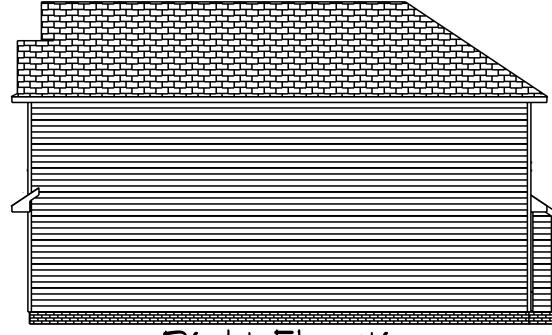


Left Elevation

Scale: 1/8"= 1'0"



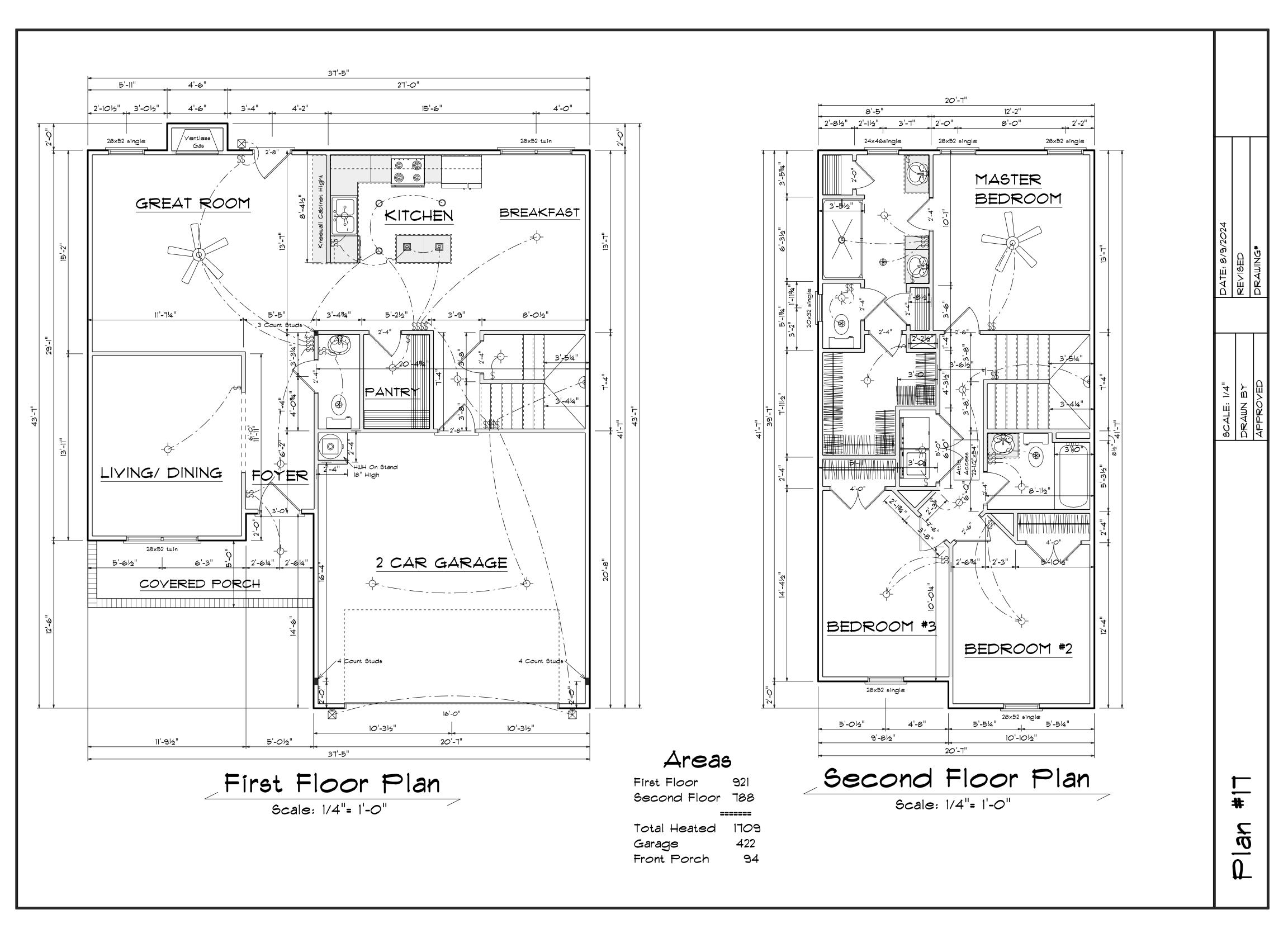
APPROVED

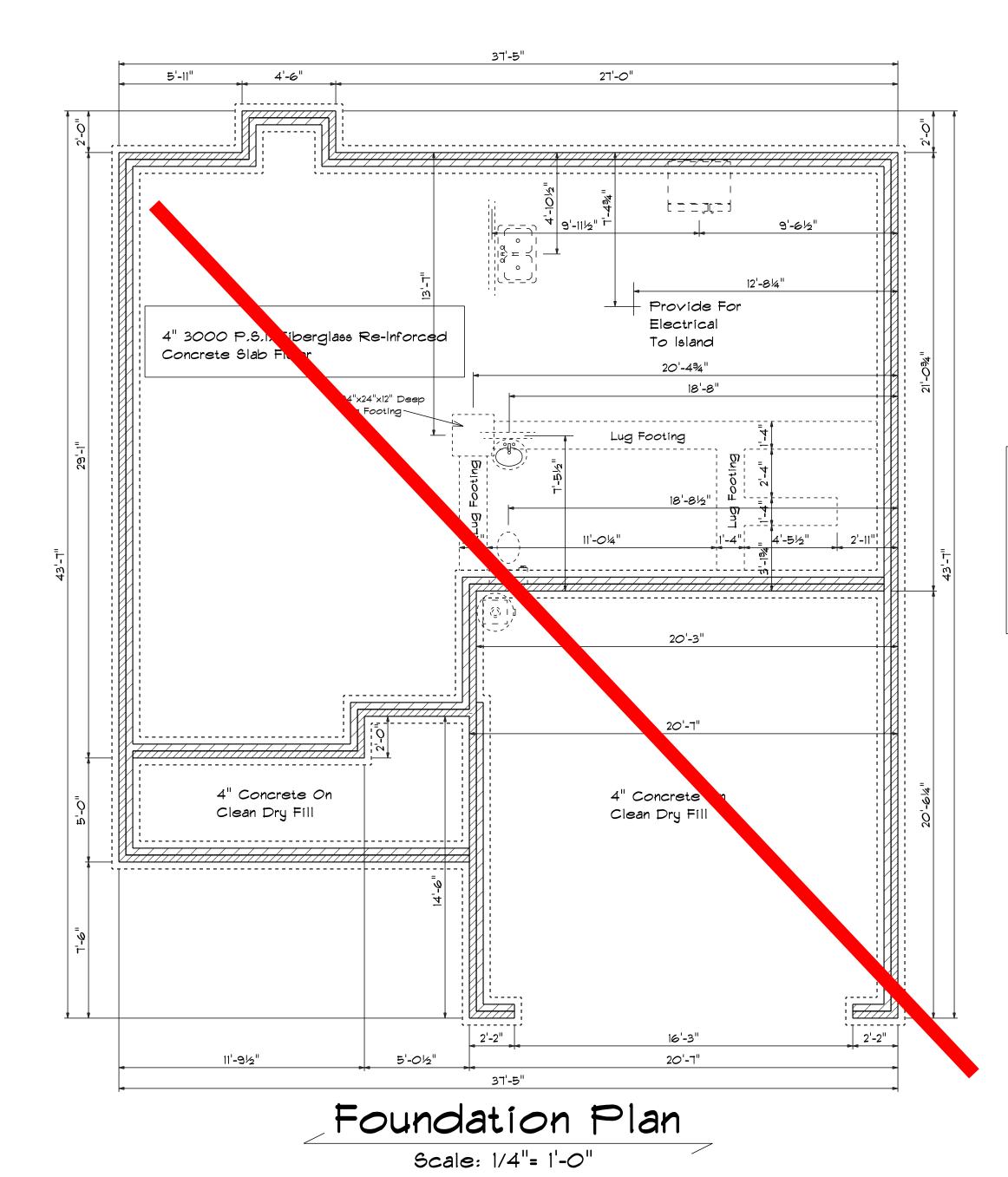
08/13/2024

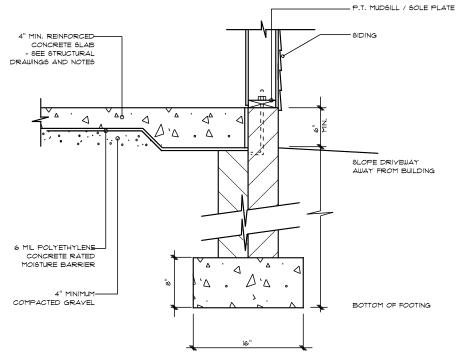
Harnett

NORTH CAROLINA

Right Elevation
Scale: 1/8"= 1'0"









STEM WALL FOOTING DETAIL

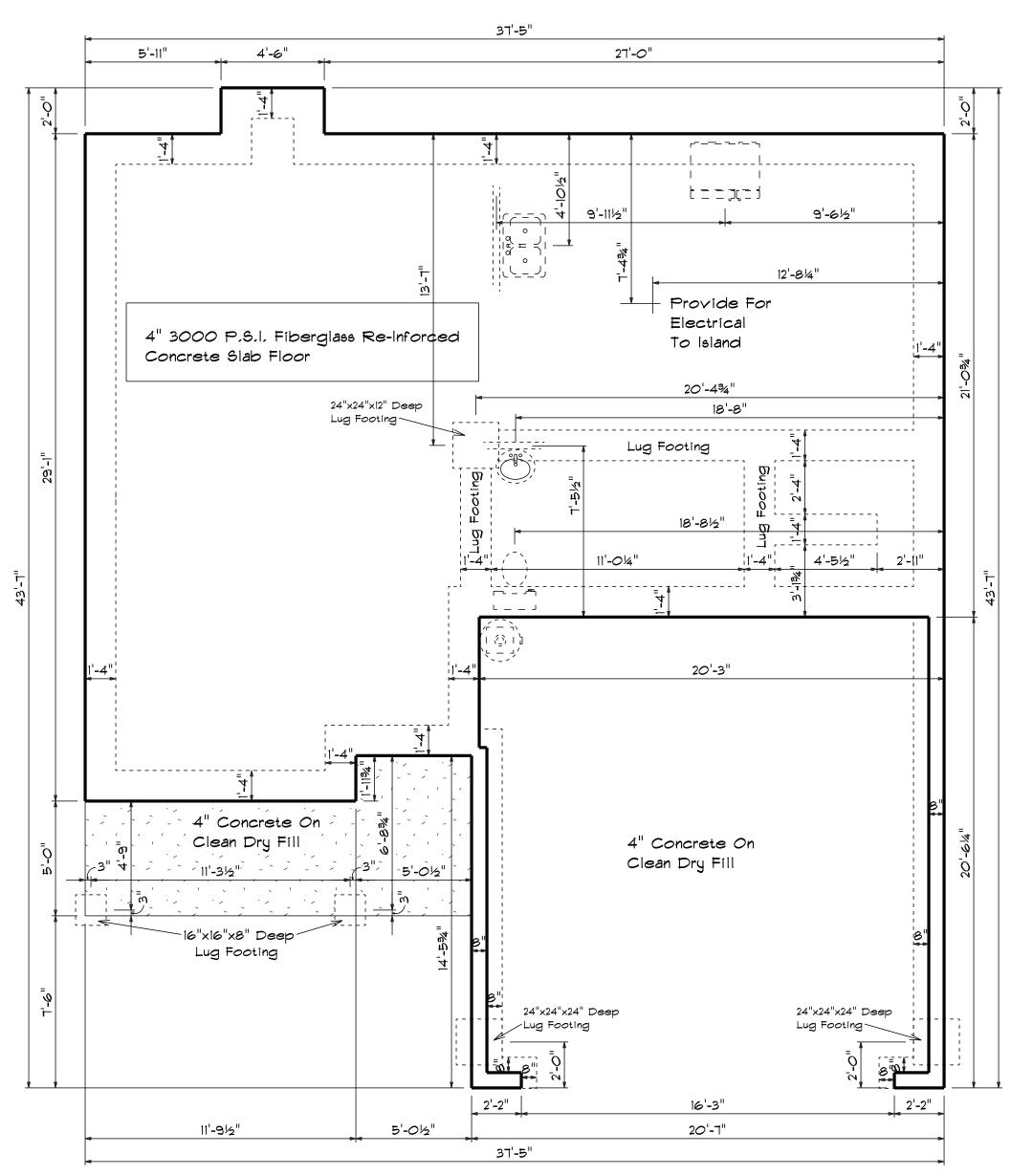
FIRST FLOOR OPENING SCHEDULE								
PRODUCT CODE	SIZE	HINGE	REVERSED	COUNT				
2-4 Door Unit	2'-4"	L	NO	1				
2-4 Door Unit	2'-4"	R	NO	2				
2-8 Door Unit	2'-8"	L	NO	1				
28x52 single	2'-8" x 5'-2"	N	NA	1				
28x52 twin	5'-4" x 5'-2"	NN	NA	2				
32X80 FRENCH A 1	2'-8"	L	NO	1				
36X80 COLONIAL A 1	3'-0"	R	NO	1				
192X84 - 8 PANEL GARAGE D	OR 16'-0"	U	NO	1				

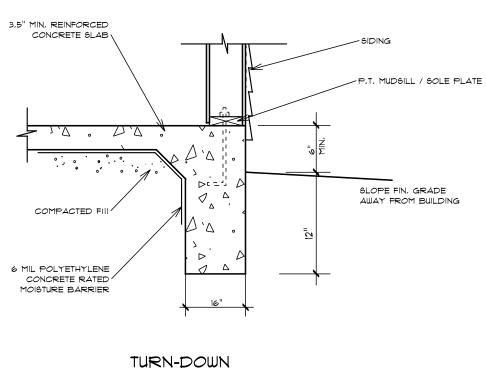
SECOND FLOOR OPENING SCHEDULE									
PRODUCT CODE	SIZE	HINGE	REVERSED	COUNT					
2-0 Door Unit	2'-0"	L	NO	1					
2-4 Door Unit	2'-4"	L	NO	1					
2-4 Door Unit	2'-4"	R	NO	4					
2-6 Door Unit	2'-6"	L	NO	1					
2-6 Door Unit	2'-6"	R	NO	2					
4-0 Doublehung Door Unit	4'-0"	LR	NO	2					
5-0 Doublehung Door Unit	5'-0"	LR	NO	1					
20x32 single	2'-0" x 3'-2"	N	NA	1					
24x46single	2'-4" x 4'-6"	N	NA	1					
28x52 single	2'-8" x 5'-2"	N	NA	4					

See Next Page

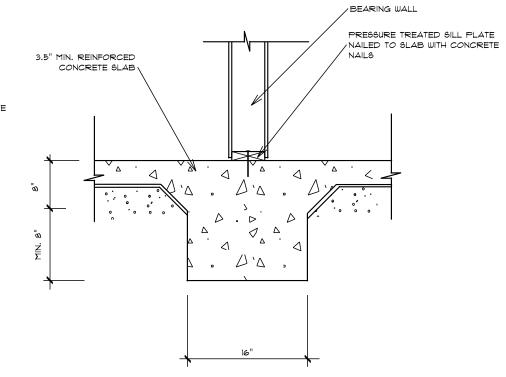
0 an #

SCALE: 1/4" DRAWN BY APPROVED





FOOTING DETAIL



INTEGRAL SLAB FOOTING

DETAIL AT BEARING WALL

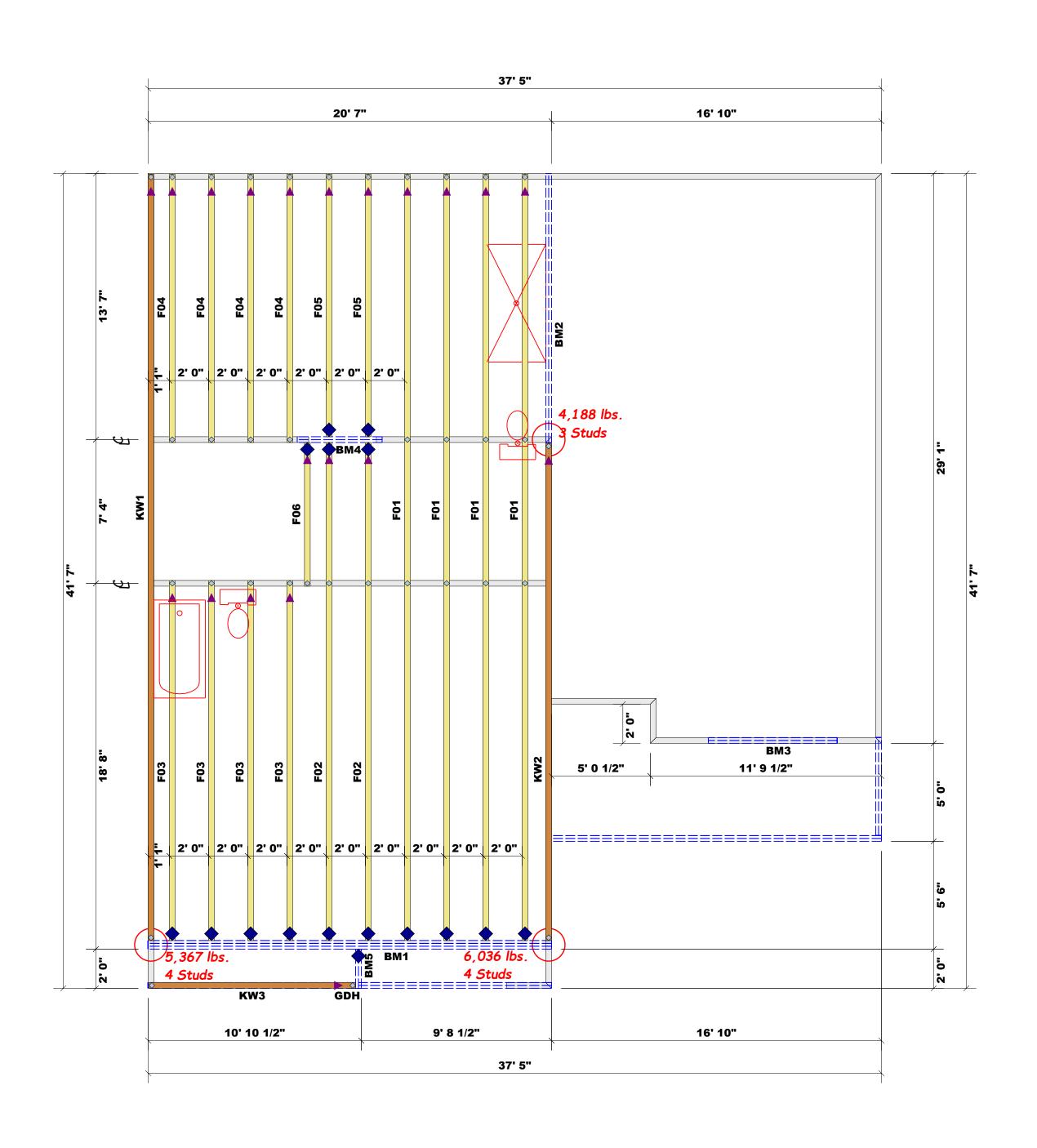
FIRST FLOOR OPENING SCHEDULE									
PRODUCT CODE	SIZE	HINGE	REVERSED	COUNT					
2-4 Door Unit	2'-4"	L	NO	1					
2-4 Door Unit	2'-4"	R	NO	2					
2-8 Door Unit	2'-8"	L	NO	1					
28x52 single	2'-8" x 5'-2"	N	NA	1					
28x52 twin	5'-4" x 5'-2"	NN	NA	2					
32X80 FRENCH A 1	2'-8"	L	NO	1					
36X80 COLONIAL A 1	3'-0"	R	NO	1					
192X84 - 8 PANEL GARAGE DR	16'-0"	U	NO	1					

SECOND FLOOR OPENING SCHEDULE									
PRODUCT CODE	SIZE	HINGE	REVERSED	COUNT					
2-0 Door Unit	2'-0"	L	NO	1					
2-4 Door Unit	2'-4"	L	NO	1					
2-4 Door Unit	2'-4"	R	NO	4					
2-6 Door Unit	2'-6"	L	NO	1					
2-6 Door Unit	2'-6"	R	NO	2					
4-0 Doublehung Door Unit	4'-0"	LR	NO	2					
5-0 Doublehung Door Unit	5'-0"	LR	NO	1					
20x32 single	2'-0" x 3'-2"	N	NA	1					
24x46single	2'-4" x 4'-6"	N	NA	1					
28x52 single	2'-8" x 5'-2"	N	NA	4					

|--|

Scale: 1/4"= 1'-0"

SCALE: 1/4"
DRAWN BY
APPROVED



HANGER LEGEND

= USP JUS414 / Single 4x Hanger

соттесн

ROOF & FLOOR

TRUSSES & BEAMS

Reilly Road Industrial Park

Fayetteville, N.C. 28309

Phone: (910) 864-8787 Fax: (910) 864-4444

▲ = Denotes Left End of Truss(Reference Engineered Truss Drawing)Do Not Erect Trusses Backwards

3400 1 6800 2

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

2550 1 5100 2

7650 3

10200 4 12750 5

15300 6

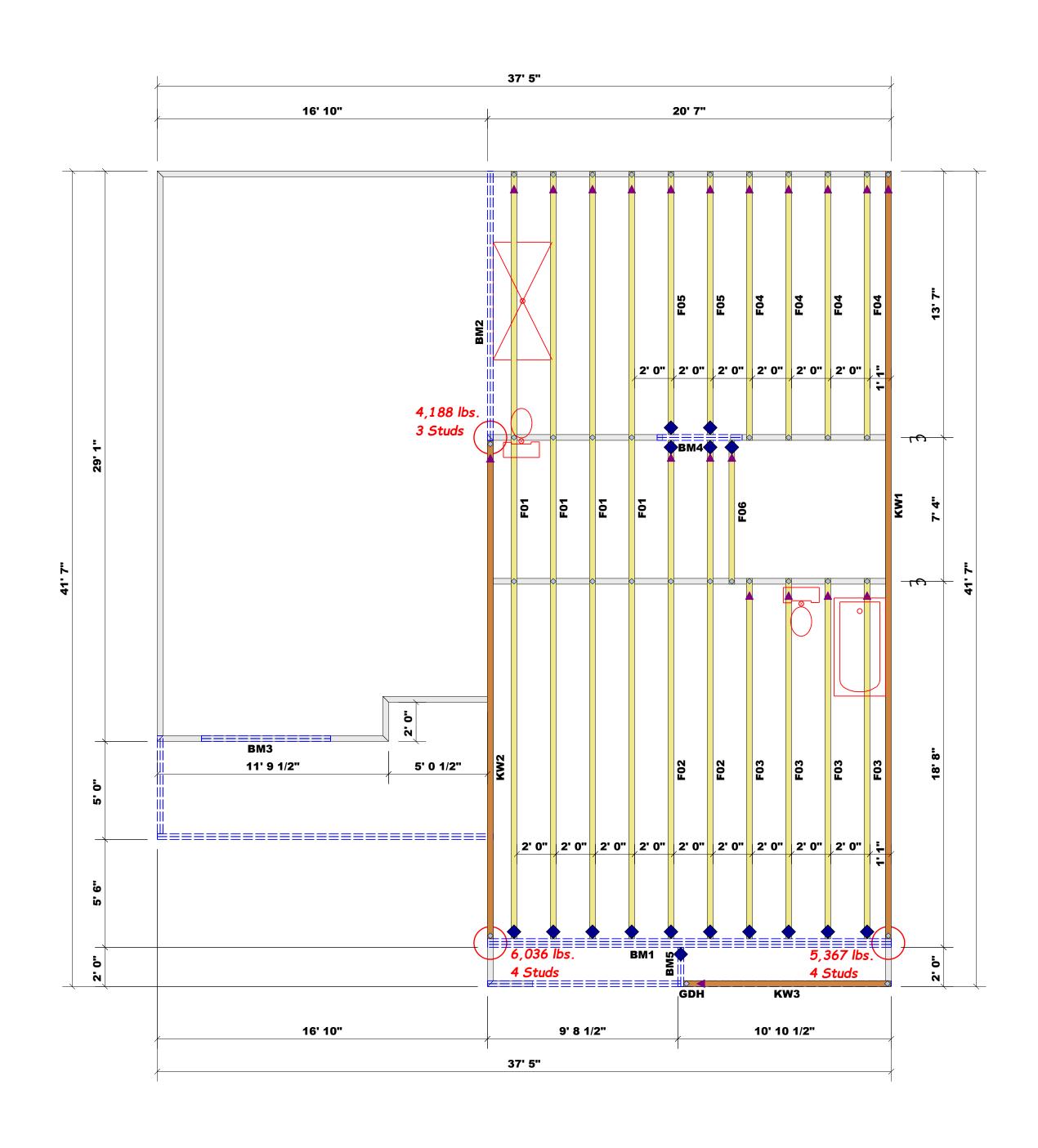
All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Truss Placement Plan SCALE: 1/4" = 1'

		Beam Legend			
PlotID	Length	Product	Plies	Net Qty	Fab Type
ВМ3	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
GDH	21' 0"	1-3/4"x 14" LVL Kerto-S	2	2	FF
BM2	14' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM4	5' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM5	2' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM1	21' 0"	1-3/4"x 18" LVL Kerto-S	3	3	FF

BUILDER	Wellco Contractors	CITY / CO.	Spring Lake / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer
JOB NAME	Lot 9 Overhills Creek	ADDRESS	98 Onslow Ct.	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package
PLAN	Plan 17	MODEL	Floor	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables
SEAL DATE	Seal Date	DATE REV.	05/02/24	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those
QUOTE#	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#. Signature Signature
JOB#	J0524-2606	SALES REP.	Lenny Norris	Curtis Quick



HANGER LEGEND

= USP JUS414 / Single 4x Hanger

соттесн

ROOF & FLOOR

TRUSSES & BEAMS

Reilly Road Industrial Park

Fayetteville, N.C. 28309

Phone: (910) 864-8787 Fax: (910) 864-4444

▲ = Denotes Left End of Truss(Reference Engineered Truss Drawing)Do Not Erect Trusses Backwards

6800 2

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS

(BASED ON TABLES P502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

5100 2

7650 3

10200 4 12750 5

15300 6

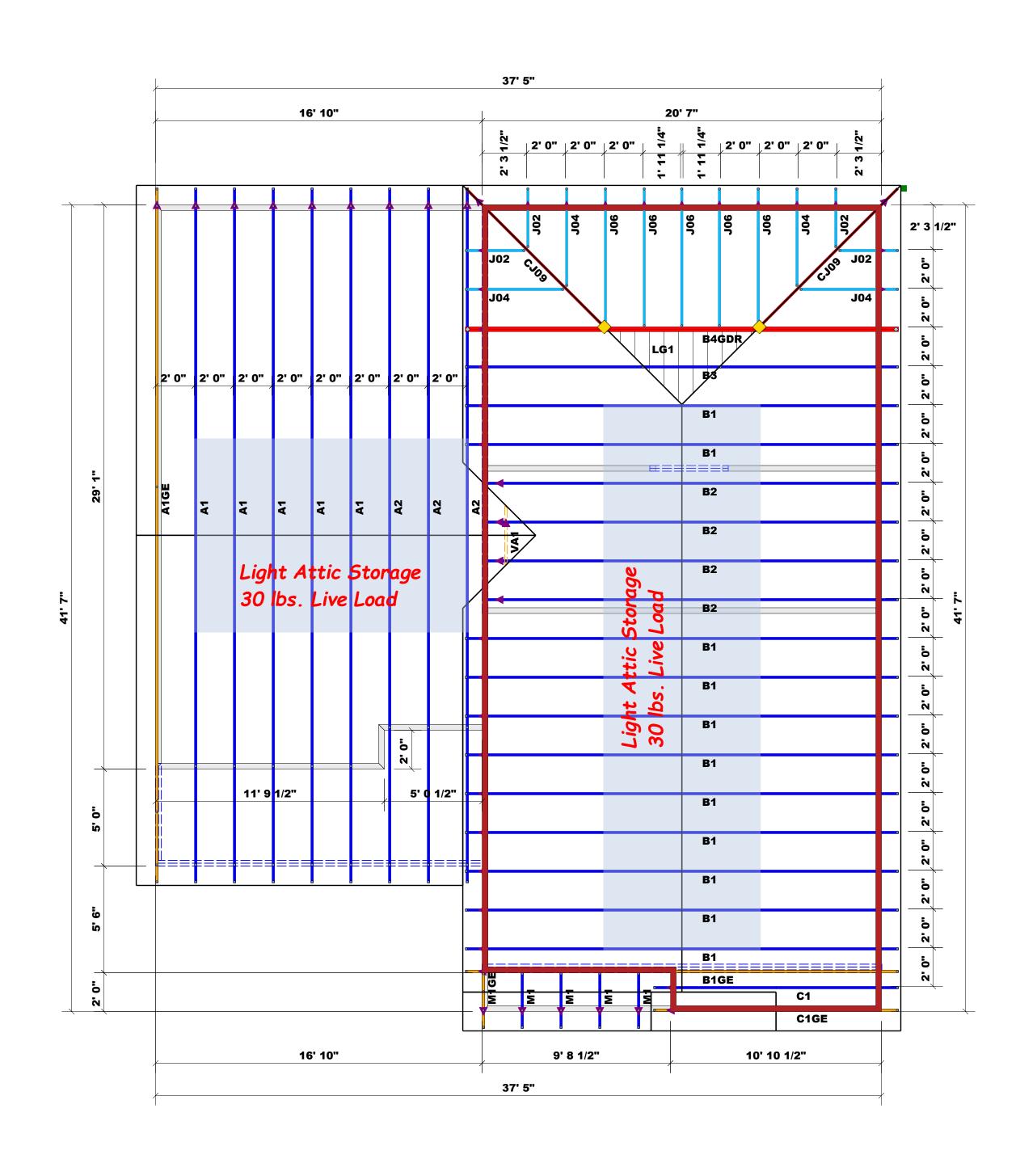
All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

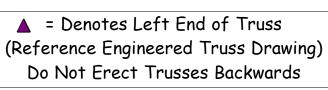
-- Denotes Reaction Greater than 3,000 lbs.

Truss Placement Plan SCALE: 1/4" = 1'

		Beam Legend			
PlotID	Length	Product	Plies	Net Qty	Fab Type
ВМ3	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
GDH	21' 0"	1-3/4"x 14" LVL Kerto-S	2	2	FF
BM2	14' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM4	5' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM5	2' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM1	21' 0"	1-3/4"x 18" LVL Kerto-S	3	3	FF

				·
BUILDER	Wellco Contractors	CITY / CO.	Spring Lake / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer
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PLAN	Plan 17	MODEL	Floor	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables
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QUOTE#	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.
JOB#	J0524-2606	SALES REP.	Lenny Norris	Curtis Quick





3400 1

6800 2

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS

(BASED ON TABLES P502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

5100 2

7650 3

10200 4

12750 5

15300 6

1700 1

3400 2

5100 3

6800 4

8500 5

10200 6

11900 7

13600 8

15300 9

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Truss Placement Plan SCALE: 1/4" = 1' Hatch Legend
2nd Floor Bearing Walls @ 8' 1-1/2"

HANGER LEGEND

= USP HJC26 / Hip Hanger

				SCALL. 1/4 = 1	
	BUILDER	Wellco Contractors	CITY / CO.	Spring Lake / Harnett	THIS IS A These trusse the building disheets for each
EADER	JOB NAME	Lot 9 Overhills Creek	ADDRESS	98 Onslow Ct.	is responsible the overall str walls, and co regarding bra
(+) P. - <u>-</u> <u>-</u>	PLAN	Plan 17	MODEL	Roof	or online @ s Bearing read prescriptive
	SEAL DATE	Seal Date	DATE REV.	05/02/24	(derived fro foundation s than 3000# i be retained
	QUOTE#	Quote #	DRAWN BY	Curtis Quick	specified in retained to d
	JOB#	J0524-2605	SALES REP.	Lenny Norris	Signatur

IS IS A TRUSS PLACEMENT DIAGRAM ONLY.

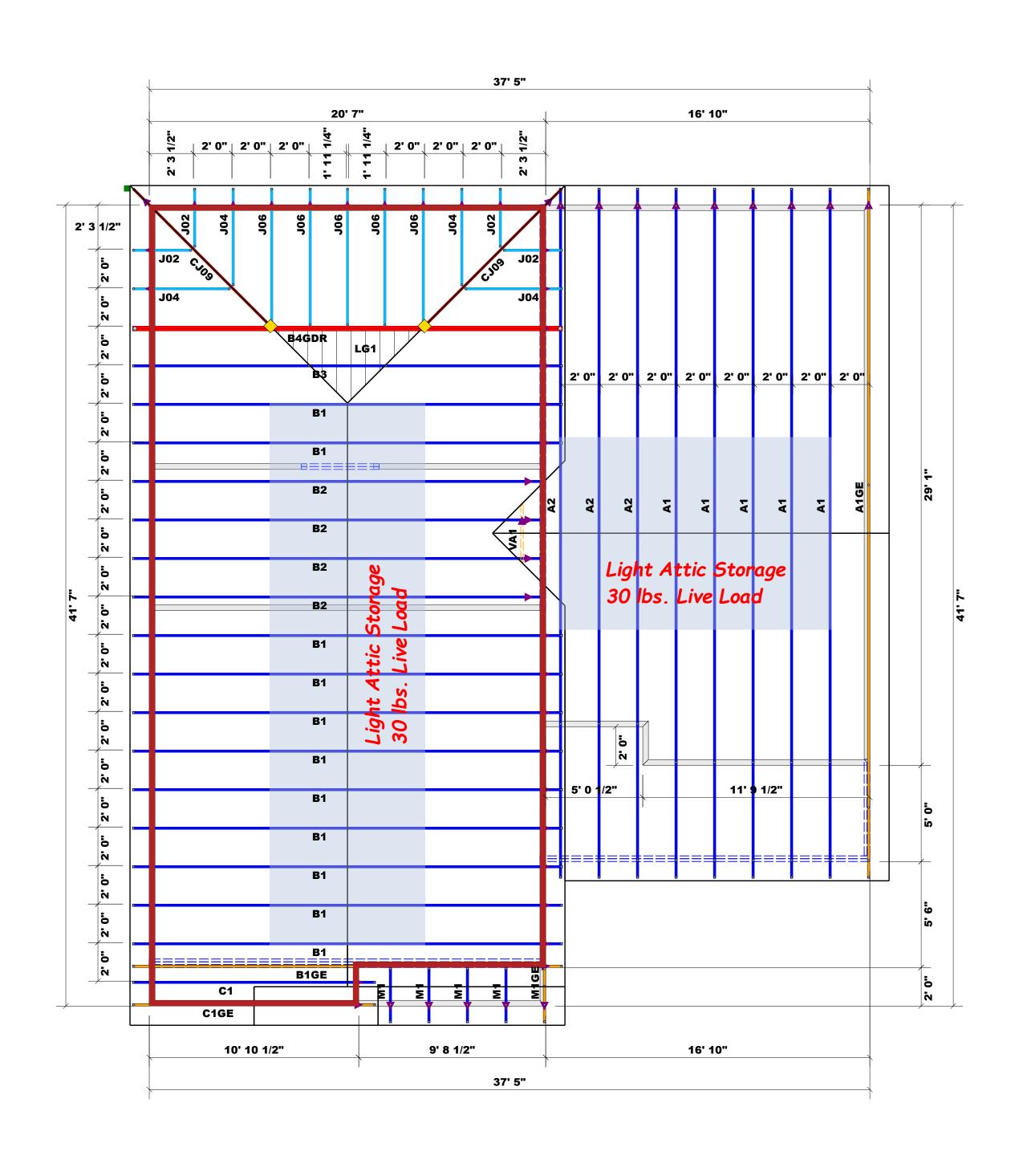
see trusses are designed as individual building components to be incorporated into building design at the specification of the building designer. See individual design ets for each truss design identified on the placement drawing. The building designer seponsible for temporary and permanent bracing of the roof and floor system and for overall structure. The design of the truss support structure including headers, beams, is, and columns is the responsibility of the building designer. For general guidance arding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package in the general guidance with the scriptive Code requirements. The contractor shall refer to the attached Tables the prived from the prescriptive Code requirements) to determine the minimum and ton size and number of wood studs required to support reactions greater in 3000# but not greater than 15000#. A registered design professional shall retained to design the support system for any reaction that exceeds those cified in the attached Tables. A registered design professional shall be timed to design the support system for all reactions that exceed 15000#.

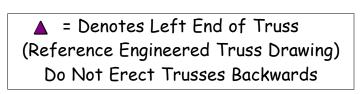
Signature

Curtis Quick



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444





3400 1

6800 2

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

5100 2

7650 3

10200 4

12750 5

15300 6

1700 1 3400 2

5100 3

6800 4 8500 5

10200 6

11900 7 13600 8 15300 9 All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Truss Placement Plan SCALE: 1/4" = 1' Hatch Legend
2nd Floor Bearing Walls @ 8' 1-1/2"

HANGER LEGEND

= USP HJC26 / Hip Hanger

BUILDER Wellco Contractors		CITY / CO. Spring Lake / Harnett		THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer			
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SEAL DATE	Seal Date	DATE REV.	05/02/24	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those			
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JOB#	J0524-2605	SALES REP.	Lenny Norris	Signature Curtis Quick			



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444



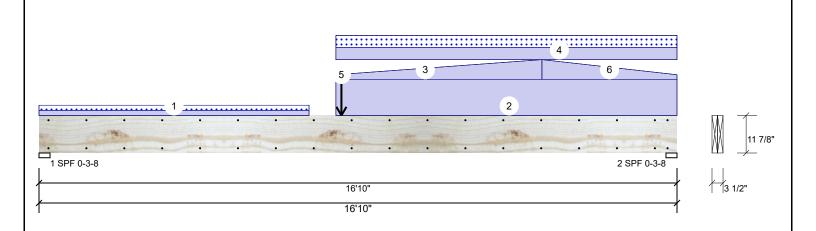
Project: Address:

5/8/2024 Input by: Curtis Quick Job Name: Plan 17 Beams Page 1 of 14

Project #:

1.750" X 11.875" 2-Ply - PASSED **Kerto-S LVL GDH**

Level: Level



Member Infor	mation			Read	ctions UNP	ATTERN	IED Ib	(Uplift)			
Type:	Girder	Application:	Floor	Brg	Direction	Live		Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	Vertical	0		764	190	0	0
Moisture Conditio	n: Dry	Building Code:	IRC 2018	2	Vertical	0		1549	291	0	0
Deflection LL:	480	Load Sharing:	No								
Deflection TL:	360	Deck:	Not Checked								
Importance:	Normal - II										
Temperature:	Temp <= 100°F										
				Bear	rings						
				Bea	aring Length	Dir.	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
				1 -	SPF 3.500"	Vert	18%	764 / 190	955	L	D+S
				2 -	SPF 3.500"	Vert	35%	1549 / 291	1840	L	D+S

Analysis Results

_						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5342 ft-lb	9'6 7/8"	17919 ft-lb	0.298 (30%)	D	Uniform
Unbraced	6333 ft-lb	9'6 13/16"	6340 ft-lb	0.999 (100%)	D+S	L
Shear	1311 lb	15'6 5/8"	7980 lb	0.164 (16%)	D	Uniform
LL Defl inch	0.050 (L/3912)	8'8 15/16"	0.409 (L/480)	0.123 (12%)	S	L
TL Defl inch	0.314 (L/626)	8'9 15/16"	0.546 (L/360)	0.575 (58%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 15'8 7/16" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

		3 1 7								
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Part. Uniform	0-0-0 to 7-1-8		Тор	17 PLF	0 PLF	17 PLF	0 PLF	0 PLF	M1
2	Part. Uniform	7-10-0 to 16-10-0		Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
3	Tapered Start	7-10-0		Тор	15 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Gable
	End	13-3-4			65 PLF	0 PLF	0 PLF	0 PLF	0 PLF	

Continued on page 2...

Notes			
Calculated Stru	ctured Desig	ns is respon	sible
structural adec	nuacy of this	component	has

structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. **Lumber**

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

isDesign

Client: Wellco Contractors

Project: Address: Date: 5/8/2024 Input by:

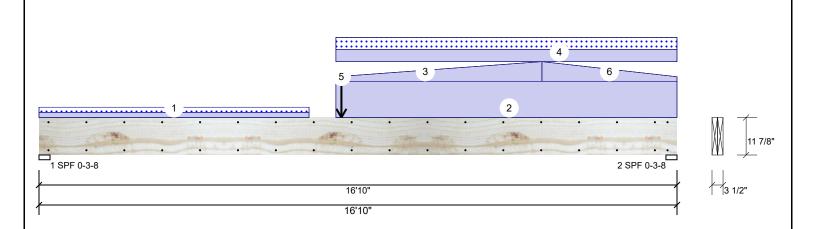
Curtis Quick Job Name: Plan 17 Beams

Project #:

Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED **GDH**

Level: Level

Page 2 of 14



Continued from p	.Continued from page 1									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
4	Part. Uniform	7-10-0 to 16-10-0		Тор	40 PLF	0 PLF	40 PLF	0 PLF	0 PLF	Overhang
5	Point	7-11-12		Тор	237 lb	0 lb	0 lb	0 lb	0 lb	BM6
	Bearing Length	0-3-8								
6	Tapered Start	13-3-4		Тор	65 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Gable
	End	16-10-0			15 PLF	0 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				9 PLF					

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

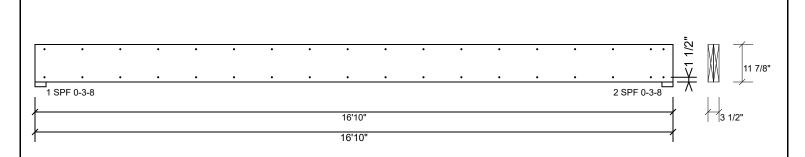
Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Client: Wellco Contractors 5/8/2024 Project: Input by: Curtis Quick isDesign Address: Job Name: Plan 17 Beams Project #:

1.750" X 11.875" 2-Ply - PASSED **GDH Kerto-S LVL**

Level: Level

Page 3 of 14



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

1 3	,
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
CM	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info



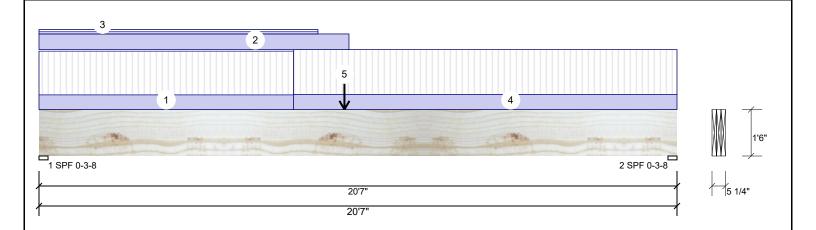
Project: Address: Date: 5/8/2024

Input by: Curtis Quick Job Name: Plan 17 Beams Page 4 of 14

Project #:

1.750" X 18.000" Kerto-S LVL 3-Ply - PASSED BM₁

Level: Level



Member Information Reactions UNPATTERNED Ib (Uplift) Application: Brg Direction Snow Wind Const Type: Floor Live Dead Plies: 3 Design Method: ASD 3481 2555 0 Vertical 120 0 1 Moisture Condition: Dry **Building Code:** IRC 2018 2 Vertical 3526 1841 33 0 0 Deflection LL: 480 Load Sharing: Yes Deflection TL: 240 Deck: Not Checked Importance: Normal - II Temperature: Temp <= 100°F **Bearings** Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.500" D+L Vert 2555 / 3481 6036 L 2 - SPF 3.500" Vert 69% 1841 / 3526 5367 L D+I

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	28788 ft-lb	9'10 1/4"	67051 ft-lb	0.429 (43%)	D+L	L
Unbraced	28788 ft-lb	9'10 1/4"	28935 ft-lb	0.995 (99%)	D+L	L
Shear	4983 lb	1'9 1/2"	20160 lb	0.247 (25%)	D+L	L
LL Defl inch	0.269 (L/900)	10'3 5/8"	0.504 (L/480)	0.534 (53%)	L	L
TL Defl inch	0.443 (L/546)	10'1 3/4"	1.007 (L/240)	0.439 (44%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 6'10 5/8" o.c.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width

7 Laterar 3	deliacificas fallo basca of	i single ply width.								
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Part. Uniform	0-0-0 to 8-2-8		Тор	112 PLF	335 PLF	0 PLF	0 PLF	0 PLF	F01
2	Part. Uniform	0-0-0 to 10-0-0		Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
3	Part. Uniform	0-0-0 to 9-0-0		Тор	17 PLF	0 PLF	17 PLF	0 PLF	0 PLF	M1
4	Part. Uniform	8-2-8 to 20-7-0		Тор	115 PLF	344 PLF	0 PLF	0 PLF	0 PLF	F02
5	Point	9-10-4		Тор	268 lb	0 lb	0 lb	0 lb	0 lb	BM5
	Bearing Length	0-3-8								
	Self Weight				21 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us



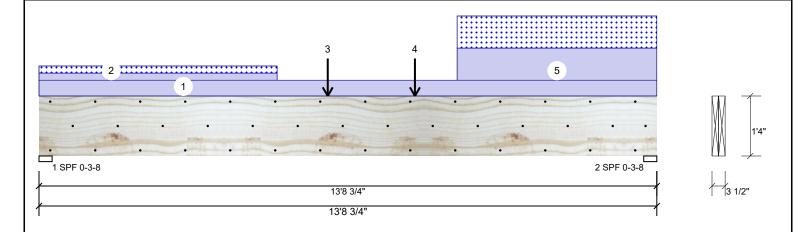
Project: Address: Date: 5/8/2024

Input by: Curtis Quick Job Name: Plan 17 Beams Page 5 of 14

Project #:

Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED BM₂

Level: Level



Member Information Reactions UNPATTERNED Ib (Uplift) Application: Live Wind Type: Floor Brg Direction Dead Snow Const Plies: 2 Design Method: ASD 0 1929 1020 0 Vertical 0 1 Moisture Condition: Dry **Building Code:** IRC 2018 2 Vertical 0 2549 1640 0 0 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temp <= 100°F Temperature: **Bearings** Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. D+S 1-SPF 3.500" Vert 1929 / 1020 2949 L

2 - SPF 3.500"

Vert

80%

2549 / 1640

4188 L

D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	13402 ft-lb	6'5"	39750 ft-lb	0.337 (34%)	D+S	L
Unbraced	13402 ft-lb	6'5"	13432 ft-lb	0.998 (100%)	D+S	L
Shear	3197 lb	12'1 1/4"	13739 lb	0.233 (23%)	D+S	L
LL Defl inch	0.076 (L/2091)	6'11 9/16"	0.332 (L/480)	0.230 (23%)	S	L
TL Defl inch	0.197 (L/808)	6'11 5/16"	0.443 (L/360)	0.446 (45%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 8'10 9/16" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

o Eateral sieriderness ratio based on single pry width.											
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall	
2	Part. Uniform	0-0-0 to 5-3-8		Тор	54 PLF	0 PLF	54 PLF	0 PLF	0 PLF	J04	
3	Point	6-5-0		Тор	839 lb	0 lb	839 lb	0 lb	0 lb	B4GDR	
	Bearing Length	0-3-8									

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

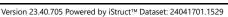
Manufacturer Info

Metsä Wood
301 Merritt 7 Building, 2nd Floor
Norwalk, CT 06851
(800) 622-5850
www.metsawood.com/us

Client: Wellco Contractors Date: 5/8/2024 Page 6 of 14 Project: Input by: Curtis Quick isDesign Address: Job Name: Plan 17 Beams Project #: 1.750" X 16.000" 2-Ply - PASSED Level: Level **Kerto-S LVL BM2** 5 1 1 SPF 0-3-8 2 SPF 0-3-8 13'8 3/4" 13'8 3/4" .Continued from page 1 Trib Width Dead 0.9 ID Load Type Location Side Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments Point 8-4-4 Тор 452 lb 0 lb 452 lb 0 lb ВЗ 4 Bearing Length 0-3-8 Part. Uniform 244 PLF 0 PLF 244 PLF 0 PLF 0 PLF B2 5 9-3-8 to 13-8-12 Тор Self Weight 12 PLF 6. For flat roofs provide proper drainage to prevent ponding Manufacturer Info NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Handling & Installation







Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

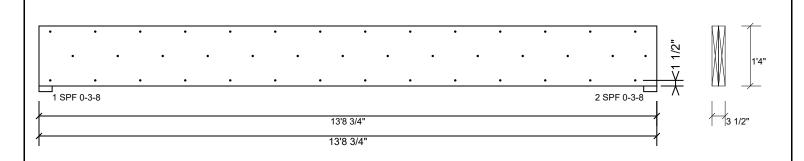
This design is valid until 6/28/2026

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Client: Wellco Contractors 5/8/2024 Page 7 of 14 Project: Input by: Curtis Quick isDesign Address: Job Name: Plan 17 Beams Project #:

1.750" X 16.000" 2-Ply - PASSED **Kerto-S LVL** BM₂

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1 00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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Manufacturer Info

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851



Project: Address: Date: 5/8/2024

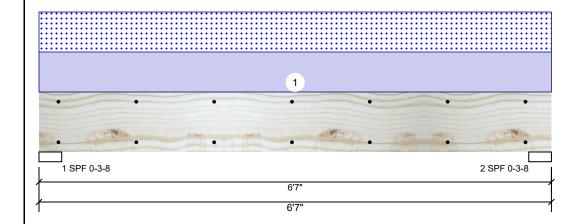
Input by: Curtis Quick Job Name: Plan 17 Beams

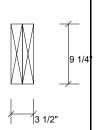
Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** BM₃

Level: Level

Reactions UNPATTERNED Ib (Uplift)





Page 8 of 14

Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application: Design Method: ASD **Building Code:** IRC 2018 Load Sharing: No

Not Checked

			` '			
Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1449	1425	0	0
2	Vertical	0	1449	1425	0	0

Bearings

Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	55%	1449 / 1425	2874	L	D+S
2 - SPF	3.500"	Vert	55%	1449 / 1425	2874	L	D+S

Analysis Results

ĺ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	4095 ft-lb	3'3 1/2"	14423 ft-lb	0.284 (28%)	D+S	L
l	Unbraced	4095 ft-lb	3'3 1/2"	10451 ft-lb	0.392 (39%)	D+S	L
l	Shear	1952 lb	1' 3/4"	7943 lb	0.246 (25%)	D+S	L
l	LL Defl inch	0.037 (L/1991)	3'3 1/2"	0.153 (L/480)	0.241 (24%)	S	L
l	TL Defl inch	0.074 (L/987)	3'3 1/2"	0.204 (L/360)	0.365 (36%)	D+S	L

Deck:

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	433 PLF	0 PLF	433 PLF	0 PLF	0 PLF	A1	
	Self Weight				7 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 2 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

isDesign

Client: Wellco Contractors

Project: Address: Date: 5/8/2024 Input by:

Curtis Quick Job Name: Plan 17 Beams

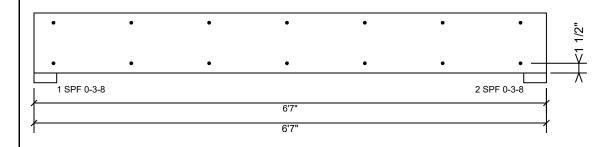
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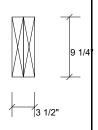
Kerto-S LVL BM3

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 9 of 14

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1 00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850

Manufacturer Info

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Client: Project:

Wellco Contractors

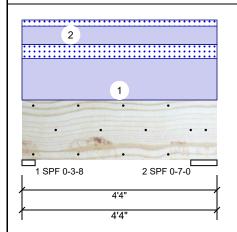
Date: 5/8/2024 Input by: Curtis Quick Job Name: Plan 17 Beams

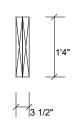
Project #:

1.750" X 16.000" 2-Ply - PASSED **Kerto-S LVL** BM4

Address:

Level: Level





Page 10 of 14

Member Information

Type: Plies: Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II

Temperature: Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	817	265	0	0
2	Vertical	0	935	303	0	0

Bearings

Bearing	Length	Dir.	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	21%	817 / 265	1082	L	D+S
2 - SPF	7.000"	Vert	12%	935 / 303	1238	L	D+S

Analysis Results

,						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	869 ft-lb	2' 1/4"	39750 ft-lb	0.022 (2%)	D+S	L
Unbraced	869 ft-lb	2' 1/4"	29979 ft-lb	0.029 (3%)	D+S	L
Shear	229 lb	2'5"	13739 lb	0.017 (2%)	D+S	L
LL Defl inch	0.001 (L/66978)	2' 5/16"	0.090 (L/480)	0.007 (1%)	S	L
TL Defl inch	0.003 (L/16387)	2' 5/16"	0.120 (L/360)	0.022 (2%)	D+S	L

Application:

Design Method:

Building Code:

Load Sharing:

Deck:

Floor

ASD

No

IRC 2018

Not Checked

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	272 PLF	0 PLF	91 PLF	0 PLF	0 PLF	F05
2	Uniform			Тор	120 PLF	0 PLF	40 PLF	0 PLF	0 PLF	F02
	Self Weight				12 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

This design is valid until 6/28/2026



Client:

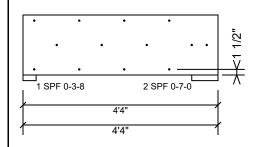
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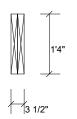
Input by: Curtis Quick Job Name: Plan 17 Beams

Project #:

1.750" X 16.000" 2-Ply - PASSED **Kerto-S LVL BM4**

Level: Level





Page 11 of 14

Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

	,	
Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
См	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

(800) 622-5850

Manufacturer Info

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 www.metsawood.com/us



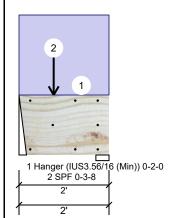
Project: Address: Date: 5/8/2024

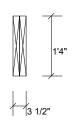
Input by: Curtis Quick
Job Name: Plan 17 Beams

Project #:

BM5 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED

Level: Level





Page 12 of 14

Member Info	rmation			Rea	ctions UNP	ATTERN	IED Ib	(Uplift)			
Type:	Girder	Application:	Floor	Brg	Direction	Live		Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	Vertical	0		196	72	0	0
Moisture Condition	on: Dry	Building Code:	IRC 2018	2	Vertical	0		189	48	0	0
Deflection LL:	480	Load Sharing:	No								
Deflection TL:	360	Deck:	Not Checked								
Importance:	Normal - II										
Temperature:	Temp <= 100°F										
				Bea	rings						
				Bea	aring Length	Dir.	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
				1 -	2.000"	Vert	5%	196 / 72	268	L	D+S
				Hai	nger						
Analysis Resu	lts			2 -	SPF 3.500"	Vert	5%	189 / 48	237	L	D+S
Analysis A	etual Leastion	Allowed Conce	ity Comb Co	20							

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	143 ft-lb	9 1/4"	39750 ft-lb	0.004 (0%)	D+S	L
Unbraced	143 ft-lb	9 1/4"	39187 ft-lb	0.004 (0%)	D+S	L
Shear	52 lb	1'6"	10752 lb	0.005 (0%)	D	Uniform
LL Defl inch	0.000 (L/187413)	9 1/4"	0.042 (L/480)	0.003 (0%)	S	L
TL Defl inch	0.000 (L/63485)	9 1/4"	0.056 (L/360)	0.006 (1%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Fill all hanger nailing holes.
- 5 Left Header: SPF, Thickness: 3 1/2"
- 6 Girders are designed to be supported on the bottom edge only.
- 7 Top loads must be supported equally by all plies.
- 8 Top must be laterally braced at end bearings.
- 9 Bottom must be laterally braced at end bearings.
- 10 Lateral slenderness ratio based on single ply width.

Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended	Handling & Installation 1. LVL beams must not be cut or drilled 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code	ponding	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850	
application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	approvals approvals about the state of the s	This design is valid until 6/28/2026	www.metsawood.com/us	



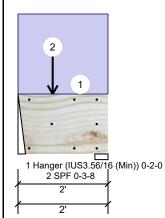
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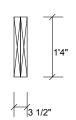
Input by: Curtis Quick Job Name: Plan 17 Beams

Project #:

Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED BM5

Level: Level





Page 13 of 14

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
2	Point	0-9-4		Тор	120 lb	0 lb	120 lb	0 lb	0 lb	C1
	Bearing Length	0-3-8								
	Self Weight				12 PLF					

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. IVI. beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850

Manufacturer Info

www.metsawood.com/us



Project: Address: Date: 5/8/2024 Input by:

Curtis Quick Job Name: Plan 17 Beams

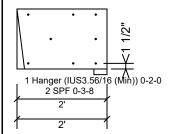
Project #:

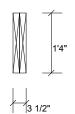
Kerto-S LVL BM₅

1.750" X 16.000"

2-Ply - PASSED

Level: Level





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Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

	/	
Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
См	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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