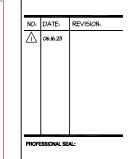
#### **ABBREVIATIONS** INDEX L LENGTH LA LAUNDRY LAY LAYATORY LYR LOUVER MAX MAXIMIM MECH MECHANICAL MER. MANUFACTURER MINIMUM MISC MISCELLANEOUS AD. AREA DRAIN AD.J ADJUSTABLE ALT ALTERNATE ALIMINM ARCH. ARCHITECTURAL BA BATHROOM BD BOARD BF BI-FOLD (DOOR) FRONT ELEVATION 'K' TITLE SHEET / COVER SHEET QUICK VIEW I.I K ROOF PLAN 'K' SIDE AND REAR ELEVATIONS 'K' 0.2 QUICK VIEW MECELLANGUS NORTH NT.5. NOT TO SCALE NOT TO SCALE OF THE NOT FRONT FLEVATION 'A' SIDE AND REAR ELEVATIONS 'K'-ΙA ROOF PLAN 'A' W CRANL SPACE DE BEANGE (DOOR) BOT BOTTOM BINN BETWEEN BOT BOTTOM BINN BETWEEN CAB CABNET CAB CABNET CAB CARNOL JOINT OR CONSTRUCTION JOINT CL (LOSET OR CENTER LINE CL COLLEGE CALL COMMETE MASOINTY UNIT CAN COMMETE CAN COMMETE CAR CORRESION RESISTANT CAT. CERAMIC TILE CAT. CERAMIC TILE CAT. CERAMIC TILE D DRYER SIDE AND REAR ELEVATIONS 'A' SIDE AND REAR ELEVATIONS 'K'-2 A SIDE AND REAR ELEVATIONS 'A'-W/ BASEMENT 2.I A W CRAWL SPACE MONOLITHIC SLAB PLAN 'K' SIDE AND REAR ELEVATIONS 'A'-2.2 A STEM WALL PLAN 'K' 3 SW K W/ BASEMENT 3 CS K CRAWL SPACE PLAN K 3 MS A MONOLITHIC SLAB PLAN 'A' BASEMENT PLAN 'K' STEM WALL PLAN 'A' 4 K IST FLOOR PLAN 'K' 3 SW A CRAWL SPACE PLAN 'A' 3 CS A 5 K 2ND FLOOR PLAN 'K' C.I. CERAMIC IILE D PRYTER DBL DOUBLE DH DOUBLE HANS DIM DINENSION DISP DISPOSAL DN DOON DR DOON DR DOON DS DOWNSPOUT DW DISH MASHER DWG DRAWING E EAST EA EACH BUY ELEVATION BASEMENT PLAN 'A' 4 A IST FLOOR PLAN 'A' ΙP FRONT ELEVATION 'P' 2ND FLOOR PLAN 'A' 5 A HP ROOF PLAN 'P' SIDE AND REAR ELEVATIONS 'P' ΙB FRONT ELEVATION 'B' SIDE AND REAR ELEVATIONS 'P'-I.I B ROOF PLAN 'B W/ CRAWL SPACE 2 B SIDE AND REAR ELEVATIONS 'B' SIDE AND REAR ELEVATIONS 'P'-2.I B SIDE AND REAR ELEVATIONS 'B' W/ BASEMENT W CRAWL SPACE MONOLITHIC SLAB PLAN 'P' SIDE AND REAR ELEVATIONS 'B'-3 SW P STEM WALL PLAN 'P' W/ BASEMENT CRAWL SPACE PLAN 'P' V.B. VAPOR BARRIER VERT VERTICAL V.T.R. VENT THRU ROOF 3 MS B MONOLITHIC SLAB PLAN 'B' 3 BS P BASEMENT PLAN 'P' V.T.R. VENT THRU ROOF IN MASHING MACHINE ND MOOD NDM INIDOM NH MATER HEATER IN WOOD NIC MALK-IN CLOSET W WO WITH OR NITHOUT NP MATERREPROOF(ING) WHM WELDED WIRE MESH 3 SW B STEM WALL PLAN 'B' IST FLOOR PLAN 'P' CRAWL SPACE PLAN 'B' 2ND FLOOR PLAN 'P' 3 (SB 5 P 3 BS B BASEMENT PLAN 'B' IST FLOOR PLAN 'B' 4 B FRONT ELEVATION 'R' GL GLASS OR GLAZING OFF PD OFF STAM BOARD HB HOSE DIBB HD HEAD OR HARD HICK HEADER HT HEADER HAVE HEATHS/NETITLATING/AIR COND. HICK INTEROR JUNI JUNI JUNI KIT KITCHEN #L PROPERTY LINE Ø ROUND / DIAMETER 2ND FLOOR PLAN 'B' 5 B IJ₽ ROOF PLAN 'R' SIDE AND REAR ELEVATIONS 'R' FRONT ELEVATION 'F' SIDE AND REAR ELEVATIONS 'R'-ROOF PLAN 'F' LLE W/ CRAWL SPACE SIDE AND REAR ELEVATIONS 'F' 2 F SIDE AND REAR ELEVATIONS 'R'-2.I F SIDE AND REAR ELEVATIONS 'F'-W/ BASEMENT BUILDING CODE COMPLIANCE / W/ CRAWL SPACE MONOLITHIC SLAB PLAN 'R' SIDE AND REAR ELEVATIONS 'B'-STEM WALL PLAN 'R' 3 SW R PROJECT INFORMATION W/ BASEMENT 3 CS R CRAWL SPACE PLAN 'R' ALL CONSTRUCTION TO COMPLY WITH LOCAL CODES AND ORDINANCES CURRENTLY IN USE WITH THE LOCAL JURISDICTION. MONOLITHIC SLAB PLAN 'F' BASEMENT PLAN 'R' 3 BS R STEM WALL PLAN 'F' IST FLOOR PLAN 'R' CRAWL SPACE PLAN 'F' 2ND FLOOR PLAN 'R' 3 CS F 5 R FOLLOW ALL APPLICABLE STATE AND LOCAL CODES. 2018 NORTH CAROLINA STATE SUPPLEMENTS AND AMENDMENTS BASEMENT PLAN 'E' 3 BS F 4 F IST FLOOR PLAN 'F' BUILDING SECTIONS 2ND FLOOR PLAN 'F' BUILDING SECTIONS LLAS 1.1.2 A S BUILDING SECTIONS I.I.3 A S BUILDING SECTIONS BASEMENT UTILITY PLAN IST FLOOR UTILITY PLAN 2ND FLOOR UTILITY PLAN OPTIONAL EXT. GOVERED PORCH 9 B OPTIONAL EXT GOVERED PORCH SINGLE FAMILY RESIDENCE 9 K OPTIONAL EXT. GOVERED PORCH OCCUPANCY CLASSIFICATION ARCHITECTURAL SHEETS RESIDENTIAL R-3 CONSTRUCTION TYPE ALL CONSULTANT DRAWINGS ACCOMPANYING THESE ARCHITECTURAL DRAWINGS HAVE NOT BEEN PREPARED BY OR UNDER THE DIRECTION OF GMD DESIGN GROUP, INC. GMD DESIGN GROUP INC. THEREFORE ASSUMES NO LIABILITY FOR THE COMPLETENESS OR CORRECTNESS OF THESE DRAWINGS

# EXPRESS HOMES 40'SFRIFS MODEL 'HAYDEN' - RH

Woodgrove Lot 115 420 Blue Aspen Drive Fuquay Varina, NC 27526



BOJECT TITLE:

40' Series

PLAN CHANGES DESCRIPTION INITIAL PLAN RELEASE CLIENT REVISIONS 03.10.21 04.14.21 CLIENT REVISIONS CLIENT REVISIONS 04.15.21 CLIENT REVISIONS CLIENT REVISIONS ADDED LIGHT OVER TUB/SHOWER IN BATH 2 04 25 22 09.22.22 PULL DOWN STAIRS LOCATION UPDATE ADDED ELEVATION 'Q' TO SET CLIENT REVISIONS CLIENT REVISIONS 013023 CONSULTANTS:

# GENERAL NOTES DESIGNER NORTH CAROLINA:

THESE DOCUMENTS ARE THE PROPERTY OF THE DESIGNER AND SHALL NOT BE COPIED, DUPLICATED, ALTERED, MODIFIED OR REVISED IN ANY WAY WITHOUT THE EXPRESSED WRITTEN APPROVAL OF THE DESIGNER.

CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE AND ALL INCONSISTENCES SHALL BE BROUGHT TO THE ATTENTION OF THE DEVELOPER AND THE DESIGNER BEFORE PROCEEDING WITH WORK.

ANY ERRORS OR OMISSIONS FOUND IN THESE DRAWINGS SHALL BE BROUGHT TO

DO NOT SCALE DRAWINGS. WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED

ALL DIMENSIONS ARE TO FACE OF STUD OR TO FACE OF FRAMING UNLESS

ALL TRUSS DRAWINGS TO BE REVIEWED AND APPROVED BY THE STRUCTURAL ALL OR EQUAL SUBSTITUTIONS MUST BE SUBMITTED TO AND APPROVED BY CITY BUILDING OFFICIAL PRIOR TO INSTALLATION.

ALL ANGLED PARTITIONS ARE 45 DEGREES UNLESS OTHERWISE NOTED.

PROVIDE FIREBLOCKING, (PER LOCAL CODES.)

ALL ELECTRICAL AND MECHANICAL EQUIPMENT AND METERS ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS, CONTRACTOR TO VERIFY.

PROVIDE BLOCKING AND/OR BACKING AT ALL TOWEL BAR, TOWEL RING AND/OR TOILET PAPER HOLDER LOCATIONS, AS SHOWN PER PLAN. TYPICAL AT ALL BATHROOMS AND POWDER ROOMS. VERIFY LOCATIONS AT FRAMING WALK.

ELASTOMERIC SHEET WATERPROOFING: FURNISH AND INSTALL ALL WATERPROOFING ASPHALT INTEGRALLY BONDED TO POLYETHYLENE SHEETING, OR EQUAL INSTALL PER MANUFACTURE'S AND TRADE ASSOCIATION'S PRINTED INSTALLATION INSTRUCTIONS. 6" MINIMUM LAP AT ALL ADJACENT WALL SURFACES.

TO THE BEST OF THE DESIGNER'S KNOWLEDGE THESE DOCUMENTS ARE IN CONFORMANCE WITH THE REQUIREMENTS OF THE BUILDING AUTHORITIES HAVING JURISDICTION OVER THIS TYPE OF CONSTRUCTION AND OCCUPANCY.

SHOP DRAWING REVIEW AND DISTRIBUSTION, ALONG WITH PRODUCT SUBMITTALS. RESPONSIBILITY OF THE GENERAL CONTRACTOR, UNLESS DIRECTED OTHERWISE

DEVIATIONS FROM THESE DOCUMENTS IN THE CONSTRUCTION PHASE SHALL BE REVIEWED BY THE DESIGNER AND THE OWNER PRIOR TO THE START OF WORK IN QUESTION. ANY DEVIATIONS FROM THESE DOCUMENTS WITHOUT PRIOR REVIEW, SHALL BE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR

THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL WORK AND MATERIALS REPRESENTED ON THESE DOCUMENTS INCLUDING THE WORK AND MATERIALS FURNISHED BY SUBCONTRACTORS AND VENDORS.

## THE BUILDER SHALL FURNISH ANY AND ALL REPORTS RECEIVED FROM THE

THE DUILDER SYMELT INVANCES AND AND ALL RECEIVED FROM THE SECTECHNICAL ENGINEER (SOILS REPORT), ON THE STUDY OF THE PROPROSED SITE, TO THE DESIGNER, STRUCTURAL ENGINEER, AND GENERAL CONTRACTOR. IN THE EVENT THE GEOTECHNICAL REPORTS DO NOT EXIST, THE SOILS CONDITION SHALL BE ASSUMED TO BE A MINIMUM DESIGN SOIL PRESSURE STATED BY THE STRUCTURAL ENGINEER OF RECORD FOR THE PURPOSE OF STRUCTURAL DESIGN GENERAL CONTRACTOR SHALL ASSURE THE SOIL CONDITIONS MEET OR EXCEED

ALL WORK PERFORMED BY THE GENERAL CONTRACTOR SHALL COMPLY AND CONFORM WITH LOCAL AND STATE BUILDING CODES, ORDINANCES AND THE REPORT OF THESE REQUIREMENTS. AND GOVERNING REGULATIONS

PROVIDE AN APPROVED WASHER DRAIN PAN AT SECOND FLOOR ONLY

WINDOM SUPPLIER TO VERIFY AT LEAST ONE MINDOM IN ALL BEDROOMS TO HAVE A CLEAR OPENABLE AREA OF 4,0 SQ FT. THE MINIMM NET CLEAR OPENING HEIGHT SHALL BE 22' AND THE MINIMM NET CLEAR OPENING HEIGHT SHALL BE 22' AND THE MINIMM NET CLEAR OPENING WIDTH SHALL BE 20'. GLAZING TOTAL AREA OF NOT LESS THAN 5.0 SQ FT IN THE CASE OF A GROUND MINDOM AND NOT LESS THAN 5.7 SQ FT IN THE

ALL HANDRAIL BALLUSTERS TO BE SPACED SUCH THAT A 4" SPHERE CANNOT PASS PROVIDE STAIR HANDRAILS AND GUARDRAILS PER

# BUILDER SET:

OF CONSTRUCTION DOCUMENTS AND GENERAL NOTES HEREINAFTER REFERRED TO AS "PLANS" THIS SET OF PLANS IS SUFFICIENT TO OBTAIN A BUILDING PERMIT, HOWEVER, ALL MATERIALS AND METHODS OF CONSTRUCTION NECESSARY TO COMPLETE THE PROJECT ARE NOT AND HELHOUS OF CONSTRUCTION INCLESSANT TO COMPLETE HE PROJECT ARE NOT NECESSANTLY DESCRIBED. THE PLANS CHINATTE AND DESCRIBE ONLY LICATIONS, DIMENSIONS, TYPES OF MATERIALS, AND GENERAL METHODS OF ASSEMBLING OR FASTENING. THEY ARE NOT INTENDED TO SPECIFY PARTICULAR PRODUCTS OR OTHER METHODS OF ANY SPECIFIC MATERIALS, PRODUCT OR METHOD. THE IMPLEMENTATION OF THE PLANS REQUIRES A CLIENT / CONTRACTOR THROUGHLY KNOWLEDGEABLE WITH THE APPLICABLE BUILDING CODES. AND METHODS OF CONSTRUCTION SPECIFIC TO THIS PRODUCT TYPE AND TYPE OF CONSTRUCTION

CONSTRUCTION REQUIREMENTS AND QUALITY: PROVIDE WORK OF THE SPECIFIC QUALITY WHERE QUALITY LEVEL IS NOT INDICATED, PROVIDE WORK OF QUALITY CUSTOMARY IN SIMILAR TYPES OF WORK. WHERE THE PLANS AND SPECIFICATIONS, CODES, LAWS, REGULATIONS, THESE OF MORN. THERE THE FLAND AND SPECIFICATIONS, CODIES, LAND, REDUCTIONS, MANUFACTURERS RECOMMENDATIONS OR INDUSTRY STANDARDS REQUIRE WORK OF HIGHER QUALITY OR PERFORMANCE, PROVIDE WORK COMPLYING WITH THOSE REQUIREMENTS AND QUALITY WERER TWO OR MORE QUALITY PROVISIONS OF THOSE REQUIREMENTS COMFLICT WITH THE MOST STRINGENT REQUIREMENTS, WHERE REQUIREMENTS ARE DIFFERENT BUT APPARENTLY EQUAL, AND HEREE IT IS UNCERTAIN MICH REQUIREMENT IS MOST STRINGENT, OBTAIN CLARIFICATION FROM THE 6MD DESIGN GROUP BEFORE PROCEEDING.

# AREA CALCULATIONS:

MODEL 'HAYDEN' SQUARE FOOTAGES					
AREA	· .	ELEV 'F', 'K'			
Ist FLOOR		1066 SF			
2nd FLOOR		1445 SF			
TOTAL LIVING		2511 SF			
GARAGE		422 SF			
PORCH		109 SF			
OPT. COVERED PORCH		80 SF			
OPT. BASEMENT		1006 SF			
**BASEMENT AREA IS TA	**BASEMENT AREA IS TAKEN TO INSIDE OF CONCRETE WALL**				

D·R·HORTON America's Build

PROJECT NO: GMD17049

TITLE SHEET

January 22, 2021









Front Elevation 'B' SCALE: 1/4'=1'-0" AT 22"X34" LAYOUT 1/8'=1'-0" AT 11"X1T" LAYOUT

NO: DATE: REVISION:

06.16.23 PROFESSIONAL SEAL:

PROJECT TITLE:

40' Series

FOR CONSTRUCTION

CLIENTS NAME:



PROJECT NO: GMD17049

QUICK VIEW

PRINT DATE:
January 22, 2021

0.1

## AVAILABLE WITH OPTIONAL 9'-1" FIRST FLOOR PLATE

## NOTES AT OPT 9'-1" PLT:

- WDW HT SET AT 7'-6"
- INTERIOR SOFFITS AT 8'-0"
- EXTERIOR SOFFITS AT 8'-0"

## NOTES:

- GRADE CONDITIONS MAY VARY FOR INDIVIDUAL SITE FROM THAT SHOWN.
BILLIDER SHALL VERIFY AND COORDINATE PER ACTUAL SITE CONDITIONS.
HINDOW HEAD HEIGHTS.
IST FLOOR = 6-6\* UNJO. ON ELEVATIONS.
2ND FLOOR = 1-0\* UNJO. ON ELEVATIONS.
- ROOFING, PITCHED SHINGLES PER DEVELOPER.
- ROOFING, PITCHED SHINGLES PER DEVELOPER.

- WINDOWS: MANUFACTURER PER DEVELOPER, DIVIDED LITES AS SHOWN ON THE EXTERIOR ELEVATIONS
- ENTRY DOOR: AS SELECTED BY DEVELOPER. GARAGE DOORS: AS SELECTED BY DEVELOPER, RAISED PANEL AS SHOWN.
- ALL EXTERIOR MATERIALS TO BE INSTALLED PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
- PROTECTION AGAINST DECAY:
  (ALL PORTIONS OF A PORCH, SCREEN PORCH OR DECK FROM THE BOTTOM OF THE HEADER DOWN, INCLUDING POST, RAILS, PICKETS, STEPS AND FLOOR STRUCTURE.)
- INEL HEADEN DOWN INCLUDING POST, KAILS, PICKETS, SHE'S .
  INBLATION FOR TABLE NIOZZI.
  EXTERIOR MALLS . R.15 BATTS MINIMM. VERIFY
  FLOOR OVER GARAGE.
  ATTIC KREENALLS . R.14 BATTS MINIMM. VERIFY
  CRANL SPACE FLOORING. R.14 BATTS MINIMM. VERIFY
  R.14 BATTS MINIMM. VERIFY
  R.14 BATTS MINIMM. VERIFY
  R.14 BATTS MINIMM. VERIFY

## KEY NOTES:

### MASONRY:

- ADHERED STONE VENEER AS SELECTED BY DEVELOPER, HEIGHT AS NOTED. 2 MASONRY FULL BRICK AS SELECTED BY DEVELOPER, HEIGHT AS NOTED.
- MASONRY FULL STONE AS SELECTED BY DEVELOPER, HEIGHT AS NOTED.
- 4 8' SOLDIER COURSE.
- 5 ROWLOCK COURSE

- N/A
   TYPICALS:
   CORROSION RESISTANT SCREEN LOWERED VENTS, SIZE AS NOTED. [8] CODE APPROVED TERMINATION CHIMNEY CAP.
- 4 CORROSION RESISTANT ROOF TO WALL FLASHING, CODE COMPLIANT FLASHING PER NCRC R905-2,6:3
- O STANDING SEAM METAL ROOF, INSTALL PER MANUFCATURER'S WRITTEN INSTRUCTIONS. DECORATIVE WROUGHT IRON, SEE DETAILS.

- SIDING.

  IZ VINTL SHAKE SIDING FER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER.
  (AT SPECIFIED LOCATIONS.
  FIBER CEMENT SHAKE SIDING PER DEVELOPER W IX4 CORNER TRIM BOARD.)
- 3 VINYL LAP SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER.
- (AT SPECIFIED LOCATIONS: FIBER CEMENT LAP SIDING PER DEVELOPER W/ IX4 CORNER TRIM BOARD.)
- 4 VINYL WAVY SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT WAYY SIDING PER DEVELOPER W IX4 CORNER TRIM BOARD.)
- FIBER CEPTENT PAVIT SOUND FER DEVELOPER WI NA CONTREX TRIN BOARDJ

  [3] NINTL BOADD AND BATT SONNO FER DEVELOPER WITH VINYL CORNER TRIM PER DÉVELOPER

  (AT S'PECIFIED LOCATIONS.
  FIBER CIPENT PANEL SOINS WI I/OS BATTS AT 12" O.C. PER DEVELOPER WI I/O CORNER TRIM BOARDJ

  [4] NINYL TRIM SIZE AS NOTED

  (AT S'PECIFIC LOCATIONS.

  IN PIECE CEPTENT TRIM OR EQUAL, UN.O. SIZE AS NOTED

- FYPON SHUTTERS, TYPE AS SHOWN, SIZE AS NOTED.

  (AT SPECIFIC LOCATIONS: FALSE VINYL SHUTTERS, TYPE AS SHOWN, SIZE AS NOTED.)

ALL INIDONG MOSE CIPIENING IS LESS THAN 24" ABOVE THE FINISH FLOOR AND MIOSE OPENING IS GREATER THAN 12" ABOVE THE OUTSIDE MALKING SURFACE MIST HAVE NINDOW OPENING LIMING DEVICES COMPLYING WITH THE NORC SECTION R312.21 AND R312.22.

6:12 PITCH <u>|</u>|12 6" BAND FYPON: 5H3POI2X5OS, FASCIA MDW HD 17 FYPO 5H3POI8X62 FYPON: IT SH3POI8X625, IT OR OTHER -13 4:12 PITCH , WDW HD - 5 4 \_\_\_2 COLUMN DETAIL Front Elevation 'F' 

NO: DATE: REVISION: 06.16.23

PROFESSIONAL SEAL:

PROJECT TITLE:

40' Series

FOR CONSTRUCTION

D·R·HORTON<sup>®</sup> America's Builder

PROJECT NO: GMD17049

SHEET TITLE: 'HAYDEN' **EXTERIOR** 

**ELEVATIONS** 

'4EPF-F'

PRINT DATE:

January 22, 2021

1F

## ATTIC VENT CALCULATION FOR PLAN 'HAYDEN': 1:150 RATIO.

THE NET FREE VENTILATING AREA SHALL NOT BE LESS THAN 1/50 OF THE AREA OF THE SPACE VENTILATED, PROVIDED THAT AT LEAST 50 PERCENT AND NOT MORE THAN 80 PERCENT OF THE REQUIRED VENTILATING AREA IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE SPACE O BE VENTILATED AT LEAST 3 FEET ABOVE THE EAVE OR CORNICE VENTS WITH THE BALANCE OF THE REQUIRED VENTILATION PROVIDED BY EAVE OR CORNICE VENTS.

EXCEPTIONS:

1. EXCLOSED ATTIC/RAFTER SPACES REQUIRING LESS THAN

1. SQ FT OF VENTILATION MAY BE VENTED WITH CONTINUOUS

SOFFIT VENTILATION ONLY.

2. ENCLOSED ATTIC/RAFTER SPACES OVER UNCONDITIONED SPACE MAY BE VENTED WITH CONTINUOUS SOFFIT VENT ONLY

GENERAL CONTRACTOR SHALL VERIFY THE NET FREE GENERAL CONTRACTOR SHALL VERIFY THE NET FREE VENTILATION OF THE VENT PRODUCT SELECTED BY OWNER. VERIFY WITH MANUFACTURER OF HIGH AND LOW VENTS TO BE USED FOR MINIMAN ACLULATED VENTS REQUIRED. THE REQUIRED VENTILATION SHALL BE MAINTAINED PROVIDE INSULATION STOP SUCH THAT INSULATION DOES NOT OBSTRUCT FREE AIR MOVEMENT AS REQUIRED. BY THE BUILDING OFFICIAL.

BY THE BUILDING OFFICIAL.

ALL OVERLAP FRAMED ROOF AREAS SHALL HAVE OPENINGS BETWEEN THE ADJACENT ATTICS IN THE ROOF SHEATHING (AS) ALLOWED BY THE STRUCTURAL ENGINEER) TO ALLOW PASSAGE AND ATTIC VENTILATION BETWEEN THE TWO OR ISOLATED ATTIC SPACES SHALL BE VENTED INDEPENDENTLY TO CASC REQUIREMENTS.

PER DEVELOPER, AT ALL CANTILEVERED FLOORS, CANTILEVERED ARCHITECTURAL POP-OUTS, AND ANY DOUBLE CANTILLEVERED ACCITICATIONS THAT ARE SEPARATED FROM THE YEARTING CALCULATIONS SHOWN ABOVE, PROVIDE A CONTINUOUS 2" CORROSION RESISTANT SOFFIT VENT AT UNDERSIDE OF FRANCE ELEMENT.

## (PER SECTION R806.2)

I SQUARE INCH VENT FOR EVERY 150 SQUARE INCHES OF CEILING \*144 SQ. IN. = 1 SQ. FT.

BLDG. CEILING (SF) X 144 = BLDG (SQ. IN.)

BLDG. (SQ. IN.) / I50 = SQ. IN. OF VENT REQUIRED

ROOF AREA I:= 1488 SF. 1488 SQ. FT. X 144 = 214272 SQ. IN. 214272 SQ. IN. / ISO = 1428.48 SQ. IN. OF VENT REQ'D

**ROOF AREA 2:=** 34 SF 34 SQ. FT. X I44 = 5616 SQ. IN. 5616 SQ. IN. / I50 = 371.44 SQ. IN. *O*F VENT REQ'D

**ROOF AREA 3:=** | 180 SF | 180 Sq. FT. X | 144 = 25920 Sq. IN. | 25920 Sq. IN. | 150 = | 172.80 Sq. IN. | 0F VENT REQ'D

-TRUSS MANUFACTURER SHALL SUBMIT STRUCTURAL CALCS AND SHOP DRAWINGS TO THE BUILDER'S GENERAL CONTRACTOR AND BUILDING DEPARTMENT FOR REVIEW PRIOR TO FABRICATIONS. - ALL ROOF DRAINAGE SHALL BE PIPED TO STREET OR APPROVED DRAINAGE FACILITY.

- DASHED LINES INDICATE WALL BELOW. - LOCATE GUTTER AND DOWNSPOUTS PER BUILDER. PITCHED ROOFS AS NOTED.

- ALL PLUMBING VENTS SHALL BE COMBINED INTO A MINIMUM AMOUNT OF ROOF PENETRATIONS, ALL ROOF PENETRATIONS SHALL OCCUR TO THE REAR OF THE MAIN RIDGE,

## ATTIC VENT CALCULATION FOR PLAN 'HAYDEN': 1:300 RATIO.

AS AN ALTERNATE TO THE 1/150 RATIO LISTED ABOVE. THE NET FREE CROSS-VENTILATION AREA MAY BE REDUCED TO 1/300 WHEN A CLASS I OR II VAPOR RETARDER IS INSTALLED ON THE WARM - IN - WINTER SIDE OF THE CEILING.

GENERAL CONTRACTOR SHALL VERIEY THE NET FREE GENERAL CONTRACTOR SHALL VERIFY THE NET FREE VENTILATION OF THE VENT PRODUCT SELECTED BY OWNER. VERIFY WITH MANIFACTURER OF HIGH AND LOW VENTS TO BE USED FOR MINIMM CALCULATED VENTS REQUIRED, THE REQUIRED VENTILATION SHALL BE WAINTAINED. PROVIDE INSULATION 5TOP SUCH THAT INSULATION DOES NOT OBSTRUCT FREE AIR MOVEMENT AS REQUIRED BY THE BUILDING OFFICIAL.

ALL OVERLAP FRAMED ROOF AREAS SHALL HAVE OPENINGS BETWEEN THE ADJACENT ATTICS IN THE ROOF SHEATHING (AS ALLOWED BY THE STRUCTURAL ENGINEER)
TO ALLOW PASSAGE AND ATTIC VENTILATION
BETWEEN THE TWO OR ISOLATED ATTIC SPACES SHALL
BE VENTED INDEPENDENTLY TO CBC REQUIREMENTS.

PER DEVELOPER, AT ALL CANTILEVERED FLOORS, CANTILEVERED ARCHITECTURAL POP-OUTS, AND ANY DOUBLE CANTILLEYERED ARCHITECTURAL POF-CUIS, AND ANT OFFI FRAMING PROJECTIONS THAT ARE SEPARATED FROM THE VENTING CALCULATIONS SHOWN ABOVE, PROVIDE A CONTINUOUS 2" CORROSION RESISTANT SOFFIT VENT AT UNDERSIDE OF FRAMED ELEMENT.

## (PER SECTION R806.2)

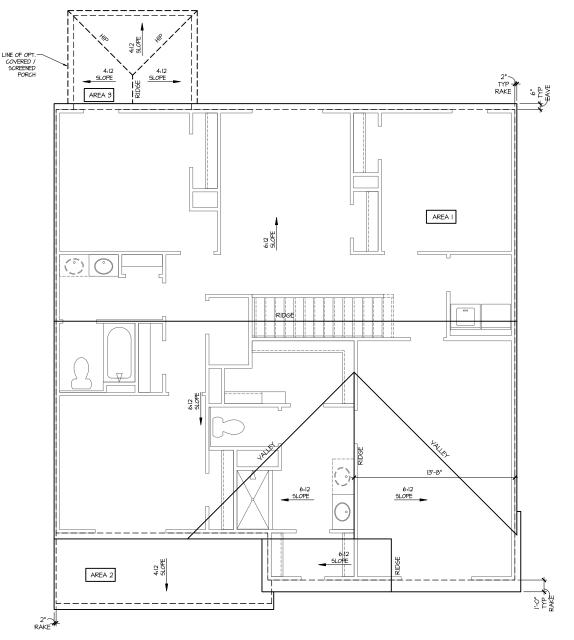
I SQUARE INCH VENT FOR EVERY 300 SQUARE INCHES OF CEILING \*144 SQ. IN. = 1 SQ. FT.

REA I: = 1488 SF. 1488 SQ. FT. X 144 = 214272 SQ. IN. 214272 SQ. FT. / 300 = 114.24 SQ. IN. OF VENT REA'D 114.24 SQ. IN. / 2 = 357112 SQ. IN. OF VENT AT LOW REQUIRED. ROOF AREA I: =

ROOF AREA 3: = 180 SF 180 Sq. FT. X 144 = 25420 Sq. IN. 05 VENT REQTD 25420 Sq. IF. / 300 = 86.40 Sq. IN. 05 VENT REQTD 86.40 Sq. IN. / 2 = 43.20 Sq. IN. 05 VENT AT LOW REQUIRED.

BUILDER TO PROVIDE (2) LAYERS OF UNDERLAYMENT AT ANY ROOF W/ A SLOPE FROM 2:12 TO LESS THAN 4:12

> AT SINGLE FAMILY DETACHED PLANS: PREFINISHED VENTED SOFFIT AT EAVE PER MANUFACTURER. (VERIFY FIRE SEPARATION DISTANCE FOR SOFFIT PROTECTION PER NCRC SECTION R302.1.1 AND TABLE R302.1)



NO: DATE: REVISION: 06.16.23 PROFESSIONAL SEAL:

PROJECT TITLE:

40' Series

FOR CONSTRUCTION

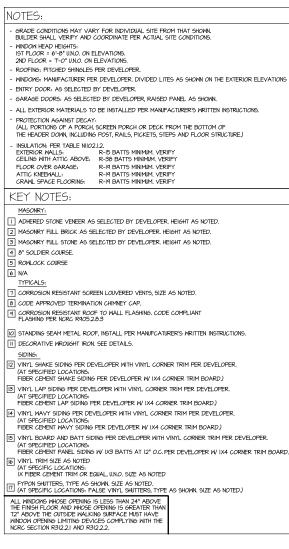
D·R·HORTON America's Builder

PROJECT NO: GMD17049

'HAYDEN' **ROOF PLAN** '4EPF-F'

January 22, 2021

1.1 F



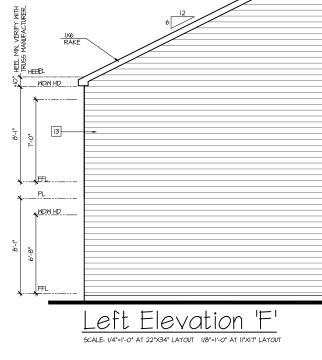
Right Elevation 'F'

SCALE: 174"=1'-0" AT 22"X34" LAYOUT 1/8"=1'-0" AT 11"X17" LAYOUT

AVAILABLE WITH OPTIONAL 9'-I" FIRST FLOOR PLATE

NOTES AT OPT 9'-1" PLT:

- WDW HT SET AT 7'-6"
- INTERIOR SOFFITS AT 8'-0"
- EXTERIOR SOFFITS AT 8'-0"



FASCIA

MDW HD

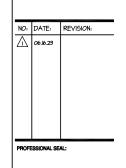
13

, WDW HD



6:12 PITCH





PROJECT TITLE:

40' Series

FOR CONSTRUCTION

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America's Builder
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PROJECT NO: 6MD17049

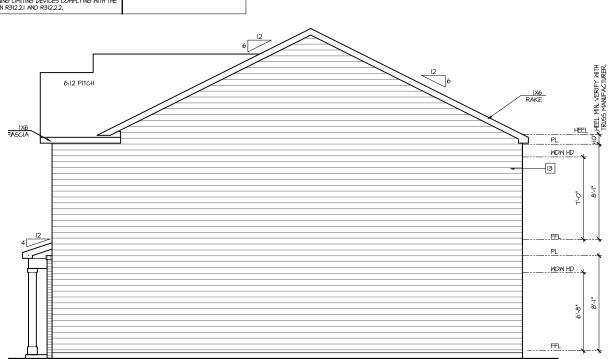
SHEET TITLE:
'HAYDEN'
EXTERIOR

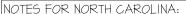
ELEVATIONS '4EPF-F'

January 22, 2021

PRINT DATE:

2 F





IRRIGATION SYSTEM SHALL BE DESIGNED TO PREVENT THE SATURATION OF SOIL ADJACENT TO BUILDING.

THIS PERIMETER DIMENSION PLAN IS FOR DIMENSIONAL INFORMATION ONLY.

SLOPE ALL STOOPS AND HARDSCAPE MATERIAL AWAY FROM BUILDING - TYPICAL.

SLOPE GARAGE FLOOR I/8" PER FOOT TO GARAGE DOOR OPENING.

VERIFY CURB CUT BLOCKOUT WITH GARAGE DOOR MANUFACTURER.

REFER TO CIVIL DRAWINGS FOR FINISH SURFACE ELEVATIONS.

- FINISH GRADE SHALL PROVIDE POSITIVE DRAINAGE AWAY FROM BUILDING, REFER TO SOILS REPORT FOR ANY SPECIFIC REQUIREMENTS.

REFER TO STRUCTURAL DRAWINGS FOR HOLDDOWNS, FOOTING DETAILS, CURB THICKNESS, AND INFORMATION NOT SHOWN ON THIS PLAN.

PLUMBING FIXTURES, VENT LOCATIONS, ETC. ARE APPROXIMATE. CONTRACTOR TO VERIFY COUNT AND LOCATION. VERIFY THE SUPPLY FOR SEPARATE CONDUITS TO ANY ISLAND FOR GAS, WATER OR ELECTRIC.

· VERIFY ALL DOOR THRESHOLD HEIGHTS TO HARD SURFACES, δ 1/4" MAX AT INSMING DOORS, (PER NORG SECTION R311,3.1,)

- TYP STOOP AT INSWING/SLIDER DOORS: 36" DEEP BY THE WIDTH OF THE DOOR SERVED, MINIMUM. (PER NORG SECTION R311.3.) PROVIDE A SLIP-RESISTANT FINISH.

FOR THE USE OF EXPOSED GAS MATER HEATERS IN THE GARAGE, PROTECT THE WATER HEATER WITH 3" DIA CONCRETE FILLED STEEL PIPE EMBEDDED INTO CONCRETE FOOTING.

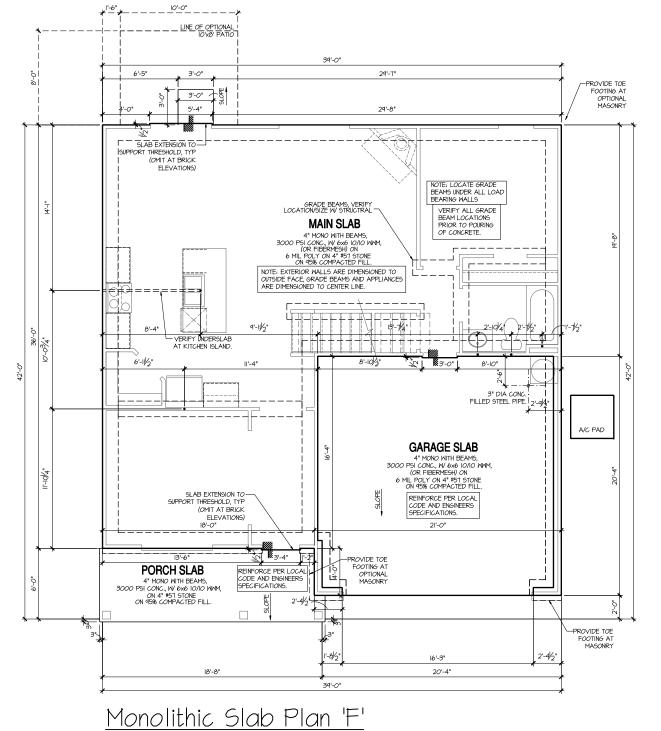
S DIA CONCRETE FILLED STEEL FIFE EMBEDDED INTO CONCRETE FOOTING.

SOLA STREATMENT:

BORACARE TERMITE TO BE APPLIED TO FRAMING PER PRODUCT SPECIFICATIONS.

(PROVIDE CHEMICAL TREATMENT FOR PROTECTION FROM TERMITE INVESTATION ACCORDING TO THE STANDARDS OF THE NC DEPT OF AGRICULTURE).

HOOD CONTACTING CONCRETE OR MASONRY OR LESS THAN CODE REQUIRED SEPARATION TO GRADE SHALL BE PRESEQUE TREATED OR FOUNDATION GRADE REDWOOD. SET ALL EXTERIOR WALL SILLS IN MASTIC.



SCALE: I/4"=I'-0" AT 22"X34" LAYOUT I/8"=I'-0" AT II"XI7" LAYOUT

NO: DATE: REVISION: 06.16.23 PROFESSIONAL SEAL:

PROJECT TITLE: 40' Series

FOR CONSTRUCTION



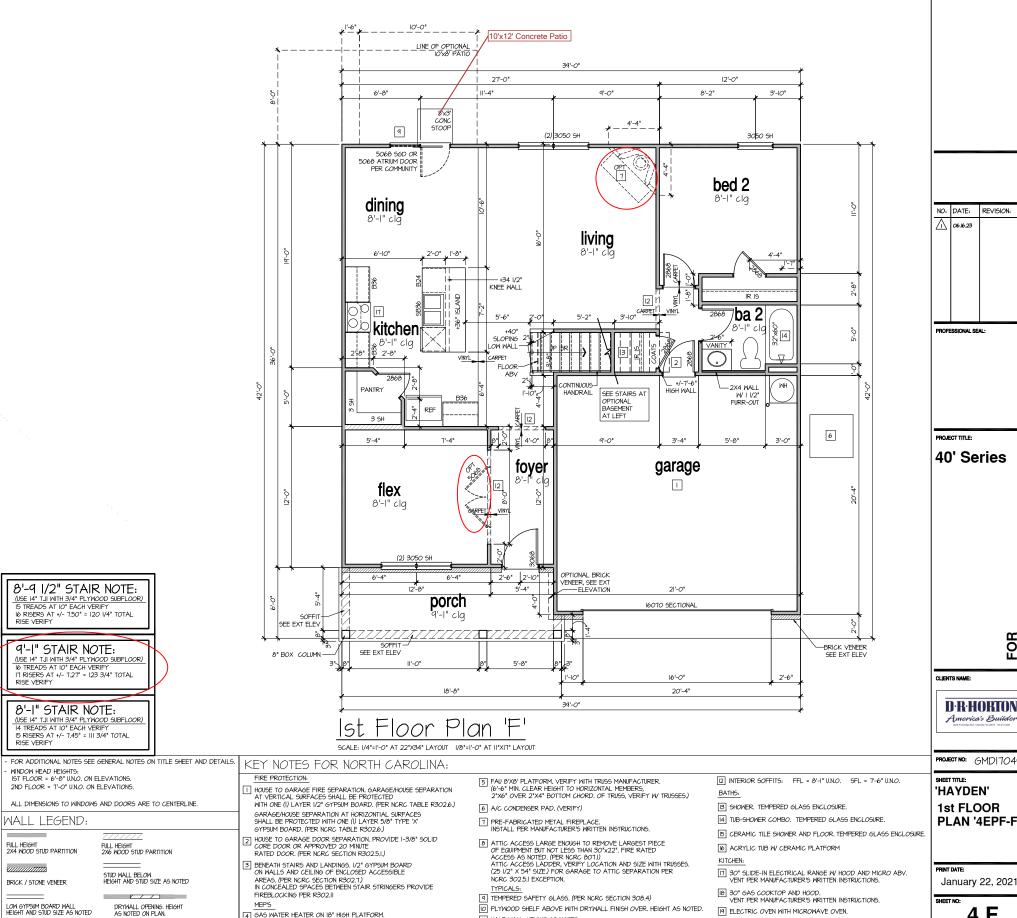
PROJECT NO: GMD17049

'HAYDEN' MONOLITHIC SLAB

PLAN '4EPF-F'

January 22, 2021

3 MS F



PLYWOOD SHELF ABOVE WITH DRYWALL FINISH OVER. HEIGHT AS NOTED.

II HALF WALL, HEIGHT AS NOTED.

LOW GYPSUM BOARD WALL HEIGHT AND STUD SIZE AS NOTED

FOR CONSTRUCTION

CLIENTS NAME:



PROJECT NO: GMD17049

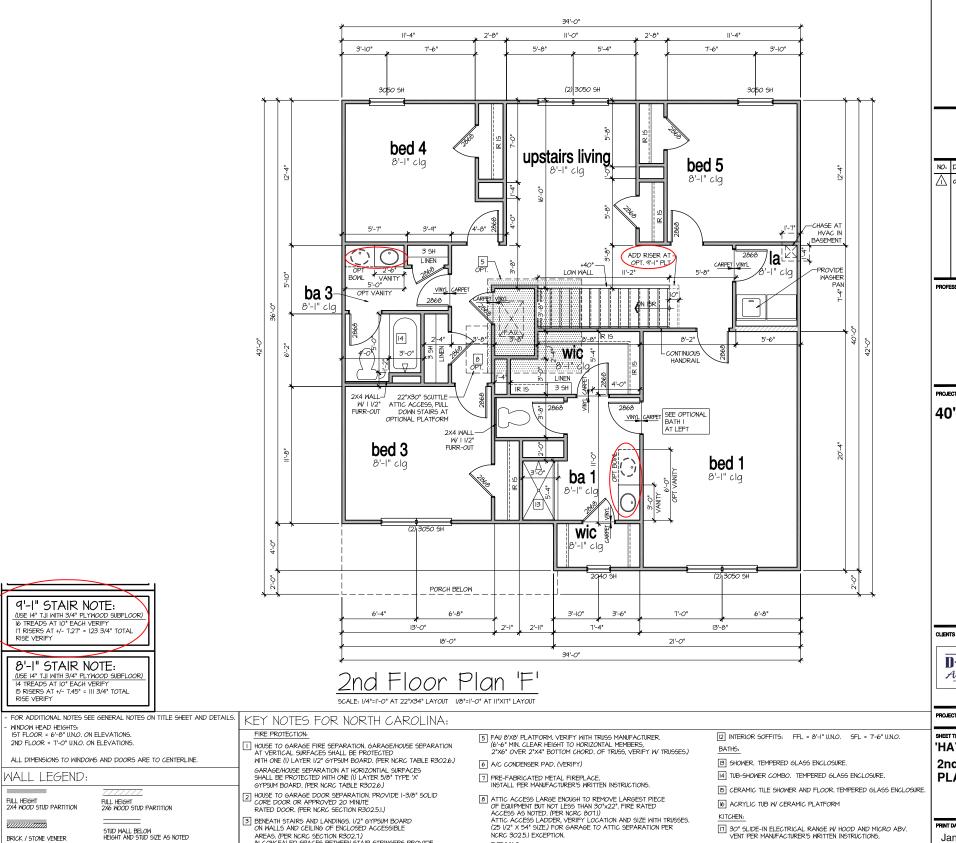
'HAYDEN' 1st FLOOR

PLAN '4EPF-F'

19 ELECTRIC OVEN WITH MICROWAVE OVEN.

January 22, 2021

4 F



TYPICALS:

TEMPERED SAFETY GLASS. (PER NORC SECTION 308.4)

II HALF WALL, HEIGHT AS NOTED.

PLYWOOD SHELF ABOVE WITH DRYWALL FINISH OVER. HEIGHT AS NOTED.

AREAS. (PER NORC SECTION R302.7.)
IN CONCEALED SPACES BETWEEN STAIR STRINGERS PROVIDE

FIREBLOCKING PER R302.II

GAS WATER HEATER ON 18" HIGH PLATFORM. (PER CHAPTER 5 NCRC-PLUMBING)

MEP'S

BRICK / STONE VENEER

LOW GYPSUM BOARD WALL HEIGHT AND STUD SIZE AS NOTED

DRYWALL OPENING. HEIGHT AS NOTED ON PLAN.

NO: DATE: REVISION: 06.16.23 PROFESSIONAL SEAL:

PROJECT TITLE:

40' Series

FOR CONSTRUCTION

CLIENTS NAME:



PROJECT NO: GMD17049

'HAYDEN'

2nd FLOOR PLAN '4EPF-F'

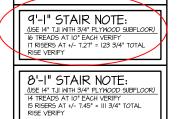
TI 30" SLIDE-IN ELECTRICAL RANGE W HOOD AND MICRO ABV. VENT PER MANUFACTURER'S WRITTEN INSTRUCTIONS.

B 30" GAS COOKTOP AND HOOD.

VENT PER MANUFACTURER'S WRITTEN INSTRUCTIONS. 19 ELECTRIC OVEN WITH MICROWAVE OVEN.

January 22, 2021

5 F



- REFER TO FLOOR PLAN NOTES FOR TYPICAL FIRE PROTECTION NOTES AND LOCATIONS.
- THESE BUILDING SECTIONS MAY YAR" AT ALTERNATE ELEVATION STYLES AND AT "PLAN OPTION" CONDITIONS, REFER TO MAIN FLOOR PLAN AND ALTERNATE FLOOR PLANS FOR INFORMATION NOT SHOWN HERE.
- BUILDING SECTIONS SHOWN HERE DEPICT VOLUMA SPACES WITHIN THE STRUCTURE. REFER TO STRUCTURAL DRAWINGS, TRUSS DRAWINGS, STRUCTURAL DETAILS AND CALCULATIONS BY OTHER FOR ALL STRUCTURAL INFO.

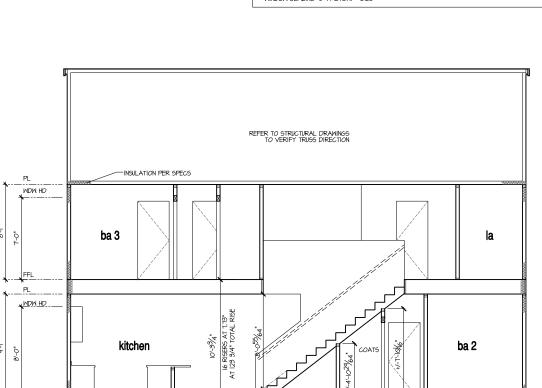
   ROOFING: PITCHED SHINGLE ROOF, REFER TO ROOF PLAN FOR TYPICALS.

PER STATE RESIDENTIAL CODE COMPLIANCE METHOD TO BE DETERMINED BY BUILDER.

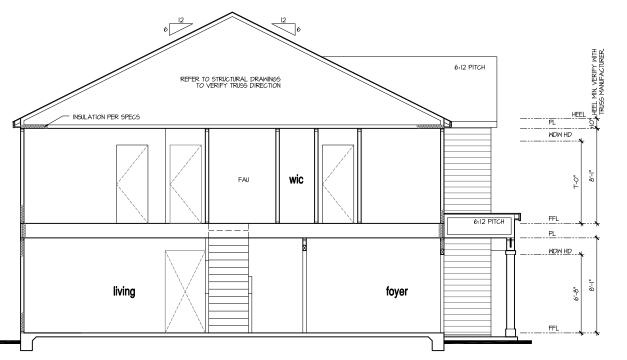
- MOOD FLOORS: FLOOR SHEATHING OVER FLOOR JOIST, REFER TO STRUCTURAL AND TRUSS DRAWINGS BY OTHERS.
- VERIFY STAIRS MINIMUM AND MAXIMUM REQUIREMENTS FOR CONSTRUCTION CLEARANCES WITH LOCAL CODES.
- INSULATION:

- INSULATION:
  EXTERIOR MALLS ZONE 3: R-13 BATTS MINIMUM. VERIFY
  EXTERIOR MALLS ZONE 4: R-15 BATTS MINIMUM. VERIFY
  CEILING WITH ATTIC ABOVE COMPRESSED INSULATION:
  R-36 BATTS MINIMUM. VERIFY
  CEILING WITH ATTIC ABOVE UNCOMPRESSED INSULATION (HEELS IN TRUSSES):
  R-30 BATTS MINIMUM. VERIFY
- FLOOR OVER GARAGE: R-I9 BATTS MINIMUM, VERIFY ATTIC KNEEWALL: CRAWL SPACE FLOORING:

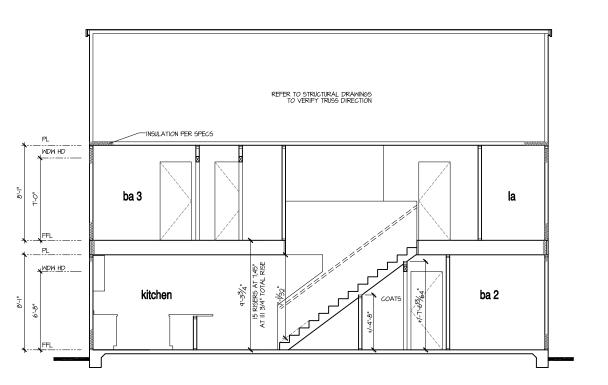
WINDOW GLAZING "U" FACTOR: 0.35



Building Section 2 at Optional 9' Plt



Building Section Lat Monolithic Slab



Building Section 2 at Monolithic Slab

NO: DATE: REVISION:

06.16.23

PROJECT TITLE:

40' Series

FOR CONSTRUCTION

CLIENTS NAME:

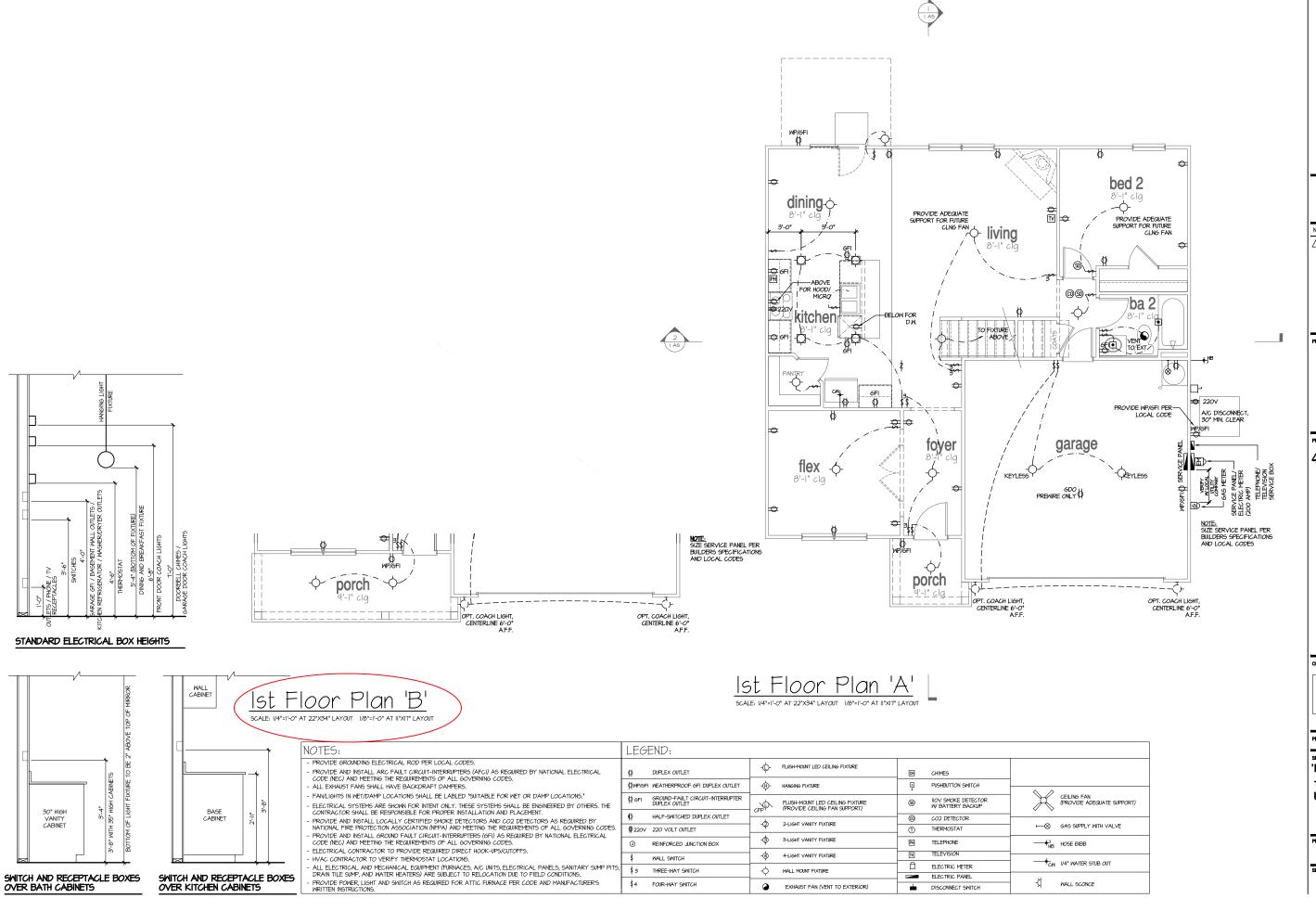


PROJECT NO: GMD17049

'HAYDEN' **BUILDING SECTIONS** 

January 22, 2021

1A S



NO: DATE: REVISION:

C6.16.23

PROFESSIONAL SEAL:

PROJECT TITLE:

40' Series

FOR CONSTRUCTION

CLIENTS NAMI



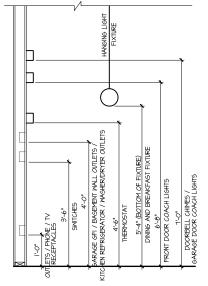
PROJECT NO: GMD17049

SHEET TITLE:
'HAYDEN'
1st FLOOR
UTILITY PLAN

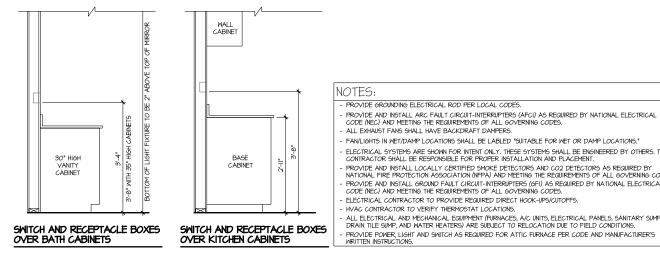
T DATE:

January 22, 2021

7



### STANDARD ELECTRICAL BOX HEIGHTS



## bed 4 PROVIDE ADEQUATE BOTH SUPPORT FOR FUTURE CLING FAN upstairs living 8'-1" 519 PROVIDE ADEQUATE SUPPORT FOR FUTURE CLING FAM PROVIDE ADEQUATE SUPPORT FOR FUTURE CLNG FAN (SD) PROVIDE 2ND GFI/LIGHT AT OPTIONAL BOWL la ba 3-8'-1" clq (SD) VENT TO EXT (SD) VENT TO EXT PROVIDE ADEQUATE SUPPORT FOR FUTURE CLNG FAN PROVIDE 2ND GFI/LIGHT AT OPTIONAL BOWL bed 3 -VENT TO EXIT bed 1 **ba 1** 8'-1" clg **\** WIC,

# 2nd Floor Plan 'A' Scale: 1/4"=1"-0" at 22"x34" layout 1/8"=1"-0" at 11"x11" layout

	LEGI	END:						
AL.	ф	DUPLEX OUTLET	φ-	FLUSH-MOUNT LED CEILING FIXTURE	CH	CHIMES		
	Фир/6FI	WEATHERPROOF GFI DUPLEX OUTLET	ф-	HANGING FIXTURE	P	PUSHBUTTON SWITCH	1	
5. THE	∯ 6FI	GROUND-FAULT CIRCUIT-INTERRUPTER DUPLEX OUTLET	CFP C	FLUSH-MOUNT LED CEILING FIXTURE (PROVIDE CEILING FAN SUPPORT)	99	IIOV SMOKE DETECTOR W BATTERY BACKUP		CEILING FAN (PROVIDE ADEQUATE SUPPORT)
	ф	HALF-SWITCHED DUPLEX OUTLET			(0)	CO2 DETECTOR		
CODES.	<b>₽</b> 220∨	220 VOLT OUTLET	-\$	2-LIGHT VANITY FIXTURE	(T)	THERMOSTAT	⊢⊗	GAS SUPPLY WITH VALVE
CAL	0	REINFORCED JUNCTION BOX	-\$	3-LIGHT VANITY FIXTURE	PH	TELEPHONE	→ <sub>HB</sub>	HOSE BIBB
	\$	WALL SWITCH	-49	4-LIGHT VANITY FIXTURE	īV	TELEVISION		
IMP PITS,	\$3	THREE-WAY SWITCH	-0	WALL MOUNT FIXTURE		ELECTRIC METER	CM	I/4" WATER STUB OUT
5	\$4	FOUR-WAY SWITCH	<b>a</b>	EXHAUST FAN (VENT TO EXTERIOR)		ELECTRIC PANEL DISCONNECT SWITCH	-∤ -∤	WALL SCONCE

NO:	DATE:	REVISION:			
$\overline{\mathbb{A}}$	06.16.23				

PROJECT TITLE:
40' Series

FOR CONSTRUCTION

CLIENTS NAME:



PROJECT NO: GMD17049

HAYDEN'
2nd FLOOR
UTILITY PLAN

T DATE:

January 22, 2021

8

Construction Type: Commerical ☐ Residential ☒

Applicable Building Codes:

• 2018 North Carolina Residential Building Code with All Local Amendments

ASCE 7-10: Minimum Design Loads for Buildings and Other Structures

# Design Loads: ! Roof Live Loads

	I.I. Cor	ventional 2x	 20 F	×
	I.2. Trus	5	 20 F	ķ
	1.2.	I. Attic Truss	60 i	-
2.	Roof Dea	d Loads		
	2.1. Cor	ventional 2x	 10 P	S
3.	Snow		 15 P	ô
	3.1. Impa	ortance Factor	 1.0	
4.	Floor Live	Loads		
	41 Tue	Durallina	10 5	0

42. Sleeping Areas .... 30 PSF 40 PSF 5. Floor Dead Loads Conventional 2x 10 PSF

6. Ultimate Design Wind Speed (3 sec. gust).
6.1. Exposure ...... 6.2. Importance Factor... 63 Wind Base Shea

6.3.1. Vx = 6.3.2. Vy = onent and Cladding (in PSF)

1,	Component and Cladding (III ) 317						
	MEAN ROOF HT.	UP TO 30'	30'1"-35'	35'1"-40'	40'1"-45'		
	ZONE 1	16.7,-18.0	17.6,-18.9	18.3,-19.7	18.8,-20.2		
	ZONE 2	16.7,-21.0	17.6,-22.1	18.3,-22.9	18.8,-23.6		
	ZONE 3	16.7,-21.0	17.6,-22.1	18.3,-22.9	18.8,-23.6		
	ZONE 4	18.2,-19.0	19.2,-20.0	19.9,-20.8	20.4,-21.3		
	ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.2	20.4,-26.9		

Seismi	S	
8.1.	Site Class	D
82.	Design Category	С
8.3.	Importance Factor	1.0
8.4	Seismic Use Group	1

86. Seismic Base Shea

8.62.Vu = 8.7. Basic Structural Sustem (check one) □ Bearing Wall
 □ Building Frame
 □ Moment Frame

Dual w/ Special Moment Frame Dual w/ Intermediate R/C or Special Steel

□ Inverted Pendulum Wind 🖂

9. Assumed Soil Bearing Capacity 20000sf



## HAYDEN RH

PROJECT ADDRESS

DR Horton, Inc. 8001 Arrowridge Blvd. Charlotte, NC 28273

DESIGNER: GMD Design Group 102 Fountain Brook Circle Suite C Cary, NC 27511

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory 4 Testing, INC. before construction begins.

#### PLAN ABBREVIATIONS

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	9C	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
DSP	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
EE	EACH END	SYP	SOUTHERN YELLOW PINE
ΕW	EACH WAY	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
oc	ON CENTER	TYP	TYPICAL
P6F	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
P5I	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory 4 Testing, INC. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by <u>DR Horton, Inc.</u> Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify SUMMIT immediately.

#### SHEET LIST:

4.19.21

5 12.06.26

6.14.21 10177

11,23,21 TØ177

Ø82123 TØITT

TØ177

Sheet No.	Description
C9I	Cover Sheet, Specifications, Revisions
S1.Øm	Monolithic Slab Foundation
S1.Øs	Stem Wall Foundation
51.0c	Crawl Space Foundation
S1.0b	Basement Foundation
52.0	Basement Framing Plan
63.0	First Floor Framing Plan
54.0	Second Floor Framing Plan
S5.Ø	Roof Framing Plan
S6.Ø	Basement Bracing Plan
ST.Ø	First Floor Bracing Plan
58.0	Second Floor Bracing Plan

	51.Øc	;	Crawl Space Foundation				
	51.0k	,	Basement Foundation				
	52.0			Basement Framing Plan			
	63.0			First Floor Framing Plan			
	54.0			Second Floor Framing Plan			
	95.0			Roof Framing Plan			
	56.0	)	Basement Bracing Plan				
	ST.Ø		First Floor Bracing Plan				
	58.0		Second Floor Bracing Plan				
_							
_							
	REVISION I	LIST:					
	Revision No.	Date	Project No.	Description			

Updated elevation names

Added Stem Wall, Crawlspace, and Basement

Foundations
Added OX-IS option and Updated OX-IS table framing

Updated the engineering in the first floor framing

Added Front Porch Extension

Resealing

	Signature
Operations	
Operations System	
Operations Product Development	

DR HORTON PROJECT SIGN-OFF

SUMMIT



DR Horton, Inc. 8001 Arrowridge Blv. Charlotte, NC 28213

NEW S. BEN 106.23 STRUCTURAL MEMBER

PROJECT 4 528-TØITT DRAWN BY: EO CHECKED BY: JOEF

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

DATE: 12/06/2023 9CALE: 22x34 1/4"+1"-@" lbd1 1/8"+1"-@"

## GENERAL STRUCTURAL NOTES:

- The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, and the periormance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory 4 Testing, INC. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entity.

  The structure is only stable in its completed form. The contractor
- shall provide all required temporary bracing during construction to stabilize the structure.
- The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents.
- should any non-conformities occur.

  Any structural elements or details not fully developed on the construction drawings shall be completed under the direction o a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as i relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.

  Verification of assumed field conditions is not the responsibility
- of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before
- construction begins.

  The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically
- noted on the structural drawings.

  This structure and all construction shall conform to all applicable sections of the international residential code. This structure and all construction shall conform to all
- applicable sections of local building codes.

  All structural assemblies are to meet or exceed to requirements of the current local building code.

The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding

- The bottom of all footings shall extend below the frost line for rise portion of all rootings and extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade. Any fill shall be placed under the direction or recommendation of a licensed professional engineer.

  The resulting soil shall be compacted to a minimum of 95% extending the control of the procession of the provider of the procession of the provider o
- maximum dry density.

  Excavations of footings shall be lined temporarily with a 6 mill polystylene membrane if placement of concrete does not occur within 24 hours of excavation.
- No concrete shall be placed against any subgrade containing water, ice, frost, or loose materia

#### STRUCTURAL STEEL

Structural steel shall be fabricated and erected in accordance with the American institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design"

- Structural steel shall receive one coat of shop applied
- rust-inhibitive paint. All steel shall have a minimum yield stress  $\langle F_u \rangle$  of 36 ksi unless otherwise noted.
- Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS DIL. Electrodes for shop and field welding shall be class ETØXX. All welding shall be performed by a certified welder per the above

<u>ICREIE:</u>

Concrete shall have a normal weight aggregate and a minimum compressive strength (f'<sub>c</sub>) at 28 days of 3000 psi, unless otherwise noted on the plan.

- Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301:
- "Specifications for Structural Concrete for Buildings".

  Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows:
  - 3.1. Footings: 5% 32.Exterior Slabs: 5%
- No admixtures shall be added to any structural concrete without written permission of the SER.

- Concrete slabs-on-grade shall be constructed in accordance with ACI 302.IR-96: "Guide for Concrete Slab and Slab
- The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pcl and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions
  - Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted.
  - Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished Reinforcing steel may not extend through a control joint.
  - Reinforcing steel may extend through a saw cut joint.

    All welded wire fabric (W.W.F.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The WWF, shall be securely supported during the concrete pour.

#### CONCRETE REINFORCEMENT:

- Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered
- oue to strinkage and tremal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength. Floetmesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed clefin materials and specifically manufactured for use as concrete secondary reinforcement.
- Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard)
  Fibermesh shall comply with ASTM CIII6, any local building code
  requirements, and shall meet or exceed the current industry
- Steel reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"

  Horizontal footing and wall reinforcement shall be continuous
- and shall have 90° bends, or corner bars with the same size/spacing as the horizontal reinforcement with a class B tension splice. Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in

masonru shall be a minimum of 48 bar diameters.

- Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.
- Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted

WOOD FRAMING Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Southern-Yellow-Pine (SYP)  $^{\rm 12}$ 2.

LVL or PSL engineered wood shall have the following minimum

- design values: 2.1. E = 1,900,000 psi
- 22. Fb = 2600 psi 23. Fv = 285 psi 2.4.Fc = 700 psi
- Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AUPA standard C-I5. All other moisture exposed wood shall be treated in accordance with AWPA standard C-2
- Nails shall be common wire nails unless otherwise noted.

  Lag screws shall conform to ANSI/ASME standard B182.1-1381.

  Lead holes for lag screws shall be in accordance with NDS specifications
- All beams shall have full bearing on supporting framing members unless otherwise noted.
- Exterior and load bearing stud walls are to be 2x4 SYP \*2 @ 16 O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- Ning studs shall be continuous, individual studs forming a column shall be attached with one lod nail 6 6" OC. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfe Multi-ply beams shall have each ply attached with (3) 10d nails @
- Four and five ply beams shall be bolted together with (2) rows of  $12^n$  diameter through bolts staggered e [6" OC. unless noted otherwise.

- The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for
- the wood trusses.

  The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided folloads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to
- The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
- Anu chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

#### EXTERIOR WOOD FRAMED DECKS: Decks are to be framed in accordance with local building

codes and as referenced on the structural plans, either through code references or construction details.

#### WOOD STRUCTURAL PANELS:

- Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial" and all other applicable APA
- All structurally required wood sheathing shall bear the mark of the APA.

Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
Roof sheathing shall be APA rated sheathing exposure 1 or 2.
Roof sheathing shall be continuous over two supports and

attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over fraining. Apply building paper over the sheathing as required by the state Building Code.

Wood floor sheathing shall be APA rated sheathing exposure 1

or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshark nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T4G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by th

Sheathing shall have a 1/8" gap at panel ends and edges as

#### STRUCTURAL FIBERBOARD PANELS:

Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards. All structurally required fiberboard sheathing shall bear the

mark or the AFA.

Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more interesting the properties.

Sheathing shall have a 1/8" gap at panel ends and edges are

#### FOUNDATION NOTES:

- FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AND STATE AMENDMENTS
- STATE AMENDMENTS

  STRUCTURAL CONCRETE TO BE F<sub>c</sub> = 3000 PSI, PREPARED AND PLACED IN ACCORDANCE WITH ACT STANDARD 318.

  FOOTINGS TO BE PLACED ON UNDISTURBED EARTH, BEARING A MINIMUM OF 12" BELOW ADJACENT HINGHED GRADE, OR AS OTHERWISE DIRECTED BY THE CODE ENFORCEMENT OFFICIAL.

  FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF 2000 PSF. CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFYING THE SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION, FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE ELEMENTS, PROVIDE 2" MINIMUM FOOTING PROJECTION FROM THE FACE OF MASONEY.

  MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS

- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS SPECIFIED IN SECTION R4041 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE.
- PILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL
- PROVIDE FOUNDATION MATERPROPING, AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS, PROVIDED PERIMETER INSULATION FOR ALL FOUNDATIONS PER 2018 NORTH
- CAROLINA RESIDENTIAL BUILDING CODE.

  10. CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK
- VENEERS.
- VENERES.

  CRAUL 5PACE TO BE GRADED LEVEL, AND CLEARED OF ALL DEBRIS.
  FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH
  CAROLINA RESIDENTIAL CODE SECTION RADS.16, MINIMUM IN" DIA, BOLTS
  SPACED AT 6'-0" ON CENTER WITH A 1" MINIMUM EMBEDMENT INTO MASONRY
  OR CONCRETE, MINIMUM (2) ANCHOR BOLTS FER PLATE SECTION AND (1)
  LOCATED NOT MORE THAN 12" FROM THE CORNER ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE. ABBREVIATIONS:

D.L.: DOUBLE JOIST SJ = SINGLE JOIST FT = FLOOR TRUSS EE = EACH END SC = STUD COLUMN TJ = TRIPLE JOIST CL = CENTER LINE PL = POINT LOAD OC = ON CENTER

- 14. ALL PIERS TO BE 16 "x16" MASONRY AND ALL PILASTERS TO BE 8"x16" MASONEY, TYPICAL. (UNO)
  WALL FOOTINGS TO BE CONTINUOUS CONCRETE, SIZES PER STRUCTURAL PLAN.
- A FOUNDATION EXCAVATION OBSERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER, OR HIS QUALIFIED REPRESENTATIVE. IF ISOLATED AREAS OF YIELDING MATERIALS AND/OR POTENTIALLY EXPANSIVE SOILS ARE OBSERVED IN THE FOOTING EXCAVATIONS AT THE TIME OF CONSTRUCTION, SUMMIT ENGINEERING, LABORATORY & TESTING, INC. MUST BE PROVIDED THE OPPORTUNITY TO REVIEW THE FOOTING DESIGN PRIOR TO CONCRETE PLACEMENT.
- ALL FOOTINGS & SLABS ARE TO BEAR ON UNDISTURBED SOIL OR 95% COMPACTED FILL, VERIFIED BY ENGINEER OR CODE OFFICIAL.

REFER TO BRACED WALL PLAN FOR PANEL LOCATIONS AND ANY REQUIRED HOLD-DOWNS.
ADDITIONAL INFORMATION PER SECTION R602.10.8
AND FIGURE R602.10.1 OF THE 2018 NCRC.

NOTE: ALL EXTERIOR FOUNDATION DIMENSIONS ARE TO FRAMING AND NOT BRICK VENEER UNO

NOTE: A 4" CRUSHED STONE BASE COURSE IS NOT REQUIRED WHEN SLAB IS INSTALLED ON WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS CLASSIFIED AS GROUP I PER TABLE R4051

NOTE: FOUNDATION ANCHORAGE HAS BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R602.3.5 OF THE 2018 NCRC.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DR HORTON
COMPLETED/REVISED ON 4/15/2]. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY 4 TESTING, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

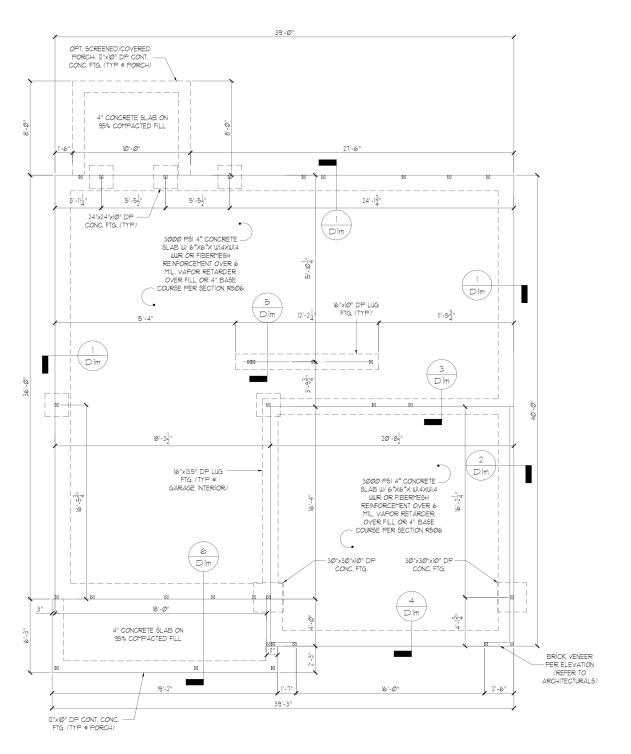
## STRUCTURAL MEMBERS ONLY

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

MONOLITHIC SLAB FOUNDATION PLAN

9CALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"



ELEVATION BEK





DR Horton, Inc. 8001 Arrowrldge Blvc Charlotte, NC 28213

3 ithic <u>\$</u> <u>0</u> Hayden



DATE: 12/06/2023 8CALE: 22x34 |/4"+|'-0" |kr| |/8"+|'-0" PROJECT 9 528-TØTT DRAWN BY: EO CHECKED BY: JOEF

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

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REQUIRED BRACED WALL PANEL CONNECTIONS					
			REQUIRED CONNECTION		
METHOD	MATERIAL	MIN. THICKNESS	@ PANEL EDGES	@ INTERMEDIATE SUPPORTS	
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS	6d COMMON NAILS @ 12" O.C.	
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS… ⊕ 7° O.C.	5d COOLER NAILS** 9 7" O.C.	
W6P	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS	6d COMMON NAILS @ 12" O.C.	
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.6.4	PER FIGURE R602.10.6.4	
	"OR EQUIYALENT PER TABLE RT0235				

#### BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE WITH ALL LOCAL AND STATE AMENDMENTS
- WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND MAXIMUM WIND SPEEDS UP TO
- REFER TO ARCHITECTURAL PLAN FOR DOOR/IJINDOULOPENING SIZES
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R602.10.1
- TADLE ROUSIDI ALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED Ø FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.10.1.
- THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD (UNO). FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON
- FOR COMMISSION OFFICIALS (INCLUDING INFILL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS, FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION
- OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.

  10. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 12 FEET OF EACH END OF A BRACED WALL LINE
- THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 21 FEET.
- 12. MASONRY OR CONCRETE STEM WALLS W/ A LENGTH OF 48" OR LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.4.3 OF THE 2018 NCRC.

  13. BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN
- ACCORDANCE WITH SECTION R602/10/44

  14. BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602/10/45
- CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10.4.6
- 16. PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.1 (UNO)

GB = GYPSUM BOARD

WSP = WOOD STRUCTURAL PANEL 

### GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL
- BUILDING CODE WITH ALL LOCAL AND STATE AMENDMENTS.
  CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONTRACTOR SHALL
  COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM
- 3. CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING
- REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION PROPERTIES USED IN THE DESIGN ARE 45 FOLLOWS: PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS:
  MICROLLAM (LVL), F<sub>B</sub> = 1600 PSI, F<sub>1</sub> = 285 PSI, E = 1.9x10<sup>6</sup> PSI
  PARALLAM (PSI), F<sub>1</sub> = 2900 PSI, F<sub>2</sub> = 290 PSI, E = 125x10<sup>6</sup> PSI
  ALL WOOD MEMBERS SHALL BE '2 SYP UNLESS NOTED ON PLAN, ALL
  STUD COLUMNS AND JOISTS SHALL BE '2 SYP (WNO).
  ALL BEAM'S SHALL BE SUPPORTED WITH A (2) 2x4 '2 SYP STUD COLUMN
  AT EACH END UNLESS NOTED OTHERWISE.
  ALL REINFORCING STEEL SHALL BE GRADE 60 BAR'S CONFORMING TO
  ASTM AGE AND SAULK LUGGE A MINIMARY COURSE DE 31"

- ASTM AGIS AND SHALL HAVE A MINIMUM COVER OF 3".

  8. CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN
- PERPENDICULAR TO RAFTERS. FERT-ENDIQUAR TO PATIEND.

  FLITCH BEANS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH 1/2" DIA. THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/D3f.
- MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.

  10. ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP 72, DROPPED FOR NON-LOAD BEARING HEADERS EXCEEDING 8"-0" IN WIDTH AND/OR WITH MORE THAN 2"-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 6YP 12, DROPPED. (UNLESS NOTED OTHERWISE)

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DR HORTON
COMPLETED/REVISED ON 4/15/21. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY 4 TESTING, CLIENTO NOTIFI SUITH ENGINEERING, LABORATORY & 12911NG, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

### STRUCTURAL MEMBERS ONLY

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ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO
BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, INC. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

FIRST FLOOR BRACING (FT)

CONTINUOUS SHEATHING METHOD

11.6

BWL 1-1

BWL 1-2 BWL 1-A REQUIRED

PROVIDED

24.8

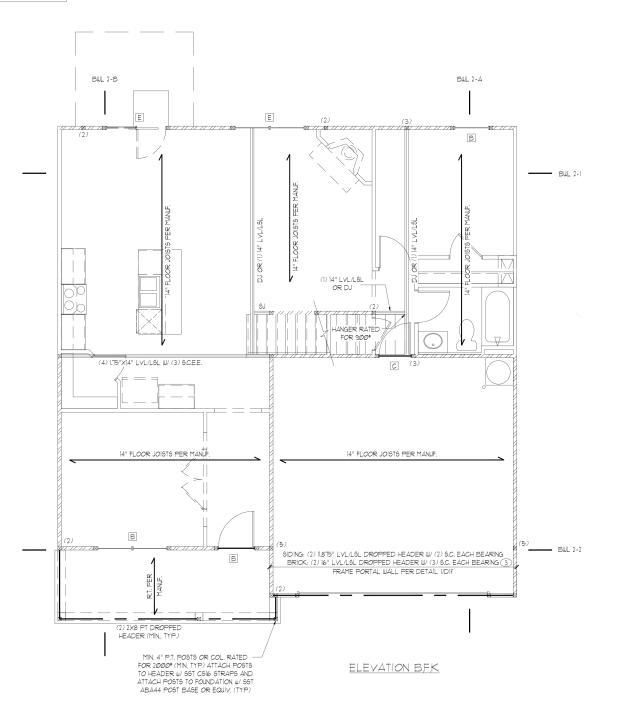
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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR FRAMING PLAN

SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"



HEA	HEADER SCHEDULE						
TAG	SIZE	JACKS (EACH END)					
А	(2) 2x6	(1)					
В	(2) 2x8	(2)					
С	(2) 2xlØ	(2)					
D	(2) 2x12	(2)					
E	(2) 9-1/4" LSL/LVL	(3)					
F	(3) 2x6	(1)					
G	(3)2x8	(2)					
H	(3) 2xlØ	(2)					
I	(3) 2xl2	(2)					

I. HEADER SIZES SHOWN ON PLANS ARE MINIMUMS, GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION.

2. ALL HEADERS TO BE DROPPED (UN.O.). 3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD COLUMNS LISTED ABOVE (UN.O.)

KING STUD	SCHEDULE
MAXIMUM HEADER SPAN	MINIMUM KING STUDS E.E.
4'-Ø"	(1)
6'-Ø"	(2)
8'-Ø"	(2)
10'-0"	(3)
12'-Ø"	(3)
14'-∅"	(3)
16'-0"	(4)
18'-∅"	(4)

WALL STUD SCHEDULE (10 FT HEIGHT)				
STUD SIZE	STUD SPACING (O.C.)			
	ROOF ONLY	ROOF € 1 FLOOR	ROOF & 2 FLOORS	NON-LOAD BEARING
2×4	24"	16"	12"	24"
2x6	24"	24"	16"	24"

1. BRACED WALLS STUDS SHALL BE A MAX. OF 16" O.C. 2. STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX OF 16" OC

3, TWO STORY WALLS SHALL BE FRAMED W/ 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED W/ HORIZ. BLOCKING @ 6'-0" O.C. VERTICALLY.

LINTEL SCHEDULE				
TAG	SIZE	OPENING SIZE		
①	L3x3x1/4"	LESS THAN 6'-0"		
2	L5x3x1/4"	6'-0" TO 10'-0"		
3	L5x3-1/2x5/16"	GREATER THAN 10'-0		
4	L5x3-1/2x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS		
SECURE LINITE	TO HEADER/ (2) 1/2"	DIAMETER LAG		

SCREWS STAGGERED @ 16" O.C. (TYP FOR ) ALL HEADERS WHERE BRICK IS USED, TO BE:

SHADED WALLS INDICATED LOAD BEARING WALLS

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

JOIST & BEAM SITES SHOUN ARE MINIMUMS BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

NOTE: MEMBERS NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDE! THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION.

INSTALL HOLD-DOWNS FOR BRACED WALL END CONDITIONS PER SECTION R602.10.8 & FIG. R602.10.7 OF THE 2018 NCRC.

NOTE: WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIET LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R602.3.5 OF THE 2018 NCRC.

SUMMIT ENGINEERING - LABORATORY - TESTIN
A Universal Engineering Sciences Compar
2520 Whitehall Park Dr. Suite 250
Charlotte, NC, 28273
Office: 704.504.1717
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www.summit-companies.com



DR Horton, Inc. 8001 Arrowridge E Charlotte, NC 2821;

Ω QĬ. rst



DATE: 12/06/2023 8CALE: 22x34 |/4"\*|"-@" |bdT |/8"\*|"-@" PROJECT 4 528-TØITT DRAWN BY: EO CHECKED BY: JOEF

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S3.1

REQUIRED BRACED WALL PANEL CONNECTIONS				
			REQUIRED (	CONNECTION
METHOD	MATERIAL	MIN. THICKNESS	@ PANEL EDGES	@ INTERMEDIATE SUPPORTS
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS	6d COMMON NAILS 8 12" O.C.
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS** ⊕ 1° O.C.	5d COOLER NAILS** @ 7" O.C.
WSP WOOD STRUCTURAL PANEL  PF WOOD STRUCTURAL PANEL		3/8"	6d COMMON NAILS	6d COMMON NAILS @ 12" O.C.
		7/16"	PER FIGURE R602.10.6.4	PER FIGURE R602.10.6.4
"OR FOLIVALENT PER TABLE RT02.35				

#### BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE WITH ALL LOCAL AND STATE AMENDMENTS
- WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND MAXIMUM WIND SPEEDS UP TO
- REFER TO ARCHITECTURAL PLAN FOR DOOR/JUNDOULOPENING SIZES
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R602.10.1
- ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED IØ FEET FOR ISOLATED PANEL METHOD AND IZ FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.10.1.
- THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD (UNO), FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES NCLUDING INFILL AREAS BETWEEN BRACED WALL
  PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS.

  9. FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION
- OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.

  10. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 12 FEET OF EACH END OF A BRACED WALL LINE.
- THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT
- 12. MASONRY OR CONCRETE STEM WALLS W/ A LENGTH OF 48" OR LESS SUPPORTING A 12. MASONRY OR CONCRETE STEM WALLS W/A LENGTH OF 48" OR LESS SUPPORTING A
  BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE
  R602.10.4.3 OF THE 2010 NCRC.

  13. BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN
  ACCORDANCE WITH SECTION R602.10.4.4

  14. BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN
  ACCORDANCE WITH SECTION R602.10.4.5

  COORDANCE WITH SECTION R602.10.4.5

- Ib. CRIPPLE WALLS AND WALK OUT BASETHENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.104.6
- PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.1 (UNO)
- 11 ABBREVIATIONS:

GB = GYPSUM BOARD

WSP = WOOD STRUCTURAL PANEL GB = GYP5UM BOARD

C5-XXX = CONT. SHEATHED

FF = PORTAL FRAME

WOT = WOOD O TRUCKINGLE | ASSETTED SOLUTION

FF = ROSTAL FRAME

FF-ENG = ENG. PORTAL FRAME

### GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL
- BUILDING CODE WITH ALL LOCAL AND STATE AMENDMENTS.

  CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONTRACTOR SHALL

  COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM
- 3 CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING
- REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION. PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS: PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS:
  MICROLLAM (LVL): F<sub>B</sub> = 2600 PSI, Fv = 238 PSI, E = 1.9x10<sup>6</sup> PSI
  PARALLAM (19L): F<sub>B</sub> = 2900 PSI, Fv = 239 PSI, E = 1.25x10<sup>6</sup> PSI
  ALL WOOD MEMBERS SHALL BE 12 SYP UNLESS NOTED ON PLAN, ALL
  STUD COLUMNS AND JOISTS SHALL BE 12 SYP (UND).
  ALL BEAYIS SHALL BE SUPPORTED WITH A (2) 2x4 12 SYP STUD COLUMN
  AT EACH END UNLESS NOTED OTHERWISE.
  ALL RENFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO
  ASTMACKER AND SHALL USES AND MISSING COLUMN COLUMN.

- ASTM A615 AND SHALL HAVE A MINIMUM COVER OF 3".

  8. CONTRACTOR TO PROVIDED LOCKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- PERPENDICULAR 10 KAPIERS.

  9. FLITCH BEAMS, 4-PLY LYLS AND 3-PLY SIDE LOADED LYLS SHALL BE BOLTED TOSETHER WITH 1/2" DIA, THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/D3f. MIN EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.

  10. ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP 12,
- DROPPED FOR NON-LOAD BEARING HEADERS EXCEEDING 8"-0" IN WIDTH AND/OR WITH MORE THAN 2"-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 6YP 12, DROPPED. (UNLESS NOTED OTHERWISE)

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DR HORTON
COMPLETED/REVISED ON 4/15/21. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY & TESTING, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

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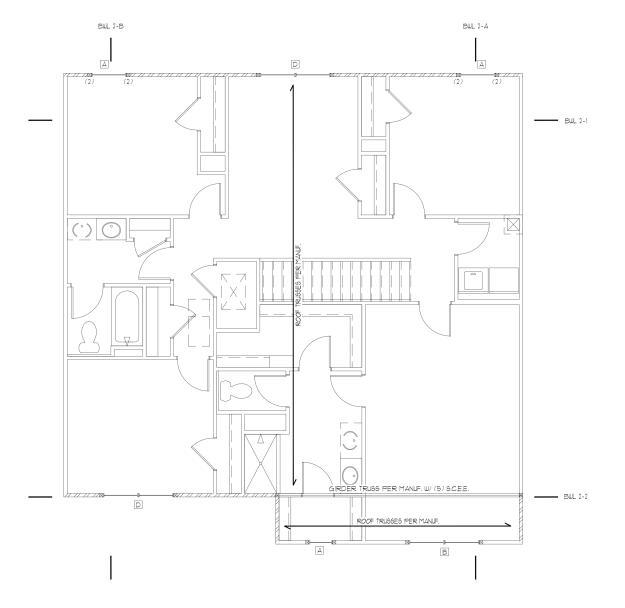
ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO
BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, INC FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

SECOND FLOOR FRAMING PLAN

SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"





ELEVATION B.F.K.

HE	JLE	
TAG	SIZE	JACKS (EACH EN
Д	(2) 2x6	(1)
В	(2) 2x8	(2)
С	(2) 2x1Ø	(2)
D	(2) 2x12	(2)
E	(2) 9-1/4" LSL/LVL	(3)
F	(3) 2x6	(1)
G	(3) 2x8	(2)
Н	(3) 2xlØ	(2)
	(3) 2xl2	(2)

NOTES: I. HEADER SIZES SHOWN ON PLANS ARE MINIMUMS, GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION. 2. ALL HEADERS TO BE DROPPED (UN.O.). 3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD

COLUMNS LISTED ABOVE (UN.O.).

KING STUD	SCHEDULE
MAXIMUM HEADER SPAN	MINIMUM KING STUDS E.E.
4'-Ø"	(1)
6'-0"	(2)
8'-0"	(2)
10'-0"	(3)
12'-Ø"	(3)

WALL STUD SCHEDULE (10 FT HEIGH				
STUD SIZE STUD SPACING (O.C.)				
	ROOF ONLY	ROOF \$ 1 FLOOR	ROOF & 2 FLOORS	NON-LOAD BEARING
2×4	24"	16"	12"	24"
2x6	24"	24"	16"	24"
	STUD SIZE	STUD SIZE  ROOF ONLY  2x4 24"	STUD SIZE	STUD SIZE   STUD SPACING (O.C.)   ROOF ONLY   ROOF 4   ROOF 4   1 FLOOR 2 FLOORS   2 FLOORS   12"

L BRACED WALLS STUDS SHALL BE A MAX. OF 16" O.C. 2. STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX OF 16" OC

3, TWO STORY WALLS SHALL BE FRAMED W/ 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED W/ HORIZ. BLOCKING @ 6'-0" O.C. VERTICALLY.

	LINTEL SCHED	DULE	
TAG	SIZE	OPENING SIZE	
①	L3x3x1/4"	LESS THAN 6'-0"	
2	L5x3x1/4"	6'-0" TO 10'-0"	
3	L5x3-1/2x5/16"	GREATER THAN 10'-0	
4	L5x3-1/2x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS	
SECURE LINTEL TO HEADER #/ (2) 1/2" DIAMETER LAG			

SCREWS STAGGERED @ 16" O.C. (TYP FOR 3) ALL HEADERS WHERE BRICK IS USED, TO BE:

SHADED IIIALLS INDICATED LOAD BEARING IIIALLS

JOIST & BEAM SIZES SHOWN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: MEMBERS NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDE THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION.

INSTALL HOLD-DOWNS FOR BRACED WALL END CONDITIONS PER SECTION R602:10.8 4 FIG. R602:10.7 OF THE 2018 NCRC.

NOTE: WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLET LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R60235 OF THE 2018 NCRC.





DR Horton, Inc. 8001 Arrowridge E Charlotte, NC 2821;

QĬ. Floor  $\sigma$ Hayden RH Secon



DATE: 12/06/2023 8CALE: 22x34 |/4"=|"-@" |bd7 |/8"=|"-@" PROJECT 9 528-TØTT DRAWN BY: EO CHECKED BY: JOEF

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S4.1

TRUSS UPLIFT CONNECTOR SCHEDULE					
MAX. UPLIFT ROOF TO WALL FLOOR TO FLOOR FLOOR					
600 LBS	ØLBS H2.5A PER WALL SHEATHING & FASTENE				
1200 LBS	(2) H2.5A	CSI6 (END = II")	DTT2Z		
1450 LBS	HTS2Ø	CSI6 (END = II")	DTT2Z		
2000 LBS	(2) MT52Ø	(2) CSI6 (END = II")	DTT2Z		
2900 LBS	(2) HTS2Ø	(2) CSI6 (END = II")	HTT4		
3685 LBS	LGT3-5D52.5	MSTC52	HTT4		

JOBB LBS LEGIS-505/5 MSICS2

1. ALL PRODUCTS LISTED ARE SIMPSON STRONG-TIE. EQUIVALENT PRODUCTS MAY BE USED PER MANUFACTURER'S SPECIFICATIONS.

2. UPLIFT VALUES LISTED ARE FOR STP TO GRADE MEMBERS.

3. REFER TO TRUISS LATOUT PER MANUF, FOR UPLIFT VALUES AND TRUSS TO TRUSS CONNECTIONS. CONNECTOR'S SPECIFIED BY TRUSS MANUFACTURER OVERRIDE THOSE LISTED ABOVE.

4. CONTACT SUMMIT FOR REQUIRED CONNECTOR'S UHEN LOADS EXCEED THOSE LISTED ABOVE.

NOTE: 19T PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, WNO)

NOTE: ROOF TRUSSES SHALL BE SPACED TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

REFER TO DETAIL 5/D3F FOR EYEBROW, RETURN OR SHED ROOF FRAMING REQUIREMENTS, (TYP FOR ROOFS PROTRUDING MAXIMUM 24" FROM STRUCTURE)

NOTE: TRUSS UPLIFT LOADS SHALL BE DETERMINED PER TRUSS MANUFACTURER IN ACCORDANCE WITH SECTION REGILIL, WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION REGILIS OF THE 2018 NCRC, REFER TO BRACED WALL PLANS FOR SHEATHING AND FASTENER REQUIREMENTS.

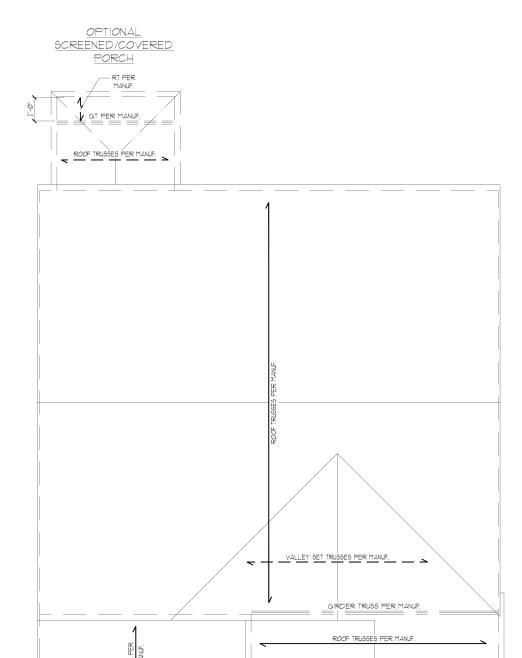
THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL, PLANS PROVIDED BY DR. HORTON COMPLETED/REVISED ON A/B/2/. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY & TESTING, INC., IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

## STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, INC. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

ROOF FRAMING: PLAN
9CALE: 1/4":1"-0" ON 22"×34" OR 1/0":1"-0" ON 11"x11"









DR Horton, Inc. 8001 Arrowridge Blvc Charlotte, NC 28213

Hayden RH Roof Framing Plan



22869 5/5/9

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S5.1

## Applicable Building Codes:

• 2018 North Carolina Residential Building Code with All Local Amendments ASCE 7-10: Minimum Design Loads for Buildings and Other Structures

# Design Loads:

sign Loa	105:	
1. ₹	Roof Live Loads	
	I.I. Conventional 2x	20 PSF
	1.2. Truss	20 PSF
	1.2.1. Attic Truss	60 PSF
2. ₹	Roof Dead Loads	
	2.l. Conventional 2x1	Ø PSF
	2.2. Truss2	20 PSF
3. S	now1	5 PSF
	3.l. Importance Factor1	.Ø
4. F	loor Live Loads	
	4.1. Typ. Dwelling	4Ø PSF

4.2. Sleeping Areas ... 40 PSF 4.3. Decks . 4.4. Passenger Garage .. .50 PSF 5. Floor Dead Loads 5.1. Conventional 2x 5.2. I-Joist .... . 15 PSF

MEAN ROOF HT.	UP TO 30'	3Ø'1"-35'	35'1"-40'	40'1"-45'
ZONE 1	16.7,-18.0	17.5,-18.9	18.2,-19.6	18.7,-20.2
ZONE 2	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 3	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 4	18.2,-19.0	19.2,-20.0	19.9,-20.7	20.4,-21.3
ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.1	20.4,-26.9

## 8 Seismic

Seismi	C	
8.1.	Site Class	D
8.2.	Design Category	C
	Importance Factor	1.0
8.4.	Seismic Use Group	1
8.5.	Spectral Response Acceleration	

8.5.1. Sms = %a 8.5.2. Sml = %a 8.6. Seismic Base Shear

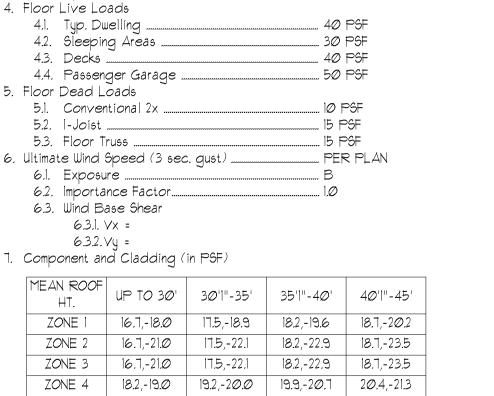
8.6.1. Vx = 8.7. Basic Structural System (check one)

9. Assumed Soil Bearing Capacity .....

 Bearing Wall
 ■ ☐ Building Frame

☐ Moment Frame ☐ Dual w/ Special Moment Frame ☐ Dual w/ Intermediate R/C or Special Steel

□ Inverted Pendulum 8.8. Arch/Mech Components Anchored. 8.9. Lateral Design Control: Seismic 🗆 Wind 🖂





STRUCTURAL PLANS PREPARED FOR:

# STANDARD DETAILS

PROJECT ADDRESS:

DR Horton Carolinas Division 8001 Arrowridge Blvd Charlotte, NC 28273

ARCHITECT/DESIGNER: GMD Design Group 1845 Satellite Blvd. Duluth, GA 30097

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory & Testing, P.C. before construction begins.

## PLAN ABBREVIATIONS:

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	SC	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
DSP	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
EE	EACH END	SYP	SOUTHERN YELLOW PINE
ΕW	EACH WAY	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
<i>0</i> C	ON CENTER	TYP	TYPICAL
PSF	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
PSI	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by <u>DR Horton</u>, <u>Inc.</u> Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify SUMMIT immediately.

## SHEET LIST:

Sheet No.	Description
CS1	Cover Sheet, Specifications, Revisions
Dlm	Monolithic Slab Foundation Details
Dls	Stem Wall Foundation Details
Dic	Crawl Space Foundation Details
Dlb	Basement Foundation Details
DIf	Framing Details

# DR HORTON PROJECT SIGN-OFF:

Manager	Signature
Operations	
Operations System	
Operations Product Development	

REVISION LIST:

Revision No.	Date	Project No.	Description
1	5.11.17		Added box bay detail (2/D2f). Added deck options with basement. Revised deck options with stem wall and crawl space foundations
2	T.12.1T		Revised stem wall insulation note.
3	2.15.18		Revised garage door detail, NC only
4	2.28.18		Added high-wind foundation details
5	12.19.18		Revised per 2018 NCRC
6	2.19.19		Revised per Mecklenburg County Comments
٦	3.1.19		Revised stem wall deck attachment and roof sheathing on wall sections.
8	3.6.19		Corrected dimensions at perimeter footings
9	3.2.20		Added tall turndown detail
100	3.18.20		Added balloon framing detail
11	10.20.20		Added alternate two-pour detail for slab and added note for crawl girder above grade
12	3.1.21		Added OX-16 Standard Details
13	5.18.21		Updated OX-19 Standard Details
14	Ø2.14.23		Added 4/D2m - Tall Slab Detail w/ Siding

Revision No.	Date	Project No.	Description
1	5.11.17		Added box bay detail (2/D2f). Added deck options with basement. Revised deck options with
			stem wall and crawl space foundations
2	T.12.1T		Revised stem wall insulation note.
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5	12.19.18		Revised per 2018 NCRC
6	2.19.19		Revised per Mecklenburg County Comments
7	3.1.19		Revised stem wall deck attachment and roof sheathing on wall sections.
8	3.6.19		Corrected dimensions at perimeter footings
9	3.2.2Ø		Added tall turndown detail
10	3.18.20		Added balloon framing detail
11	10.20.20		Added alternate two-pour detail for slab and added note for crawl girder above grade
12	3.1.21		Added OX-15 Standard Details
13	5.18.21		Updated OX-IS Standard Details
14	Ø2.14.23		Added 4/D2m - Tall Slab Detail w/ Siding
			-

- GENERAL STRUCTURAL NOTES: The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT
- shall be considered the same entity. 2. The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction
- to stabilize the structure. 3. The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur
- 4. Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.
- 5. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.
- 6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- 7. This structure and all construction shall conform to all applicable sections of the international residential code. 8. This structure and all construction shall conform to all
- applicable sections of local building codes. 9. All structural assemblies are to meet or exceed to requirements of the current local building code.

# FOUNDATIONS:

The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.

- 2. The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
- 3. Any fill shall be placed under the direction or recommendation of a licensed professional engineer. 4. The resulting soil shall be compacted to a minimum of 95%
- maximum dry density. 5. Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
- 6. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.

- STRUCTURAL STEEL: Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design" latest editions.
- Structural steel shall receive one coat of shop applied rust-inhibitive paint.
- 3. All steel shall have a minimum yield stress  $(F_{ij})$  of 36 ksi unless otherwise noted.
- 4. Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D.I. Electrodes for shop and field welding shall be class ETØXX. All welding shall be performed by a certified welder per the above standards.

- Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- 2. Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- 3. Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows: 3.1. Footings: 5%

# 3.2.Exterior Slabs: 5%

4. No admixtures shall be added to any structural concrete without written permission of the SER.

- 5. Concrete slabs-on-grade shall be constructed in accordance with ACI 302.IR-96: "Guide for Concrete Slab and Slab Construction".
- 6. The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions.
- 7. Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted.
- 8. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished

supported during the concrete pour.

9. Reinforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut joint. 10. All welded wire fabric (W.W.F.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The W.W.F. shall be securely

# CONCRETE REINFORCEMENT:

- Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- 2. Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- 3. Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard) 4. Fibermesh shall comply with ASTM CIII6, any local building code requirements, and shall meet or exceed the current industry
- 5. Steel reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- 6. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures" Horizontal footing and wall reinforcement shall be continuous

and shall have 90° bends, or corner bars with the same

size/spacing as the horizontal reinforcement with a class B tension splice. 8. Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.

- 9. Where reinforcing dowels are required , they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters
- 10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.
- WOOD FRAMING: 1. Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Spruce-Yellow-Pine (SYP) #2.
- 2. LVL or PSL engineered wood shall have the following minimum design values:
  - 2.1. E = 1,9*00,000* psi  $2.2.\,F_{\rm b} = 2600\,$  psi  $2.3.F_{V} = 285 \text{ psi}$
- 2.4.Fc = 700 psi 3. Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance with AWPA standard C-2
- 4. Nails shall be common wire nails unless otherwise noted. 5. Lag screws shall conform to ANSI/ASME standard B18.2.1-1981. Lead holes for lag screws shall be in accordance with NDS
- specifications. 6. All beams shall have full bearing on supporting framing members
- 7. Exterior and load bearing stud walls are to be 2x4 SYP #2 @ 16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- 8. Individual studs forming a column shall be attached with one 10d nail @ 6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfer.

9. Multi-ply beams shall have each ply attached with (3) 10d nails a

10. Flitch beams, 4-ply beams and 3-ply side loaded beams shall be bolted together with (2) rows of 1/2" diameter through bolts staggered @ 16" O.C. unless noted otherwise. Min. edge distance shall be 2" and (2) bolts shall be located a min. 6" from each end of the beam.

# WOOD TRUSSES:

- l. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- 2. The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-05), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to
- 3. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- 4. The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
- 5. Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

# EXTERIOR WOOD FRAMED DECKS:

 Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

# WOOD STRUCTURAL PANELS:

- Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA
- 2. All structurally required wood sheathing shall bear the mark of the APA.

- 3. Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Wood floor sheathing shall be APA rated sheathing exposure I or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T&G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- 6. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

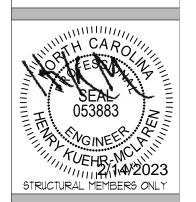
# STRUCTURAL FIBERBOARD PANELS:

Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards. 2. All structurally required fiberboard sheathing shall bear the

- mark of the AFA. 3. Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more
- 4. Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

SUMMIT 120 PENMARC DR., SUITE 108 RALEIGH, NC 27603 OFFICE: 919.380.9991 FAX: 919.380.9993

WWW.SUMMIT-COMPANIES.COM



DATE: 02/14/2023 SCALE: 22x34 |/4"=1'-0" ||x|T |/8"=1'-0" PROJECT \*: 528-06R DRAWN BY: JCEF CHECKED BY: BCP

> PRIGINAL INFORMATION PROJECT \*

> > REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

UNDISTURBED SOIL

6A COVERED PATIO DETAIL

STANDARD - BRICK

CHARTS

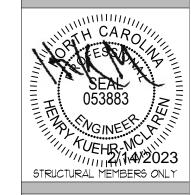
STANDARD - SIDING

6 PATIO SLAB DETAIL

SUMMIT 120 PENMARC DR., SUITE 108 RALEIGH, NC 27603 OFFICE: 919.380.9991 FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM

& Testing, Inc.

CLIENT: DR Horton Carolina Divis 8001 Arrowridge Blvd. Charlotte, NC 28273



DRAWING DATE: Ø2/14/2Ø23 PROJECT \*: 528-06R DRAWN BY: JCEF CHECKED BY: BCP

ORIGINAL INFORMATION

4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR

5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL

AMENDMENTS AND REQUIREMENTS NOT SHOWN

CONNECTIONS

BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND

6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE

ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

Dm

PER PLAN CONTINUOUS

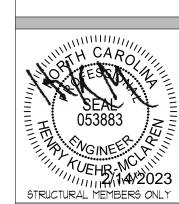
4 TALL SLAB DETAIL W/ SIDING

1. REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET

- FOR ADDITIONAL INFORMATION. 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE. 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.
- 4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND CONNECTIONS
- 5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN
- 6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC

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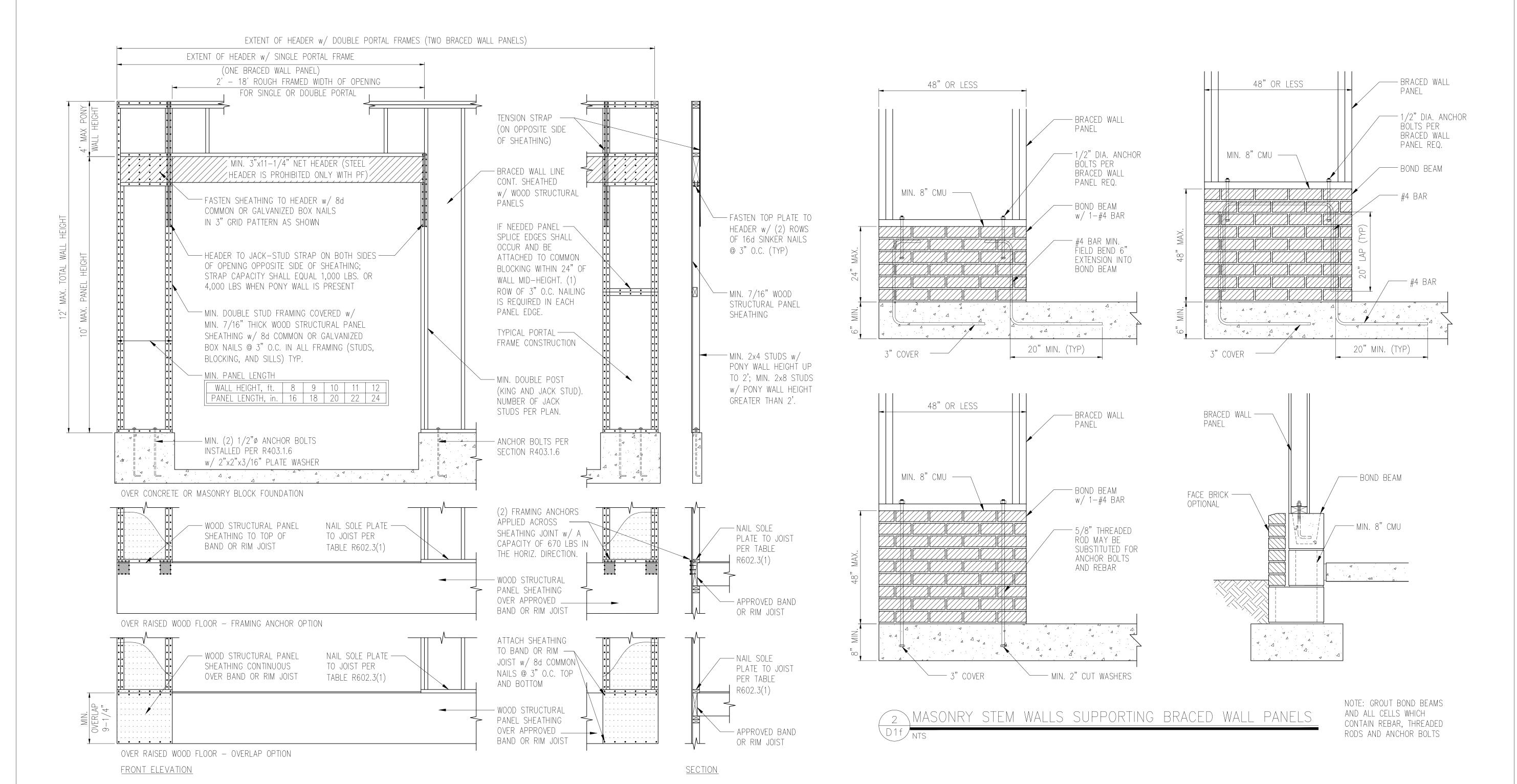


DRAWING DATE: Ø2/14/2Ø23 PROJECT \*: 528-06R DRAWN BY: JCEF CHECKED BY: BCP

> ORIGINAL INFORMATION PROJECT • DATE 1/31/2017

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

D2m







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& Testing, Inc. No. F-1454

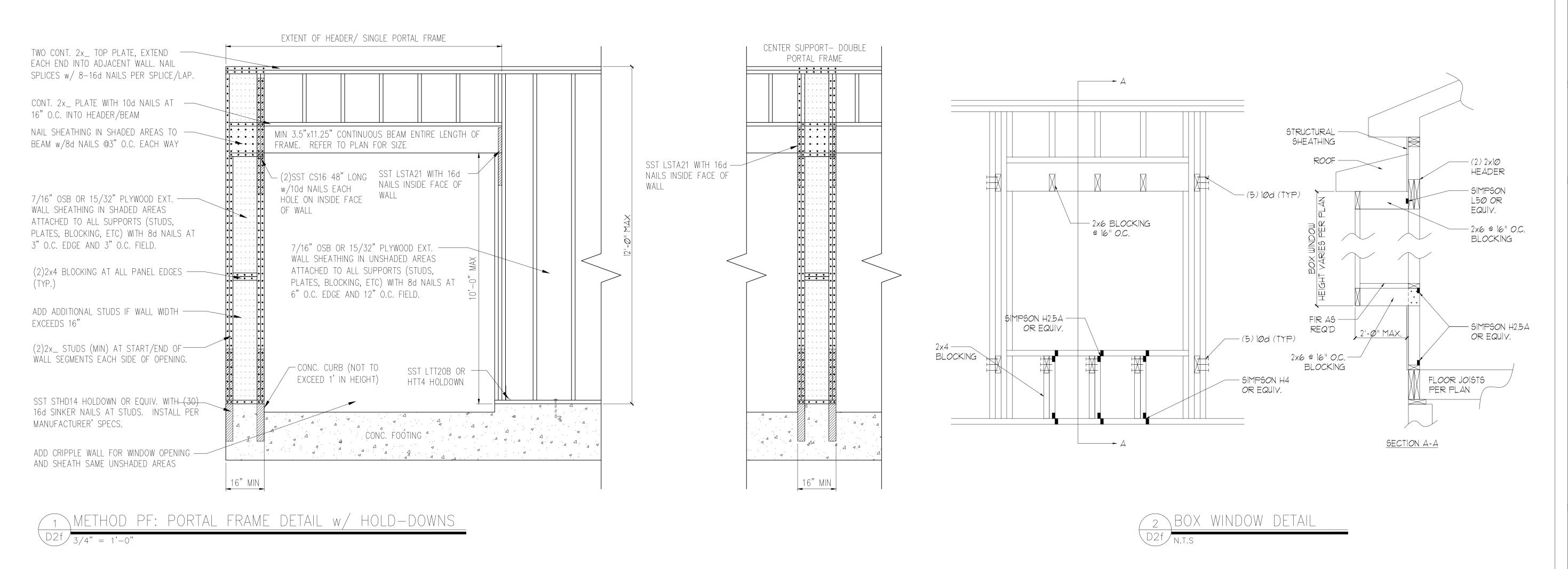


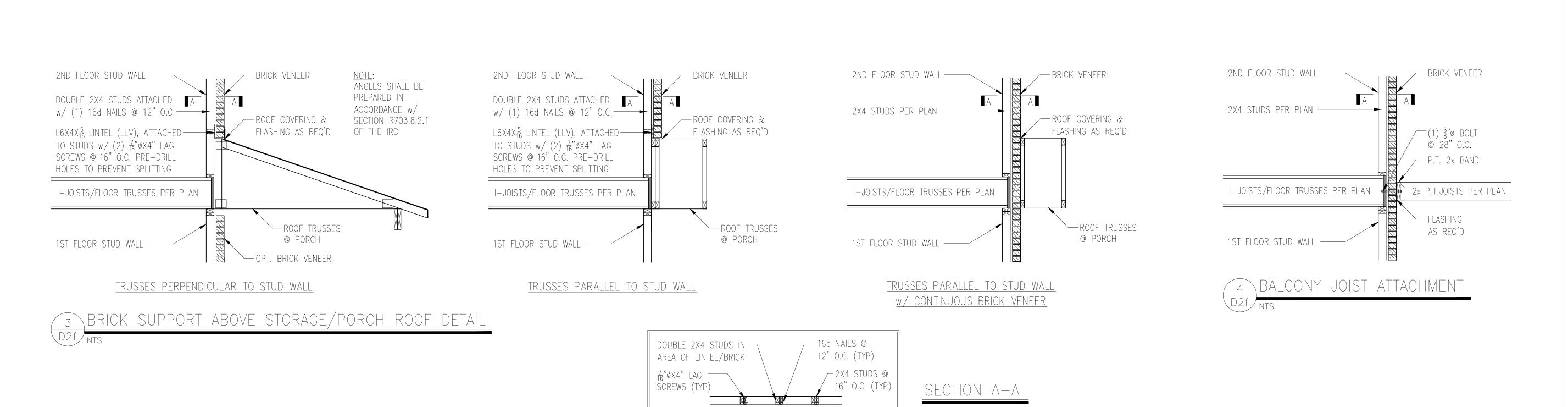
DRAWING DATE: Ø2/14/2Ø23 SCALE: 22x34 1/4"=1'-0" ||x|7 ||/8"=1'-0" PROJECT \*: 528-06R DRAWN BY: JCEF CHECKED BY: BCP

ORIGINAL INFORMATION

PROJECT • DATE 1/31/2017

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

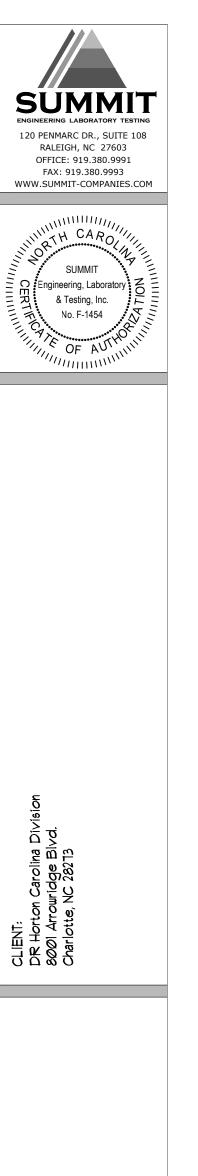




 $L6X4X_{\overline{16}}^{5}$  LINTEL (LLV), ATTACHED

\_ TO STUDS w/ (2)  $\frac{7}{16}$  "øx4" LAG SCREWS @ 16" O.C. PRE-DRILL

HOLES TO PREVENT SPLITTING



ille (0x-15) □@ţâ∐s

 $\bigcirc$ 

STRUCTURAL MEMBERS ONLY

9CALE: 22x34 |/4"=1'-@" ||x|1 |/6"=1'-@" |PROJECT \*: 528-06R

PROJECT • DATE 1/31/2017

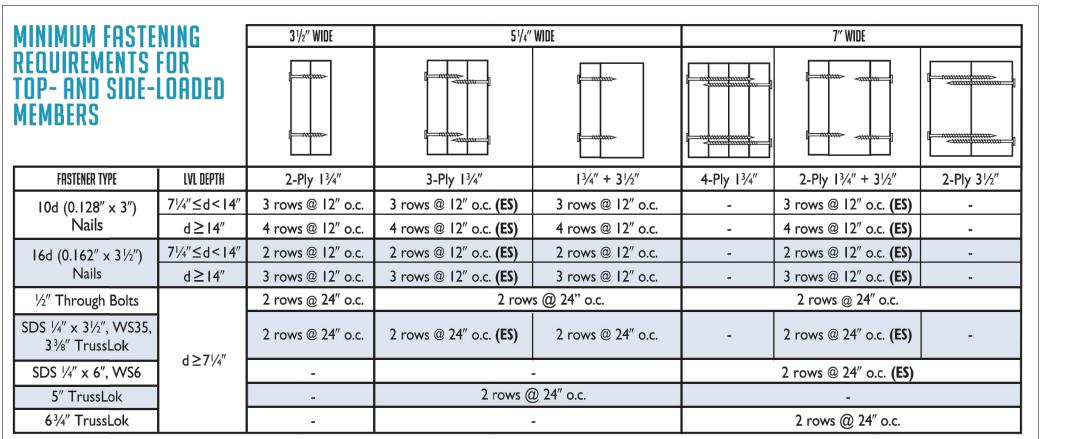
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

DRAWING

DATE: Ø2/14/2Ø23

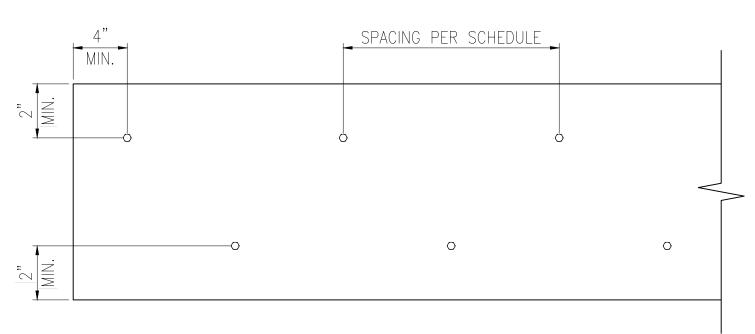
DRAWN BY: JCEF
CHECKED BY: BCP

ORIGINAL INFORMATION



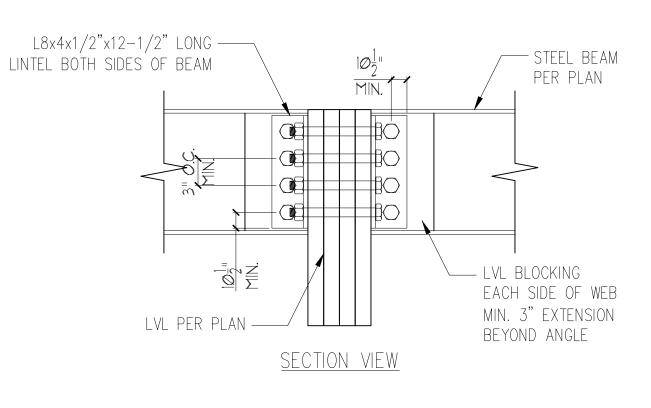
## NOTES:

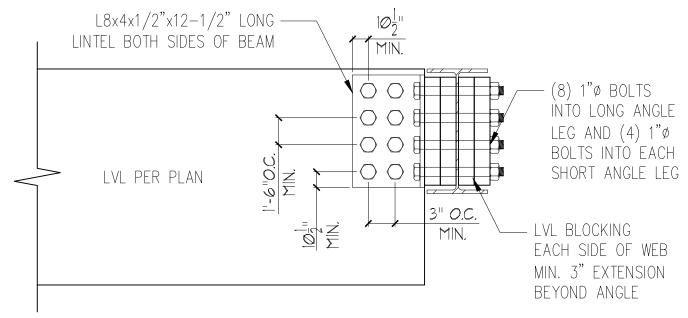
- I.All fasteners must meet the minimum requirements in the table above. Side-loaded multiple-ply members must meet the minimum fastening and side-loading capacity requirements given on page 48.
- 2. Minimum fastening requirements for depths less than  $7\frac{1}{4}$ " require special consideration. Please contact your technical representative.
- 3. Three general rules for staggering or offsetting for a certain fastener schedule:
- (1) if staggering or offsetting is not referenced, then none is required;(2) if staggering is referenced, then fasteners installed in adjacent rows on the front side are to be staggered up to one-half the o.c. spacing, but maintaining the fastener
- clearances above; and
  (3) if "ES" is referenced, then the fastener schedule must be repeated on each side, with the fasteners on the back side offset up to one-half the o.c. spacing of the front side (whether or not it is staggered).



ELEVATION VIEW

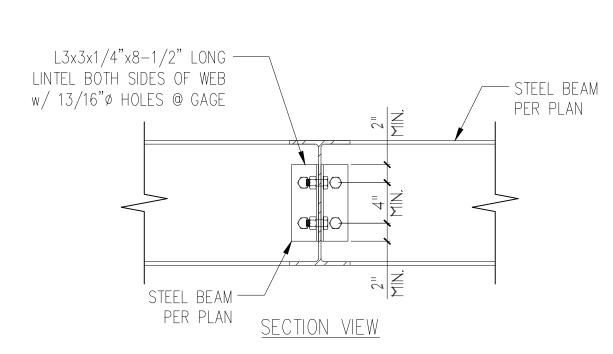
# MULTI-PLY BEAM CONNECTION DETAIL

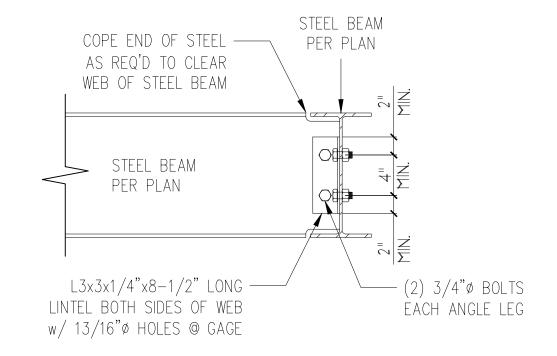




ELEVATION VIEW

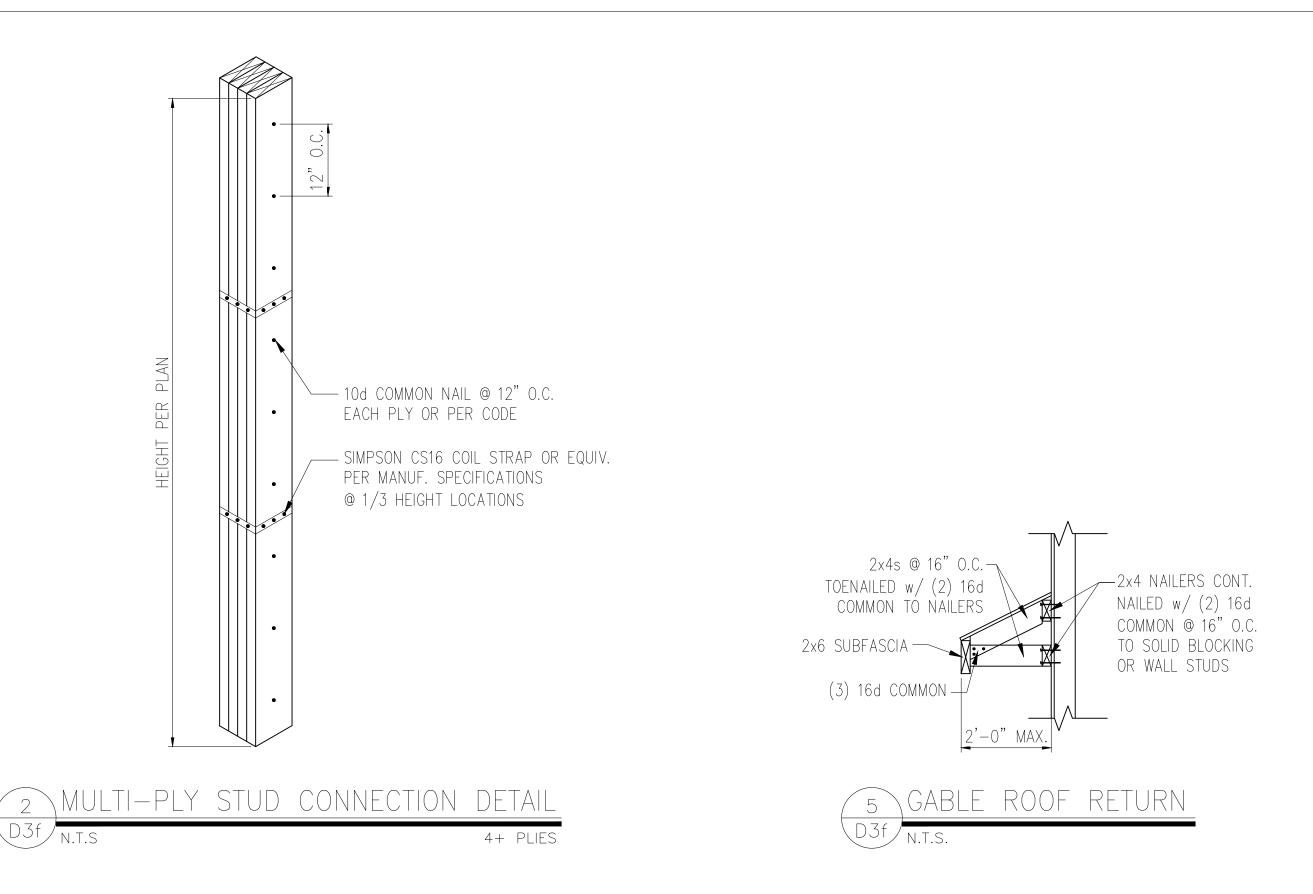


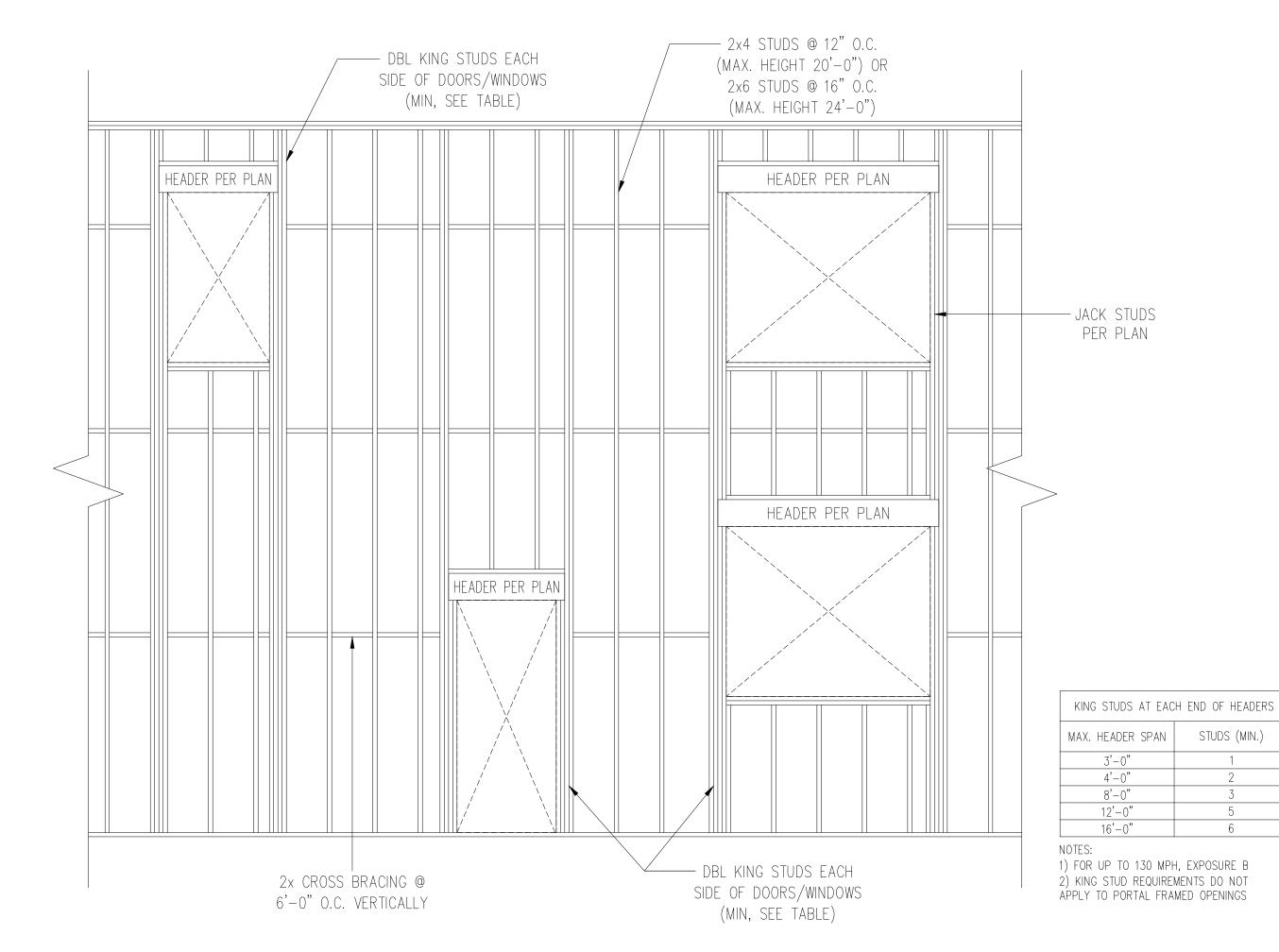




ELEVATION VIEW



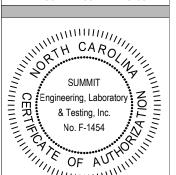




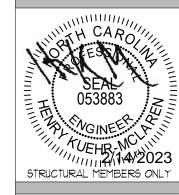
6 TYP. BALLOON FRAMING DETAIL

D3f N.T.S





CLIEN!: DR Horton Carolina Division 8001 Arrowridge Blvd. Charlotte, NC 28273



DRAWING

DATE: 02/14/2023

SCALE: 22x34 |/4"=|'-0"
||x|T |/8"=|'-0"

PROJECT \* 528-06R

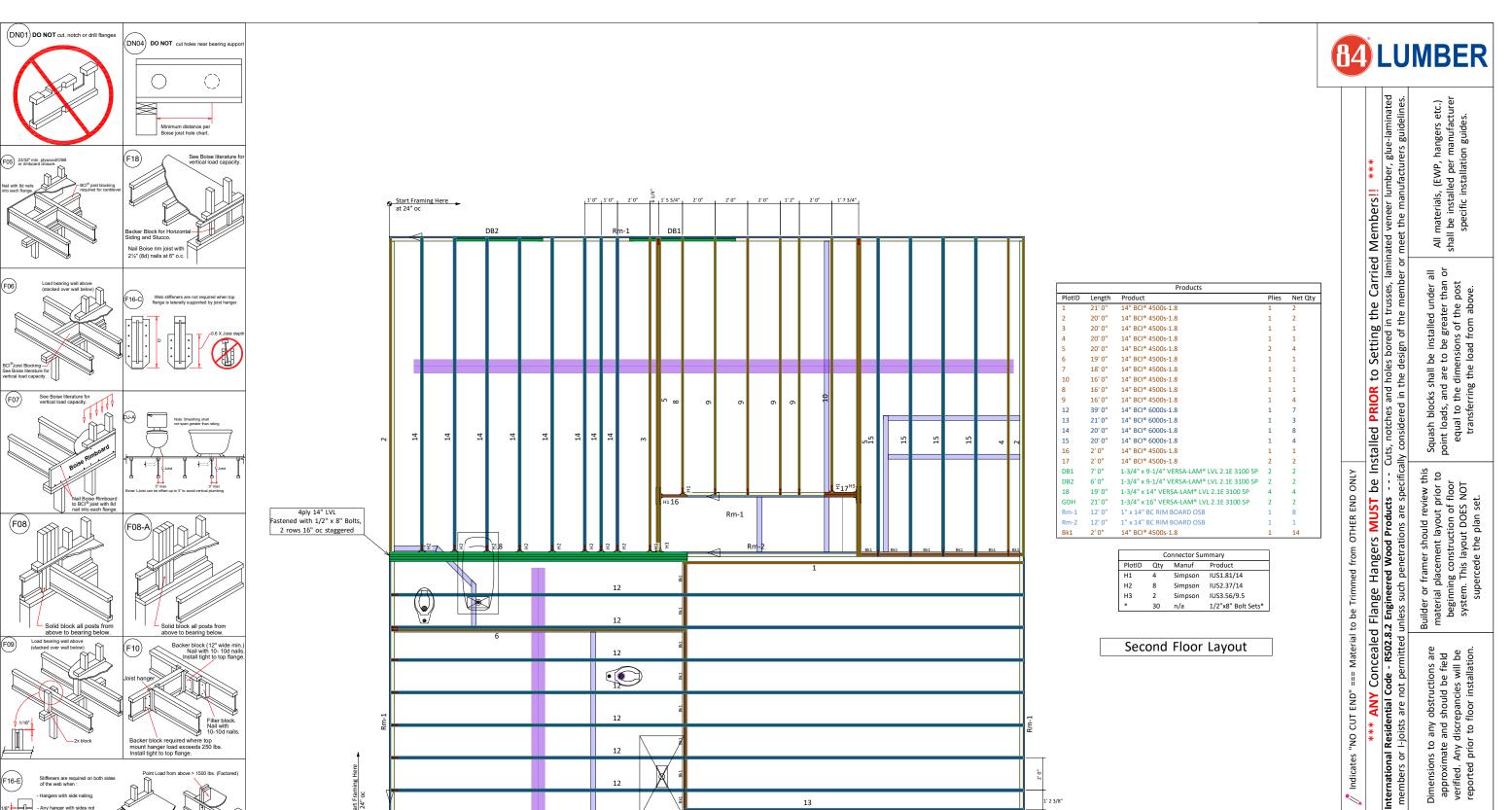
DRAWN BY: JCEF

CHECKED BY: BCP

1/31/2Ø11

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

D3f



13

13

13

GDH

12

12

Backer block required where top mount hanger load exceeds 250 lbs. Install tight to top flange.

Install web stiffeners tight against top flange with 1/8'

Start Framing at 24" oc

2x block

- Hangers with side nailing.

- Web stiffener nailed with 3 - 3" (10d) nails for 9 1/2" and 11 7/8" joists, and 5 - 3" (10d) nails for 14" & 16" joists.

Double BCI®Joist Connection

Connection valid for all applications.
Contact Boise EWP Engineering for specific conditions

F16-E)

F58-B

Dimensions to any obstructions are approximate and should be field verified. Any discrepancies will be reported prior to floor installation.

# Plan Information

Lot Number: 115 Woodgrove Model: Hayden F

Builder: DR Horton Express

Boise BC FRAMER II / SAPPHIRE Structure

Plan Date: 01/22/21

Structural Date: 11/24/21

Not To Scale By: CK

Indicates '

