

Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0724-4004 Weaver Development/Lot 6 West Preserve

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I66870477 thru I66870497

My license renewal date for the state of North Carolina is December 31, 2024.

North Carolina COA: C-0844



July 17,2024

# Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



		8-5-12	1	20-1-4	28-	9-6	1	37-9-0	
		8-5-12	I	11-7-8	8-8	3-2	8	-11-10	
Plate Offs	ets (X,Y)	[4:0-4-0,0-4-8], [8:0-2-6,Edge	], [11:0-5-8,0-3-8]						
LOADING	i (psf)	SPACING- 2-	0-0 CSI.		DEFL. in	(loc)	l/defl L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1	.15 TC	0.50	Vert(LL) -0.12	10 >	>999 360	MT20	244/190
TCDL	10.0	Lumber DOL 1	.15 BC	0.44	Vert(CT) -0.25	10-11 >	>999 240		
BCLL	0.0 *	Rep Stress Incr Y	'ES WB	0.99	Horz(CT) 0.15	8	n/a n/a		
BCDL	10.0	Code IRC2015/TPI20	14 Matr	ix-S	Wind(LL) 0.11	10 >	>999 240	Weight: 348 lb	FT = 20%

#### LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2 *Except*
	14-15,15-16,16-17,17-18: 2x6 SP No.1
OTHERS	2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

WEBS

Structural wood sheathing directly applied or 4-4-14 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt 7-11

REACTIONS. (size) 8=0-3-8, 13=0-3-8 Max Horz 13=392(LC 11) Max Uplift 8=-259(LC 13), 13=-403(LC 12) Max Grav 8=1106(LC 1), 13=1997(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-477/674, 3-5=-835/342, 5-7=-875/292, 7-8=-2664/461

BOT CHORD 2-13=-435/500, 11-13=-567/488, 10-11=-246/2258, 8-10=-248/2253

WEBS 3-13=-1737/755, 3-11=-271/1036, 5-11=-93/354, 7-11=-1958/574, 7-10=-53/1589

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-9 to 3-8-4, Interior(1) 3-8-4 to 18-10-8, Exterior(2) 18-10-8 to 23-3-5, Interior(1) 23-3-5 to 38-5-9 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=259, 13=403.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)





	8-5-12	20-1-4	I	28-9-6	37-9-0			
	8-5-12	11-7-8		8-8-2	8-11-10			
Plate Offsets (X,Y)	[8:0-2-6,Edge], [11:0-5-8,0-3-8]							
LOADING (pst)	SPACING- 2-0-0	CSI.	DEFL.	in (loc) I/defl	L/d	PLATES	GRIP	
TCLI 20.0	Plate Grin DOI 115	TC 0.50	Vert(LL)	-0.12 10 \000	360	MT20	244/190	

TOP CHORD

BOT CHORD

WEBS

LUMBER	-			BRACING-				
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL)	0.08 10	>999	240	Weight: 267 lb
BCU	0.0 *	Ren Stress Incr YES	WB 0.99	Horz(CT)	0.15 8	>333 n/a	240 n/a	
TCDI	10.0	Lumber DOI 115	BC 0.44	Vort(CT)	-0.25 10-11	~000	240	
	20.0		10 0.00		=U.1Z 1U	2333	300	

#### LUMBER-

2x6 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2

REACTIONS. (size) 8=0-3-8, 13=0-3-8 Max Horz 13=313(LC 11) Max Uplift 8=-86(LC 13), 13=-116(LC 12) Max Grav 8=1106(LC 1), 13=2020(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-477/674, 3-5=-835/206, 5-7=-875/223, 7-8=-2664/235

BOT CHORD 2-13=-435/500, 11-13=-528/488, 10-11=-50/2258, 8-10=-52/2253

WFBS 3-13=-1737/755, 3-11=-271/1036, 5-11=-34/354, 7-11=-1958/326, 7-10=0/1589

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-9 to 3-8-4, Interior(1) 3-8-4 to 18-10-8, Exterior(2) 18-10-8 to 23-3-5, Interior(1) 23-3-5 to 38-5-9 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf. 5) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 13=116

# CHARLEN WINDOW SEAL 036322 G minin July 17,2024

FT = 20%

Structural wood sheathing directly applied or 4-4-14 oc purlins.

7-11

Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt

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818 Soundside Road



4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 12=116.



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-458/579, 3-5=-1054/293, 5-7=-1125/322, 7-8=-3181/453

BOT CHORD 2-12=-371/485, 10-12=-183/590, 9-10=-266/2701, 8-9=-266/2702

WEBS 3-10=-62/454, 5-10=-58/618, 7-10=-2174/452, 7-9=-65/1863, 3-12=-1683/732

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-9 to 3-8-4, Interior(1) 3-8-4 to 18-10-8, Exterior(2) 18-10-8 to 23-3-5, Interior(1) 23-3-5 to 37-7-4 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 12=107.



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-458/579, 3-5=-1053/292, 5-7=-1123/318, 7-8=-3175/415

BOT CHORD 2-13=-371/485, 11-13=-178/595, 10-11=-206/2693, 8-10=-207/2695

WEBS 3-11=-62/454, 5-11=-52/616, 7-11=-2171/418, 7-10=-31/1860, 3-13=-1683/732

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-9 to 3-8-4, Interior(1) 3-8-4 to 18-10-8, Exterior(2) 18-10-8 to 23-3-5, Interior(1) 23-3-5 to 38-5-9 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 13=107.



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A MiTek Affilia 818 Soundside Road



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-694/251, 2-4=-781/236, 4-5=-2612/271, 1-9=-955/307

BOT CHORD 8-9=-277/326. 7-8=-111/2214. 5-7=-114/2209

WEBS 2-8=0/325, 4-8=-1120/364, 4-7=0/1563, 1-8=-119/677

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-1, Interior(1) 4-9-1 to 7-6-8, Exterior(2) 7-6-8 to 11-11-5, Interior(1) 11-11-5 to 27-1-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 9.



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Job	Truss	Truss Type	Qty	Ply	Weaver Development/Lot 6 West Preserve			
J0724-4004	A08	COMMON	1	2	leb Reference (optional)	36870484		
Comtech, Inc, Fayett	teville, NC - 28314,	10.1	8 19kby A 2\\A/Tik	.530 s Au	g 2 2023 MiTek Industries, Inc. Mon Jul 15 14:05:07 2024 F	'age 1		
		9-1-8 18-2-15 9-1-8 9-1-7		2	8-0-0 28-10-8 -9-9 0.10-8	(050		
		5×5 —			Sce	ale = 1:85.9		
		- cxc				10 - 1.00.0		
	8.00 T	2 16 16 17 17 17 17 17 17 17 17 17 17	4x6 = 3 2x4 3 2x4 4 4 9 20	21				
	15 <sup>14</sup> 8 2×6 II	x8 = 6x8 = 5x8    5x12	II		2x4			
	5.0 11	5x	8		3x4 =			
	3-0-9	15-2-7 16-2-	12	25-10-0	28-0-0			
Plate Offsets (X,Y) [5	<u>3-0-9</u> ::0-6-0,0-1-10], [9:0-4-0,0-1-8],	<u>12-1-14</u> 1-0- [10:0-10-4,0-2-4], [11:0-4-0,0-1-8], [13:0-4-0,	5 0-4-12]	9-7-4	2-2-0			
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 4-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NC Code IRC2015/TPI2014	CSI.         DEFL           TC         0.85         Vert(I           BC         0.63         Vert(I           WB         0.44         Horz(           Matrix-S         Wind	in .L) -0.20 CT) -0.42 CT) 0.18 (LL) 0.12	(loc) 5-10 5-10 6 8	I/defi         L/d         PLATES         GRIP           >999         360         MT20         244/190           >793         240         n/a         n/a           >999         240         Weight: 511 lb         FT = 20%			
LUMBER- TOP CHORD 2x6 SP N BOT CHORD 2x10 SP 6-8: 2x4 WEBS 2x4 SP N 1-14,5-8: REACTIONS. (size) Max Hor Max Upi	Io.1 No.1 *Except* SP No.1, 5-11: 2x6 SP No.1 Io.2 *Except* 2x6 SP No.1 14=0-3-8, 6=0-3-8 z 14=-606(LC 13) ift 14=-151(LC 13), 6=-105(LC	BRAC TOP ( BOT ( WEB: 13)	CHORD CHORD CHORD	2-0-0 oc (Switche Rigid ce 6-0-0 oc 1 Row a	purlins (5-8-12 max.), except end verticals ed from sheeted: Spacing > 2-0-0). iling directly applied or 10-0-0 oc bracing, Except: bracing: 6-8. t midpt 2-13			
Max Gra           FORCES.         (lb) - Max. C           TOP CHORD         1-2=-11           BOT CHORD         13-14=           WEBS         2-13=-1	v 14=2656(LC 20), 6=2659(LC 00), 6=2659(LC 00), 000, 000, 000, 000, 000, 000, 000	; 20) ) (lb) or less except when shown. 3741/555, 5-6=-1719/320, 1-14=-2733/447 175/3024 0=-1424/722, 1-13=0/2009						
<ul> <li>VOTES-</li> <li>1) 2-ply truss to be connected together with 10d (0.131*X3<sup>°</sup>) nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc, Webs connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.</li> <li>2) All loads are considered equally applied to all piles, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.</li> <li>3) Unbalanced roof live loads have been considered for this design.</li> <li>4) Winck ASC 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15f; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-1, Interior(1) 4-9-1 to 9-1-8, Exterior(2) 9-1-8 to 13-6-5, Interior(1) 13-6-5 to 28-8-9 zone; C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60</li> <li>5) This truss has been designed for a 10.0 psf bottom chord ine load nonconcurrent with any other live loads.</li> <li>6) "This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other remebers, with BCL = 10.0psf.</li> <li>7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 14=151, 6=105.</li> <li>8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.</li> </ul>								

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Job	Truss	Truss Type	Qty	Ply	Weaver Development	/Lot 6 West Preserve	
J0724-4004	A09	COMMON	4	1			166870485
Comtech, Inc, Fayette	eville, NC - 28314,		8	.530 s Au	Job Reference (option ug 2 2023 MiTek Indust	al) ries, Inc. Mon Jul 15 14:0	05:08 2024 Page 1
		9-1-8 18	ID:M8kbvA?WTIhi1vS 8-2-15	eQ5lcK_y	/WBWQ-btkRPeD_RDS 28-0-0 28-10	AAxArTL_hYPHwPImAO -8	Skd_hml9?yxoSP
		9-1-8	9-1-7		9-9-1 0-10-	8	
		5x5 =					Scale = 1:85.9
		— 2					
	4x8 = 1 025 1 025 1 025 1 025 1 0 025 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	16 Å	17 4x6 × 3 2x4 4	+ //	18		
						π <mark>ο</mark> 14	
		13 19 12 1	1 10 9 20	21	• 5		
	15 <sup>14</sup> 8	x8 = 6x8 =	5x12		2x4		
	5x0	5>	ĸ8    5x8		3x4 =		
	, 3-0-9	15-2-7	16-2-12	25-10-0	6x12 — 28-0-0		
Plate Offsets (X Y) [5:	<u>3-0-9</u> 0-6-0 0-1-10] [9 <sup>.</sup> 0-4-0 0-1-8]	12-1-14 [10:0-10-4 0-2-4] [11:0-4-0 0-1-4]	1-0-5	9-7-4	2-2-0		
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.73 BC 0.57 WB 0.59 Matrix-S	DEFL.         in           Vert(LL)         -0.20           Vert(CT)         -0.42           Horz(CT)         0.18           Wind(LL)         0.12	(loc) 5-10 5-10 6 8	l/defl L/d >999 360 >792 240 n/a n/a >999 240	PLATES MT20 Weight: 255 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP N BOT CHORD 2x10 SP 1	o.1 No.1 *Except*		BRACING- TOP CHORD	Structur	ral wood sheathing dire	ectly applied or 3-7-0 oc	; purlins,
6-8: 2x4 S WEBS 2x4 SP N 1-14,5-8:	SP No.1, 5-11: 2x6 SP No.1 o.2 *Except* 2x6 SP No.1		BOT CHORD WEBS	Rigid ce 6-0-0 oc 1 Row a	eiling directly applied of c bracing: 6-8. at midpt 2-	r 10-0-0 oc bracing, E: 13	xcept:
REACTIONS. (size) Max Horz Max Uplif Max Grav	14=0-3-8, 6=0-3-8 : 14=-303(LC 13) :t 14=-75(LC 13), 6=-53(LC 13) / 14=1328(LC 20), 6=1329(LC	) ; 20)					
FORCES.         (lb) - Max. Co           TOP CHORD         1-2=-58           BOT CHORD         13-14=-           WEBS         2-13=-6	mp./Max. Ten All forces 25( 0/204, 2-4=-1725/375, 4-5=-1 238/312, 10-13=0/795, 5-10=- 97/140, 2-10=-181/1489, 4-10	) (lb) or less except when shown. 371/278, 5-6=-859/160, 1-14=-1366 87/1512 =-712/361, 1-13=0/1004	6/223				
NOTES- 1) Unbalanced roof live lo 2) Wind: ASCE 7-10; Vult and C-C Exterior(2) 0-4	ads have been considered for =130mph Vasd=103mph; TCI I-4 to 4-9-1, Interior(1) 4-9-1 to	this design. DL=6.0psf; BCDL=6.0psf; h=15ft; C 9-1-8, Exterior(2) 9-1-8 to 13-6-5,	at. II; Exp C; Enclosed; Interior(1) 13-6-5 to 28	MWFR 3-8-9 zon	S (envelope) e;C-C for		

members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14, 6.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCEL Building Component Stability and proposed to component development description. and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road



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A MiTek At 818 Soundside Road





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5) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



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<sup>818</sup> Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Weaver Developmen	nt/Lot 6 West Preserve				
.10724-4004	C1	ATTIC	1	1			l66870491			
					Job Reference (option	nal)				
Comtech, Inc, Faye	etteville, NC - 28314,		ID:M8kbvA?WTIhi1v	8.530 s Au /SeQ5lcK	ug 2 2023 MiTek Indus vWBWQ-?SPa1gFsk8	tries, Inc. Mon Jul 15 14 ak10vQ8TXOA2vSMVis	4:05:11 2024 Page 1 SblL4gf?PmKvxoSM			
		-0-11-0 4-5-12 7-11-13	<u>11-8-8</u> <u>15-5-3</u>	18-11	-4 23-5-0 24	-4-0	·g··· ·)····			
		0-11-0 4-5-12 3-6-1	3-0-11 3-0-11	3-0-	1 4-5-12 0-1	1-0				
			6x8 =				Scale = 1:82.7			
			6							
			奥							
		$12.00   12 \\ 2x6 =$								
		_ /		2x6	. =					
		5 🗲		<u> </u>						
		8x8	_		8x8					
	Ę	3	4	-	T 1 2 9					
	4.40	· · · · · · · · · · · · · · · · · · ·	8-2-		4x6	~				
	440	2				10				
	~ ~		14-ρ-0	2						
	2-3-4					√   <sup>2</sup> / <sub>2</sub> <sup>1</sup> / <sub>1</sub>				
	1 - 1			14		1 -				
		16 15		6x8 =	13 12					
		3x6    10x10 =	18-11-4		10x10 = 3x6	II				
Plata Offeate (X X)	4-5-12 10-11-4 25-5-U 4-5-12 14-5-8 4-5-12									
	2.0 0 12,0 1 12j, [0.0 4 0,Euge	j, [4.0 0 0,Euge], [0.0 0 0,Euge], [	0.0 4 0,Eugej, [10.0 0	12,0 1 12	<u>, [10.0 0 0,0 7 0], [10</u>					
LOADING (psf) TCLL 20.0	Plate Grip DOI 11	D <b>CSI.</b> 5 TC 0.62	DEFL. ir Vert(LL) -0.26	ו (loc) 13-15 נוסב	l/defl L/d	PLATES MT20	GRIP 244/190			
TCDL 10.0	Lumber DOL 1.1	5 BC 0.86	Vert(CT) -0.40	) 13-15	>685 240		2.1.,100			
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	S WB 0.85 Matrix-S	Horz(CT) 0.01 Wind(LL) 0.07	12 13-15	n/a n/a >999 240	Weight: 274 lb	FT = 20%			
						5				
TOP CHORD 2x10 SF	P No.1 *Except*		TOP CHORD	Structur	ral wood sheathing dir	rectly applied or 4-5-0	oc purlins,			
1-3,9-1	1: 2x6 SP No.1			except of	end verticals.	ar 7 11 0 as brasing				
WEBS 2x6 SP	No.1 *Except*		BOTCHORD	Rigiu ce	aning unectly applied t					
2-15,10	-13: 2x4 SP No.2									
REACTIONS. (size	) 16=0-3-8, 12=0-3-8									
Max Ho Max Gi	orz 16=-441(LC 10) av 16=1655(LC 21) 12=1655(	I C 20)								
TOP CHORD 2-4=-	Comp./Max. Ten All forces 25 1913/0, 4-5=-1210/193, 7-8=-12	0 (lb) or less except when shown. 210/193, 8-10=-1912/0, 2-16=-200	3/0,							
10-12	=-2004/1									
WEBS 5-7=-'	=-430/553, 13-15=0/1249 1329/217, 4-15=0/965, 8-13=0/9	965, 2-15=0/1214, 10-13=0/1218								
NOTES.										
1) Unbalanced roof live	loads have been considered for	r this design.								
2) Wind: ASCE 7-10; Vi	ult=130mph Vasd=103mph; TC C-C Corner(3) -0-9-8 to 3-7-5	DL=6.0psf; BCDL=6.0psf; h=15ft;	Cat. II; Exp C; Enclosed	d; MWFRS	S (envelope)					
24-2-8 zone; end ver	tical left and right exposed;C-C	for members and forces & MWFR	S for reactions shown;	Lumber D	OL=1.60					
plate grip DOL=1.60 3) This truss has been of	designed for a 10.0 psf bottom	chord live load nonconcurrent with	any other live loads				000			
4) * This truss has been	designed for a live load of 30.0	Opsf on the bottom chord in all are	as where a rectangle 3-	6-0 tall by	2-0-0 wide	UNTH C	ARO			
will fit between the bo 5) Ceiling dead load (10	ottom chord and any other mem	bers. 5-7: Wall dead load (5 0psf) on r	nember(s).4-15 8-13			N'OR	S.L.M.			
6) Bottom chord live loa	d (40.0 psf) and additional bott	om chord dead load (10.0 psf) app	plied only to room. 13-1	5		A DEFS	THE			
<ol> <li>Attic room checked for</li> </ol>	or L/360 deflection.				-	and -	and the			
						: : .	AL : -			



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Job	Truss	Truss Type			Qty	Ply	Weaver Developmen	nt/Lot 6 West Preserve	
J0724-4004	C2	ATTIC			5	1			166870492
		-					Job Reference (optio	nal)	4.05.44.0004 D
Comtech, Inc, Faye	etteville, NC - 28314,			ID:M8kbv/	ہ WTIhi1v	8.530 s Au SeQ5lcK	g 22023 Milek Indus vWBWQ-?SPa1qFsk8	stries, Inc. Mon Jul 15 1 lgk10vQ8TXOA2vSMVj	4:05:11 2024 Page 1 SblL4gf?PmKyxoSM
		-0-11-0 4-5-12	7-11-13	11-8-8	15-5-3	18-11-	4 23-5-0 24	-4-0 11 0	<b>J</b>
		0-11-0 4-5-12	3-0-1	3-0-11	3-0-11	3-0-1	4-5-12 0-	11-0	
				6x8	=				Scale = 1:82.7
				6					
	I			Ţ.					
		12.00 1	2		$\langle \rangle$				
			2x0 —			2x6	=		
			5	/		7			
					T	<u> </u>			
	Ģ	8x8    6x8 1/ /	// Τ			$\sim$	8x8		
	13-7	17				_			
		<sup>3</sup>			2-4				
	4xi	54 //	-0-2		ά	5	4x6	~	
		2				5-1-		10	
	8.7			14-0-					
	-0 4-0					- <b>D</b>		0.4	
		₩ 16 15				14	13 12		
		3ve II 10v10	_			6x8 =	10×10 - 3×6		
		4-5-12		18-11-	-4		23-5-0	П	
Plate Offsets (X,Y)	[2:0-0-12,0-1-12], [3:0-4-0,Edge	4-5-12 ], [4:0-9-5,Edge], [8:0-9	9-5,Edge], [9	-14-5 [0-4-0,Edge]	<u>s</u> [10:0-0-1	2,0-1-12]	<u>4-5-12</u> , [13:0-5-0,0-7-8], [15	5:0-5-0,0-7-8]	
	SPACINC 2.0			DEEL	in	(100)	l/dofl l/d		
TCLL 20.0	Plate Grip DOL 1.1	5 TC 0	0.62	Vert(LL)	-0.26	13-15	>999 360	MT20	244/190
TCDL 10.0	Lumber DOL 1.1	5 BC 0	0.86	Vert(CT	) -0.40	13-15	>685 240		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	S	Wind(LL	.) 0.06	13-15	>999 240	Weight: 274 lb	FT = 20%
				BRACIN	IG-				
TOP CHORD 2x10 SI	P No.1 *Except*			TOP CH	IORD	Structur	al wood sheathing di	rectly applied or 4-5-0	oc purlins,
1-3,9-1 2×10 SI	1: 2x6 SP No.1					except e	end verticals.	or 7 11 0 oc bracing	
WEBS 2x6 SP	No.1 *Except*			BOTO	IOND	Rigiu ce	aning unectly applied	or 7-11-9 oc bracing.	
2-15,10	)-13: 2x4 SP No.2								
REACTIONS. (size	) 16=0-3-8, 12=0-3-8								
Max Ho Max G	orz 16=-352(LC 10)								
	1001(LC 21), 12-1001	20)							
FORCES. (lb) - Max.	Comp./Max. Ten All forces 25	60 (lb) or less except wh	nen shown.	(0					
10-12	2=-1990/0	100/100, 0 10= 1000/0,	2 10= 1303/	0,					
BOT CHORD 15-16	=-332/458, 13-15=0/1218	065 2 15-0/1172 10 1	2-0/1176						
WEBS 5-7=-	1329/130, 4-13=0/903, 0-13=0/	903, 2-13=0/1173, 10-1	3=0/11/0						
NOTES-	loade have been considered fo	r this docian							
2) Wind: ASCE 7-10; V	ult=130mph Vasd=103mph; TC	DL=6.0psf; BCDL=6.0p	osf; h=15ft; C	at. II; Exp C;	Enclosed	; MWFRS	6 (envelope)		
and C-C Exterior(2) -	-0-9-8 to 3-7-5, Interior(1) 3-7-5	to 11-8-8, Exterior(2) 1	1-8-8 to 15-1	11-3, Interior	(1) 15-11-	3 to 24-2-	8 zone; end		
3) This truss has been	designed for a 10.0 psf bottom	chord live load noncond	current with a	any other live	loads.	, plate gli	5 DOL=1.00		
<ol> <li>4) * This truss has been will fit between the be</li> </ol>	n designed for a live load of 30. ottom chord and anv other men	Opsf on the bottom chor obers.	rd in all areas	s where a red	tangle 3-	6-0 tall by	2-0-0 wide		

5) Ceiling dead load (10.0 psf) on member(s). 4-5, 7-8, 5-7; Wall dead load (5.0psf) on member(s).4-15, 8-13

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 13-15

7) Attic room checked for L/360 deflection.



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818 Soundside Road Edenton, NC 27932

GINEERING

Job	Truss	Truss Type	Qty	Ply	Weaver Development/Lot 6 West Preserve	100070400
J0724-4004	СЗ	ATTIC	6	1		166870493
Comtech, Inc, Fay	etteville, NC - 28314,			3.530 s Au	Job Reference (optional) g 2 2023 MiTek Industries, Inc. Mon Jul 15 14:05:12	2 2024 Page 1
· · · · · , · · , · · ,		4-5-12 7-11-13	ID:M8kbvA?WTIh	i1vSeQ5lc	K_yWBWQ-UezyE0GUVSybfYUciB3djFSZXv0eKBzI	DuJkyInyxoSL
		4-5-12 3-6-1	3-8-11 3-8-11	3-6-1	4-5-12	
			6x8 =			Scale = 1:82.7
			5			
			<b>P</b>			
		$\frac{12.00}{2x6} =$		2x6	=	
		4		6		
	0-Z	6x8 // 3	Γ		8x8    7 6x8 \\	
	<del>د</del>	2	_	Т	8	
			8-2-4	_	4x6 N	
	4x6			5-1-11	9	
			14-0-0			
	-2-3				5.3	
		⊠ 14 13 <sup>15</sup>		12	요 11 10	
		3x6    10x10 =		6x8 =	$10 \times 10 = 3 \times 6 \parallel$	
		4-5-12 4-5-12	<u>18-11-4</u> 14-5-8		<u>4-5-12</u>	
Plate Offsets (X,Y)	[1:0-1-4,0-2-0], [2:0-4-0,Edge], [	3:0-9-5,Edgej, [7:0-9-5,Edgej, [8:	:0-4-0,Edge], [9:0-1-4,0-2	2-0], [11:0-	-5-0,0-7-8], [13:0-5-0,0-7-8]	
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	) <b>CSI.</b> 5 TC 0.85	DEFL. in Vert(LL) -0.28	(loc) 11-13	I/defl         L/d         PLATES         GRI           >994         360         MT20         244	i <b>P</b> /190
TCDL 10.0	Lumber DOL 1.15 Rep Stress Incr. NO	5 BC 0.99	Vert(CT) -0.43	11-13 10	>636 240	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.06	11-13	>999 240 Weight: 269 lb F	Γ = 20%
LUMBER-		·	BRACING-	_		
TOP CHORD 2x10 S 1-2,8-9	P No.1 *Except* ): 2x6 SP No.1		TOP CHORD	Structur except e	al wood sheathing directly applied or 3-7-5 oc pur and verticals.	'lins,
BOT CHORD 2x10 S WEBS 2x6 SE	P No.1 No 1 *Except*		BOT CHORD	Rigid ce	iling directly applied or 7-3-12 oc bracing.	
1-13,9-	-11: 2x4 SP No.2					
REACTIONS. (size	e) 14=0-3-8, 10=0-3-8					
Max H Max G	lorz 14=315(LC 11) 6rav 14=1746(LC 21), 10=1673(	_C 20)				
FORCES. (lb) - Max.	Comp./Max. Ten All forces 25	0 (Ib) or less except when shown				
TOP CHORD 1-3=-	2019/0, 3-4=-1249/156, 4-5=-13	3/269, 5-6=-141/256, 6-7=-1264/	153, 7-9=-1987/0,			
BOT CHORD 13-14	4=-314/383, 11-13=0/1282	000 4 40 0/4000 0 44 0/4000				
WEBS 4-6=-	1452/157, 3-13=0/1045, 7-11=0	989, 1-13=0/1269, 9-11=0/1306				
NOTES- 1) Unbalanced roof live	e loads have been considered fo	this design.				
2) Wind: ASCE 7-10; V	/ult=130mph Vasd=103mph; TC	DL=6.0psf; BCDL=6.0psf; h=15ft;	Cat. II; Exp C; Enclosed	; MWFRS	6 (envelope)	
vertical right expose	d;C-C for members and forces 8	MWFRS for reactions shown; Lu	umber DOL=1.60 plate gi	rip DOL=1	.60	
4) * This truss has bee	n designed for a live load of 30.0	psf on the bottom chord in all are	eas where a rectangle 3-6	6-0 tall by	2-0-0 wide	11.
will fit between the b 5) Ceiling dead load (1	oottom chord and any other mem 0.0 psf) on member(s). 3-4, 6-7,	bers. 4-6; Wall dead load (5.0psf) on i	member(s).3-13, 7-11		TH CAR	Olin
<ul> <li>6) Bottom chord live log</li> <li>7) Hanger(s) or other c</li> </ul>	ad (40.0 psf) and additional botto connection device(s) shall be pro	Ib up at	Nin			
6-10-8 on bottom ch	ord. The design/selection of suc	ch connection device(s) is the res	ponsibility of others.			and I
9) In the LOAD CASE(	S) section, loads applied to the f	ace of the truss are noted as fron	nt (F) or back (B).		E SEAL	
LOAD CASE(S) Stan	dard				= 036322	2 : E
1) Dead + Roof Live (b Uniform Loads (plf)	alanced): Lumber Increase=1.15	, Plate Increase=1.15			E A	1 E
Vert: 13-14	=-20, 11-13=-40, 10-11=-20, 1-3	=-60, 3-4=-80, 4-5=-60, 5-6=-60,	6-7=-80, 7-9=-60, 4-6=-2	20	E CA WOINFE	8:13
Drag: 3-13= Concentrated Loads	s (lb)					BELIN
Vert: 15=-1	00(B)				A. GIL	inn
					July 1	7,2024

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Plate Offsets (X,Y)	[2:0-1-0,0-1-8], [4:0-1-0,0-1-8]									
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Des Citage Jack	CSI. TC 0.29 BC 0.15	DEFL. Vert(LL) Vert(CT)	in -0.01 -0.02	(loc) 4-6 4-6	l/defl >999 >999	L/d 360 240	PLATES MT20	<b>GRIP</b> 244/190	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL)	0.00	2-6	>999	240	Weight: 70 lb	FT = 20%	
LUMBER-	BRACING-	<b>D</b>	Christer		ah a ath in a di	in athe analiad as C.O.O				

 TOP CHORD
 2x4 SP No.1

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x4 SP No.2

 OTHERS
 2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 4=0-3-8 Max Horz 2=137(LC 11) Max Uplift 2=-119(LC 12), 4=-119(LC 13) Max Grav 2=530(LC 1), 4=530(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-568/165, 3-4=-568/165

BOT CHORD 2-6=-24/387, 4-6=-24/387

WEBS 3-6=0/300

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 6-0-0, Corner(3) 6-0-0 to 10-4-13, Exterior(2) 10-4-13 to 12-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 2x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=119, 4=119.



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BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. 1=8-0-5, 3=8-0-5, 4=8-0-5 (size) Max Horz 1=57(LC 11) Max Uplift 1=-24(LC 12), 3=-30(LC 13) Max Grav 1=155(LC 1), 3=155(LC 1), 4=260(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3. 6) Non Standard bearing condition. Review required.

![](_page_20_Picture_11.jpeg)

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![](_page_21_Figure_0.jpeg)

3x4 🥢

3x4 📎

Structural wood sheathing directly applied or 4-1-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Plate Offsets (X,Y)	0-0-9 0-0-9 [2:0-2-0,Edge]		<u>4-1-7</u> 4-0-14	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.03 BC 0.09 WB 0.00 Matrix-P	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         n/a         -         n/a         999           Vert(CT)         n/a         -         n/a         999           Horz(CT)         0.00         3         n/a         n/a           Weight:         12 lb         FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1

REACTIONS. (size) 1=4-0-5, 3=4-0-5 Max Horz 1=-25(LC 8) Max Uplift 1=-7(LC 12), 3=-7(LC 13) Max Grav 1=125(LC 1), 3=125(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.6) Non Standard bearing condition. Review required.

![](_page_21_Picture_15.jpeg)

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![](_page_21_Picture_17.jpeg)

![](_page_22_Figure_0.jpeg)