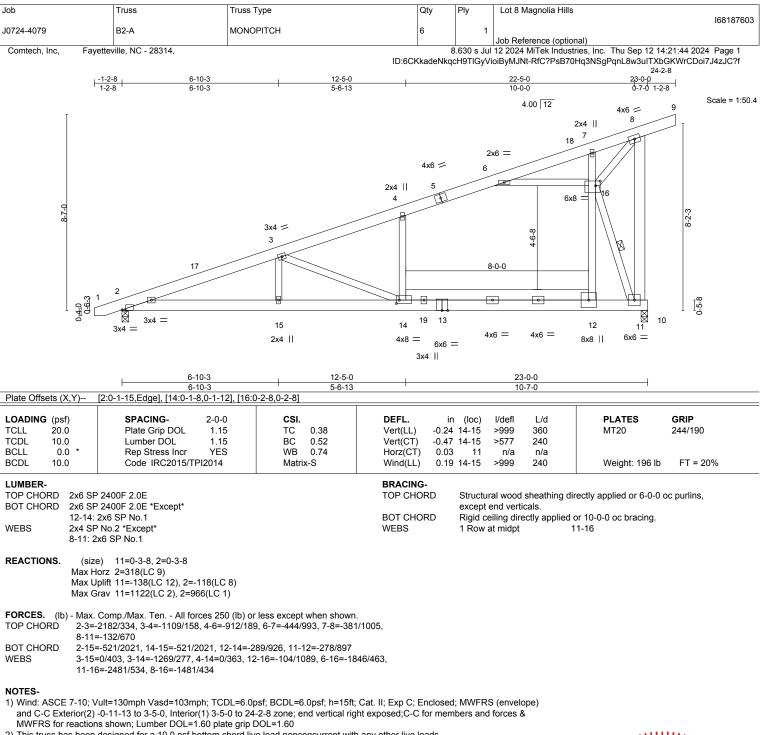
6-9, 3-10, 5-9

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

1 Row at midpt

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

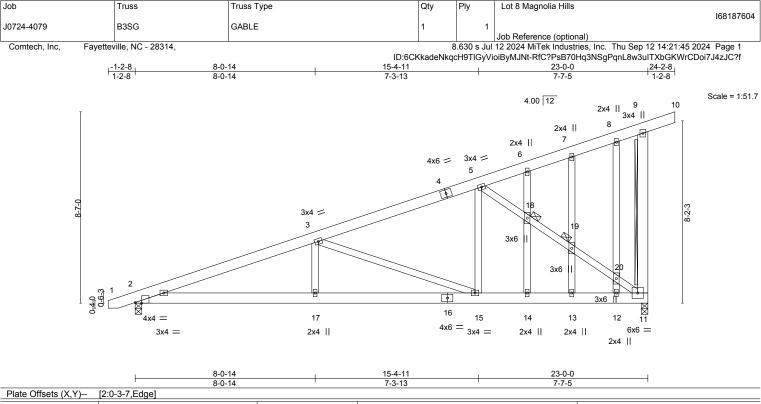
3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=138, 2=118.



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LOADING (psf) TCLL 20.0 TCDL 10.0 3CLL 0.0 * 3CDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.28 BC 0.35 WB 0.98 Matrix-S	DEFL. in Vert(LL) -0.06 Vert(CT) -0.12 Horz(CT) 0.03 Wind(LL) 0.07	6 17 >99 2 2-17 >99 3 11 r	99 360 99 240 n/a n/a	PLATES MT20 Weight: 194 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP No.1		Maulix-S	BRACING- TOP CHORD	Structural w	rood sheathing dir	ectly applied or 5-1-8	
BOT CHORD 2x6 SP No.1 VEBS 2x4 SP No.2 9-11: 2x6 SP			BOT CHORD WEBS	T-Brace:	directly applied o 2	or 8-5-15 oc bracing. x4 SPF No.2 - 9-11	
OTHERS 2x4 SP No.2			JOINTS	(0.131"x3")	nails, 6in o.c.,with cover 90% of wel	o narrow edge of web a 3in minimum end dist o length.	
Max Horz 2= Max Uplift 1	=0-3-8, 2=0-3-8 =420(LC 9) =-337(LC 12), 2=-280(LC 8) =992(LC 1), 2=972(LC 1)				n(3). 10, 10		
OP CHORD 2-3=-1986/	- , ,		2-13=-476/932,				
VEBS 3-17=0/326		8, 5-18=-1125/472, 18-19=-110	1/462,				

Lumber DOL=1.60 plate grip DOL=1.60 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

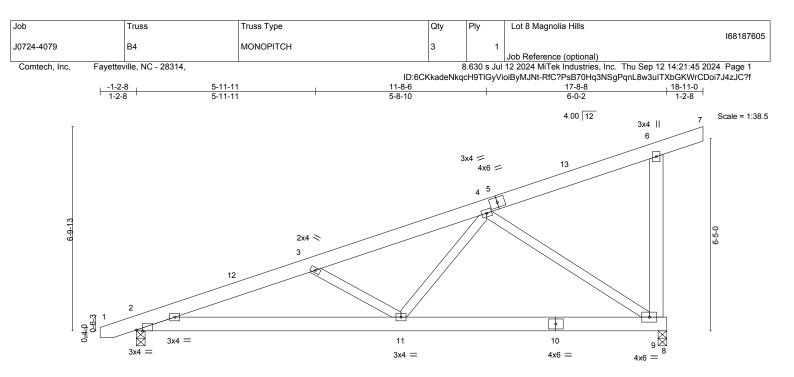
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=337, 2=280.

7) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



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		 		<u>-10-0</u> -10-0					<u>17-</u> 8-1	8-8 0-8		
Plate Off	Plate Offsets (X,Y) [2:0-2-7,Edge]											
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEF	L. i	n (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.13	Vert	(LL) -0.04	2-11	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.29	Vert	(CT) -0.0	3 2-11	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.64	Horz	(CT) 0.0	2 9	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matrix	-S	Win	d(LL) 0.0	2 11	>999	240	Weight: 125 lb	FT = 20%

LUMBER-		BRACING-		
TOP CHORD	2x6 SP No.1	TOP CHORD	Structural wood sheathing dir	ectly applied or 6-0-0 oc purlins,
BOT CHORD	2x6 SP No.1		except end verticals.	
WEBS	2x4 SP No.2 *Except*	BOT CHORD	Rigid ceiling directly applied of	or 10-0-0 oc bracing.
	6-9: 2x6 SP No.1			

REACTIONS. (size) 9=0-3-8, 2=0-3-8 Max Horz 2=250(LC 9) Max Uplift 9=-111(LC 12), 2=-99(LC 8) Max Grav 9=791(LC 1), 2=754(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD

2-3=-1447/263, 3-4=-1085/145 2-11=-404/1331, 9-11=-248/677 BOT CHORD

WEBS 3-11=-417/223, 4-11=-12/578, 4-9=-802/219

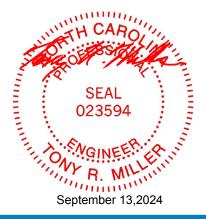
NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-13 to 3-5-0, Interior(1) 3-5-0 to 18-11-0 zone; end vertical right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

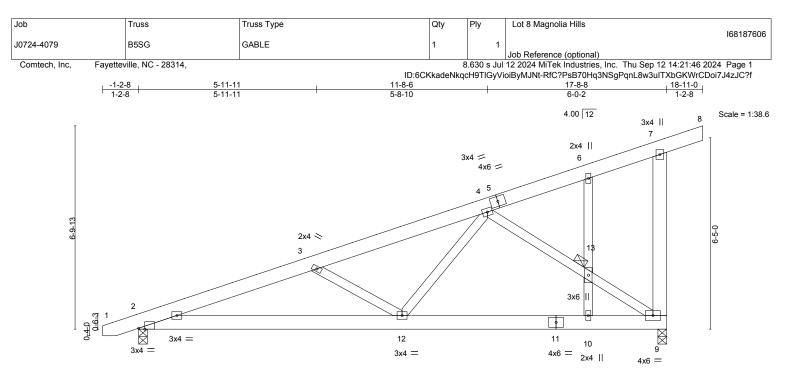
3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 9=111.



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				10-0 10-0						17-8 8-10				
Plate Offs	sets (X,Y)	[2:0-2-7,Edge]												
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DE	FL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.15	TC	0.13	Ver	rt(LL) -	-0.04	2-12	>999	360	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.28	Ver	rt(CT) -	-0.09	2-12	>999	240			
BCLL	00 *	Ren Stress Incr	YES	WB	0.15	Ho	$r_{z}(CT)$	0.02	9	n/a	n/a	1		

Wind(LL)

0.03

12 >999

240

LUMBER-	-		BRACING-		
TOP CHO	DRD 2x6 SP	? No.1	TOP CHORD	Structural wood sheathing dir	ectly applied or 6-0-0 oc purlins,
BOT CHO	DRD 2x6 SP	? No.1		except end verticals.	
WEBS	2x4 SP	No.2 *Except*	BOT CHORD	Rigid ceiling directly applied of	or 9-4-4 oc bracing.
	7-9: 2x	6 SP No.1	JOINTS	1 Brace at Jt(s): 13	
OTHERS	2x4 SP	2 No.2			

Matrix-S

REACTIONS. (size) 9=0-3-8, 2=0-3-8 Max Horz 2=330(LC 9) Max Uplift 9=-269(LC 12), 2=-226(LC 8) Max Grav 9=781(LC 1), 2=760(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1462/479, 3-4=-1102/301, 7-9=-254/200

Code IRC2015/TPI2014

BOT CHORD 2-12=-708/1344, 10-12=-377/698, 9-10=-377/698

WEBS 3-12=-412/325, 4-12=-94/538, 4-13=-822/367, 9-13=-818/363

NOTES-

BCDL

10.0

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=269, 2=226.

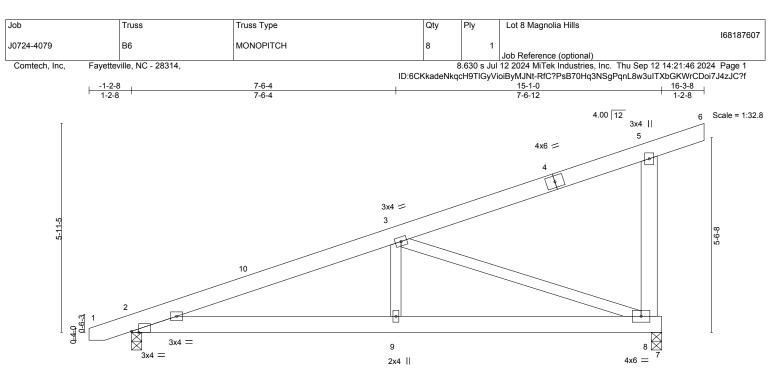


FT = 20%

Weight: 132 lb

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				-6-4 -6-4						<u>1-0</u> -12		4	
Plate Of	fsets (X,Y)	[2:0-2-7,Edge]				_							
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	-0.03	2-9	>999	360	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.22	Vert(CT)	-0.06	2-9	>999	240			

	0.0 * 0.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.95 Matrix-S	Horz(CT) 0.0 Wind(LL) 0.0		n/a >999	n/a 240	Weight: 103 lb	FT = 20%
LUMBER-				BRACING-					
TOP CHORE				TOP CHORD	Struct	ural wood	d sheathing d	irectly applied or 6-0-0 or	c purlins,
BOT CHORE	2x6 SP	No.1			excep	t end ver	ticals.		
WEBS	2x4 SP	No.2 *Except*		BOT CHORD	Rigid	ceiling dir	ectly applied	or 10-0-0 oc bracing.	
	5-8: 2x	6 SP No.1							

REACTIONS. (size) 8=0-3-8, 2=0-3-8 Max Horz 2=217(LC 9) Max Uplift 8=-98(LC 12), 2=-89(LC 8) Max Grav 8=687(LC 1), 2=649(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-1073/150, 5-8=-283/215 2-9=-282/954, 8-9=-282/954 BOT CHORD

WEBS 3-9=0/328, 3-8=-976/224

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-13 to 3-5-0, Interior(1) 3-5-0 to 16-3-8 zone; end vertical right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

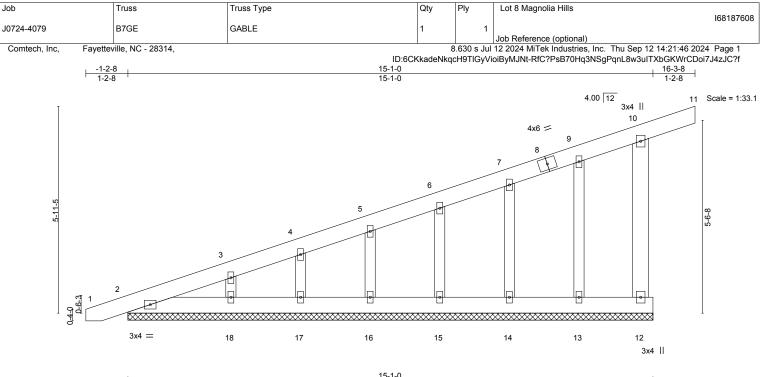
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 2.



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LOADING (psf)	SPACING-										
	SFACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	тс	0.08	Vert(LL)	0.00	<u></u> 10	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	-0.00	11	n/r	120		
BCLL 0.0	* Rep Stress Incr	YES	WB	0.03	Horz(CT)	-0.00	12	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI	12014	Matrix	x-S						Weight: 110 lb	FT = 20%

LUMBER-

 TOP CHORD
 2x6 SP No.1

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x6 SP No.1

 OTHERS
 2x4 SP No.2

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 15-1-0.

(lb) - Max Horz 2=285(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 2, 13, 14, 15, 16, 17, 18 except 12=-116(LC 9) Max Grav All reactions 250 lb or less at joint(s) 12, 2, 13, 14, 15, 16, 17, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-351/183, 3-4=-297/158, 4-5=-262/148

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) All plates are 2x4 MT20 unless otherwise indicated.

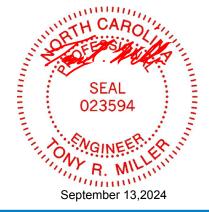
4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

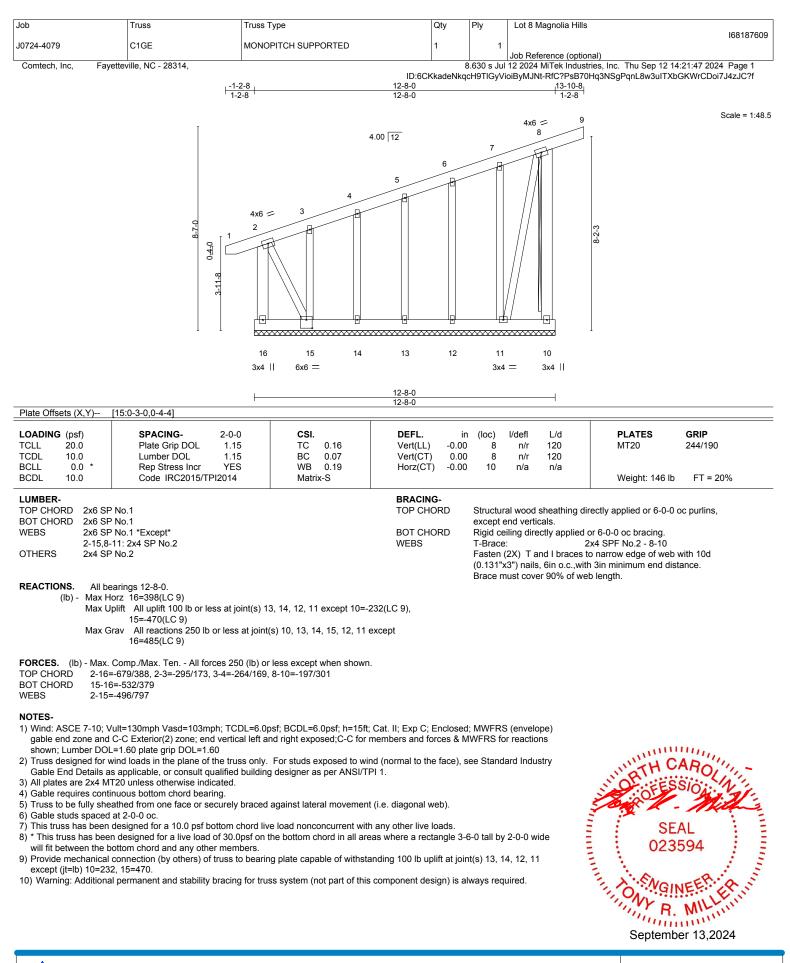
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

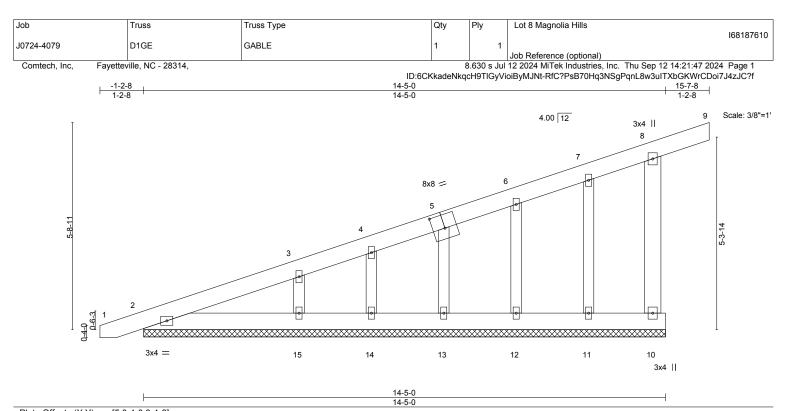
Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 13, 14, 15, 16, 17, 18 except (jt=lb) 12=116.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



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.OADING (psf) CLL 20.0 CDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15	CSI. TC 0.07 BC 0.04	DEFL. in Vert(LL) 0.00 Vert(CT) -0.00		l/defl n/r n/r	L/d 120 120	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.03 Matrix-S	Horz(CT) -0.00) 10	n/a	n/a	Weight: 103 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SP	No.1 No.1		BRACING- TOP CHORD		iral wood end vert	0	ectly applied or 6-0-0 o	oc purlins,

EXCEPT end verticals. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 14-5-0.

(lb) - Max Horz 2=274(LC 9)

2x6 SP No.1

2x4 SP No.2

Max Uplift All uplift 100 lb or less at joint(s) 2, 11, 12, 13, 14 except 10=-115(LC 9), 15=-116(LC 12) Max Grav All reactions 250 lb or less at joint(s) 10, 2, 11, 12, 13, 14 except 15=334(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

TOP CHORD

WEBS

OTHERS

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) All plates are 2x4 MT20 unless otherwise indicated.

2-3=-329/177, 3-4=-252/136

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

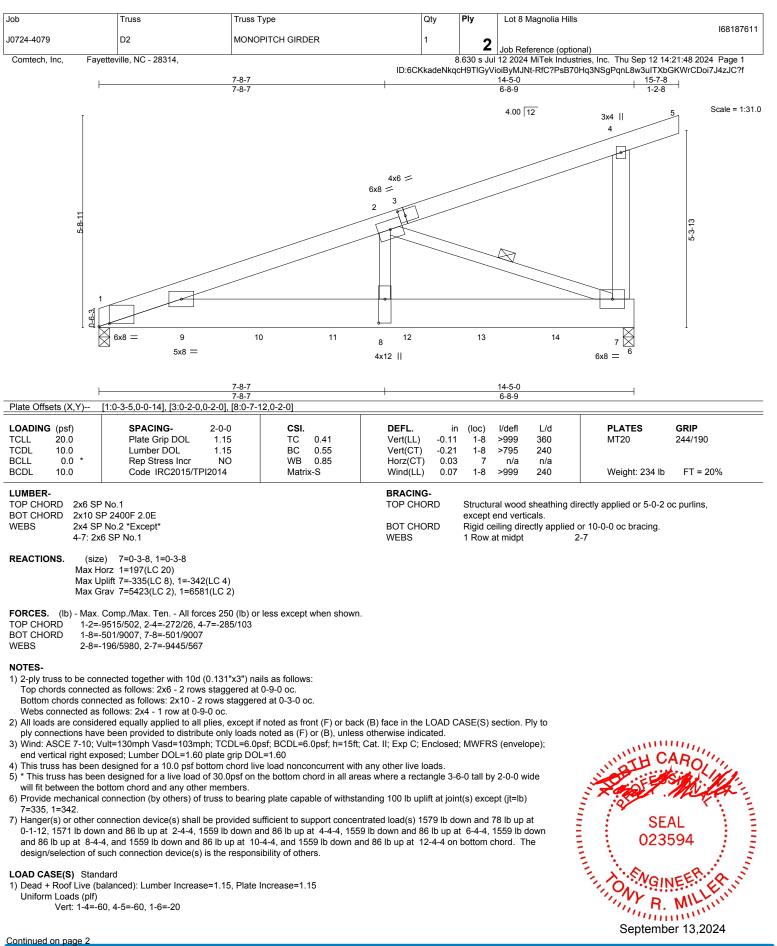
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11, 12, 13, 14

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11, 12, 13, 14 except (jt=lb) 10=115, 15=116.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Continued on page 2

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[Job	Truss	Truss Type	Qty	Ply	Lot 8 Magnolia Hills
						I68187611
	J0724-4079	D2	MONOPITCH GIRDER	1	2	
					_	Job Reference (optional)
	Comtech, Inc, Fayettev	ville, NC - 28314,			3.630 s Jul	12 2024 MiTek Industries, Inc. Thu Sep 12 14:21:48 2024 Page 2
			ID:6Ck	KadeNkq	cH9TIGyVi	piByMJNt-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 1=-1475(B) 9=-1467(B) 10=-1466(B) 11=-1466(B) 12=-1466(B) 13=-1466(B) 14=-1466(B)

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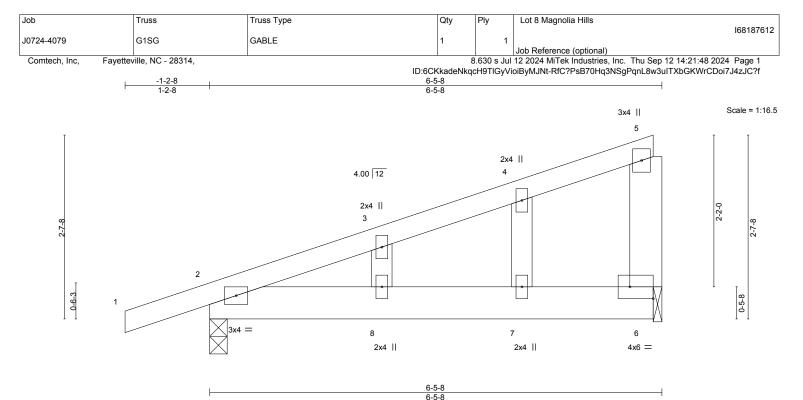


Plate Offsets (X,Y)	[6:Edge,0-2-0]	1			
LOADING (psf)	SPACING- 2-0-0	CSI.		n (loc) l/defl L/d	PLATES GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15	TC 0.14 BC 0.17	Vert(LL) 0.04 Vert(CT) -0.03		MT20 244/190
BCLL 0.0 *	Rep Stress Incr YES	WB 0.02	Horz(CT) -0.00	6 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S			Weight: 34 lb FT = 20%
LUMBER-			BRACING-		
TOP CHORD 2x4 SP			TOP CHORD	U	directly applied or 6-0-0 oc purlins,
BOT CHORD 2x6 SP	NO.1			except end verticals.	

except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 6=0-1-8 Max Horz 2=120(LC 8) Max Uplift 2=-194(LC 8), 6=-149(LC 8) Max Grav 2=333(LC 1), 6=235(LC 1)

2x6 SP No.1

2x4 SP No.2

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

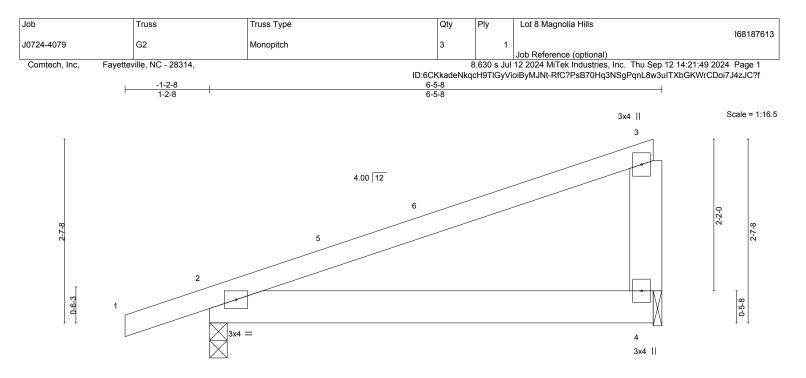
WEBS

OTHERS

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=194, 6=149.



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6-5-8										
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.50	Vert(LL)	-0.02	2-4	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.15	Vert(CT)	-0.04	2-4	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	0.00		n/a	n/a		
BCDL 10.0	Code IRC2015	TPI2014	Matrix-P	Wind(LL)	0.04	2-4	>999	240	Weight: 31 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

WEBS 2x6 SP No.1 REACTIONS. (size) 2=0-3-0, 4=0-1-8

S. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=85(LC 8) Max Uplift 2=-135(LC 8), 4=-102(LC 8) Max Grav 2=333(LC 1), 4=235(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 6-2-12 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=135, 4=102.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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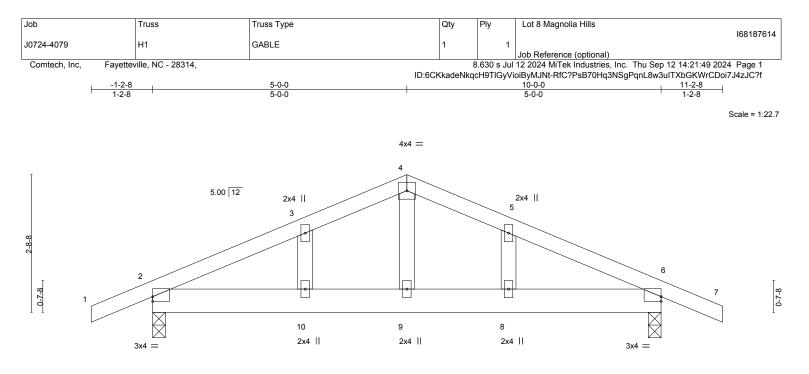


Plate Offsets (X,Y)	[2:0-0-0,0-1-2], [6:0-0-0,0-1-2]	5-0-0 5-0-0		10-0-0 5-0-0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.16 BC 0.29 WB 0.06 Matrix-S	DEFL. in (loc) Vert(LL) -0.01 10 Vert(CT) -0.02 10 Horz(CT) 0.00 6 Wind(LL) 0.02 8	>999 360 >999 240 n/a n/a	PLATES GRIP MT20 244/190 Weight: 50 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF OTHERS 2x4 SF	No.1 No.2			tural wood sheathing direct ceiling directly applied or	otly applied or 6-0-0 oc purlins. 10-0-0 oc bracing.
Max U	e) 2=0-3-0, 6=0-3-0 orz 2=52(LC 12) plift 2=-204(LC 8), 6=-204(LC 9) rav 2=470(LC 1), 6=470(LC 1)				

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-542/623, 3-4=-499/662, 4-5=-499/662, 5-6=-542/624
- BOT CHORD 2-10=-482/442, 9-10=-482/442, 8-9=-482/442, 6-8=-482/442
- WEBS 4-9=-384/234

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

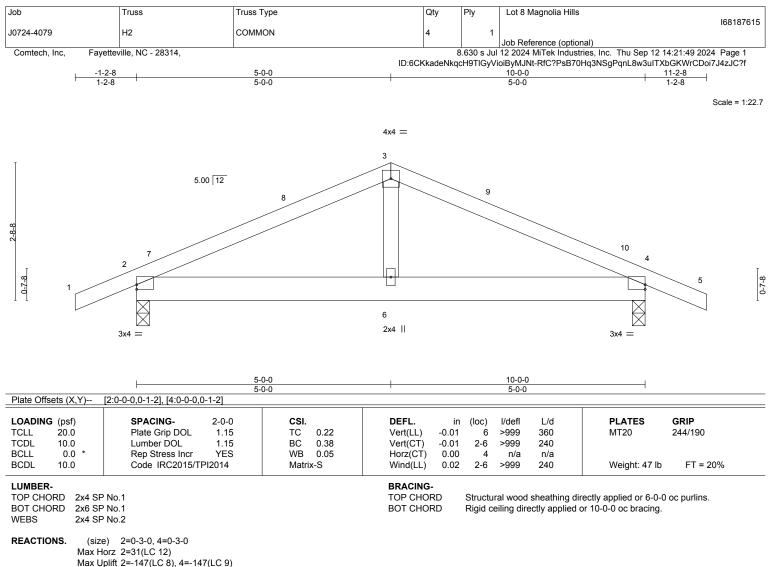
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=204, 6=204.



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Max Grav 2=470(LC 1), 4=470(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-553/619, 3-4=-553/619

BOT CHORD 2-6=-456/439, 4-6=-456/439 WEBS 3-6=-321/238

WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 5-0-0, Exterior(2) 5-0-0 to 9-4-13, Interior(1) 9-4-13 to 11-2-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

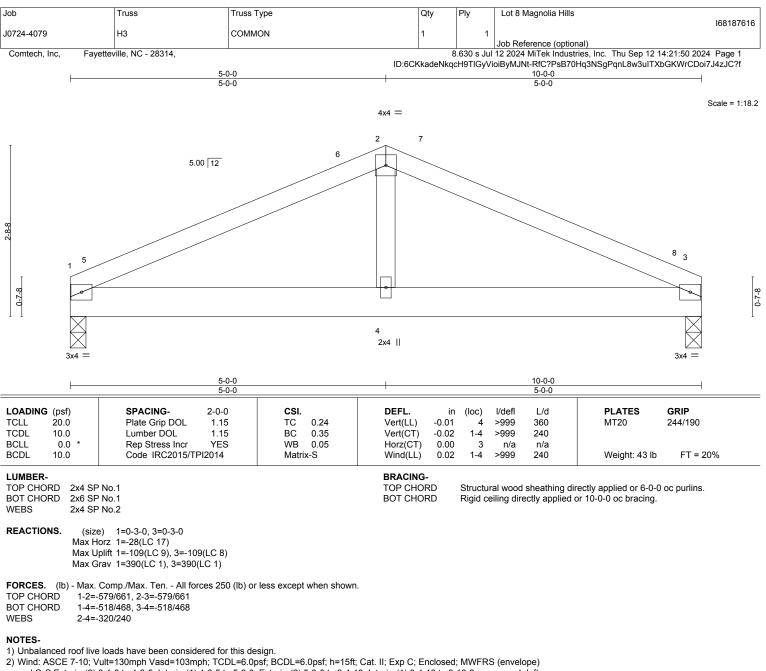
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=147, 4=147.



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- and C-C Exterior(2) 0-1-8 to 4-6-5, Interior(1) 4-6-5 to 5-0-0, Exterior(2) 5-0-0 to 9-4-13, Interior(1) 9-4-13 to 9-10-8 zone; porch left
- and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

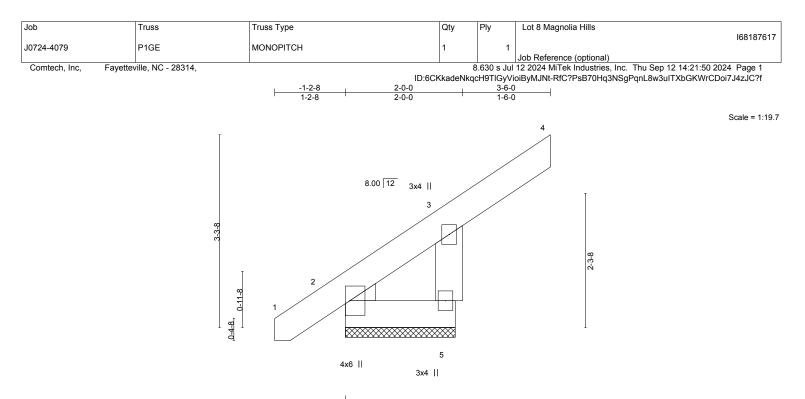
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=109, 3=109.



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LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.07	Vert(LL)	0.00	4	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.01	Vert(CT)	0.00	3	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-P						Weight: 22 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x6 SP No.1

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x6 SP No.1

 WEDGE
 Left: 2x4 SP No.2

REACTIONS. (size) 5=1-10-8, 2=1-10-8

Max Horz 2=133(LC 12) Max Uplift 5=-176(LC 12), 2=-12(LC 8) Max Grav 5=223(LC 19), 2=113(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 3-5=-253/322

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 5=176.



Structural wood sheathing directly applied or 2-0-0 oc purlins,

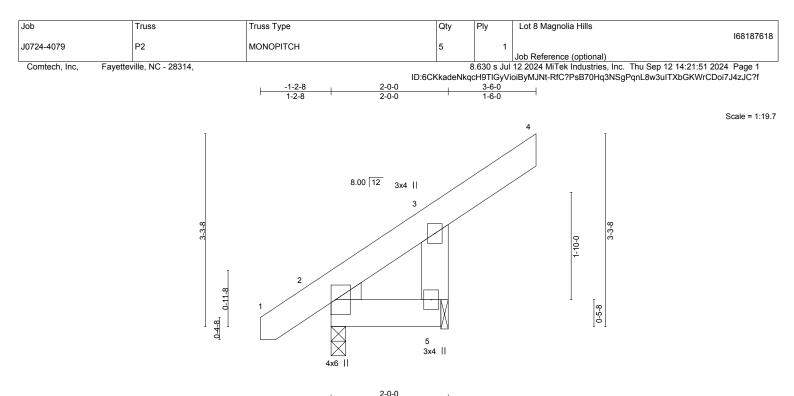
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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						2-0-0	4					
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.07	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.02	Vert(CT)	-0.00	2	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2	014	Matrix	κ-P	Wind(LL)	0.00	2	****	240	Weight: 22 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x6 SP No.1

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x6 SP No.1

 WEDGE
 Left: 2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 5=0-1-8 Max Horz 2=90(LC 12) Max Unlift 2=-11(L C 8) 5=-

Max Uplift 2=-11(LC 8), 5=-139(LC 9) Max Grav 2=106(LC 21), 5=213(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 3-5=-253/323

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 5=139.



Structural wood sheathing directly applied or 2-0-0 oc purlins,

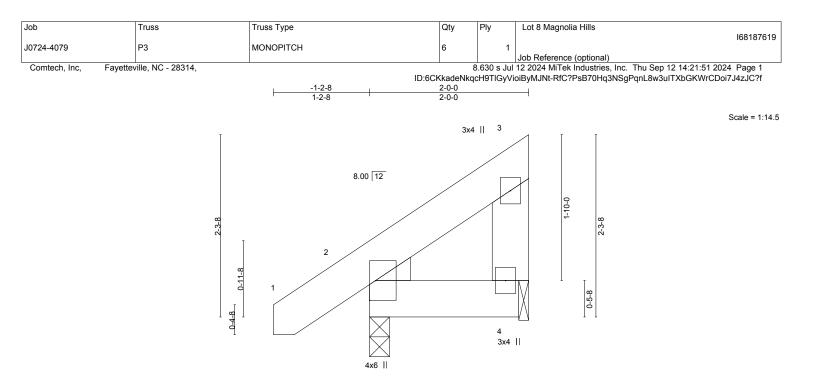
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcaccomponents.com)

ERGINEERING BY AMITEK Affiliate

818 Soundside Road



2-0-0								
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.03	DEFL. in (loc) I/defl L/d Vert(LL) -0.00 2 >999 360	PLATES GRIP MT20 244/190				
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.15 Rep Stress Incr YES	BC 0.03 WB 0.00	Vert(CT) -0.00 2 >999 240 Horz(CT) 0.00 n/a n/a					
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.00 2 **** 240	Weight: 18 lb FT = 20%				

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x6 SP No.1

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x6 SP No.1

 WEDGE
 Left: 2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=60(LC 12) Max Uplift 2=-3(LC 12), 4=-27(LC 12) Max Grav 2=157(LC 1), 4=60(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



Structural wood sheathing directly applied or 2-0-0 oc purlins,

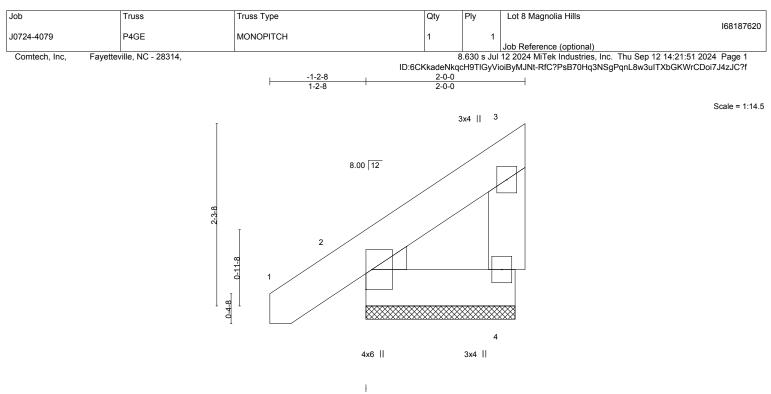
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Edenton, NC 27932



LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	тс	0.03	Vert(LL)	0.00	` í	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.01	Vert(CT)	-0.00	1	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-P						Weight: 18 lb	FT = 20%

LUMBER-

 TOP CHORD
 2x6 SP No.1

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x6 SP No.1

 WEDGE
 Left: 2x4 SP No.2

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=1-10-8, 2

(size) 4=1-10-8, 2=1-10-8 Max Horz 2=87(LC 12) Max Uplift 4=-46(LC 12), 2=-25(LC 12) Max Grav 4=64(LC 19), 2=155(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

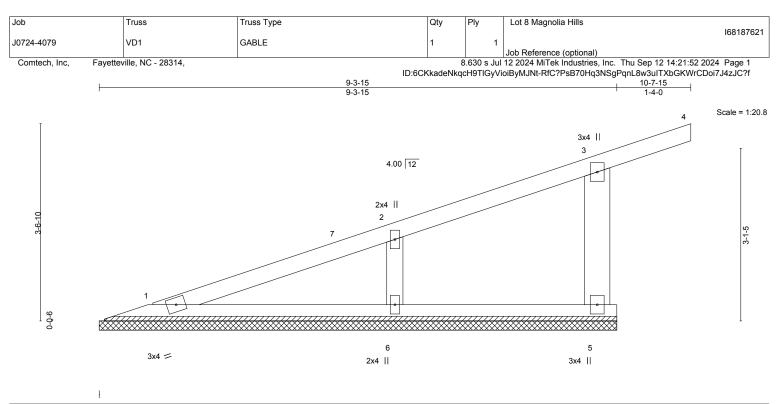
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.



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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.17 BC 0.11 WB 0.04 Matrix-S	DEFL. Vert(LL) 0.0 Vert(CT) -0.0 Horz(CT) 0.0	0 4	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 37 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x6 SF	° No.1		BRACING- TOP CHORD BOT CHORD	except	end vert	icals.	irectly applied or 6-0-0 or 6-0-0 oc bracing.	oc purlins,

2x6 SP No.1 VEBS OTHERS 2x4 SP No.2

REACTIONS.

(size) 1=9-3-15, 5=9-3-15, 6=9-3-15 Max Horz 1=102(LC 8)

Max Uplift 5=-68(LC 9), 6=-55(LC 8)

Max Grav 1=137(LC 1), 5=226(LC 1), 6=378(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 2-6=-272/205

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-11-11 to 5-3-15, Interior(1) 5-3-15 to 10-7-15 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6.

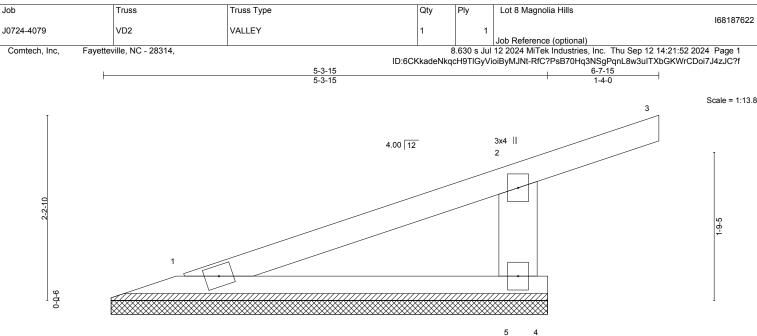


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3x4 🚍

3	x4	Ш

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

	0- <u>1-2</u> 0-1-2	<u>5-3-15</u> 5-2-13						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-1 Plate Grip DOL 1.1: Lumber DOL 1.1: Rep Stress Incr YES Code IRC2015/TPI2014	5 TC 0.15 5 BC 0.14 6 WB 0.00	DEFL. in Vert(LL) 0.00 Vert(CT) 0.01 Horz(CT) 0.00	3	l/defl n/r n/r n/a	L/d 120 120 n/a		GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SI	Р No.1	· · · · ·	BRACING- TOP CHORD	Structura	Il wood s	heathing o	directly applied or 5-3-15 d	oc purlins,

BOT CHORD

TOP CHORD 2x4 SP No.1

2x4 SP No.1 BOT CHORD WEBS 2x6 SP No.1

REACTIONS. (size) 1=5-2-13, 5=5-2-13 Max Horz 1=59(LC 8) Max Uplift 5=-68(LC 9) Max Grav 1=138(LC 1), 5=290(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-5=-243/295

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-11-11 to 5-4-8, Interior(1) 5-4-8 to 6-7-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.

5) Non Standard bearing condition. Review required.

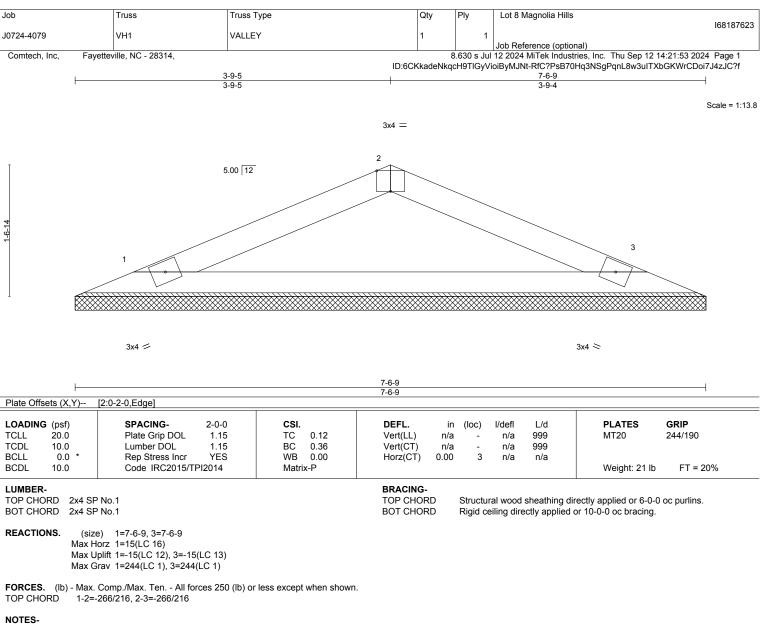


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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

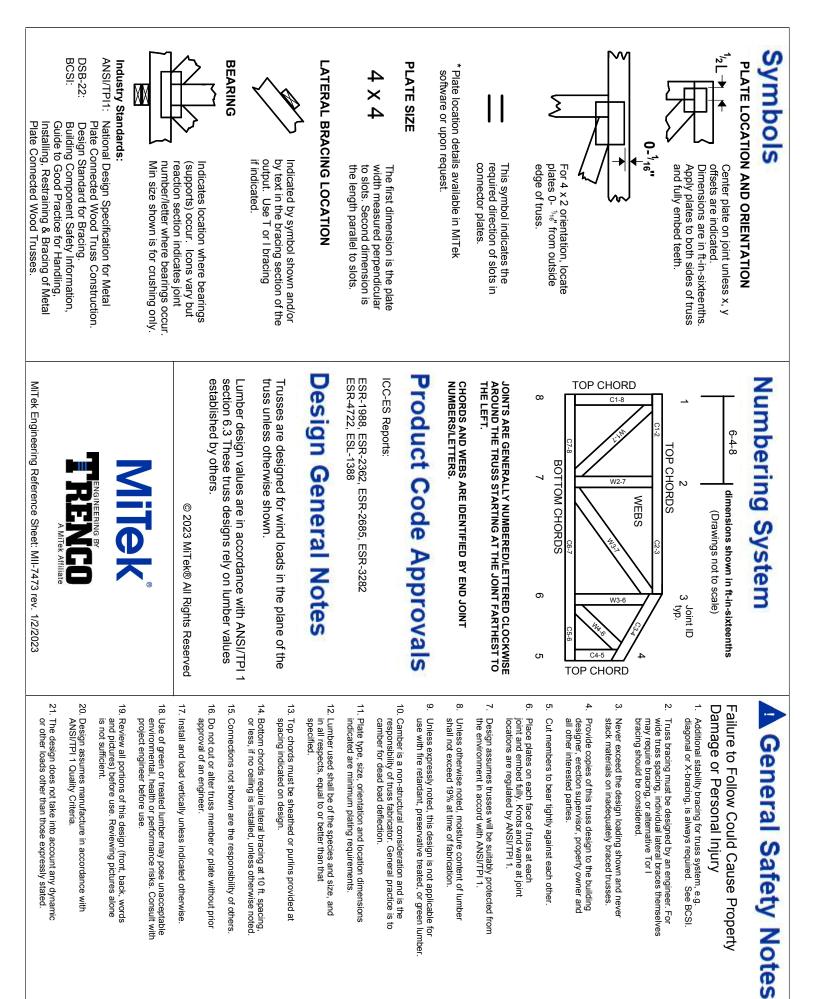
will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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818 Soundside Road Edenton, NC 27932





Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0724-4080 Lot 8 Magnolia Hills

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I68196596 thru I68196602

My license renewal date for the state of North Carolina is December 31, 2024.

North Carolina COA: C-0844



September 13,2024

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job		Truss	Truss	Туре			Qty	Ply	Lot 8 Magnolia Hills			100400500
J0724-4080		ET1	GABL	E			1	1		- D		168196596
Comtech, Inc,	Fayette	ville, NC - 28314,							Job Reference (option ul 12 2024 MiTek Indus	tries, Inc. Fr		
0 ₁₁ 8						ID:6Cł	KadeNkq	cH9TIGyV	ioiByMJNt-RfC?PsB70	Hq3NSgPqnl	_8w3uITXbG	KWrCDoi7J4zJC?f 0 ₁ 1 ₁ 8
- 11-												11
												Scale = 1:24.7
1	2	3	4	5	6	7		8	9	10	11	12 13
26	•	0		<u> </u>	•	•		•	0	•	•	
26												
					^^^^^		******					~~~~~~~~
25	24	23	22	21	20	19		18	17	16	15	14
3x4 =												3x6 =
1-4-0		2-8-0 4-0-0	5-4-			8-0-0	9-4-0		0-8-0 12-0-0	13-4		<u>14-8-0 15-0-8</u> 1-4-0 0-4-8
1-4-0		1-4-0 1-4-0	1-4-) '	1-4-0	1-4-0	<u> </u>	-4-0 1-4-0	1-4-		
LOADING (psf) TCLL 40.0		SPACING- Plate Grip DOL	2-0-0 1.00	CSI. TC 0	.07	DEFL. Vert(LL)	in n/a	. ,	l/defl L/d n/a 999	PLA MT20		GRIP 244/190
TCDL 10.0		Lumber DOL	1.00 YES	BC 0	.02	Vert(CT)	n/a	ı -	n/a 999			
BCLL 0.0 BCDL 5.0		Rep Stress Incr Code IRC2015/TPI		Matrix-R	.03	Horz(CT) 0.00	14	n/a n/a	Weig	ht: 68 lb	FT = 20%F, 11%E
LUMBER-						BRACIN	G-					
	k4 SP No k4 SP No					TOP CH	ORD		al wood sheathing dir and verticals.	ectly applied	d or 6-0-0 o	c purlins,
WEBS 22	k4 SP No k4 SP No	o.3(flat)				BOT CH	ORD		eiling directly applied	or 10-0-0 oc	bracing.	
		ngs 15-0-8.		W-> 05 44 04 0	0 00 04	20 10 19 17	10 15					

(lb) - Max Grav All reactions 250 lb or less at joint(s) 25, 14, 24, 23, 22, 21, 20, 19, 18, 17, 16, 15

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

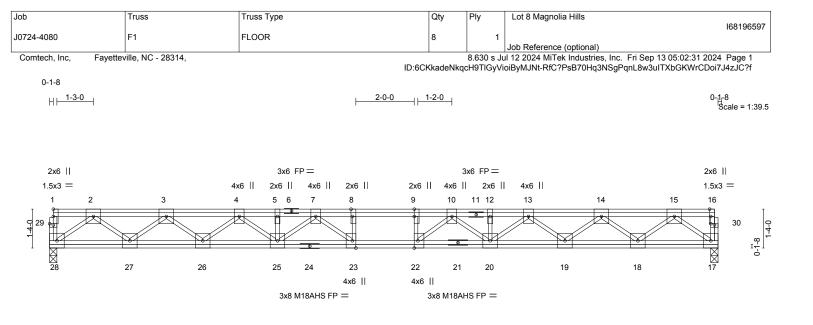
5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



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			22-11-0 22-11-0						
Plate Offsets (X,Y)	[8:0-3-0,Edge], [9:0-3-0,0-0-0], [16:0-3-			29:0-1-8	3,0-0-8]	, [30:0-1-	8,0-0-8]		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.16 BC 0.62 WB 0.65 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)			l/defl >846 >615 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 181 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
BOT CHORD 2x4 S WEBS 2x4 S REACTIONS. (si	SP 2400F 2.0E(flat) SP No.1(flat) SP No.3(flat) ze) 28=0-3-0, 17=0-3-0 Grav 28=1240(LC 1), 17=1240(LC 1)	-	BRACING- TOP CHOR BOT CHOR	D	except	end vert	icals.	irectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,
TOP CHORD 2-33 9-10 BOT CHORD 27-1 19- WEBS 2-24 15-1 13-1	 c. Comp./Max. Ten All forces 250 (ib) o =-2545/0, 3-4=-4416/0, 4-5=-5726/0, 5-7 0=-6351/0, 10-12=-5726/0, 12-13=-5726/ 28=0/1512, 26-27=0/3635, 25-26=0/5170 -20=0/5170, 18-19=0/3635, 17-18=0/151 8=-1869/0, 2-27=0/1370, 3-27=-1442/0, 2 17=-1869/0, 15-18=0/1370, 14-18=-1443 20=0720, 10-20=-555/0, 7-25=-546/0, 7-22=-192/772 	=-5726/0, 7 ⁻ 8=-6351/0, 8- 0, 13-14=-4416/0, 14-15= , 23-25=0/6105, 22-23=0/ 2 -26=0/1034, 4-26=-999/0, /0, 14-19=0/1034, 13-19=-	9=-6351/0, -2544/0 6351, 20-22=0/61(, 4-25=0/721, -998/0,	04,					
2) All plates are MT20	ive loads have been considered for this d 0 plates unless otherwise indicated.	esign.							

3) All plates are 6x6 MT20 unless otherwise indicated.

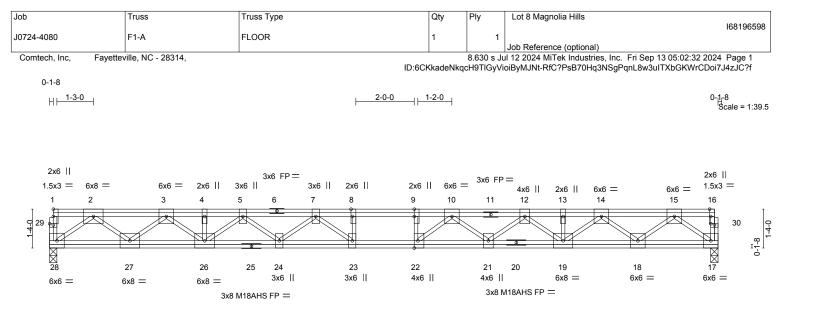
4) Plates checked for a plus or minus 1 degree rotation about its center.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oclapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of frusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCI8. Building component forth. Information, and information. Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

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l			22-11-0 22-11-0					
Plate Offsets (X,Y)	[8:0-3-0,Edge], [9:0-3-0,0-0-0], [16:0-3-			80:0-1-8,0-0-8				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-1-4-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrNOCode IRC2015/TPI2014	CSI. TC 0.36 BC 0.56 WB 0.85 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.35 23-24 -0.47 23-24 0.05 17	l/defl >783 >570 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 181 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
BOT CHORD 2x4 SI WEBS 2x4 SI REACTIONS. (siz	P 2400F 2.0E(flat) P 2400F 2.0E(flat) P No.3(flat) re) 28=0-3-0, 17=0-3-0		BRACING- TOP CHOR BOT CHOR	D Struct excep	t end verti	cals.	ectly applied or 6-0-0 c or 10-0-0 oc bracing.	oc purlins,
FORCES. (lb) - Max TOP CHORD 2-3= 9-10 BOT CHORD 27-2 19-2 WEBS 2-28 14-1 5-26	Grav 28=1777(LC 1), 17=1106(LC 1) . Comp./Max. Ten All forces 250 (lb) of -3515/0, 3-4=-5984/0, 4-5=-5984/0, 5-7= =-7099/0, 10-12=-5730/0, 12-13=-4301// 8=0/2165, 26-27=0/4967, 24-26=0/6854 21=0/5135, 18-19=0/3392, 17-18=0/1366 =-2677/0, 2-27=0/1791, 3-27=-1921/0, 3 8=-1382/0, 14-19=0/1179, 12-19=-1081/ =-1127/0, 5-24=0/493, 7-24=-367/0, 7-2: =-549/0	7227/0, 7 ⁻ 8=-7099/0, 8-9 0, 13-14=-4301/0, 14-15=- , 23-24=0/7495, 22-23=0/7 -26=0/1319, 15-17=-1688 0, 12-21=0/788, 10-21=-85	9=-7099/0, 2347/0 7099, 21-22=0/639 /0, 15-18=0/1302, 98/0,	96,				
 2) All plates are MT20 3) Plates checked for 4) Recommend 2x6 st 	ve loads have been considered for this d plates unless otherwise indicated. a plus or minus 1 degree rotation about i rrongbacks, on edge, spaced at 10-0-0 c attached to walls at their outer ends or re	ts center. oc and fastened to each tru	uss with 3-10d (0.1	131" X 3") nail	S.			

LOAD CASE(S) Standard

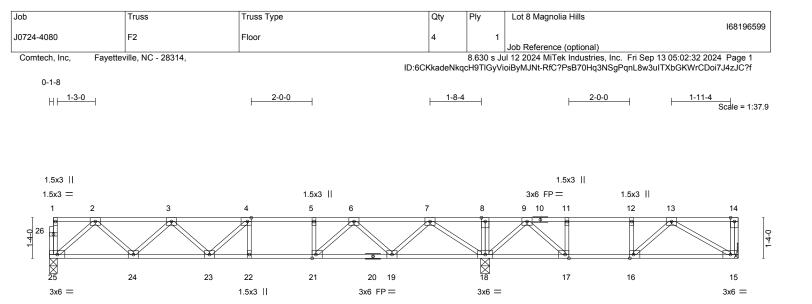
1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

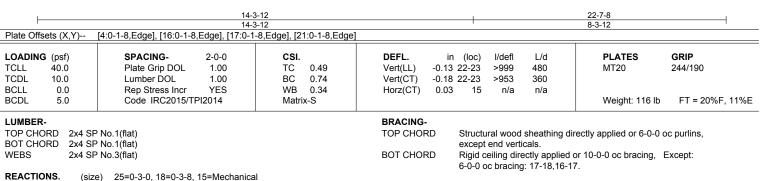
Uniform Loads (plf) Vert: 17-28=-7, 1-8=-187, 8-16=-67 SEAL 036322 September 13,2024

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REACTIONS. (size) 25=0-3-0, 18=0-3-8, 15=Mechanical Max Grav 25=749(LC 10), 18=1330(LC 1), 15=434(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1304/0, 3-4=-1975/0, 4-5=-2098/0, 5-6=-2098/0, 6-7=-1281/0, 7-8=0/541,

8-9=0/539, 9-11=-673/24, 11-12=-673/24, 12-13=-673/24

BOT CHORD	24-25=0/794, 23-24=0/1788, 22-23=0/2098, 21-22=0/2098, 19-21=0/1762, 18-19=0/789,
	17-18=-237/339, 16-17=-24/673, 15-16=0/563
WEBS	7-18=-1305/0, 7-19=0/711, 6-19=-712/0, 6-21=0/611, 5-21=-276/0, 2-25=-1055/0,
	2-24=0/708, 3-24=-674/0, 3-23=0/298, 4-23=-303/25, 9-18=-654/0, 9-17=0/615,
	11-17=-337/0, 13-15=-652/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

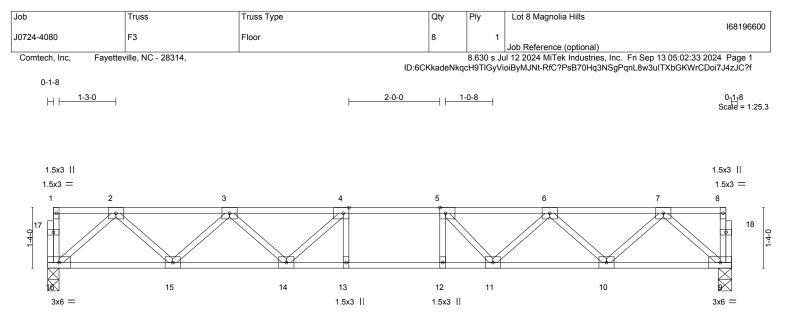
6) CAUTION, Do not erect truss backwards.



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			<u>15-0-8</u> 15-0-8							
Plate Offsets (X,Y)	[4:0-1-8,Edge], [5:0-1-8,Edge]									
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.37 BC 0.71 WB 0.37 Matrix-S	Vert(LL) -0.13	3 13-14 >999 4 7 13-14 >999 3	//d PLATES 80 MT20 60 1/a Weight: 78 lb	GRIP 244/190 FT = 20%F, 11%E				
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. WEBS 2x4 SP No.3(flat) BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.										
Max C FORCES. (lb) - Max. TOP CHORD 2-3= BOT CHORD 15-1 9-10 WEBS 2-16	BOT CHORD 15-16=0/863, 14-15=0/1966, 13-14=0/2465, 12-13=0/2465, 11-12=0/2465, 10-11=0/1958, 9-10=0/866									
NOTES- 1) Unbalanced floor liv 2) All plates are 3x4 M	=0/781, 6-10=-738/0, 6-11=0/441, 5-11= ve loads have been considered for this de IT20 unless otherwise indicated. a plus or minus 1 degree rotation about i	esign.								

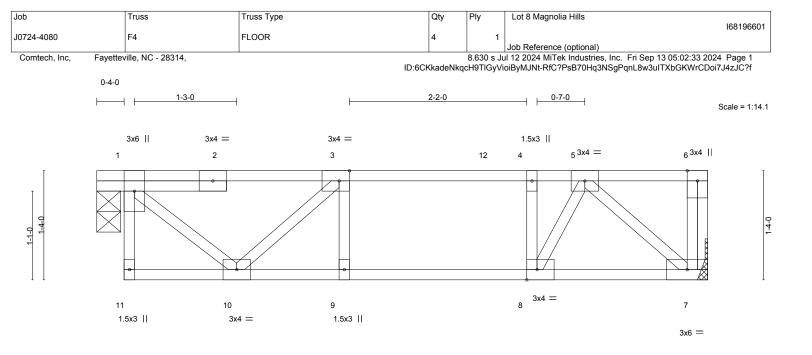
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



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0	4-0 4-0		7-5-8 7-1-8			
Plate Offsets (X,Y)	[3:0-1-8,Edge], [8:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00	CSI. TC 0.30 BC 0.32	DEFL. ir Vert(LL) -0.03 Vert(CT) -0.03		PLATES MT20	GRIP 244/190
BCLL 0.0 BCDL 5.0	Rep Stress Incr NO Code IRC2015/TPI2014	WB 0.23 Matrix-S	Horz(CT) 0.01		Weight: 41 lb	FT = 20%F, 11%E
	² No.1(flat) ² No.1(flat)		BRACING- TOP CHORD	Structural wood sheathing dire	ectly applied or 6-0-0	oc purlins,
WEBS 2x4 SP	PNo.3(flat)		BOT CHORD	Rigid ceiling directly applied o	r 10-0-0 oc bracing.	
REACTIONS. (size Max G	e) 7=Mechanical, 1=0-3-8 trav 7=628(LC 1), 1=436(LC 1)					
FORCES. (Ib) - Max.	Comp./Max. Ten All forces 250 (lb) or	less except when shown.				

TOP CHORD

1-3=-364/0, 3-4=-685/0, 4-5=-685/0

BOT CHORD 9-10=0/685, 8-9=0/685, 7-8=0/557

WEBS 1-10=0/477, 3-10=-443/0, 5-7=-742/0, 5-8=0/333, 4-8=-256/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

6) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

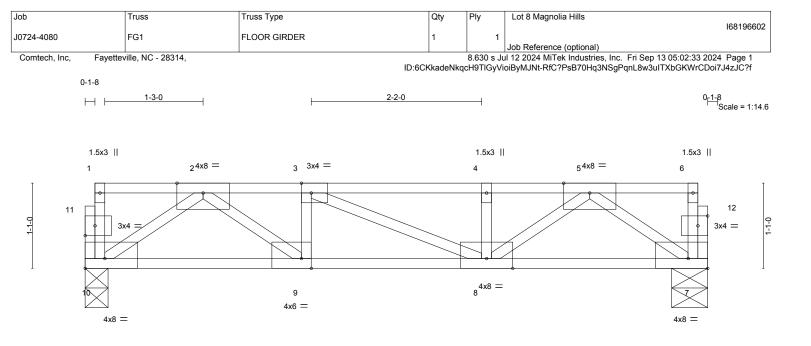
Uniform Loads (plf)

Vert: 7-11=-10, 1-12=-100, 6-12=-220



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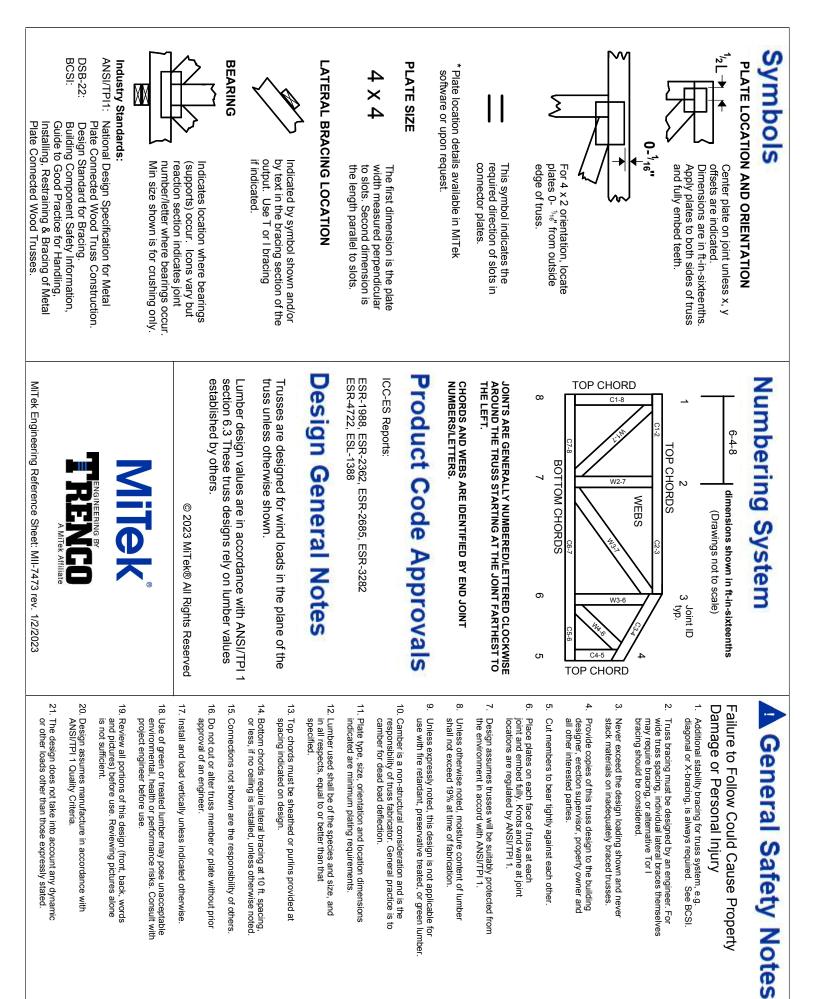
L			7-11-0				
Plate Offsets (X,Y)	[3:0-1-8,Edge], [7:Edge,0-1-8], [9:0-1-8,	Edge], [10:Edge,0-1-8], [11:0	7-11-0 D-1-8,0-1-8], [12:0-1-8	3,0-1-8]			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.68 BC 0.71 WB 0.74 Matrix-P	DEFL. ir Vert(LL) -0.06 Vert(CT) -0.08 Horz(CT) 0.03	8-9 >999	L/d 480 360 n/a	PLATES MT20 Weight: 42 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SF	⊃ 2400F 2.0E(flat) ⊃ No.1(flat) ⊃ No.3(flat)		BRACING- TOP CHORD BOT CHORD	except end ver	ticals.	ectly applied or 6-0-0 r 10-0-0 oc bracing.	oc purlins,
(-	e) 10=0-3-8, 7=0-5-8 Grav 10=1735(LC 1), 7=1735(LC 1)						
TOP CHORD 1-10 BOT CHORD 9-10	Comp./Max. Ten All forces 250 (lb) or =-261/0, 6-7=-261/0, 2-3=-3357/0, 3-4=- =0/2110, 8-9=0/3357, 7-8=0/2110 =-2551/0, 2-9=0/1544, 3-9=-888/0, 5-7=-	3359/0, 4-5=-3359/0	90/0				
2) Recommend 2x6 st	a plus or minus 1 degree rotation about i rongbacks, on edge, spaced at 10-0-0 c attached to walls at their outer ends or re	c and fastened to each truss	with 3-10d (0.131" X	3") nails.			
LOAD CASE(S) Stan 1) Dead + Floor Live (I Uniform Loads (plf)	dard balanced): Lumber Increase=1.00, Plate	Increase=1.00					

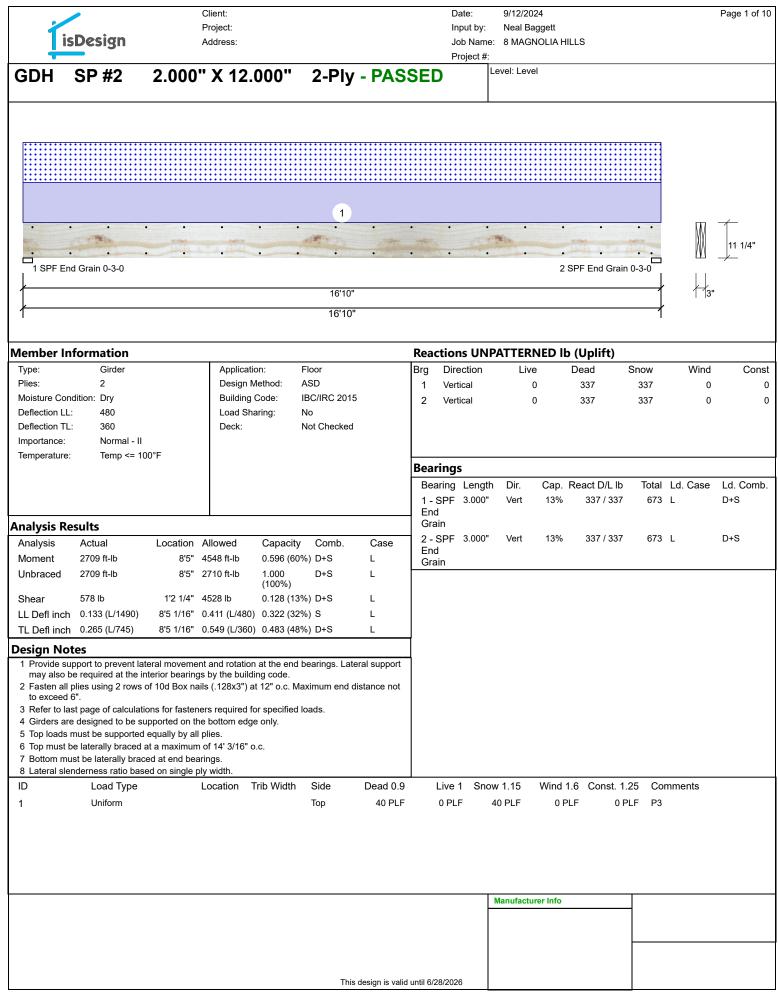
Vert: 7-10=-10, 1-6=-450



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GDH SP #2 2.000" X 12.000"	2-Ply -	PASSE	Project #:	Level: Level			
							.
•••••	• •	• •	• •	•	• •	••	1 1/2
	• •	• •		•	• •	••+	<u> </u>
1 SPF End Grain 0-3-0					2 SPF End G	Grain 0-3-0	$\overline{\Lambda}$
ł	16'10"						3"
f	16'10"					/	•

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	202.6 PLF
Yield Limit per Fastener	101.3 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

This design is valid until 6/28/2026

is	Design	Client: Project: Address:					Date: Input I Job N Projec	by: ame:	9/12/202 Neal Bag 8 MAGN	gett	LS				Page 3 o	of 1
DB1 k	Kerto-S LVI	_ 1.750	" X 18	.000" 2	2-Ply -	- PA	SSE	DLev	vel: Level							
•••••		2			•••••			3	3	· · ·				m ·	-{	
1 SPF 0-3-4		tin en fi		all and a	and the	-			Ha.		2 5	SPF 0-3	• -8	₩.	1'6"	
<u> </u>				23'									7	1 3 1	/2"	
ł				23'									┦			
lember Inf	ormation					Read	tions L	JNPA	TTERN	IED Ib	(Uplift)					
Туре:	Girder		ation:	Floor		Brg	Directio	on	Live		Dead	Sno		Wind	Co)r
Plies: Moisture Cond Deflection LL: Deflection TL:	480 360	Buildi	n Method: ng Code: Sharing:	ASD IBC/IRC 2015 No Not Checked		1 2	Vertical Vertical		278 278		1762 3241		21 21	0 0		
Importance:	Normal - II															
Temperature:	Temp <= 100°F					Bear	rings									
						1-	aring Lei SPF 3.5 SPF 3.5	500"	Dir. Vert Vert	Cap. F 47% 75%	React D/L 1762 / 67 3241 / 67	74 2	otal L 2436 L 3915 L		Ld. Cor D+0.75(D+0.75((L
nalysis Res						<u> </u>	3FF 0.0	.00	ven	1070	5241701	4 (0010 L		D10.75(<u> </u>
	15112 ft-lb 13 18767 ft-lb 12 2761 lb 2 0.107 (L/2523) 11	Decision Allowed 3'2 7/8" 38683 ft-li 111 3/4" 18779 ft-li 1'2 1/2" 12096 lb 6 1/16" 0.564 (L/4	0.999 (100%) 0.228 (2 80) 0.190 (1	39%) D D+0.75(L+5 23%) D 19%) 0.75(L+S)	Uniform L											
	0.522 (L/519) 11'11	11/16" 0.752 (L/3	60) 0.694 (6	59%) D+0.75(L+8	5) L	┥										
 may also be Fasten all p to exceed 6 Refer to last Girders are Top loads m Top must be Bottom must 	port to prevent lateral i e required at the interio lies using 3 rows of 10	r bearings by the bi d Box nails (.128x3 or fasteners require ted on the bottom e illy by all plies. naximum of 7'1 3/8' end bearings.	uilding code. ') at 12" o.c. d for specifie dge only.	Maximum end dis												
ID	Load Type	Location	Trib Widt	h Side	Dead 0.9	1	Live 1	Snow	1.15	Wind 1	.6 Const.	1.25	Com	ments		_
1	Uniform			Near Face	54 PLF		0 PLF	54	PLF	0 PL	F	0 PLF	P TRI	JSSES		
2	Tie-In Far	0-0-0 to 23-0-0	0-7-4	Far Face	15 PSF		0 PSF		PSF	0 PS) PSF		RLOADIN		
2 3	Tie-In Near Part. Uniform Self Weight	0-0-0 to 23-0-0 10-4-0 to 23-0-0	0-0-0	Тор Тор	15 PSF 255 PLF 14 PLF	4	0 PSF 0 PLF		PSF PLF	0 PS 0 PL) PSF 0 PLF		R LOADIN . & C1GE	IG	
structural adequacy o design criteria and esponsibility of the cu ensure the compone application, and to verif Lumber 1. Dry service condition	Designs is responsible only of the f this component based on the loadings shown. It is the ustomer and/or the contractor to suitability of the intended fy the dimensions and loads. ons, unless noted otherwise ed with fire retardant or corrosive	 LVL beams must not b Refer to manufac regarding installatic fastening details, bea approvals Damaged Beams must Design assumes top e 	e cut or drilled urer's product n requirements, m strength values t not be used dge is laterally rest rt at bearing poir	ponding information multi-ply , and code rained ts to avoid	roofs provide p			Me 30 No (8)	anufacture etsä Wood 11 Merritt 7 orwalk, CT 00) 622-58 ww.metsav	Building, 06851 850	2nd Floor /us					

	Client: Project:	Date: Input by:	9/12/2024 Neal Baggett	Page 4 of 10
isDesign	Address:		8 MAGNOLIA HILLS	
DB1 Kerto-S	LVL 1.750" X 18.00	00" 2-Ply - PASSED	evel: Level	
· · · · ·	· · · · · · · · ·	· · · · · · · ·	↓ · · · · · · · · · · · · · · · · · · ·	1'6"
1 SPF 0-3-8		001	2 SPF 0-3-8 ↓ ∧	
<u> </u>		23' 23'		13 1/2"
Multi-Ply Analysis Fasten all plies using 3 r	ows of 10d Box nails (.128x3") at ⁻	12" o.c Maximum end distance not	t to exceed 6".	
apacity	19.1 %			
oad ield Limit per Foot	54.0 PLF 282.4 PLF			
ield Limit per Fastener	94.1 lb.			
w eld Mode	1 IV			
dge Distance	1 1/2"			
lin. End Distance Dad Combination	3" D+S			
uration Factor	1.15			

Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the interded application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	1. LVL beams must not be cut or drilled		Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	

		Client:					Date:	9/12/202	4				Page 5 of 10
		Project:					Input by:						l age e el li
i	sDesign	Address:						e: 8 MAGN	OLIA HILL	S			
		4 7 5 6					Project #	: Level: Level					
FB2	Kerto-S LVL	. 1.750 ⁻	X 16.0	00 2	2-Piy	- PA:	SSED						
	2		3										
· ·		1 · · ·											1'4"
1 SPF 0-	-3-8		2 SPF 0-3-8										_ <u>/</u>
/	6	5'8"		-								3	1/2"
/	6	5'8"		\rightarrow									
Member Ir	nformation					Reac	tions UN	PATTERN	IFD lb (I	Unlift)			
Туре:	Girder	Applic		loor		Brg	Direction	Live		ead	Snow	Wind	Cons
Plies:	2			SD		1	Vertical	540		558	937	0	C
Moisture Cor Deflection LL			ng Code: IE Sharing: N	3C/IRC 2015		2	Vertical	540	1	558	937	0	(
Deflection TL		Deck:	-	ot Checked									
Importance:	Normal - II												
Temperature	: Temp <= 100°F					Deser							
						Beari	-		<u> </u>				
							ring Lengt		•	act D/L lb		Ld. Case	Ld. Comb.
							PF 3.500' PF 3.500'			558 / 1108 558 / 1108	2666 2666		D+0.75(L+8 D+0.75(L+8
Analysis Re	esults					2-3	PF 3.300	ven	5176 1	5567 1106	2000	L	D+0.73(L+0
Analysis	Actual Lo	cation Allowed	Capacity	Comb.	Case								
Moment	3879 ft-lb	3'4" 39750 ft-lb	0.098 (10%) D+0.75(L+	S) L								
Unbraced	3879 ft-lb	3'4" 18821 ft-lb	0.206 (21%) D+0.75(L+	S) L								
Shear	1617 lb	5' 1/2" 13739 lb) D+0.75(L+	-								
	0.008 (L/9314)	3'4" 0.156 (L/48	, , ,	· · /									
TL Defl inch	n 0.019 (L/3870)	3'4" 0.208 (L/36	60) 0.093 (9%)	D+0.75(L+	S) L	4							
Design No	tes												
may also b 2 Fasten all to exceed 3 Refer to la 4 Girders and	ast page of calculations for e designed to be support	bearings by the bu Box nails (.128x3" or fasteners required ed on the bottom ed	ilding code.) at 12" o.c. Max d for specified lo	kimum end di									
6 Top must b 7 Bottom mu	must be supported equal be laterally braced at end ust be laterally braced at enderness ratio based on	l bearings. end bearings.											
ID	Load Type	Location	Trib Width	Side	Dead 0.9	L	ive 1 Sn	ow 1.15	Wind 1.6	Const. 1	.25 Con	nments	
1	Uniform			Near Face	54 PLF	162	2 PLF	0 PLF	0 PLF	0 F	PLF F2		
	Uniform			Тор	120 PLF	() PLF	0 PLF	0 PLF	0 F	PLF WAL	L	
2	ennenn			Tem	281 PLF	() PLF	281 PLF	0 PLF	0 F	PLF B2-A	2	
2 3	Uniform			Тор	2011 LI	,						`	
				юр	12 PLF							,	
3	Uniform	chemicals			12 PLF			Manufacture	ər Info				
3 Notes Calculated Structure	Uniform Self Weight				12 PLF			Metsä Wood			-		
3 Notes Calculated Structure structural adequacy design criteria an	Uniform Self Weight ed Designs is responsible only of the of this component based on the d loadings shown. It is the	Handling & Installa 1. LVL beams must not be 2. Refer to manufactu	cut or drilled urer's product inform	6. For fla pondin	12 PLF				Building, 2r	nd Floor	-		
3 Notes Calculated Structures structural adequacy design criteria an responsibility of the ensure the compo	Uniform Self Weight	Handling & Installa 1. LVL beams must not be 2. Refer to manufacture regarding installation	cut or drilled	6. For fla pondin nation liti-ply	12 PLF			Metsä Wood 301 Merritt 7 Norwalk, CT (800) 622-58	Building, 2r 06851 50		-	`	
3 Notes Calculated Structure structural adequacy design criteria an responsibility of the ensure the compa application, and to ve Lumber	Uniform Self Weight and Designs is responsible only of the of this component based on the customer and/or the contractor to nent suitability of the intended	Handling & Installa 1. LVL beams must not be 2. Refer to manufacture regarding installation fastening details, bear	e cut or drilled urer's product inform n requirements, mu n strength values, and not be used Ige is laterally restrained	6. For fla pondin nation titi-ply code	12 PLF			Metsä Wood 301 Merritt 7 Norwalk, CT	Building, 2r 06851 50		-	` 	

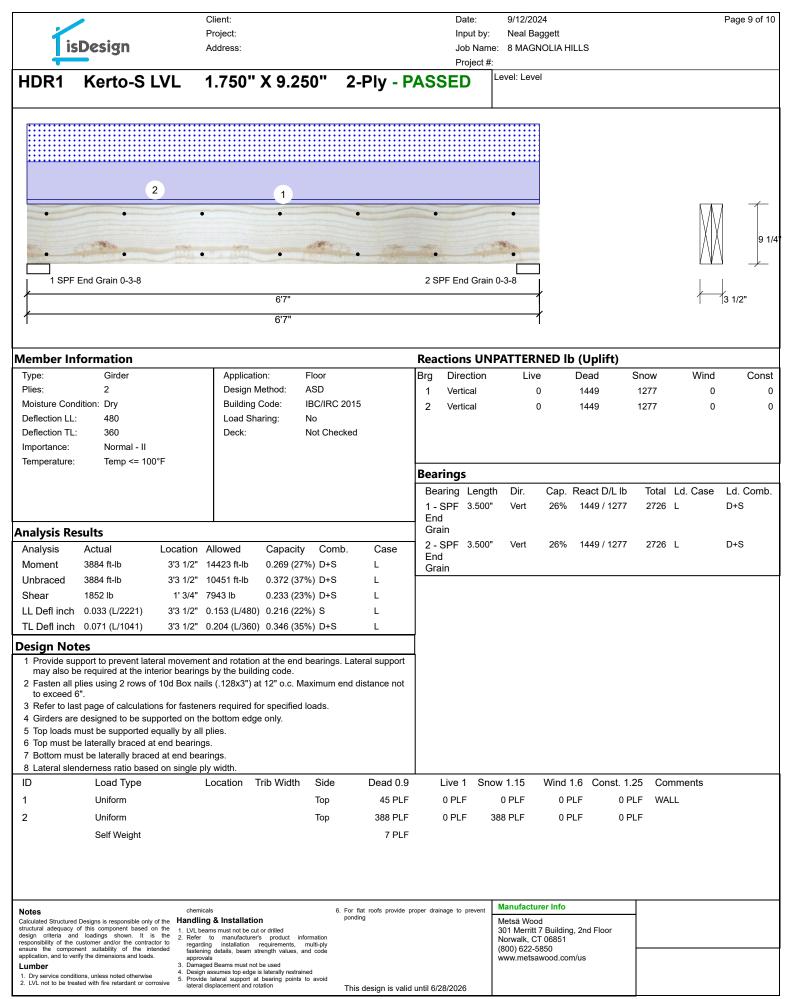
isDesig	Client: Project: Address:		Date: Input by: Job Name: Project #:	9/12/2024 Neal Baggett 8 MAGNOLIA HILLS	Page 6 of 10
FB2 Kerto	-S LVL 1.750	" X 16.000" 2-I		evel: Level	
· · · · · · · · · · · · · · · · · · ·	· · · ·	· · · · · · · · · · · · · · · · · · ·			1'4"
1 SPF 0-3-8	6'8" 6'8"	2 SPF 0-3-8			3 1/2"
Multi-Ply Analysis	13 rows of 10d Box nail	s (.128x3") at 12" o.c Ma	vimum and distance not	to exceed 6"	
Capacity Load Yield Limit per Foot Yield Limit per Fastener CM Yield Mode Edge Distance Min. End Distance	44.0 % 108.0 PLF 245.6 PLF 81.9 lb. 1 IV 1 1/2" 3"	<u>s (.120x3) at 12</u> 0.c Max			
Load Combination Duration Factor	D+L 1.00				

Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the interded application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements multi-ply	ponding This design is valid until 6/28/2026	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	

	Design	Client: Project: Address:			Project	me: 8 MAGN #:	ggett IOLIA HILLS			Page 7 of
FB1 k	(erto-S LVL	1.750" X 1	6.000" 2	2-Ply - F	PASSEE	Level: Leve	l 			
	2	1	3						-	
			Atter	2 SPF 0-3-8						1'4"
/		8'11 1/2"			ł					1/2"
/		8'11 1/2"			┥				1 1-	
/lember Inf	ormation			R	eactions U		NED lb (Uplif	t)		
Туре:	Girder	Application:	Floor		rg Direction			Snow	Wind	Со
Plies: Moisture Cond Deflection LL: Deflection TL: Importance:	2 lition: Dry 480 360 Normal - II	Design Method: Building Code: Load Sharing: Deck:	ASD IBC/IRC 2015 No Not Checked		1 Vertical 2 Vertical	1021 1021		873 873	0 0	
Temperature:	Temp <= 100°F									
					earings					
					Bearing Leng 1 - SPF 3.50	-	Cap. React D/ 62% 1807 / 1		Ld. Case L	Ld. Con D+0.75(I
nalysis Re	sults				2 - SPF 3.50	0" Vert	62% 1807 / 1	421 3228	L	D+0.75(
	5731 ft-lb 4 6541 ft-lb 4 2224 lb 1 0.022 (L/4718) 4'5'	'5 3/4" 13975 ft-lb 0.468	5 (17%) D+L 6 (47%) D+0.75(L+ 5 (19%) D+L 2 (10%) 0.75(L+S)	L L						
	, ,	13/16 0.264 (L/360) 0.173	5 (17%) D+0.75(L+	5) L						
 may also be Fasten all p to exceed 6 Refer to lass Girders are Top loads n Top must be Bottom must 	port to prevent lateral m e required at the interior lies using 3 rows of 10d ". t page of calculations fo	bearings. end bearings.	e. c. Maximum end d							
D	Load Type	Location Trib Wi	dth Side	Dead 0.9	Live 1 S		Wind 1.6 Con		nments	
1	Uniform		Near Face	76 PLF	228 PLF	0 PLF	0 PLF	0 PLF F4		
2 3	Uniform Uniform		Тор Тор	120 PLF 195 PLF	0 PLF 0 PLF	0 PLF 195 PLF	0 PLF 0 PLF	0 PLF WA 0 PLF B4	LL	
•	Self Weight		iοp	195 PLF	VILF			VILI D4		
						Manufactur	er Info			
tructural adequacy of	of this component based on the loadings shown. It is the	chemicals Handling & Installation 1. LVL beams must not be cut or drilled 2. Refer to manufacturer's produc	pondir t information	at roofs provide prope g	urainage to prevent	Metsä Wood	d 7 Building, 2nd Floo	r		
esponsibility of the c insure the compon	fy the dimensions and loads.	regarding installation requireme fastening details, beam strength val approvals 3. Damaged Beams must not be used	nts, multi-ply ues, and code			(800) 622-55 www.metsay	850 wood.com/us			

	Client: Project:			Date: Input by:	9/12/2024 Neal Baggett	Page 8 of 10
isDesign	Project: Address:	:			Neal Baggett e: 8 MAGNOLIA HILLS	
				Project #:	Level: Level	
FB1 Kerto-S	LVL 1.750)" X 16.000"	2-Ply -	PASSED		
• • •	• •	• •	• •	•		
	• •		•	1/2"		
				. 1		1'4"
1 SPF 0-3-8			2 SPF 0-3-	╔┙┼		
 	8'11 1/2	2"		\rightarrow		3 1/2"
<u> </u>	8'11 1/2			\rightarrow		
Multi-Ply Analysis						
Fasten all plies using 3 Capacity	rows of 10d Box nai	ls (.128x3") at 12" o.	c Maximum	end distance no	ot to exceed 6".	
Load	152.0 PLF					
Yield Limit per Foot Yield Limit per Fastener	245.6 PLF 81.9 lb.					
См Yield Mode	1 IV					
Edge Distance	1 1/2" 3"					
Min. End Distance Load Combination	5 D+L					
Duration Factor	1.00					
Notes	chemicals			roper drainage to prevent	Manufacturer Info	
Calculated Structured Designs is responsible structural adequacy of this component ba	sed on the 1. LVL beams must not	llation t be cut or drilled	ponding		Metsä Wood 301 Merritt 7 Building, 2nd Floor	
design criteria and loadings shown. responsibility of the customer and/or the c ensure the component suitability of the application and to verify the dimensions and	e intended fastening details, b	acturer's product information tion requirements, multi-ply eam strength values, and code			Norwalk, CT 06851 (800) 622-5850	
application, and to verify the dimensions and Lumber 1. Dry service conditions, unless noted other	loads. approvals 3. Damaged Beams mi 4. Design assumes top	ust not be used edge is laterally restrained			www.metsawood.com/us	
 Dry service conditions, unless noted other LVL not to be treated with fire retardant 	rwise E Dreutide leteral euro	port at bearing points to avoid	This design is valid	until 6/28/2026		

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Í	isDesign	Client: Project: Address:				me: 8 MAGNOLIA HILLS	Page 10 of 1
HDR1	Kerto-S L	VL 1.750'	' X 9.250"	2-Ply	Project	#: Level: Level	
•	٠	•	•	•	٠		9 1/
	• F End Grain 0-3-8	•	•	•	• 2 SPF End Gra		
			6'7" 6'7"				3 1/2"
Multi-Ply	Analysis						
Fasten all p Capacity Load Yield Limit per Yield Limit per Cm Yield Mode Edge Distance Min. End Dista	- Foot - Fastener	OT TUG BOX NAIIS 0.0 % 0.0 PLF 163.7 PLF 81.9 lb. 1 IV 1 1/2" 3"	,128x3) at 12"	o.c Maxim	um end distance i	not to exceed 6 .	
Load Combina Duration Facto		1.00					
Notes		chemicals		6. For flat roofs pro	vide proper drainage to prevent	Manufacturer Info	1
Calculated Structure structural adequace design criteria a responsibility of the	red Designs is responsible only of cy of this component based on and loadings shown. It is the customer and/or the contracto ponent suitability of the inten	the Handling & Installat the 1. LVL beams must not be the 2. Refer to manufactur regarding installation	cut or drilled er's product information requirements, multi-ply	ponding	, , <u>,</u> o portin	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850	
application, and to v Lumber 1. Dry service con	verify the dimensions and loads. Inditions, unless noted otherwise treated with fire retardant or corror	approvals 3. Damaged Beams must r 4. Design assumes top edg 5. Brouide lateral support	e is laterally restrained at bearing points to avoid	This design is	valid until 6/28/2026	www.metsawood.com/us	

		_				_													DAT	E 09/1	2/24	PAG	Ε1
Read	ction	Sumr	mary	of Ord	er		REQ. 0	QUOTE	DATE							ORD	ER #			JC)724-4	4080	
						Γ	ORDE	R DATE	Ξ	0	7/17/2	24				QUO	TE #						
						t	DELIV	ERY D	ATE							CUST	ГОМ	ER AC	CT #	00	00000	07216	
	_		00F & I	FLOOR		Ī	DATE	OF INV	OICE							CUST	ГОМ	ER PO	#				
Co	mTe	Ch TRI	USSES &	BEAMS		Ť	ORDE	RED B	Y	S	Shaun	Garderne	r			INVO	ICE	#					
Reilly R	oad Ind	ustrial Park	k P.O. Bo	x 40408		Ī	COUN	ТҮ		Н	larnet	t				TERM	/IS						
-		C. 28309				Ī	SUPE	RINTEN	IDANT	S	Shaun	Garderne	r			SALE	S R	EP		Ne	eil Ba	iggett	
						Ī	JOBSI	TE PHO	ONE #	(9	910) 9	88-8172				SALE	S A	REA		Ne	eil Ba	aggett	
		n Cust	om Hor	mos	JOB NA		ot 8 Ma	anolia I	Hille	`	,			10)T #	8	51	JBDIV:	Mad	nolia ł	Hille		
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SH Pr	ecisio	n Custo	om Hon	nes and																			
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				EDUANC		EL HEI	CUT	00.04	05	DE 2	1 41/2							0.10-		B	r	DAT	<u> </u>
		EPARTME		ERHANG	INFO HE	CL HEI	GEI	00-04	-U0	REQ.	. LAY(1018	REQ	. ENG	JINE	ERING		QUOT		NB		09/12/2	24
Floor C	Juer				-	BLE ST	TUDS	24 IN	. 00		OL,	BSITE 1			JO	BSITE	1	CUTTI		NB		09/12/2	
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FLC	DOR	TRUS	SES				.0,0.0,5.0		SS INCR.	-	FLO	OR TRUS	S SPA	CINC	3: 24	.0 IN.	0.C	(TYP.))				
FLC	OOR	QTY	DEPTH				END T		INT BE		6												
	FILE	PLY	ID	SP.		PAN						REACTIO	NS										
			01-04-0	0			F	-			Jo	nt 14	Joint	15		Joint 1	6	Joir	nt 17		Joint	18	
		1	ET1		0-08 15	-00-08	<u>ш</u>	<u> </u>				82.7 lbs.	160).8 lbs	8.	142.	7 lbs.	1	47.7 II	bs.	146	6.4 lbs.	
			01-04-0					\Box				nt 17	Joint										
		8	F1	22-1	1-00 22	-11-00	<u> </u>	<u> </u>				239.8 lbs. 09.9 lbs.		9.8 lbs 5.2 lbs									
											,	109.9 105.	000	0.2 105	.								
		I I	01 04 0	0	1							nt 17	Joint	28									
0000		1	01-04-00 F1-A		1-00 22	-11-00	₫	Ь				106.2 lbs.		20 6.9 lbs	s.								
											6	86.2 lbs.	135	4.5 lbs	s.								
			01-04-0									nt 15	Joint			Joint 2							
		4	F2	22-0	7-08 22	-07-08	<u> </u>					33.8 lbs. 60.5 lbs.		0.1 lbs			9 lbs. 6 lbs.						
												iba.	008	.0 105		201.	5 105.						
			01-04-0	0	I	1			I			nt 9	Joint	16									
\sim	M	8	61-04-00 F3		0-08 15	-00-08	₫	Ь				807.3 lbs.		7.3 lbs	S.								
							I					18.9 lbs.		.3 lbs									
K/T			01-04-0				F					nt 1	Joint										
	¥>	4	F4	07-0	5-08 07	-05-08						35.6 lbs. 65.4 lbs.		7.5 lbs 9.4 lbs									
											4		408	,.+ ID2									
			01 04 04	0	I	1			ļ			nt 7	Joint	10									1
		1	01-01-00 FG1		1-00 07	-11-00		<u></u>				nt 7 735.2 lbs.		10 5.2 lbs	s.								
ITE	MS		-			-																	I
	1		<u> </u>			-1		OT''					i										—–––
QTY		ITEM TY	PE	SIZ	Έ	1	EEN FT-IN	GTH N-16		PA	RT N	UMBER						NO	TES				
8	3	Hangers, L	JSP	HUS	\$ 410								SI	MPSC) NC	IUS410))						
-	.		(E)			I			I														I
2	2	BlueLinx ((F)	LVL, Met	sa(F) 2.0,		07-0	00-00					HC	DR1									

						DATE	E09/12/24 PAGE 2	2
Re	eaction Summary of Ord	er	REQ. QUOTE DATE	11	0	RDER #	J0724-4080	
			ORDER DATE	07/17/24	Q	UOTE #		
			DELIVERY DATE	11	C	USTOMER ACCT #	0000007216	
	ROOF & FLOOR		DATE OF INVOICE	11	C	USTOMER PO #		
I C	COMTECH TRUSSES & BEAMS		ORDERED BY	Shaun Garderner	IN	VOICE #		
Reil	ly Road Industrial Park P.O. Box 40408		COUNTY	Harnett	T	ERMS		
Fay	etteville, N.C. 28309 (910) 864-TRUS		SUPERINTENDANT	Shaun Garderner	S	ALES REP	Neil Baggett	
			JOBSITE PHONE #	(910) 988-8172	S	ALES AREA	Neil Baggett	
	Precision Custom Homes	JOB NAME:L	₋ot 8 Magnolia Hills		LOT # 8	SUBDIV: Magn	olia Hills	
S O L D	206 Shoreline Drive	MODEL: Floo	r TAG: Hai	zlitt w/CP	JOB CAT	EGORY: WCall - Wil	l Call	
D T O	Raeford, NC 28376 (910) 988-8172	DELIVERY INS 60 miles round to						
SHHP HO	Precision Custom Homes and Lot 8 Magnolia Hills Cameron, NC	SPECIAL INST Like 64 Liberty N				PLAN SI	EAL DATE: 9/10/)/24
						I LAN O	BY DATE	

BUILDING DEPARTMENT	OVERHA	ANG INFO	HEEL HEIGHT	00-04-05	R	EQ.	LAYOUTS		REQ.	ENG	GINEERING		QUOTE		11
Floor Order	END CUT	RETURN											LAYOUT	NB	09/12/24
_	PLUMB		GABLE STUDS	24 IN. OC			JOBSITE	1			JOBSITE	1	CUTTING	NB	09/12/24

ITEMS

QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES
2	BlueLinx (F)	LVL, Metsa(F) 2.0, 16"	09-00-00		FB1
2	BlueLinx (F)	LVL, Metsa(F) 2.0, 16"	07-00-00		FB2
2	BlueLinx (F)	LVL, Metsa(F) 2.0, 18"	23-00-00		DB1

															D	ATE 09	/12/24	PAGE 1
React	ion	Sun	nma	ry of	Order	Γ	REQ.	QUOT	E DATE				ORDE	<u>R</u> #			J0724	-4079
/	\sim					-	ORDE	R DA	TE	07/	17/24		QUOT	Έ#				
						Γ	DELIV	ERY	DATE	1			CUST	ОМ	ER ACCT	#	00000	007216
Í.c	т			& FLO			DATE	OF IN	IVOICE	1				-	ER PO #			
Com	iec	:n 1	RUSSE	:S & BE	AMS		ORDE		BY		aun Garderner		INVOI		#			
Reilly Roa							COUN				nett		TERM	-				
Fayettevill	le, N.C	. 2830	09 (91	0) 864-	TRUS	:	SUPE	RINTE	INDANT	Sha	aun Garderner		SALE	S R	EP			Baggett
							JOBS	ITE PI	HONE #	(91	0) 988-8172		SALE	s ai	REA		Neil E	Baggett
Prec	cisior	n Cu	stom	Home	s JOB	NAME: Lo	t 8 Ma	gnolia	a Hills			LOT #	# 8	รเ	JBDIV: Ma	agnolia	a Hills	
s 0 L D	Shor	eline	Drive)	MOD	EL:Roof			TAG:	Hazlitt w	//CP	JOB (CATEGO	DRY	:WCall -	Will C	all	
			28376		DELI	ERY INST	RUCTIO	ONS:										
) 988				60 mil	es round trip)											
Prec	cisior	ו Cus	tom H	lomes	and	IAL INSTR		NS										
	8 Mag	gnoli	a Hills	5	-	4 Liberty Me												
5 Can	neron	, NC																
															PLA		L DA BY	TE: 9/10/24 DATE
BUILDIN			MENT	OVERI	HANG INFO	HEEL HEIG	нт	00-0	04-05	REQ. I	AYOUTS	REQ. ENGINE	EERING		QUOTE			
Roof Ord				END CU											LAYOUT	NB		09/12/24
				PLUMB		GABLE ST	UDS	24	IN. OC		JOBSITE 1	JC	DBSITE	1	CUTTING	B NB		09/12/24
ROO	E TI	אווכ	SEC		ADING	TCLL-TCDL-	BCLL-BCI		RESS INCR.	_ ٦				c /		_	_	
		103		' IN	FORMATION			_	1.15		OOF TRUSS S	SPACING: 24	.0 IN. U.	.u. (
PROFI		QTY	PIT	СН	TYPE	BASE		IBER	OVE	RHANG	REACTIO	PNG						
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		1	4.00	0.00	GABLE A1GE	37-04-08		2 2 6	01-02-08		Joint 2	Joint 22	Joint			nt 24		Joint 25
		·	4.00	0.00	AIGE	01-04-00	2^0	2.00	01-02-00	1	197.5 lbs. -95.6 lbs.	184.4 lbs. -64.6 lbs.		0.4 II 1.8 II		76.6 lbs 37.9 lbs		176.6 lbs. -87.0 lbs.
										1	-30.0 IDS.	-04.0 105.	-13	1.01		501 U.S.	•	01.0 103.
					ROOF	37-04-08					Joint 2	Joint 9						
		5	4.00	0.00	A2			2 X 6	01-02-08		1555.5 lbs.	1639.6 lbs						
<u>~~ ¥</u>											-117.0 lbs.	-65.6 lbs.						
		İ																
~					ROOF	37-04-08					Joint 1	Joint 8						
A	\sim	2	4.00	0.00	A2A	37-04-08	2 × 6	2 8 6			1486.7 lbs.	1652.2 lbs -65.6 lbs.	•					
											-101.1 lbs.	-05.0 IDS.						
					ROOF	37-04-08				1	Joint 1	Joint 8						
\wedge		2	4.00	0.00	A3	37-04-08		2 X 6		01-02-0	-	1718.7 lbs						
	<u> </u>										-884.1 lbs.	-82.3 lbs.						
	İ	İ																
1					ROOF	35-05-00					Joint 2	Joint 11						
A	N	2	4.00	0.00	A4	35-04-08	2 × 6	2×6	01-02-08		1463.8 lbs.	1528.6 lbs						
											-114.6 lbs.	-47.7 lbs.						
					ROOF	35-05-00				1	Joint 1	Joint 10						
		1	4.00	0.00	A4A	35-03-00		2 X 6		1	1395.0 lbs.	1529.2 lbs						
AN	VN										-98.6 lbs.	-47.7 lbs.						
	İ	İ					1			ĺ								
17	1				ROOF	35-07-00					Joint 2	Joint 8	Joint					
N		2	4.00	0.00	A5	24-01-08	2 X 6	2 X 6	01-02-08	1	1794.7 lbs.	1050.3 lbs		94.7				
										1	-105.7 lbs.	-142.3 lbs.	-10	5.7 I	DS.			
					ROOF	35-07-00				1	Joint 2	Joint 8	Joint	10				
K	1	1	4.00	0.00	A6			2 X 6	01-02-08		550.6 lbs.	Joint 8 194.6 lbs.		10 3.3	bs.			
A	_										-90.7 lbs.	-53.4 lbs.		9.4 II				
	1	1					1	İ		1								
	4				ROOF	16-10-00				1	Joint 2	Joint 9						
A		1	4.00	0.00	A7	16-07-00	2 X 6	2 X 6	01-02-08		672.7 lbs.	710.1 lbs.						
							<u> </u>	ļ		1	-73.5 lbs.	-133.1 lbs.						
										1	lei-+ 0	10:						
-	TA	1	4.00	0.00	GABLE B1SG	23-00-00		2 X 6	01-02-08	01-02-0	Joint 2 971.7 lbs.	Joint 14 992.3 lbs.						
		'	т .00	0.00	5,00						-280.2 lbs.	-336.6 lbs.						
											200.2 100.							
					MONOPITCH	23-00-00					Joint 2	Joint 9						
	\triangleleft	5	4.00	0.00	B2	23-00-00	2 X 6	2 X 6	01-02-08	01-02-0	966.2 lbs.	1002.4 lbs						
	_										-117.8 lbs.	-138.2 lbs.						
							•											

																DA	ATE 09/12	2/24 PAGE 2
Re	action	Sun	nma	ry of	Order		REQ	<u>QUO</u>	TE DAT		11			ORDE	R #		JO	724-4079
	\wedge						ORD	ER DA	TE		07/17	/24		QUOT	Е#			
				~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~				VERY			11			CUST	OMEF	R ACCT	# 00	00007216
Í	omTer	- []		& FLO					VOICE		11					R PO #		
	omTec								BY			n Garderner		INVOI				
	y Road Indu						COU				Harne			TERM	-			
Faye	etteville, N.C	. 2830	09 (91	0) 864-	TRUS				ENDAN			n Garderner		SALE				il Baggett
							JOB	SITE P	HONE	#	(910)	988-8172		SALE	S ARE	A	Ne	il Baggett
	Precisio	n Cu	stom	Home	s JOE	B NAME: l	ot 8 N	lagnoli	a Hills				LOT #	# 8	SUB	DIV: Ma	gnolia H	lills
SOHD	206 Shoi	reline	Drive	;	мо	DEL:Root	F		TAC	: Hazl	itt w/C	P	JOB (CATEGO	ORY: V	VCall - V	Vill Call	
D T O	Raeford,	NC 2	28376			IVERY INS		IONS:										
°	(910) 988	3-8172	2		60 m	iles round t	rip											
ß	Precisio		tom l	Jomos	and													
SHHP					SPE	CIAL INST	RUCTI	ONS:										
Р Т О	Lot 8 Ma	-	a Hills	5	Like	64 Liberty N	/leadow	/S										
°	Cameror	ı, NC														PLAN	SEAL D	DATE: 9/10/2
																	BY	
		PART	MENT		HANG INFO	HEEL HE	IGHT	00-	04-05	RE	Q. LAY	OUTS	REQ. ENGINE	ERING		QUOTE		11
Roc	of Order			END CU			TURA	-						DOITE			NB NB	09/12/24
				PLUMB		GABLE S			IN. OC		J(OBSITE 1	JC	BSITE	וין	UTTING	IND	09/12/24
R	OOF TI	RUS	SES		DADING FORMATIO		L-BCLL-B	CDL ST	RESS INC	R.	RO	OF TRUSS S	SPACING: 24	0 IN. O	.C. (T)	YP.)		
-	ROFILE	QTY	PIT	CH IN	TYPE	20.0,10		0.0 IMBER	1.15	ERHA		1						
Ľ	RUFILE	PLY	TOP	BOT	ID	0/A		P BOT	LEF1		IGHT	REACTIO	ONS					
					MONOPITCH		00					Joint 2	Joint 11					
-		6	4.00	0.00	B2-A	23-00-	00 2 X	6 2 X 6	6 01-02-0	01-10	02-08	966.2 lbs.	1121.6 lbs					
									1			-117.8 lbs.	-138.2 lbs.					
					GABLE	23-00-	n					Joint 2	Joint 11					
		1	4.00	0.00	B3SG			6 2 X 6	01-02-0	01-1	02-08	971.7 lbs.	992.3 lbs.					
æ												-280.2 lbs.	-336.6 lbs.					
								1	1									
	<u></u>				MONOPITCH						00.00	Joint 2	Joint 9					
	\sim	3	4.00	0.00	B4	17-08-	ע 2 אר אר	0 2 X 6	6 01-02-0	-10 אנ	02-08	754.0 lbs.	791.3 lbs.					
												-98.7 lbs.	-111.2 lbs.					
					GABLE	17-08-	08					Joint 2	Joint 9					
		1	4.00	0.00				6 2 X 6	6 01-02-0	01-10	02-08	759.7 lbs.	781.0 lbs.					
	<u> </u>											-226.3 lbs.	-269.3 lbs.					
			4.00		MONOPITCH			6 2 4 4			02.00	Joint 2	Joint 8					
		8	4.00	0.00	B6	10-01-	50 2 X		6 01-02-0		02-00	648.6 lbs. -89.2 lbs.	686.7 lbs. -97.9 lbs.					
									1			-09.2 IDS.	-91.9 IDS.					
					GABLE	15-01-	00					Joint 2	Joint 12	Joint	t 13	Joint	14	Joint 15
		1	4.00	0.00	B7GE			6 2 X 6	6 01-02-0	01-10	02-08	167.8 lbs.	190.1 lbs.		1.8 lbs.		4.2 lbs.	159.5 lbs.
												-29.6 lbs.	-115.5 lbs.	-30).2 lbs.	-56	6.7 lbs.	-57.7 lbs.
	TIT	4	4.00	0.00	MONOPITCH C1GE			6246	6 01-02-0	18 01	02-08	Joint 10	Joint 11	Joint		Joint		Joint 14
	MI	1	4.00	0.00	UIUL	12-00-	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~				52-00	141.8 lbs. -232.2 lbs.	235.4 lbs. -22.9 lbs.		2.6 lbs. 3.6 lbs.		1.5 lbs.).7 lbs.	162.2 lbs. -60.0 lbs.
									1				22.0 103.	-+(100.	-00		50.0 105.
					GABLE	14-05-	00					Joint 2	Joint 10	Joint	t 11	Joint	12	Joint 13
-		1	4.00	0.00	D1GE	14-05-	00 2 X	6 2 X 6	6 01-02-0	01-10	02-08	208.0 lbs.	189.9 lbs.		2.2 lbs.		3.1 lbs.	174.9 lbs.
									1			-45.2 lbs.	-114.7 lbs.	-29	9.7 lbs.	-58	3.0 lbs.	-60.6 lbs.
					MONOPITE							laint 4	1.1.1.7					
		1 2 Ply	4.00	0.00	MONOPITCI D2			62 X 1	0	01-	02-08	Joint 1 6580.8 lbs.	Joint 7 5422.8 lbs					
đ		- · ·y	4.00	0.00					1			-342.1 lbs.	-335.3 lbs.					
									1									
					GABLE	06-05-	08					Joint 2	Joint 6					
		1	4.00	0.00	G1SG	06-05-	08 2 X	4 2 X 6	6 01-02-0	08		332.9 lbs.	235.4 lbs.					
-									1			-193.9 lbs.	-148.7 lbs.					
					MONOPIES								1-1-1-4					
		3	4.00	0.00	MONOPITCI G2			4 2 X F	6 01-02-0	08		Joint 2 332.9 lbs.	Joint 4 235.4 lbs.					
4		5	4.00	0.00								-135.0 lbs.	-101.7 lbs.					
									1				101.1 103.					

														DATE	09/12/24	PAGE 3
Re	action	Sun	nma	ry of	Order	R	REQ. O	τους	E DATE	11			ORDER #	ŧ	J0724	1-4079
	\wedge					C	RDE	r da'	TE	07/17	//24		QUOTE #	ŧ		
/									DATE	11				IER ACCT #	0000	007216
í.	omTor			& FLO					VOICE	11				IER PO #		
Ľ	omTec	n:	RUSSE	S & BE	AMS		RDE		BY		n Garderner		INVOICE	#		
	y Road Indu						OUN			Harne			TERMS			
Fay	etteville, N.C	. 2830	09 (91	0) 864-	TRUS		-		NDANT		n Garderner		SALES R		-	Baggett
						J	OBSI	TE PI	HONE #	(910)	988-8172		SALES A	REA	Neil E	Baggett
	Precisio	n Cu	stom	Home	s JOBI	NAME: Lot	8 Ma	gnolia	a Hills			LOT #	8 S	UBDIV: Magn	olia Hills	;
SOLD	206 Shor	reline	Drive	•	MOD	EL:Roof			TAG: H	-lazlitt w/0	CP	JOB C	ATEGOR	r: WCall - Wil	Call	
D	Raeford,	NC 2	28376		DELIV	ERY INSTR	UCTIC	ONS:								
тo	(910) 988				60 mile	es round trip										
	. ,															
S H I P	Precisio				and SPEC	IAL INSTRU		NS:								
	Lot 8 Ma	-	a Hills	5	-	Liberty Mea										
т	Cameron	ı, NC												PI AN SE		TE: 9/10/24
	<u> </u>				I										BY	DATE
BU	LDING DE	PART	MENT	OVER	HANG INFO	HEEL HEIGI	НТ	00-0	04-05	REQ. LA	YOUTS	REQ. ENGINE	ERING	QUOTE		11
	of Order			END CU											IB	09/12/24
			l	PLUMB	(GABLE STU	IDS	24	N. OC	_ J	OBSITE 1	JO	DBSITE 1		lВ	09/12/24
R	OOF TI	RUS	SES		ADING	TCLL-TCDL-B		_	RESS INCR.	RO	OF TRUSS	SPACING: 24.	0 IN. O.C.	(TYP.)		
				IN	FORMATION	20.0,10.0,0			1.15		1			· · · /		_
P	ROFILE	QTY PLY	-	CH	TYPE ID	BASE O/A		BOT		HANG	REACTIO	ONS				
		FLI	TOP	BOT	GABLE	10-00-00		BOI	LEFT	RIGHT	Joint 2	Joint 6				
		1	5.00	0.00	H1		2 X 4	2 X 6	01-02-08	01-02-08	470.0 lbs.	470.0 lbs.				
~											-204.1 lbs.	-204.1 lbs.				
					COMMON	10-00-00	0.14		04.00.00		Joint 2	Joint 4				
A		4	5.00	0.00	H2	10-00-00	2 X 4	2 X 6	01-02-08	01-02-08	470.0 lbs.	470.0 lbs.				
											-146.8 lbs.	-146.8 lbs.				
					COMMON	10-00-00					Joint 1	Joint 3				
1		1	5.00	0.00	H3	10-00-00	2 X 4	2 X 6			390.0 lbs.	390.0 lbs.				
9 7											-109.1 lbs.	-109.1 lbs.				
			0.00	0.00	MONOPITCH P1GE	02-00-00	2 4 6	2 2 6	01-02-08	01 06 00	Joint 1	Joint 2	Joint 5	Joint 23		Joint 25
	A	1	8.00	0.00	FIGE	02-00-00	2 ~ 0	2 ~ 0	01-02-00	01-00-00	121.4 lbs. -20.9 lbs.	112.5 lbs. -12.0 lbs.	222.8 -175.5			210.9 lbs. -109.6 lbs.
											-20.9 lb5.	-12.0 IDS.	-175.5	105. 1.51	05.	-109.0 lbs.
					MONOPITCH	02-00-00					Joint 2	Joint 5				
	4	5	8.00	0.00	P2		2 X 6	2 X 6	01-02-08	01-06-00	105.6 lbs.	212.7 lbs.				
											-11.2 lbs.	-139.1 lbs.				
		Ī														
				0.00	MONOPITCH	02-00-00	2 4 6	226	01 02 09	01 06 00	Joint 2	Joint 4				
A		6	8.00	0.00	P3	02-00-00	2 × 6	2 × 6	01-02-08	00-00-00	156.8 lbs.	59.5 lbs.				
											-3.1 lbs.	-27.1 lbs.				
					MONOPITCH	02-00-00					Joint 2	Joint 4				
	B	1	8.00	0.00	P4GE		2 X 6	2 X 6	01-02-08	01-06-00	155.1 lbs.	63.7 lbs.				
											-25.1 lbs.	-45.6 lbs.				
			4.00		GABLE	09-02-13		.		01 04 00	Joint 1	Joint 5	Joint 6			
-		1	4.00	0.00	VD1	09-02-13	2 X 4	² X 4		01-04-00	136.9 lbs.	226.0 lbs.	378.4			
				1							13.5 lbs.	-67.5 lbs.	-55.3	DS.		
					VALLEY	05-02-13					Joint 1	Joint 5				
	8	1	4.00	0.00	VALLEY VD2	05-02-13	2 X 4	2 X 4		01-04-00	138.2 lbs.	290.1 lbs.				
-	de la constante da la constant	.									1.1 lbs.	-67.9 lbs.				
					VALLEY	07-06-09					Joint 1	Joint 3				
								1				oonic o				
<u>_6</u>		1	5.00	0.00	VH1	07-06-09	2 X 4	2 X 4			243.5 lbs.	243.5 lbs.				
16		1	5.00	0.00			2 X 4	2 X 4								

ITEMS

	QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES
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			_											DA	TE 09/12/24	PAGE 4
Re	eaction Summa	ry of Or	der		REQ. Q	UOTE DAT	E	/ /				ORDE	R #		J0724-	4079
					ORDER	DATE	(7/17/24				QUOT	E #			
					DELIVE	RY DATE						CUST	ОМ	ER ACCT #	¢ 00000	07216
		& FLOOR			DATE O	F INVOICE						CUST	ОM	ER PO #		
C	ComTech∥ trusse	ES & BEAMS	5		ORDER	ED BY	5	Shaun Garde	rner			INVO	CE	#		
Reill	ly Road Industrial Park P.C	D. Box 40408	3		COUNT	Y	ŀ	larnett				TERM	IS			
Faye	etteville, N.C. 28309 (91	0) 864-TRUS			SUPER	INTENDAN	T S	Shaun Garde	rner			SALE	S R	EP	Neil Ba	aggett
					JOBSIT	E PHONE #	¥ (910) 988-81	72			SALE	S A	REA	Neil Ba	aggett
	Precision Custom	Homes	JOE	B NAME:	ot 8 Mag	nolia Hills					LC	T # 8	รเ	JBDIV: Mag	nolia Hills	
SOLD	206 Shoreline Drive)	мо	DEL:Roof		TAG	: Hazlit	t w/CP			JO	B CATEG	ORY	':WCall - W	/ill Call	
D HO	Raeford, NC 28376 (910) 988-8172		IVERY INS iles round to		NS:											
Like 64					RUCTIONS leadows	S:										
Ů	Cameron, NC													PLAN S	SEAL DAT	
		0/50/100	0.0050			00.04.05									BY	DATE
	LDING DEPARTMENT		G INFO RETURN	HEEL HE	IGHT	00-04-05	REC	. LAYOUTS		RÉ		INEERING		QUOTE LAYOUT	NB	/ / 09/12/24
ROC	of Order	PLUMB	REIURN	GABLE S	TUDS	24 IN. OC		JOBSITE	1			JOBSITE	1	CUTTING	NB	09/12/24
		. 20.00		U. DEE U				CODONE	· ·			CODONE	· ·		ļ	

ITEMS

QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES
7	Hangers, USP	HUS 26			SIMPSON (HUS26)

North Carolina 2018 - R402.1.5 Total UA

Property

Cameron, NC 28326 Model: Hazlitt Community: Liberty Meadows

Template - SMG Precision - Liberty Meadows lot 11 - CZ 3 slab -Liberty Meadows lot 11 Organization

Southern Energy Manager Justin Smith

Inspection Status Results are projected



Builder

SMG Precision Properties

This report is based on a proposed design and does not confirm field enforcement of design elements.

Building UA

Elements	NC Reference	As Designed
Ceilings	61.8	58.6
Above-Grade Walls	190.0	141.1
Windows, Doors and Skylights	87.7	77.5
Slab Floor:	85.0	111.1
Framed Floors	23.3	25.3
Foundation Walls	0.0	0.0
Rim Joists	9.9	8.0
Overall UA (Design must be equal or lower):	457.7	421.6

Requirements

\bigcirc	402.1.5	Total UA alternative compliance passes by 7.9%.	
\bigcirc	402.3.2	Average SHGC: 0.28 Max SHGC: 0.30	
\bigcirc	R402.4.2.2	Air Leakage Testing	Air sealing is 0.25 CFM50 / ft² Shell Area. It must not exceed 0.30 CFM50 / ft² Shell Area.
\bigcirc	R402.5	Area-weighted average fenestration SHGC	
\bigcirc	R402.5	Area-weighted average fenestration U-Factor	
\bigcirc	R404.1	Lighting Equipment Efficiency	
\bigcirc	Mandatory Checklist	Mandatory code requirements that are not checked by Ekotrope must be met.	
\bigcirc	R403.3.1	Duct Insulation	
\bigcirc	403.3.3	Duct Testing	

Design exceeds requirements for North Carolina 2018 Prescriptive compliance by 7.9%.

Name:	Justin Smith	Signature:	Justin Smith
Organization:	Southern Energy Management	Digitally signed:	8/9/22 at 11:13 AM

North Carolina 2018 Prescriptive compliance results calculated using Ekotrope RATER's energy and code compliance algorithm, including appropriate amendments. Ekotrope RATER is a RESNET Accredited HERS Rating Tool. All results are based on data entered by Ekotrope users. Ekotrope disclaims all liability for the information shown on this report.

Property Cameron, NC 28326 Model: Hazlitt Community: Liberty Meadows

Template - SMG Precision - Liberty Meadows lot 11 - CZ 3 slab - ecoSelect Liberty Meadows lot 11

Organization Southern Energy Management Justin Smith

SMG Precision Properties

Inspection Status Results are projected



General Building Informati	on
Number Of Bedrooms	4
Number Of Floors	2
Conditioned Floor Area [sq. ft.]	2,430
Has Electric Vehicle Ready Space	No
Unconditioned, attached garage?	Yes
Conditioned Volume [cu. ft.]	21,870
Total Units in Building	1
Residence Type	Single family detached
Number of Floors in Building	-
Floor Number	-
Model	Hazlitt
Community	Liberty Meadows
RESNET/IECC 2006 Climate Zone	4A
IECC 2021 Climate Zone	ЗА

Builder

Foundation Wall

None Present

Foundation Wall Library List

Slab								
Name	e Library Type	e Perimeter	Floor Grade	Carpet R	Exposed Masonry Area	Surface Area	Location	Enclosing
slab	Uninsulated	174	On Grade	1	0	1,564.0 ft²	Exposed Exterior	Conditioned Space

Slab Library I	List						
Name	Wall Construction Type	Slab Completely Insulated? Ir	Underslab nsulation Width [ft]	Perimeter Insulation Depth [ft]	Perimeter Insulation R Value	Thermal Break	Effective R-value
Uninsulated	Wood Frame / Other	No	0	0	0	No	0.00

Property Cameron, NC 28326 Model: Hazlitt Community: Liberty Meadows

Organization Southern Energy Management Justin Smith

Inspection Status Results are projected



Template - SMG Precision - Liberty Meadows lot 11 - CZ 3 slab - ecoSelect Liberty Meadows lot 11

Builder SMG Precision Properties

Framed Floor					
Name	Library Type	Carpet R	Floor Grade	Surface Area	Location
over garage	R 19, 16"OC G1 Carpet	0	Above Grade	495.0 ft²	Unconditioned, attached garage

Framed Floor Library List				
	Name	Effective R-value		
R 19, 16"OC G1	Carpet	19.566		

n Joist	Library Tree	Surface Area	Lessier
Name	Library Type	Surrace Area	Location
1st floor ambient	R 19 G1, 16"OC	144.0 ft²	Exposed Exterior
1st floor garage	R 19 G1, 16"OC	34.0 ft²	Unconditioned, attached garage



Property Cameron, NC 28326 Model: Hazlitt Community: Liberty Meadows

Template - SMG Precision - Liberty Meadows lot 11 - CZ 3 slab - ecoSelect Liberty Meadows lot 11

Organization Southern Energy Management Justin Smith

Inspection Status Results are projected



Builder SMG Precision Properties

Wall Libr	ary List	
	Name	Effective R-value
R 19 Adv. Fra	ming G1 16" O.C	17.492

Glazing

Claring									
Name	Library Type	Wall Assignment	Foundation Wall Assignment	is Operable	Overhang Depth	Overhang Ft To (Top	Overhang Ft To Bottom	Orientation	Surface Area
front 2nd unshaded	33/28	2nd floor ambient		Yes	0	0	0	North	40.1 ft²
front unshaded	33/28	1st floor ambient		Yes	0	0	0	North	13.4 ft²
rear 2nd unshaded	33/28	2nd floor ambient		Yes	0	0	0	South	26.7 ft²
rear unshaded	33/28	1st floor ambient		Yes	0	0	0	South	77.1 ft²
right unshaded	33/28	1st floor ambient		Yes	0	0	0	West	53.4 ft²

Glazing Library List			
Name	Shgc	U-factor	
33/28	0.28	0.330	

Skylight

None Present

Skylight Library List

Property Cameron, NC 28326 Model: Hazlitt Community: Liberty Meadows

Template - SMG Precision - Liberty Meadows lot 11 - CZ 3 slab - ecoSelect Liberty Meadows lot 11

Organization Southern Energy Management Justin Smith

Inspection Status Results are projected



Builder SMG Precision Properties

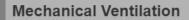
Name	Library Type	Wall Assignment	Foundation Wall Assignment		Solar Absorptance	Surface Color	Surface Area	Location
front entry	Fiberglass R-5	1st floor ambient		0.9	0.75	Medium	20.0 ft²	Exposed Exterior
garage entry	Fiberglass R-5	1st floor garage		0.9	0.75	Medium	20.0 ft ²	Unconditioned, attached garage



oof Insulatio	on					
Name	Library Type	Attic Exterior Area [ft ²]	Clay or Concrete Roof Tiles	Surface Color	r Surface Area	Location
attic	R 38 Attic BLOWN FG G1 2x10 24"OC NO Padiant Barrier	3,026.73	No	Dark	2,059.0 ft²	Attic

Roof Insulation Libr	ary List	
Name	Has Radiant Barrier	Effective R-value
R 38 Attic BLOWN FG G1 2x10 24"OC NO	No	35.115

Whole House	Infiltration		
Infiltration	Measurement Type	Shelter Class	
1750 CFM at 50 Pa	Blower-door tested	4	



Property

Cameron, NC 28326 Model: Hazlitt Community: Liberty Meadows

Template - SMG Precision - Liberty Meadows lot 11 - CZ 3 slab - ecoSelect Liberty Meadows lot 11

Organization Southern Energy Management Justin Smith

Inspection Status Results are projected



Builder SMG Precision Properties

Lighting					
% Interior Fluorescent Lighting		9 % Exterior Fluorescent Lighting		% Garage Fluorescent Lighting	% Garage LED Lighting
0	90	0	0	0	0

I	Onsite Generation	
	Nor	ne Present

Onsite Generation Library List

None Present

Solar Generation

None Present

Dehumidifier

None Present

Dehumidifier Library List

None Present

Whole House Fan

Property

Cameron, NC 28326 Model: Hazlitt Community: Liberty Meadows

Template - SMG Precision - Liberty Meadows lot 11 - CZ 3 slab - ecoSelect Liberty Meadows lot 11

Whole House Fan Library List

None Present

Organization

Justin Smith

Builder

Southern Energy Management

SMG Precision Properties

Conditioning Equipment

U						
Name	Library Type	Serial Number	Heating Percent Load	Cooling Percent Load	Hot Water Percent Load	Location
1st floor heat pump	z 24k 14 SEER 8.2hspf		64%	64%	0%	Attic
2nd floor heat pump	z 24k 14 SEER 8.2hspf		36%	36%	0%	Attic
Water Heating	z 50 gal. 0.95 EF Elec		0%	0%	100%	Unconditioned Garage

Equipment Type: z 24k 14 SEER 8.2hspf

Equipment Type	Air Source Heat Pump
Fuel Type	Electric
Distribution Type	Forced Air
Motor Type	PSC (Single Speed)
Heating Efficiency	8.2 HSPF
Heating Capacity [kBtu/h]	24
Backup Fuel Type	Electric
Switchover Temperature [°F]	0
Backup Heating Efficiency	1 COP
Use default Supplemental Heat	Yes
Cooling Efficiency	14 SEER
Cooling Capacity [kBtu/h]	24

Equipment Type: z 50 gal. 0.95 EF Elec		
Equipment Type	Residential Water Heater	
Fuel Type	Electric	
Distribution Type	Hydronic Delivery (Radiant)	
Hot Water Efficiency	0.95 Energy Factor	
Tank Capacity (gal.)	50	
Hot Water Capacity [kBtu/h]	40	
Recovery Efficiency	0.98	



Property Cameron, NC 28326 Model: Hazlitt Community: Liberty Meadows

Template - SMG Precision - Liberty Meadows lot 11 - CZ 3 slab - ecoSelect Liberty Meadows lot 11

Distribution System

Distribution Type	Forced Air
Heating Equipment	1st floor heat pump
Cooling Equipment	1st floor heat pump
Sq. Feet Served	1,564
# Return Grilles	2
Supply Duct R Value	8
Return Duct R Value	8
Supply Duct Area [ft ²]	422.28
Return Duct Area [ft²]	156.4
Leakage to Outdoors	62 CFM @ 25Pa (3.96 / 100 ft ²)
Total Leakage	62 CFM25
Total Leakage Duct Test Conditions	Post-Construction
Use Default Flow Rate	Yes
Duct 1	
Duct Location	Attic (well vented)
Percent Supply Area	70
Percent Return Area	70
Duct 2	
Duct Location	Conditioned Space
Percent Supply Area	30
Percent Return Area	30
Duct 3	
Duct Location	Conditioned Space
Percent Supply Area	0
Percent Return Area	0
Duct 4	
Duct Location	Conditioned Space
Percent Supply Area	0
Percent Return Area	0
Duct 5	
Duct Location	Conditioned Space
Percent Supply Area	0
Percent Return Area	0
Duct 6	
Duct Location	Conditioned Space
Percent Supply Area	0
Percent Return Area	0

Organization

Justin Smith

Builder

Southern Energy Management

SMG Precision Properties



Property Cameron, NC 28326 Model: Hazlitt Community: Liberty Meadows

Template - SMG Precision - Liberty Meadows lot 11 - CZ 3 slab - ecoSelect Liberty Meadows lot 11

Distribution System

Distribution Type	Forced Air
Heating Equipment	2nd floor heat pump
Cooling Equipment	2nd floor heat pump
Sa. Feet Served	866
# Return Grilles	2
Supply Duct R Value	8
Return Duct R Value	o 8
	233.82
Supply Duct Area [ft ²]	233.02 86.6
Return Duct Area [ft ²]	
Leakage to Outdoors	34 CFM @ 25Pa (3.93 / 100 ft²)
Total Leakage	34 CFM25
Total Leakage Duct Test Conditions	Post-Construction
Use Default Flow Rate	Yes
Duct 1	
Duct Location	Attic (well vented)
Percent Supply Area	100
Percent Return Area	100
Duct 2	
Duct Location	Conditioned Space
Percent Supply Area	0
Percent Return Area	0
Duct 3	
Duct Location	Conditioned Space
Percent Supply Area	0
Percent Return Area	0
Duct 4	
Duct Location	Conditioned Space
Percent Supply Area	0
Percent Return Area	0
Duct 5	
Duct Location	Conditioned Space
Percent Supply Area	0
Percent Return Area	0
Duct 6	
Duct Location	Conditioned Space
Percent Supply Area	0
Percent Return Area	0

Organization

Justin Smith

Builder

Southern Energy Management

SMG Precision Properties

HVAC Grading

HVAC Grading Not Conducted

Ceiling Fan

Has Ceiling Fan Cfm Per Watt

No 100



Property Cameron, NC 28326 Model: Hazlitt Community: Liberty Meadows

Template - SMG Precision - Liberty Meadows lot 11 - CZ 3 slab - ecoSelect Liberty Meadows lot 11

Water Distribution

Water Fixture Type	Low-flow
Use Default Hot Water Pipe Length	No
Hot Water Pipe Length [ft]	87
At Least R3 Pipe Insulation?	No
Hot Water Recirculation System?	No
Recirculation System Pipe Loop Length [ft]	20
Drain Water Heat Recovery?	No

Builder

SMG Precision Properties

Clothes Dryer

Cef	3 01
Fuel Type	Electric
Field Utilization	Timer Controls
Is Outside Conditioned Space	No
Clothes Dryer Available	Yes
Defaults Type	HERS Reference

Clothes Washer

153 kWh/Year
\$12.00
\$0.11/kWh
\$1.22/Therm
3.31
2.1547
Custom
Front-load
6
No
Yes

Dishwasher	
Dishwasher Efficiency	270 kWh
Dishwasher Size	Standard
Annual Gas Cost	\$22.23
Electric Rate	\$0.12/kWh
Gas Rate	\$1.09/Therm
Is Outside Conditioned Space	No

Organization Southern Energy Management Justin Smith



Property Cameron, NC 28326 Model: Hazlitt Community: Liberty Meadows

Template - SMG Precision - Liberty Meadows lot 11 - CZ 3 slab - ecoSelect Liberty Meadows lot 11

Organization Southern Energy Management Justin Smith

SMG Precision Properties

Inspection Status Results are projected



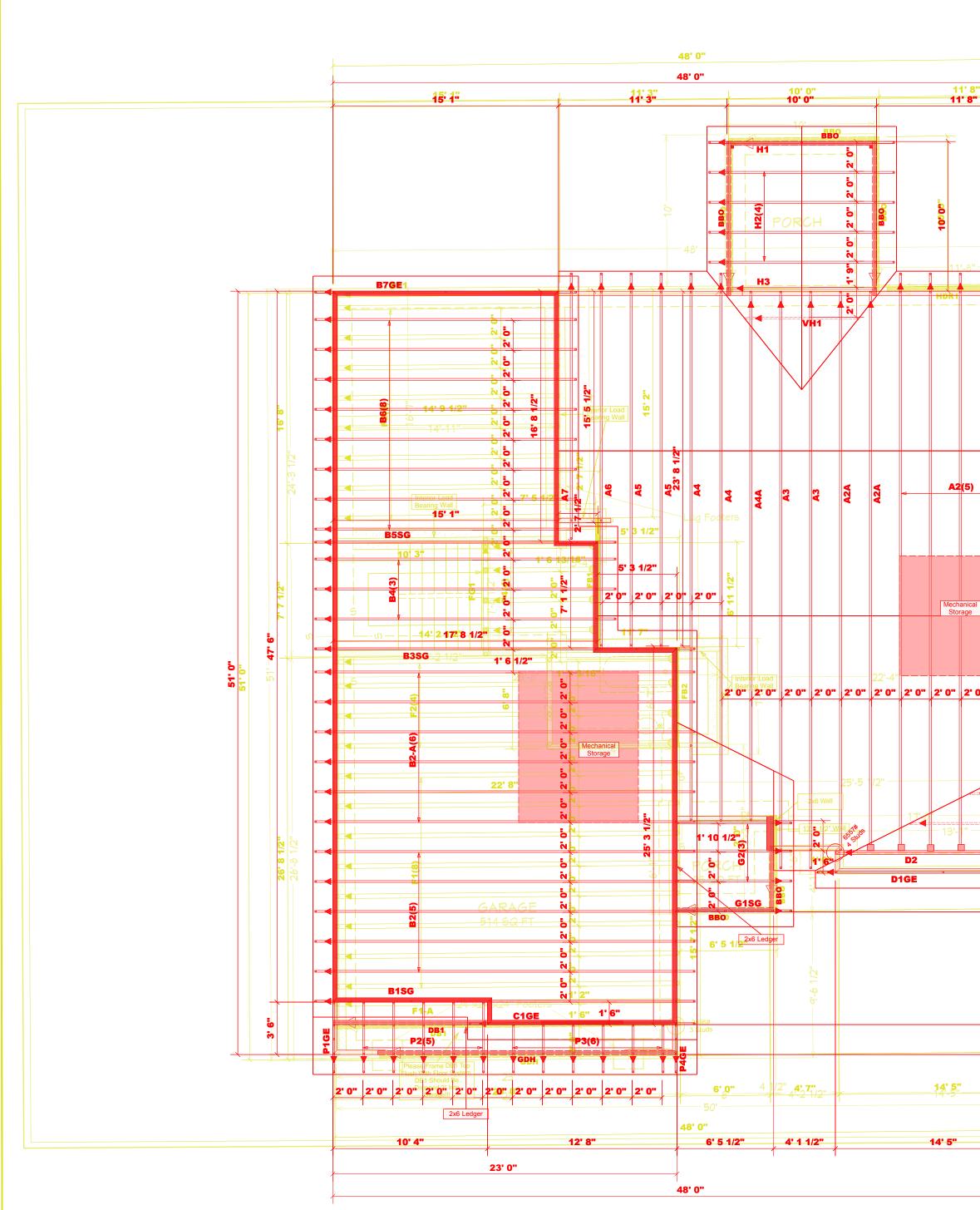
Appliances and Controls	
Thermostat Cooling Setpoint	* * * * 75.0
Thermostat Heating Setpoint	* * * * 70.0
Range/Oven Fuel	Electric
Convection Oven?	No
Induction Range?	No
Range/Oven Outside Conditioned Space?	No
Refrigerator Consumption	538 kWh/Year
Refrigerator Outside Conditioned Space?	No

Builder

Notes

Initial Inputs _____MS 06/30/22__

-confirm HVAC specs -confirm water heater specs -confirm ventilation entry, modeled as air cycler -modeled to worst case orientation -confirm cfl lighting %



11' 8" [1' 8"	PLAN: Hazlitt w/ Covered Porch	Contractions less than or equirations the prescriptives. Code requirations less than or equirations the prescriptives than the prescriptives that the determine the minimum or equiration size and number of wood study extended to be equirations that be retained to design the support system for any reactions that exceeds those specified in the attached tables. A constrained to design the support system for any reaction that exceeds those specified in the attached tables. A registered design professional shall be retained to design the support system for any reactions shall be retained to design the support system for any reaction that exceeds those and the support system for any reactions that be retained to design the support system for any reactions that be retained to design the suppor
	Plumbing Drop Notes 1. Plumbing drop locations shown are NOT exact. 2. Outractor to verify ALLI plumbing drops. 3. Adjust spacing as needed not to exceed 24 oc. 3. Adjust spacing as needed not to exceed 24 oc. 1. All evening mission Notes are to 1. All evening mission Notes are to 1. All evening wall diversions are to 1.	Signature LART FOR JACK STUDS Ideas Neith Baggett: NUMBER OF JACK STUDS RECURED @ EA END OF IDAD CHART FOR JACK STUDS @ Based on Tables R502.5(1) 4 (6) Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Wilder OF JACK STUDS REQUIRED @ EA END OF Stood 1 2550 1 Stood 2 5100 2 6800 2 10200 4
chanical torage	2 10	COUNTYHarnettADDRESSLot 8 Magnolia HillsADDRESSLot 8 Magnolia HillsADDELRoofMODELRoofBATE REV9/12/2024DATE REV9/12/2024DRAWN BYNeil BaggettSALESMANNeil Baggett
	Image: DB3 2370*, 1:3/42x18*(UV Kerto S 9 22 00 22) GD4 20*0* 2x12 SP No.2 20*0* 2x12 SP No.	BUTLDER Precision Custom Homes BUTLDER Precision Custom Homes Lot 8 Magnolia Hills Lot 8 Magn
		Composition to be a set of the se

nese trusses are designed as individual bunding omponents to be dincorplottated who the building sign at the specification of the building designers estadividual design sheets for each truss designal entified on the placement drawnig The building ng signer is responsible for temporary and ing bracing ermanent braching of the Pool and Theories yet and manent braching of the Pool and Theories yet and manent braching of the Pool and Theories yet and the soverall structure of the design of the truss try.c upport structure including headers, beams, walls, nd columns is the responsibility of the building

support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com