

Run: 8.62 S Sep 22 2022 Print: 8.620 S Sep 22 2022 MiTek Industries, Inc. Wed Jan 24 12:28:16

Page: 1 ID:jZpBnqoT4xUcdSaN3tVreOzsDtb-nqoAjqGo336IB\_Lh3?Jnjv4yprOioDuoYZ56\_mzs9TD 6-9-8 13-3-8 19-9-8 26-7-0 6-9-8 6-6-0 6-6-0 6-9-8 5x6= 5 7<sup>12</sup> 5x6 🗸 5x6<sub></sub> 6 8-7-4 5x4 5x4 -3

Plate Offsets (X, Y): [2:0-5-10,Edge], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [8:0-5-10,Edge], [8:0-0-0,0-0-0]

3x8 ı

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	-0.30	10-12	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.83	Vert(CT)	-0.44	10-12	>721	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.23	Horz(CT)	0.06	8	n/a	n/a		
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MSH							Weight: 135 lb	FT = 20%

21

17-7-8

8-8-0

2211

3x6=

10

3x4=

26-7-0

8-11-8

Structural wood sheathing directly applied or 3-7-11 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

12

3x4=

LUMBER **BRACING** 

8-11-8

8-11-8

TOP CHORD TOP CHORD 2x4 SP No.2 BOT CHORD **BOT CHORD** 2x4 SP No.2 2x4 SP No.3

SLIDER Left 2x4 SP No.3 -- 1-11-0, Right 2x4 SP No.3 -- 1-11-0

REACTIONS 2=1116/0-3-8, (min. 0-1-8), 8=1116/0-3-8, (min. 0-1-8) (lb/size)

2=-160 (LC 8) Max Horiz

Max Uplift 2=-35 (LC 10), 8=-35 (LC 11)

**FORCES** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD  $2\text{-}3\text{-}606/0, 3\text{-}4\text{-}-1516/136, 4\text{-}5\text{-}-1380/185, 5\text{-}6\text{-}-1379/185, 6\text{-}7\text{-}-1515/136, 7\text{-}8\text{-}-529/0}$ **BOT CHORD** 2-12=-181/1333, 12-21=0/903, 21-22=0/903, 11-22=0/903, 10-11=0/903, 8-10=-21/1241

4-12=-342/178. 5-12=-66/607. 5-10=-66/606. 6-10=-341/178

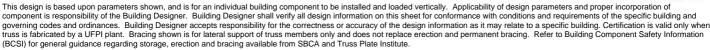
# WFBS NOTES

WEBS

- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 2 and 35 lb uplift at joint 8. 5)
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/ 6) TPI 1.



3x8 II







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 $ID: QUPztFwkj?kBq?Lle\_gB2VzsDtR-nqoAjqGo336IB\_Lh3?Jnjv40urRso41oYZ56\_mzs9TD$ 4-5 13-3-8 19-3-14 26-7-0 13-3-8 4-10-10 7-3-2 3x6= 13 12 14 712 5x6= 11 15 10 18 3x6 -3x6 9 20 7 <sup>8</sup> 22 23 3-9-7 3x4s 24 JAW2 25 ψ̈́ 26 **⊠**29 28 27 39 38 37 36 35 34 33 32 31 30 3x6= 1.5x3 ı 3x6 ı 20-1-8 13-9-12 26-7-0 13-9-12 6-3-12 6-5-8 [13:0-3-0,Edge], [23:0-2-15,Edge] Plate Offsets (X, Y):

DEFL

Vert(LL)

Vert(CT)

Horz(CT)

0.35

0.63

0.79

BOT CHORD

JOINTS

in

0.07

-0.13

0.02

(loc)

27-43

27-43

1 Brace at Jt(s): 17, 21

25

I/defI

>999

>999

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

L/d

240

180

n/a

CSI

вс

Matrix-MSH

2-0-0

1.15 TC

1.15

YES WB

IRC2015/TPI2014

LUMBER **BRACING** TOP CHORD 2x4 SP No.2 \*Except\* T3:2x4 SP No.3 TOP CHORD

**BOT CHORD** 2x4 SP No.2 2x4 SP No.3 WEBS

**OTHERS** 2x4 SP No.3 \*Except\* ST9:2x4 SP No.1

**SLIDER** Right 2x4 SP No.3 -- 1-11-0

REACTIONS All bearings 13-11-8, except 25=0-3-8, 30=0-3-8

> (lb) - Max Horiz 40=-172 (LC 8)

(psf)

20.0

10.0

0.0

10.0

Spacing

Code

Plate Grip DOL

Rep Stress Incr

Lumber DOL

Max Uplift All uplift 100 (lb) or less at joint(s) 25, 30, 32, 33, 34, 35, 36, 37, 40 except 31=-336 (LC 22), 39=-105 (LC 10)

Max Grav All reactions 250 (lb) or less at joint(s) 31, 32, 33, 34, 35, 36, 37, 38, 39,

40 except 25=587 (LC 1), 30=815 (LC 1)

**FORCES** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

23-24=-534/84, 24-25=-252/0, 16-17=-525/130, 17-19=-527/126, 19-21=-502/115, 21-23=-489/110 TOP CHORD BOT CHORD

28-29=0/461, 27-28=0/461, 25-27=0/461

WEBS 16-29=-511/120, 23-27=0/257

### NOTES

Loading

TCDL

BCLL

BCDI

TCLL (roof)

- Unbalanced roof live loads have been considered for this design. 1)
- Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; Cat. II; Exp B; Enclosed; MWFRS (envelope) 2) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS
- for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Truss designed for wind loads in the plane of the truss only. 3)
- 4) All plates are 2x3 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 1-4-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 40, 25, 32, 33, 34, 35, 36, 37, 30
- except (it=lb) 31=336, 39=104 9)
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/ 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

PRE

PLATES

Weight: 192 lb

MT20

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end

GRIP

244/190

FT = 20%

This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute





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B1

26-7-0

9-1-12

Structural wood sheathing directly applied or 2-2-0 oc purlins

5x6 =

22

Page: 1

ID:BIMa\_Ap5rEcTFc9acb04AbzsDta-nqoAjqGo336IB\_Lh3?Jnjv4t3rN0oBEoYZ56\_mzs9TD 14-7-5 13-3-8 9-1-12 11-11-11 17-5-4 26-7-0 9-1-12 2-9-15 2-9-15 9-1-12 1-3-13 5x6= 6 3x6= 3x6 7<sup>12</sup> 2x3 II 2x3 8 5x8 3 8-9-9 9-6,

Plate Offsets (X, Y): [6:0-3-0,Edge], [8:0-0-0,Edge], [10:0-0-0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.92	Vert(LL)	-0.30	12-14	>999	240	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.87	Vert(CT)	-0.42	12-14	>763	180			
BCLL	0.0*	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.03	2	n/a	n/a			
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MSH		Attic	-0.20	12-14	>497	360	Weight: 180 lb	FT = 20%	
											1		

-

3x3=

13-3-8

4-1-12

-

3x3 =

4-1-12

5x6=

13

5x6=

LUMBER **BRACING** 

TOP CHORD TOP CHORD 2x6 SP SS \*Except\* T1:2x6 SP No.2 **BOT CHORD** 2x6 SP No.2

**BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No.2 WEBS

В1

14

5x6=

21

9-1-12

9-1-12

WEDGE Left: 2x4 SP No.2 Right: 2x4 SP No.2

REACTIONS (lb/size) 2=1157/0-3-8, (min. 0-1-10), 10=1157/0-3-8, (min. 0-1-10) Max Horiz 2=-160 (LC 8)

5x6≥

Max Uplift 2=-11 (LC 10), 10=-11 (LC 11)

2=1396 (LC 18), 10=1396 (LC 19)

**FORCES** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2009/41, 3-4=-1837/66, 4-5=-1495/134, 5-6=-151/1303, 6-7=-151/1304, 7-8=-1495/134, 8-9=-1837/66, 9-10=-2009/41 TOP CHORD

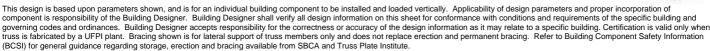
BOT CHORD 2-21=-144/1618, 14-21=0/1618, 13-14=0/1618, 12-13=0/1618, 12-22=0/1618, 10-22=0/1618

WEBS 4-14=0/646, 8-12=0/647, 5-7=-3052/336

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; Cat. II; Exp B; Enclosed; MWFRS (envelope) 2) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3)
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between 4) the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Ceiling dead load (5.0 psf) on member(s). 4-5, 7-8, 5-7
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 12-14
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 11 lb uplift at joint 2 and 11 lb uplift at joint 10.
- 8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/ **TPI 1.**
- 9) Attic room checked for L/360 deflection









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6-3-12

0-1-12

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Plate Offsets (X, Y): [3:	0-2-0,0-1-12]
---------------------------	---------------

- 1													
١	Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defI	L/d	PLATES	GRIP
١	TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.71	Vert(LL)	-0.04	5-6	>999	240	MT20	244/190
١	TCDL	10.0	Lumber DOL	1.15	BC	0.34	Vert(CT)	-0.08	5-6	>999	180		
١	BCLL	0.0*	Rep Stress Incr	YES	WB	0.10	Horz(CT)	0.01	5	n/a	n/a		
١	BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MR							Weight: 50 lb	FT = 20%
- 1													

LUMBER BRACING

TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end 2x4 SP No.2 **BOT CHORD** 

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No.3 WEBS

6-3-12

REACTIONS (lb/size) 5=569/0-3-8, (min. 0-1-8), 7=503/0-3-8, (min. 0-1-8) Max Horiz 7=-95 (LC 8)

5=-24 (LC 11), 7=-10 (LC 10) Max Uplift

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

 $1\hbox{-}2\hbox{--}574/67, 2\hbox{-}3\hbox{--}579/69, 1\hbox{-}7\hbox{--}436/76, 3\hbox{-}5\hbox{--}508/112}$ 

**BOT CHORD** 6-7=0/410, 5-6=0/410

2-6=0/266 WEBS

## NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members
- 5) Bearing at joint(s) 7, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 7 and 24 lb uplift at joint 5. 6)
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/







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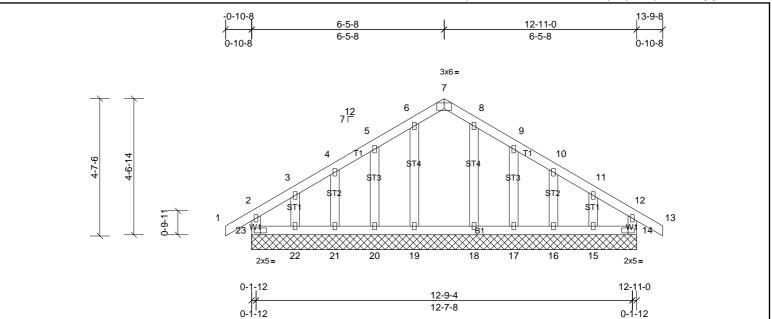


Plate Offsets (X, Y):	[7:0-3-0,Edg	je]											
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.07	Vert(LL)	n/a	-	n/a	999	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	n/a	-	n/a	999			
BCLL	0.0*	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	14	n/a	n/a			
BCDI	10.0	Code	IRC2015/TPI2014	Matrix-MR		1					Weight: 73 lh	FT = 20%	

LUMBER **BRACING** 

TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end BOT CHORD 2x4 SP No.2

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No.3 WEBS OTHERS 2x4 SP No.3

REACTIONS All bearings 12-11-0.

(lb) - Max Horiz 23=-99 (LC 8)

Max Uplift All uplift 100 (lb) or less at joint(s) 14, 15, 16, 17, 20, 21, 22, 23 Max Grav All reactions 250 (lb) or less at joint(s) 14, 15, 16, 17, 18, 19, 20, 21, 22,

**FORCES** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS 2) for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. 3)
- All plates are 1.5x3 MT20 unless otherwise indicated. 4)
- 5) Gable requires continuous bottom chord bearing
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 1-4-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 23, 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing 10
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 23, 14, 20, 21, 22, 17, 16, 15.
- 12 This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/ TPI 1.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.





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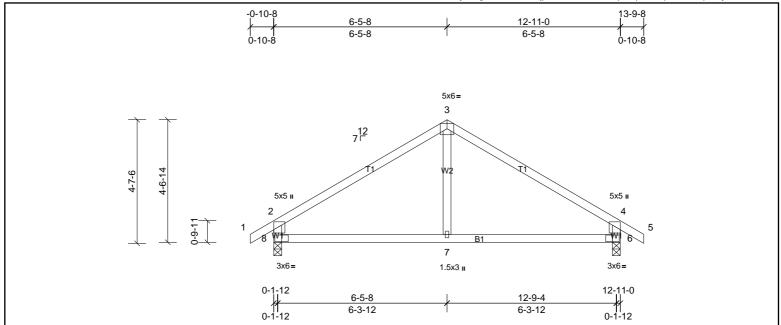


Plate Offsets (X, Y):	[2:0-2-8,0-1-12], [4:0-2-8,0-1-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.72	Vert(LL)	-0.03	7-8	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.36	Vert(CT)	-0.07	7-8	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.10	Horz(CT)	0.01	6	n/a	n/a		
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MR							Weight: 52 lb	FT = 20%

LUMBER BRACING

TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end 2x4 SP No.2 **BOT CHORD** 

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No 3 WEBS

REACTIONS (lb/size) 6=566/0-3-8, (min. 0-1-8), 8=566/0-3-8, (min. 0-1-8) Max Horiz 8=-99 (LC 8)

6=-24 (LC 11), 8=-24 (LC 10) Max Uplift

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

 $2\hbox{-}3\hbox{-}578/68,\, 3\hbox{-}4\hbox{-}578/68,\, 2\hbox{-}8\hbox{-}508/112,\, 4\hbox{-}6\hbox{-}508/112$ 

**BOT CHORD** 7-8=0/408, 6-7=0/408

3-7=0/271 WEBS

## NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members
- 5) Bearing at joint(s) 8, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 8 and 24 lb uplift at joint 6. 6)
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/







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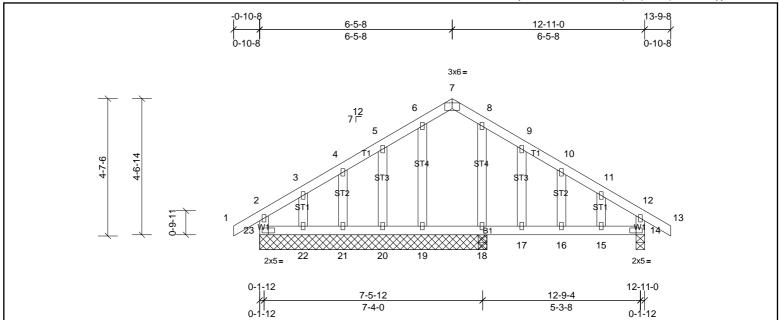


Plate Offsets (X, Y):	[7:0-3-0,Edg	e]											
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.18	Vert(LL)	0.03	15-16	>999	240	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.27	Vert(CT)	-0.05	15-16	>999	180			
BCLL	0.0*	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	14	n/a	n/a			

LUMBER BRACING

TOP CHORD 2x4 SP No.2 TOP CHORD BOT CHORD 2x4 SP No.2

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing 2x4 SP No.3 WEBS **OTHERS** 2x4 SP No.3

IRC2015/TPI2014 | Matrix-MR

REACTIONS All bearings 7-7-8. except 14=0-3-8, 18=0-3-8

23=-99 (LC 8) (lb) - Max Horiz

Code

Max Uplift All uplift 100 (lb) or less at joint(s) 14, 18, 20, 21, 22, 23 except 19=-145 (LC 22)

Max Grav All reactions 250 (lb) or less at joint(s) 19, 20, 21, 22, 23 except 14=295

(LC 1), 18=392 (LC 18)

**FORCES** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES

BCDI

1) Unbalanced roof live loads have been considered for this design.

10.0

- Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) 2) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only
- 4) All plates are 1.5x3 MT20 unless otherwise indicated.
- 5) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 6) Gable studs spaced at 1-4-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members
- Bearing at joint(s) 23, 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing 9)
- 10 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 23, 20, 21, 22, 14, 18 except (jt=lb)
- 11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/ **TPI 1.**



Weight: 73 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end

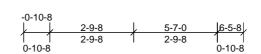
FT = 20%

This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute



Job	Truss	Truss Type	Qty	Ply	Service - 601 WEST BAY ST RF
72402286	D01	Truss	2	1	Job Reference (optional)

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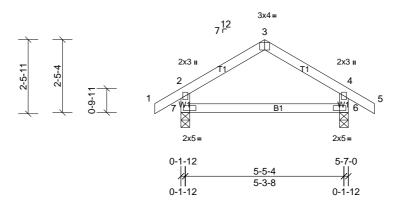


Plate Offsets (X, Y):	[3:0-2-0,Edg	e]											
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.17	Vert(LL)	-0.03	6-7	>999	240	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.23	Vert(CT)	-0.05	6-7	>999	180			
BCLL	0.0*	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	6	n/a	n/a			
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MR							Weight: 22 lb	FT = 20%	

**BRACING** 

TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 5-7-0 oc purlins, except end 2x4 SP No.2 **BOT CHORD** 

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No 3 WEBS

REACTIONS (lb/size) 6=273/0-3-8, (min. 0-1-8), 7=273/0-3-8, (min. 0-1-8)

Max Horiz 7=58 (LC 9)

Max Uplift 6=-19 (LC 11), 7=-19 (LC 10)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES

LUMBER

- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS
- for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 7, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing 5)
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 7 and 19 lb uplift at joint 6.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/ TPI 1.

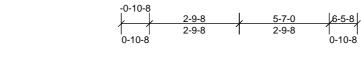




Job	Truss	Truss Type	Qty	Ply	Service - 601 WEST BAY ST RF
72402286	D02-G	Truss	1	1	Job Reference (optional)

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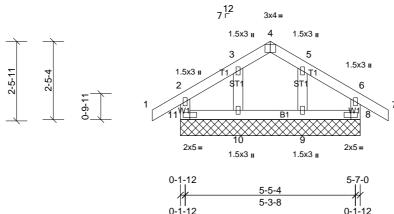


Plate Offsets (X, Y):	[4:0-2-0,Edg	e]										
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.07	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	8	n/a	n/a		
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MR							Weight: 26 lb	FT = 20%

LUMBER **BRACING** 

TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 5-7-0 oc purlins, except end BOT CHORD 2x4 SP No.2

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No.3 WEBS **OTHERS** 2x4 SP No.3

REACTIONS All bearings 5-7-0. (lb) - Max Horiz 11=58 (LC 9)

Max Uplift

All uplift 100 (lb) or less at joint(s) 8, 9, 10, 11 All reactions 250 (lb) or less at joint(s) 8, 9, 10, 11

**FORCES** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS 2) for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only
- 4) Gable requires continuous bottom chord bearing
- 5) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 6) Gable studs spaced at 1-4-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members
- 9) Bearing at joint(s) 11, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing
- 10 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 8, 10, 9.
- 11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



Job	Truss	Truss Type	Qty	Ply	Service - 601 WEST BAY ST RF
72402286	J01	Truss	6	1	Job Reference (optional)

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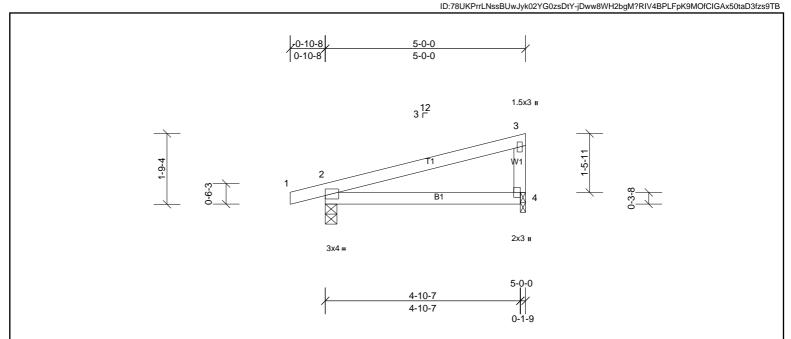


Plate Offsets (X, Y): [4:Edge,0-0-1]													
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defI	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.35	Vert(LL)	0.05	4-7	>999	240	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.25	Vert(CT)	-0.05	4-7	>999	180			
BCLL	0.0*	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.01	2	n/a	n/a			
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MP							Weight: 18 lb	FT = 20%	

LUMBER BRACING

TOP CHORD 2x4 SP No.2 TOP CHORD 2x4 SP No.2 **BOT CHORD** 

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No 3 WEBS

REACTIONS (lb/size) 2=251/0-3-8, (min. 0-1-8), 4=189/0-1-9, (min. 0-1-8) Max Horiz 2=46 (LC 6)

Max Uplift 2=-71 (LC 6), 4=-54 (LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES

- Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) 1) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 71 lb uplift at joint 2 and 54 lb uplift at joint 4.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/ TPI 1.



Structural wood sheathing directly applied or 5-0-0 oc purlins, except end

