REVISION LOG

REVISION:001

DATF: 2-9-24

 PROTO-WALK CHANGES MADE TO THE PLANS. SQUARE FOOTAGE ADJUSMENTS MADE AS A RESULT. 131 DC - 403 Duncan Creek Rd., Lillington, NC 27546

The Guilford French Country - RH

NC.

10-15-24 - Redlines - JJ

10-25-24 - replace structural sheets - JJ



Crawl Space Foundation

	ARCHITECTURAL DRAWINGS
Sheet No.	Sheet Description
0.0	Cover Sheet
1.0	Foundation (Slab)
1.0.1	Foundation (Crawl)
2.0	First Floor Plan
2.1	First Floor Plan Options
2.2	Second Floor Plan
2.3	Alternate Second Floor Plan
3.0	Front & Rear Elevations (Slab)
3.0.1	Front & Rear Elevations (Crawl)
3.1	Side Elevations (Slab)
3.1.1	Side Elevations (Crawl)
3.2	Elevation Options (Slab)
3.2.1	Elevation Options (Crawl)
3.3	Elevation Options (Slab)
3.3.1	Elevation Options (Crawl)
3.4	Elevations FOr Optional Second Floor (Crawl)
3.4.1	Elevations FOr Optional Second Floor (Slab)
4.0	Roof Plan
4.0.1	Roof Plan for Optional Second Floor
5.0	First Floor Electrical
5.0.1	First Floor Options Electrical
5.1	Second Floor Electrical
5.1.1	Second Floor Options Electrical

SOLIAR	E FOOTA	GF	1
3QU/III		TRY' ELEVATION	1
	UNHEATED	HEATED	1
FIRST FLOOR	0	1519	1
SECOND FLOOR	0	860	1
FRONT PORCH	27	0]
EQUIPMENT ROOM	61	0	
REAR PATIO/DECK	144	0	1
2 CAR GARAGE	394	0	1
SUBTOTALS	626	2379	-
TOTAL UNDER ROOF	30	05	_ 2523 Heated
			675 unheated
	PTIONS		
	UNHEATED S.F.	HEATED S.F.	
OPTIONAL CAFE/OFFICE	0	+144	
SECOND FLOOR B	Û	+21	1
ALTERNATE SECOND FLOOR A	0	+215	1
ALTERNATE SECOND	0	+238	<u></u>
FLOOR B	Ů	1230	4
OPT. 2ND FL OPTIONAL POCKET OFFICE	-70	+70	├
EQUIPMENT ROOM	+114	0	
OPTIONAL REAR COV. PATIO/DECK W/ OPT. CAFE/OFFICE	193	0]

DESIGN CRITERIA:

THIS PLAN IS TO BE BUILT IN CONFORMANCE WITH THE 2018 NORTH CAROLINA STATE BUILDING CODE: RESIDENTIAL CODE

DIMENSIONS SHALL GOVERN OVER SCALE, AND CODE SHALL GOVERN OVER DIMENSIONS.



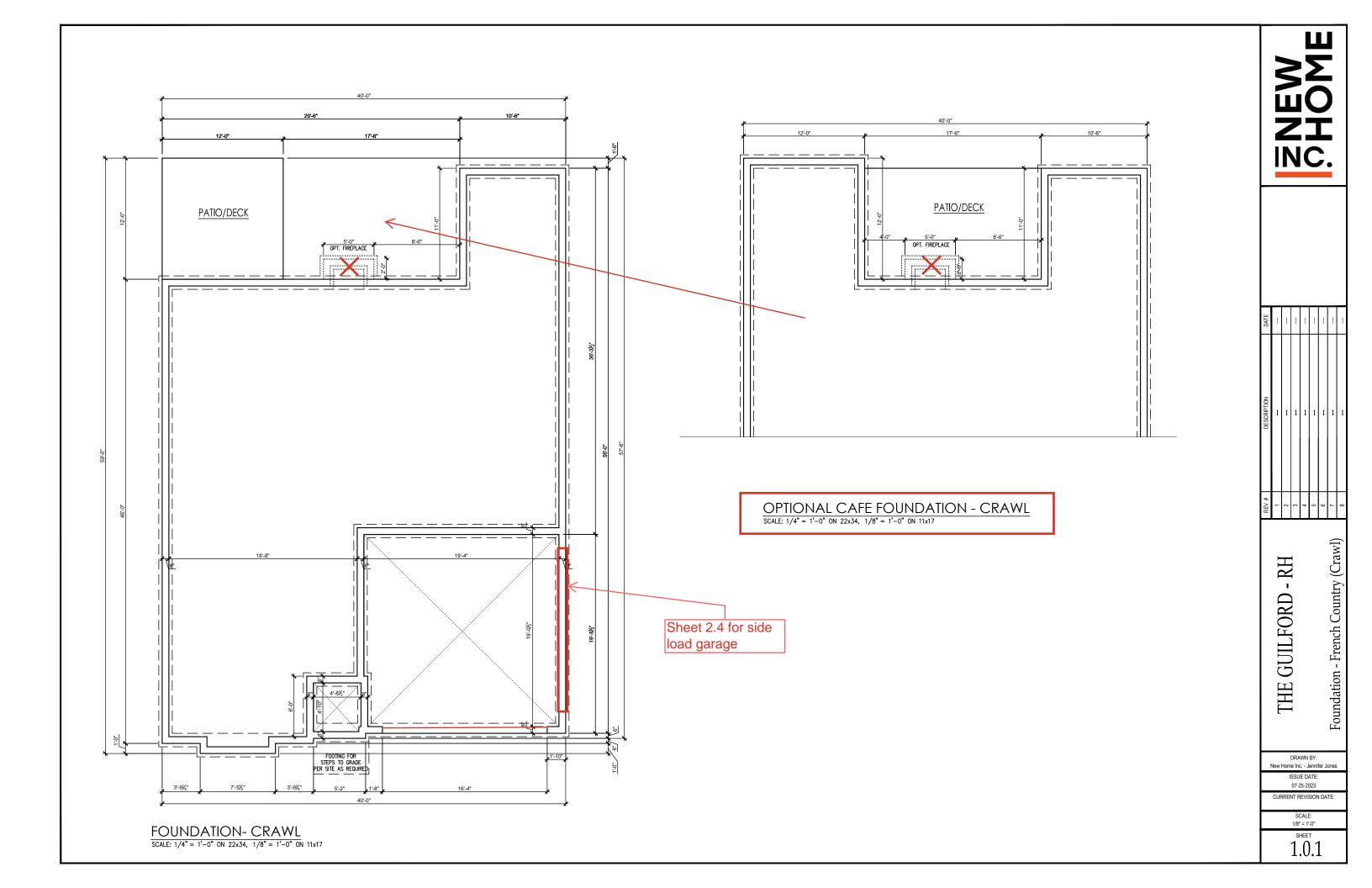
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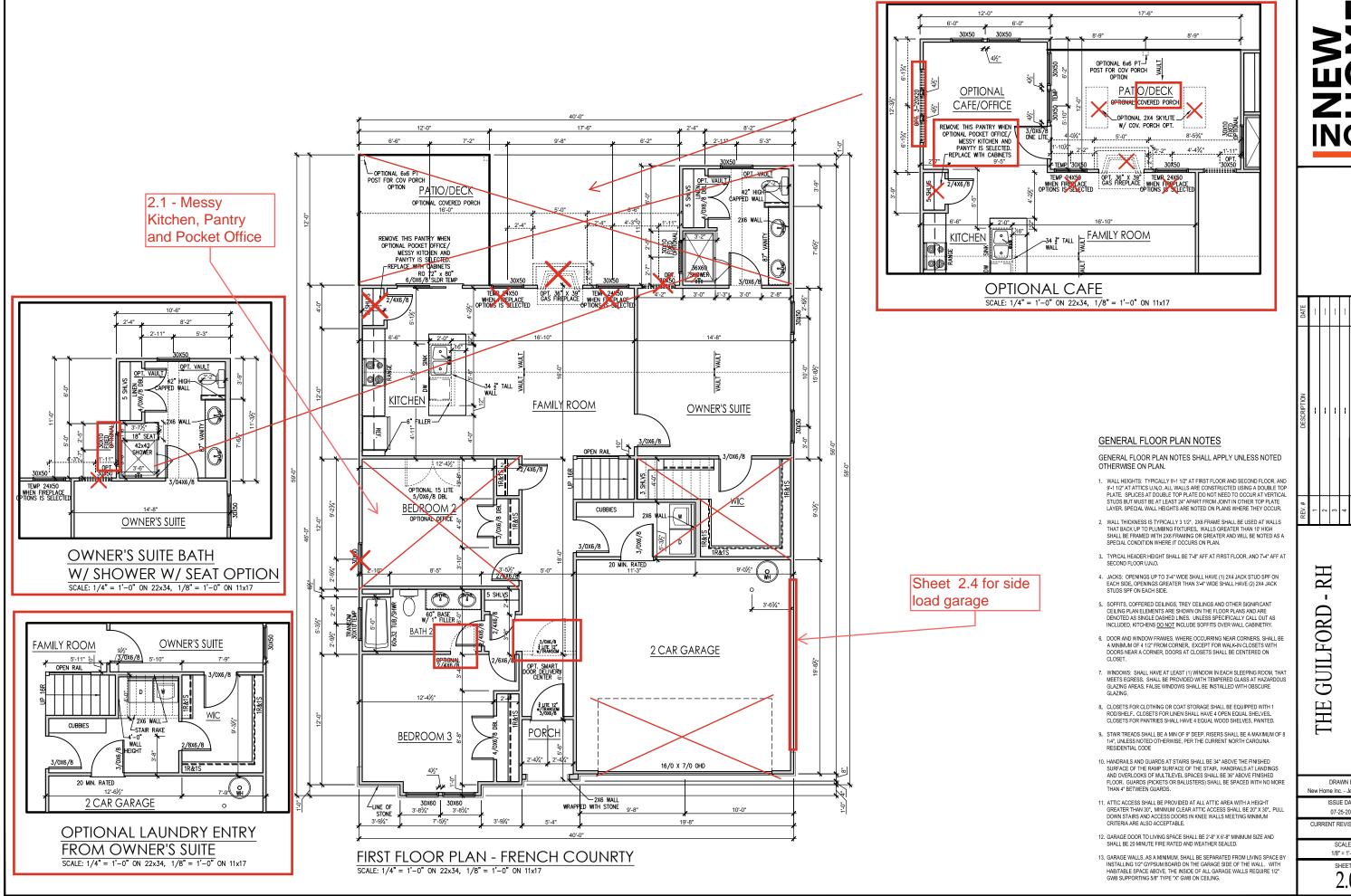
THE GUILFORD - RH

DRAWN BY:
New Home Inc. - Jennifer Jones
ISSUE DATE:

07-25-2023 CURRENT REVISION DATE

SCALE: 1/8" = 1'-0"





DRAWN BY: New Home Inc. - Jennifer Jone

Country

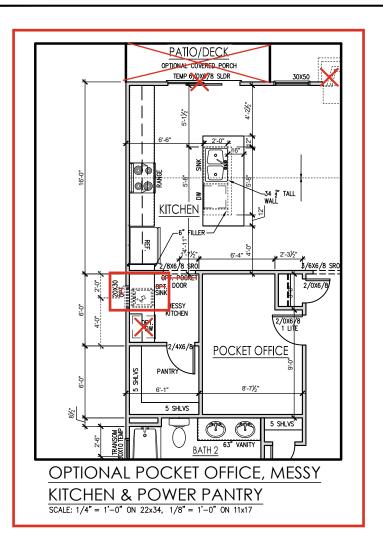
First Floor Plan - French

ISSUE DATE: 07-25-2023

CURRENT REVISION DATE:

SCALE: 1/8" = 1'-0"

SHEET 2.0



NEW HOME

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RH

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GUILFORD

THE

GENERAL FLOOR PLAN NOTES

GENERAL FLOOR PLAN NOTES SHALL APPLY UNLESS NOTED OTHERWISE ON PLAN.

- WALL HEIGHTS: TYPICALLY 9'-1 1/2" AT FIRST FLOOR AND SECOND FLOOR, AND 9'-1 1/2" AT ATTICS U.N.O. ALL WALLS ARE CONSTRUCTED USING A DOUBLE TOP PLATE. SPLICES AT DOUBLE TOP PLATE DO NOT NEED TO OCCUR AT VERTICAL STUDS BUT MUST BE AT LEAST 24" APART FROM JOIN TO OTHER TOP PLATE LAYER. SPECIAL WALL HEIGHTS ARE NOTED ON PLANS WHERE THEY OCCUR.
- WALL THICKNESS IS TYPICALLY 3 1/2". 2X6 FRAME SHALL BE USED AT WALLS THAT BACK UP TO PLUMBING FIXTURES. WALLS GREATER THAN 10' HIGH SHALL BE FRAMED WITH JOS FRAMING OR GREATER AND WILL BE NOTED AS A SPECIAL CONDITION WHERE IT OCCURS ON PLAN.
- 3. TYPICAL HEADER HEIGHT SHALL BE 7'-8" AFF AT FIRST FLOOR, AND 7'-4" AFF AT SECOND FLOOR UNIO
- 4. JACKS: OPENINGS UP TO 3'-4" WIDE SHALL HAVE (1) 2X4 JACK STUD SPF ON EACH SIDE. OPENINGS GREATER THAN 3'-4" WIDE SHALL HAVE (2) 2X4 JACK STUDS SPF ON EACH SIDE.
- SOFFITS, COFFERED CEILINGS, TREY CEILINGS AND OTHER SIGNIFICANT CEILING PLAN ELEMENTS ARE SHOWN ON THE FLOOR PLANS AND ARE DENOTED AS SINGLE DASHED LINES. UNIESS SPECIFICALLY CALL OUT AS INCLUDED, KITCHENS <u>DO NOT</u> INCLUDE SOFFITS OVER WALL CABINETRY.
- DOOR AND WINDOW FRAMES, WHERE OCCURRING NEAR CORNERS, SHALL BE A MINIMUM OF 4 1/2" FROM CORNER. EXCEPT FOR WALK-IN CLOSETS WITH DOORS NEAR A CORNER, DOORS AT CLOSETS SHALL BE CENTERED ON CLOSET.
- WINDOWS: SHALL HAVE AT LEAST (1) WINDOW IN EACH SLEEPING ROOM, THAT
 MEETS EGRESS. SHALL BE PROVIDED WITH TEMPERED GLASS AT HAZARDOUS
 GLAZING AREAS, FALSE WINDOWS SHALL BE INSTALLED WITH OBSCURE
 GLAZING.
- CLOSETS FOR CLOTHING OR COAT STORAGE SHALL BE EQUIPPED WITH 1
 RODISHELF. CLOSETS FOR LINEN SHALL HAVE 4 OPEN EQUAL SHELVES.
 CLOSETS FOR PANTRIES SHALL HAVE 4 EQUAL WOOD SHELVES, PAINTED.
- 9. STAIR TREADS SHALL BE A MIN OF 9" DEEP, RISERS SHALL BE A MAXIMUM OF 8 1/4", UNLESS NOTED OTHERWISE, PER THE CURRENT NORTH CAROLINA RESIDENTIAL CODE
- 10. HANDRAILS AND GUARDS AT STAIRS SHALL BE 34" ABOVE THE FINISHED SURFACE OF THE RAMP SURFACE OF THE STAIR, HANDRAILS AT LANDINGS AND OVERLOOKS OF MULTILEVEL SPACES SHALL BE 36" ABOVE FINISHED FLOOR, GUARDS (PICKETS OF BALUSTERS) SHALL BE SPACED WITH NO MORE THAN 4" BETWEEN GUARDS.
- 11. ATTIC ACCESS SHALL BE PROVIDED AT ALL ATTIC AREA WITH A HEIGHT GREATER THAN 30". MINIMUM CLEAR ATTIC ACCESS SHALL BE 20" X 30". PULL DOWN STAIRS AND ACCESS DOORS IN KNEE WALLS MEETING MINIMUM CRITERIA ARE ALSO ACCEPTABLE.
- 12. GARAGE DOOR TO LIVING SPACE SHALL BE 2'-8" X 6'-8" MINIMUM SIZE AND SHALL BE 20 MINUTE FIRE RATED AND WEATHER SEALED.
- 13. GARAGE WALLS, AS A MINIMUM, SHALL BE SEPARATED FROM LIVING SPACE BY INSTALLING 1/2" GYPSUM BOARD ON THE GARAGE SIDE OF THE WALL. WITH HABITABLE SPACE ABOVE, THE INSIDE OF ALL GARAGE WALLS REQUIRE 1/2" GWB SUPPORTING 5/6" TYPE" X" GWB ON CEILING.

DRAWN BY:

First Floor Options

New Home Inc. - Jennifer Jones

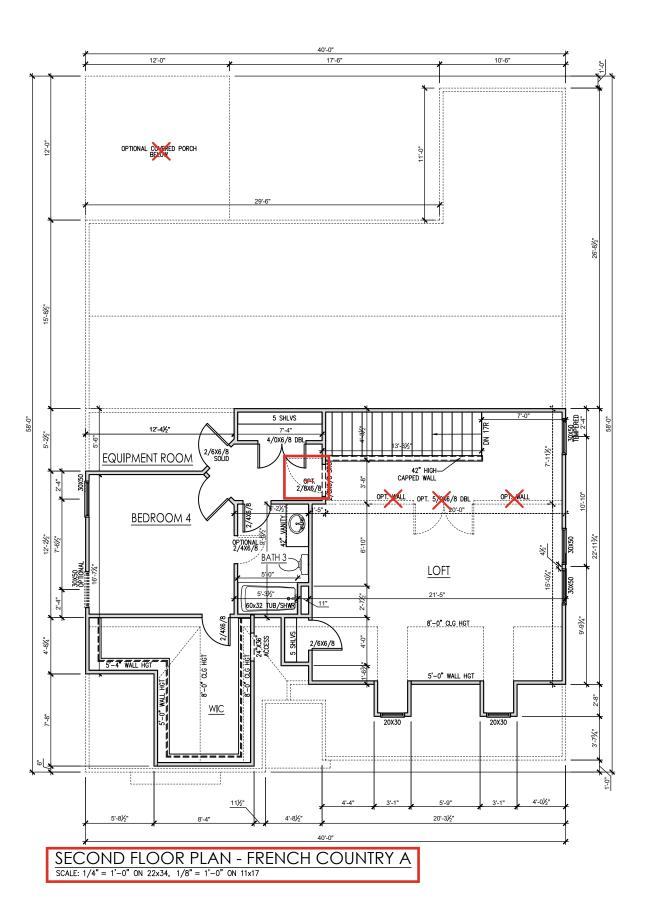
07-25-2023 CURRENT REVISION DATE:

> SCALE: 1/8" = 1'-0"

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First Floor Options

ISSUE DATE: 07-25-2023 CURRENT REVISION DATE:

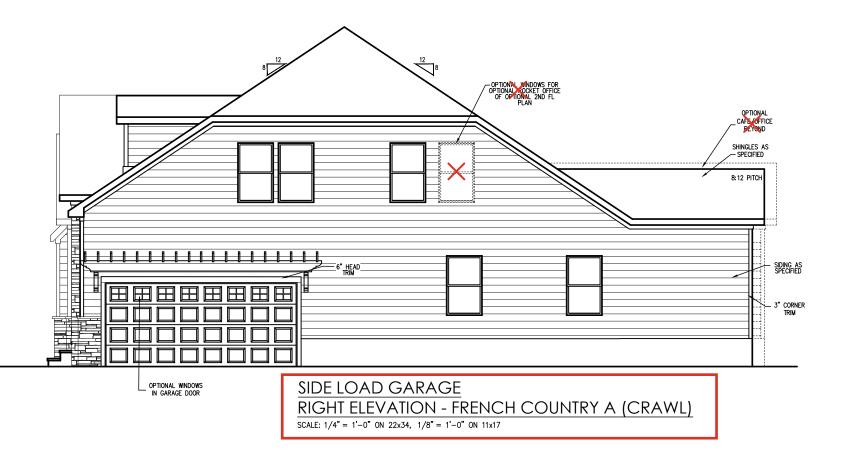
THE GUILFORD

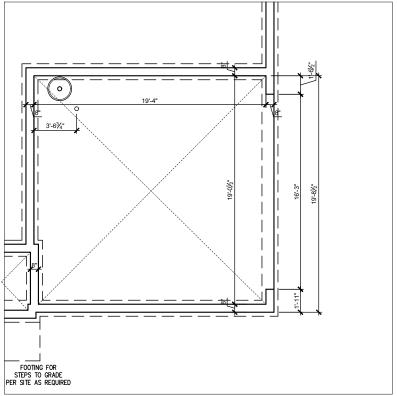
SCALE: 1/8" = 1'-0"

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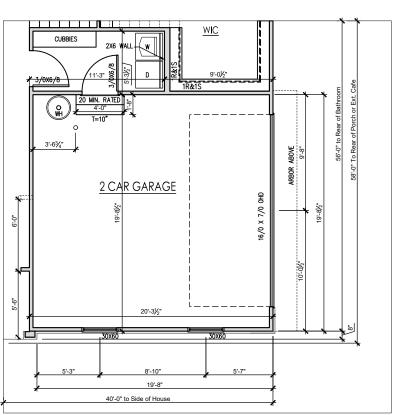


SIDE LOAD GARAGE FRONT ELEVATION FRENCH COUNTRY A (SLAB) SCALE: 1/4" = 1'-0" ON 22x34, 1/8" = 1'-0" ON 11x17





SIDE LOAD GARAGE FOUNDATION- CRAWL SCALE: 1/4" = 1'-0" ON 22x34, 1/8" = 1'-0" ON 11x17



SIDE LOAD GARAGE
FIRST FLOOR PLAN - FRENCH COUNRTY
SCALE: 1/4" = 1'-0" ON 22x34, 1/8" = 1'-0" ON 11x17

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Side Load Garage

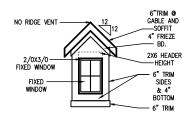
THE GUILFORD - RH

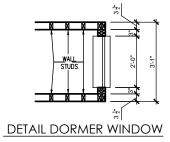
DRAWN BY:
New Home Inc. - Jennifer Jones
ISSUE DATE:

07-25-2023
CURRENT REVISION DATE:

SCALE: 1/8" = 1'-0"

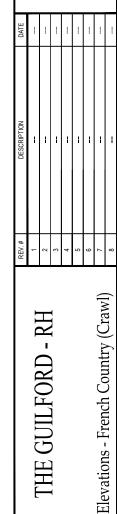






 $\frac{\mathsf{FRONT}\;\mathsf{ELEVATION}\;\mathsf{-}\;\mathsf{FRENCH}\;\mathsf{COUNTRY}\;\mathsf{A}\;\left(\mathsf{CRAWL}\right)}{\mathsf{SCALE}:\;1/4"=\;1'-0"\;\mathsf{ON}\;\;22x34,\;\;1/8"=\;1'-0"\;\;\mathsf{ON}\;\;11x17}$

Side Loade Garage Sheet 2.4



THE GUILFORD - RH

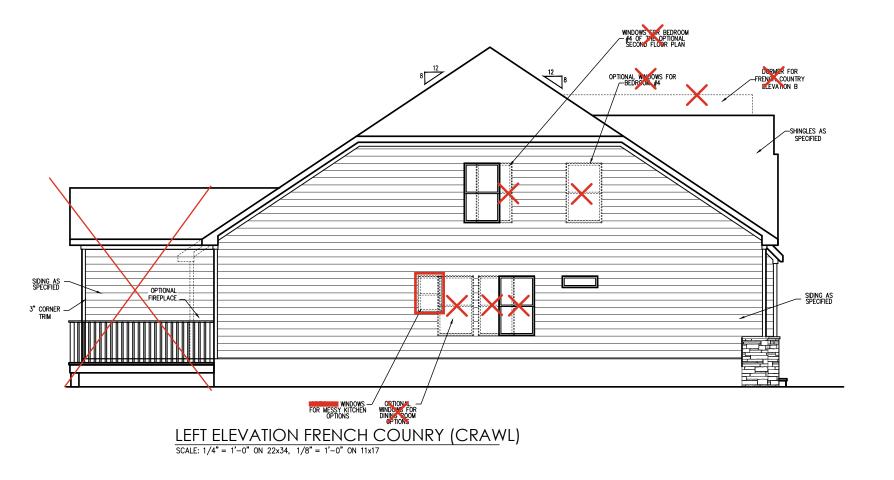
DRAWN BY: New Home Inc. - Jennifer Jones

ISSUE DATE: 07-25-2023

CURRENT REVISION DATE:

SCALE: 1/8" = 1'-0"

3.0.1



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THE GUILFORD - RH

Side Elevations - French Country (Crawl)

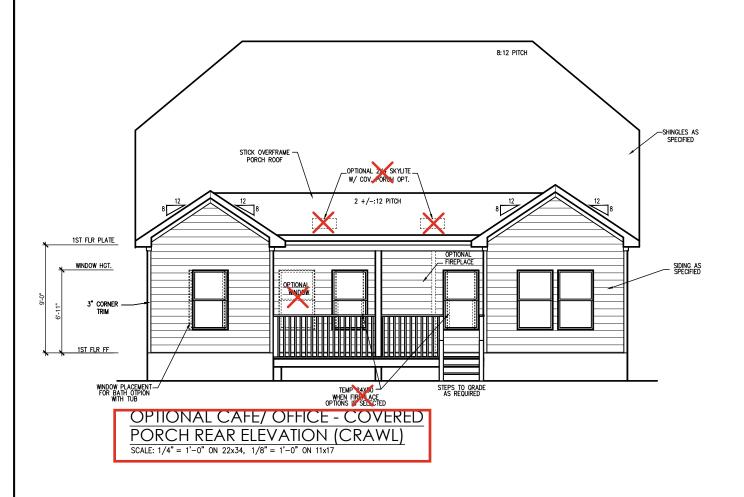
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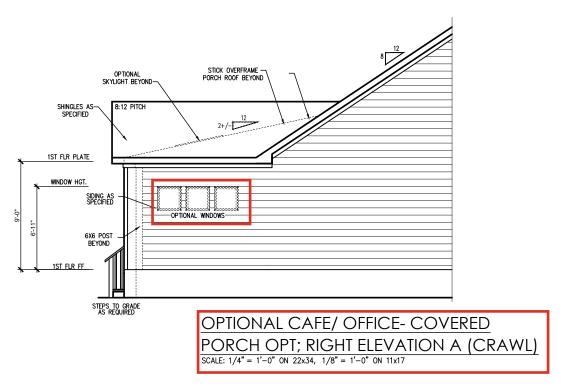
v Home Inc. - Jennifer J ISSUE DATE: 07-25-2023

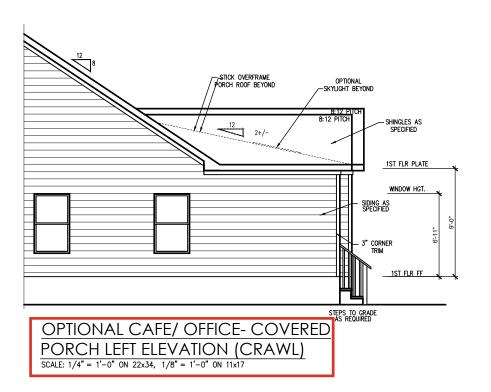
07-25-2023 CURRENT REVISION DATE:

> SCALE: 1/8" = 1'-0"

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THE GUILFORD - RH

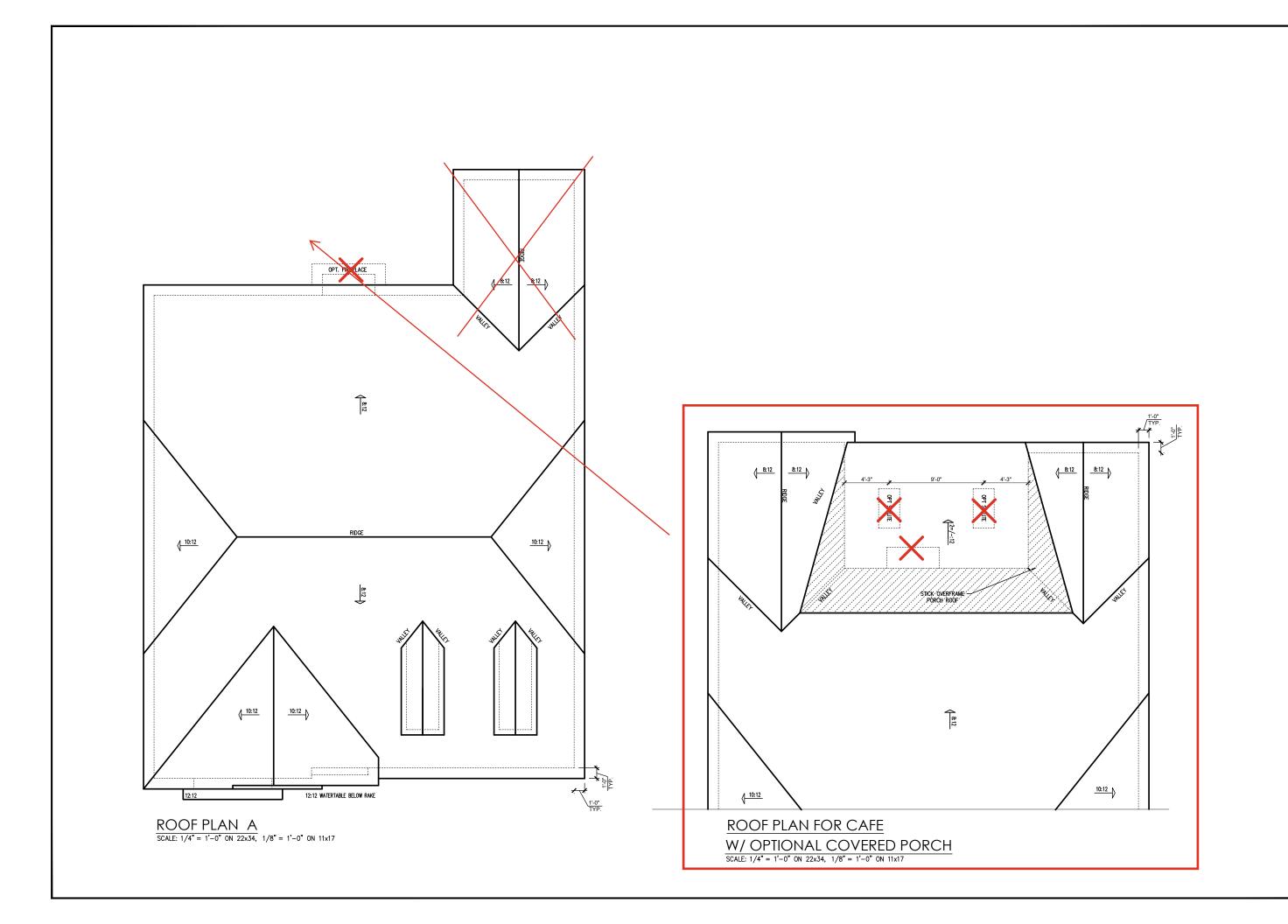
Elevation Options (Crawl)

DRAWN BY: New Home Inc. - Jennifer Jones

ISSUE DATE: 07-25-2023

CURRENT REVISION DATE:

SCALE: 1/8" = 1'-0" SHEET 3.3.1





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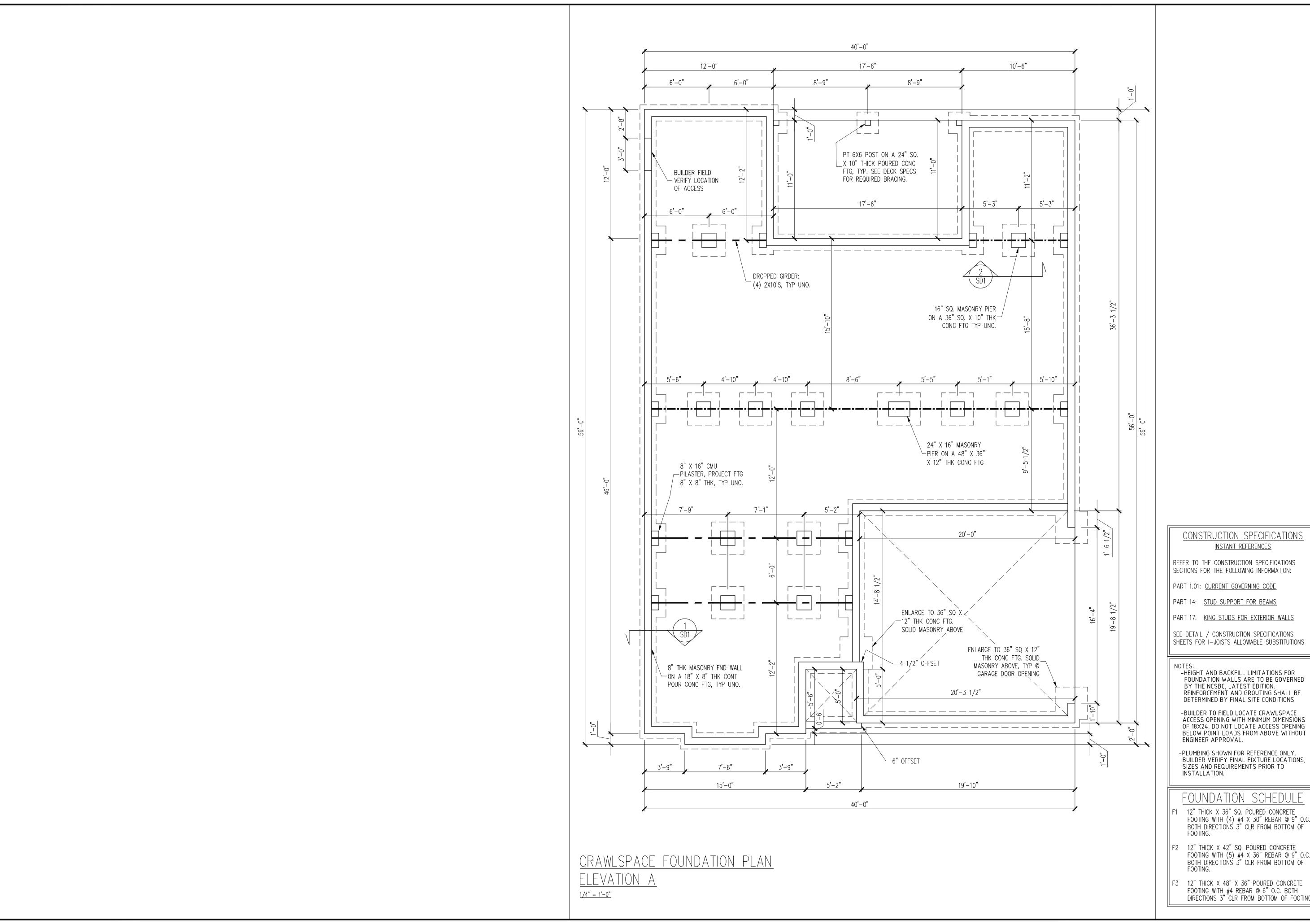
THE GUILFORD - RH

Roof Plan - French Country

DRAWN BY: New Home Inc. - Jennifer Jones

ISSUE DATE: 07-25-2023 CURRENT REVISION DATE:

> SCALE: 1/8" = 1'-0"



ENDUM

REV # REF PROJ #

1 24-66-124

2 24-66-245 NEW HON STRUCTURAL A 131 DUNCANS CREEK LILLINGTON, NC

SEE DETAIL / CONSTRUCTION SPECIFICATIONS SHEETS FOR I-JOISTS ALLOWABLE SUBSTITUTIONS -HEIGHT AND BACKFILL LIMITATIONS FOR

-BUILDER TO FIELD LOCATE CRAWLSPACE ACCESS OPENING WITH MINIMUM DIMENSIONS OF 18X24. DO NOT LOCATE ACCESS OPENING BELOW POINT LOADS FROM ABOVE WITHOUT ENGINEER APPROVAL.

-PLUMBING SHOWN FOR REFERENCE ONLY.
BUILDER VERIFY FINAL FIXTURE LOCATIONS, SIZES AND REQUIREMENTS PRIOR TO

FOUNDATION SCHEDULE

- F1 12" THICK X 36" SQ. POURED CONCRETE FOOTING WITH (4) #4 X 30" REBAR @ 9" O.C. BOTH DIRECTIONS 3" CLR FROM BOTTOM OF FOOTING.
- F2 12" THICK X 42" SQ. POURED CONCRETE FOOTING WITH (5) #4 X 36" REBAR @ 9" O.C. BOTH DIRECTIONS 3" CLR FROM BOTTOM OF FOOTING.
- F3 12" THICK X 48" X 36" POURED CONCRETE FOOTING WITH #4 REBAR @ 6" O.C. BOTH DIRECTIONS 3" CLR FROM BOTTOM OF FOOTING.

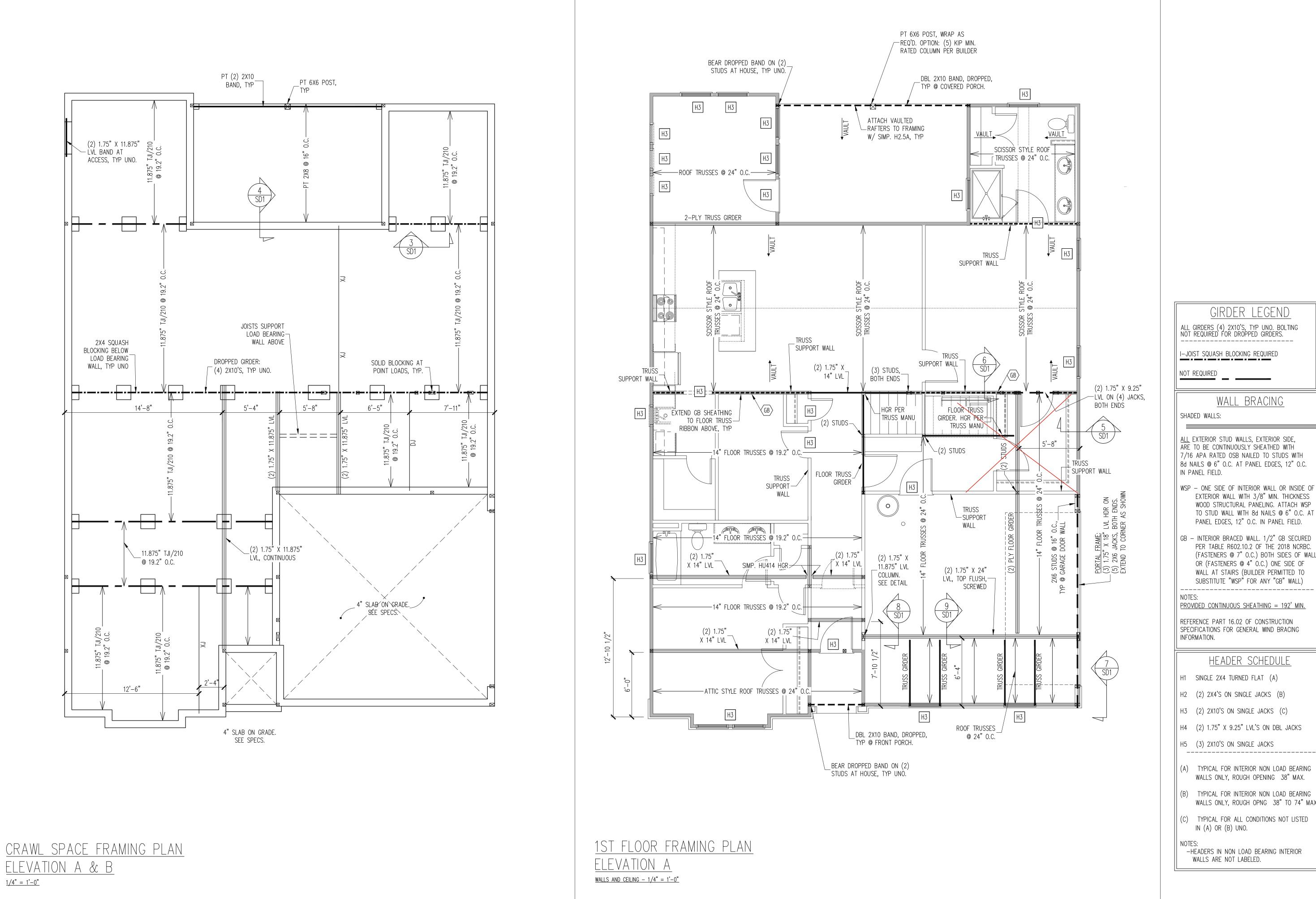
ENG: EAF/RJS

DATE: 10-25-2024

PLAN GUILFORD - FC

> PROJECT NO. 24-66-336

> > SHEET NO. of 5





ENDUM
REV # REF PROJ #
1 24-66-124
2 24-66-245

STRUCTURAL /
131 DUNCANS CREEK
LILLINGTON, NC

GB - INTERIOR BRACED WALL. 1/2" GB SECURED PER TABLE R602.10.2 OF THE 2018 NCRBC. (FASTENERS @ 7" O.C.) BOTH SIDES OF WALL OR (FASTENERS @ 4" O.C.) ONE SIDE OF WALL AT STAIRS (BUILDER PERMITTED TO SUBSTITUTE "WSP" FOR ANY "GB" WALL)

EXTERIOR WALL WITH 3/8" MIN. THICKNESS WOOD STRUCTURAL PANELING. ATTACH WSP

TO STUD WALL WITH 8d NAILS @ 6" O.C. AT PANEL EDGES, 12" O.C. IN PANEL FIELD.

GIRDER LEGEND

WALL BRACING

PROVIDED CONTINUOUS SHEATHING = 192' MIN.

REFERENCE PART 16.02 OF CONSTRUCTION SPECIFICATIONS FOR GENERAL WIND BRACING INFORMATION.

HEADER SCHEDULE

- | H1 SINGLE 2X4 TURNED FLAT (A)
- H2 (2) 2X4'S ON SINGLE JACKS (B)
- H3 (2) 2X10'S ON SINGLE JACKS (C)
- H4 (2) 1.75" X 9.25" LVL'S ON DBL JACKS
- H5 (3) 2X10'S ON SINGLE JACKS
- (A) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPENING 38" MAX.
- (B) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPNG 38" TO 74" MAX.
- (C) TYPICAL FOR ALL CONDITIONS NOT LISTED IN (A) OR (B) UNO.
- -HEADERS IN NON LOAD BEARING INTERIOR WALLS ARE NOT LABELED.

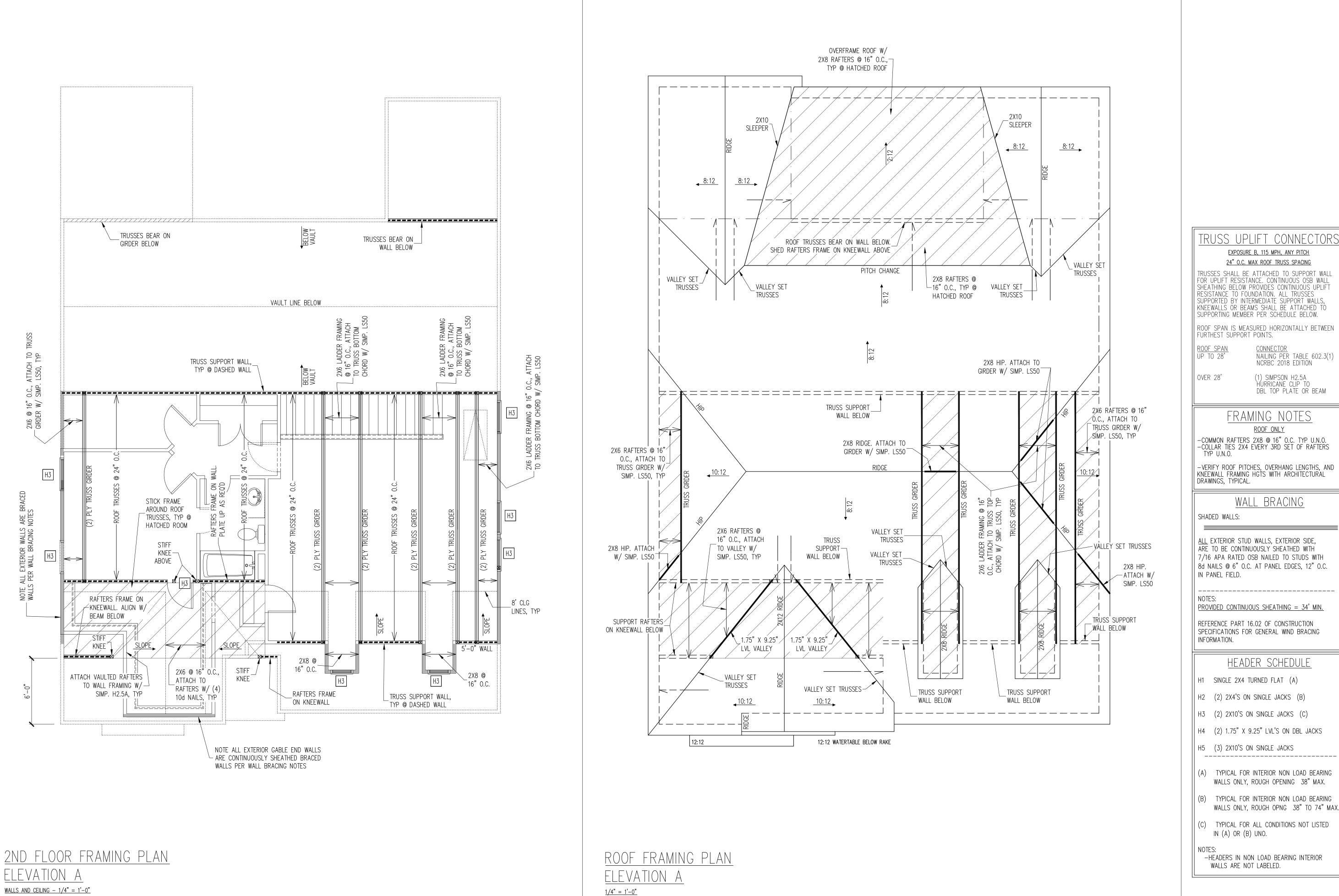
ENG: EAF/RJS DATE: 10-25-2024

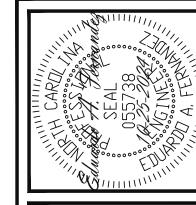
PLAN ||GUILFORD - FC

> PROJECT NO. 24-66-336

> > SHEET NO. **S2**

2 of 5





NEV # REF PROJ # 1 24-66-124 2 24-66-245

STRUCTURAL /
131 DUNCANS CREEK
LIILLINGTON, NC

PROVIDED CONTINUOUS SHEATHING = 34' MIN.

EXPOSURE B, 115 MPH, ANY PITCH 24" O.C. MAX ROOF TRUSS SPACING

NAILING PER TABLE 602.3(1)

NCRBC 2018 EDITION

(1) SIMPSON H2.5A HURRICANE CLIP TO DBL TOP PLATE OR BEAM

FRAMING NOTES

WALL BRACING

REFERENCE PART 16.02 OF CONSTRUCTION SPECIFICATIONS FOR GENERAL WIND BRACING INFORMATION.

HEADER SCHEDULE

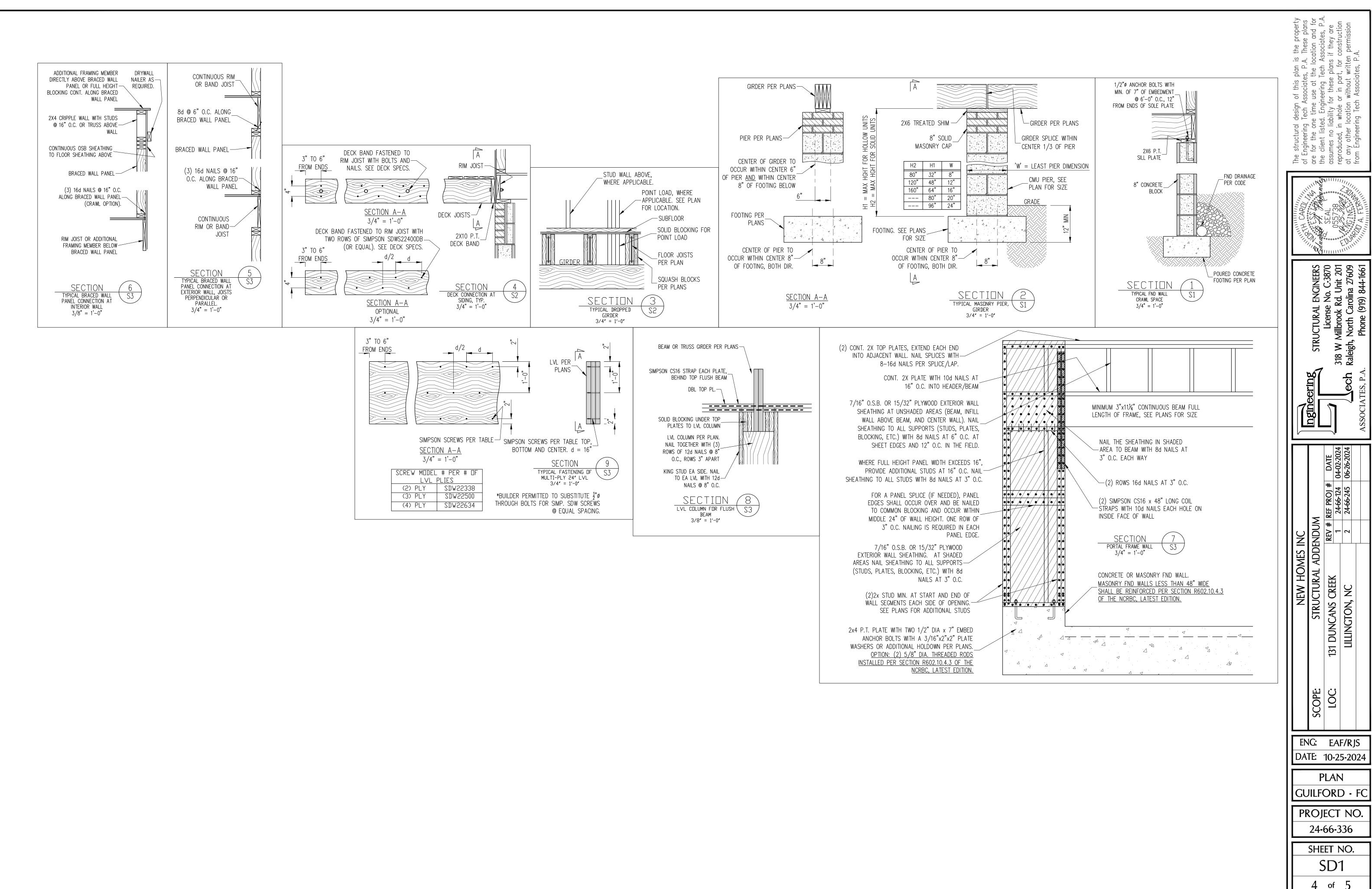
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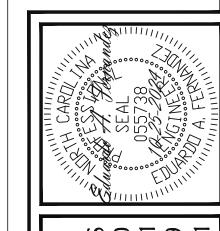
ENG: EAF/RJS DATE: 10-25-2024

PLAN ||GUILFORD - FC

> PROJECT NO. 24-66-336

> > SHEET NO. **S**3 3 of 5





4 of 5

DART LI-CENERAL 1.02 CONSTRUCTION SHALL WEET THE REQUIREMENTS OF THE NORTH CAROLINA RESIDENTIAL 1.03 CONSTRUCTION SHALL WEET THE REQUIREMENTS OF THE NORTH CAROLINA RESIDENTIAL 1.04 CONSTRUCTION SHALL CONFORM OVER SCALE ON THESE DRAWNOS. 1.05 METHORS PROCEDURES AND SECURICES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE STRUCTURE ATT ALL STAKES OF CONSTRUCTION. 1.05 METHORS PROCEDURES AND SECURITION ARE THE RESPONSIBILITY OF THE STRUCTURE ATT ALL STAKES OF CONSTRUCTION. 1.06 PART LE DESIRE LOADS. 2.01 DESIGN LOADS SHALL CONFORM WITH THE TABLE BELOW. 1.07 LEAD STAR ACCESS, STAKES, FREE ISSUERS 1.08 ALCONES, DECKS, ATTICS WITH FIXED STAR ACCESS, DIRELING ONTS INCLUMEN ATTICS WITH THE TABLE BELOW. 1.08 ALCONES, DECKS, ATTICS WITH FIXED STAR ACCESS, DIRELING ONTS INCLUMEN ATTICS WITH THE TABLE BELOW. 1.09 ATTICS (NO STORAGE, LESS THAN S' HERROROU). 1.00 ATTICS (WITH STORAGE). 2.00 ATTICS (WITH STORAGE). 2.01 ATTICS (WITH STORAGE). 2.01 ATTICS (WITH STORAGE). 2.02 AND HIGHWAY PROQUEDS HE GREATER STRESS. 2.03 AND HIGHWAY PROQUEDS HE GREATER STRESS. 2.04 AND HIGHWAY PROQUEDS HE GREATER STRESS. 2.05 ASSOCIATED AND STREET CONFORM TO ASTIM ASSOCIATION OF THE ACCESS AND THE STRUCTURAL STRUCTURAL STREET. 2.06 AND DESIGN VELOCITY OF 120 MPH. 2.07 AND THE PLANE BEAMS AND THE SECTIONS SHALL CONFORM TO ASTIM ASSOCIATION OF STRUCTURAL STREET. 2.08 AND RECTANGULAR TURING SHALL CONFORM TO ASTIM ASSOCIATION OF STRUCTURAL STREET. 2.09 AND RECTANGULAR TURING SHALL CONFORM TO ASTIM ASSOCIATION OF STRUCTURAL STREET. 2.00 AND RECTANGULAR TURING SHALL CONFORM TO ASTIM ASS MINIMUM GRADE 3.01 ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO ASTIM ASSOCIATION OF STRUCTURAL STEEL. 2.01 AND RECTANGULAR TURING SHALL CONFORM TO ASTIM ASSOCIATION OF STRUCTURAL STEEL. 2.02 AND RECTANGULAR TURING SHALL CONFORM TO ASTIM ASSOCIATION OF STRUCTURAL STEEL. 2.03 ASTELL PRE-SHALL CONFORM TO ASTIM ASSOCIATION OF STRUCTURAL STEEL. 2.04 AND SCREEN SHALL BE SECONAGE STRUCTURAL STEEL CONTINUE OF SHALL CONFORM TO ASTIM ASSOCIATION OF ST		<u>CONSTRUCTION</u>
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2.02 INTERIOR WALLS: 5 PSF LATERAL. 2.03 BASIC WIND DESIGN VELOCITY OF 120 MPH. 2.04 SOIL BEARING CAPACITY 2000 PSF (PRESUMPTIVE). PART 3: STRUCTURAL STEEL. 3.01 WIDE FLANGE BEAMS AND TEE SECTIONS SHALL CONFORM TO ASTM A992 MINIMUM GRADE. 3.02 SOUARE AND RECTANGULAR TUBING SHALL CONFORM TO ASTM A500 GRADE. 3.03 STEEL PIPE SHALL CONFORM TO ASTM A53 GRADE B, TYPE S, MINIMUM GRADE. 3.04 ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 MINIMUM GRADE. 3.05 STRUCTURAL STEEL CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE ARC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS. PART 4: WELDING. 4.01 WELDING ELECTRODES SHALL BE EFOXX AND ALL WELDING SHALL BE PERFORMED BY AN AWS CERTIFIED WELDER. PART 5: CONCRETE AND SLABS ON GRADE. 5.01 CAST IN PLACE CONCRETE SHALL BE OF NORMAL WEIGHT, 8% AIR ENTRAINMENT, AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 25 DAYS TYP UND. 5.02 REINFORCED CAST IN PLACE CONCRETE SHALL BE PROPORTIONED, MIXED AND PLACED IN ACCORDANCE WITH THE SPECIFICATIONS OF ACI 318, LATEST EDITION. 5.03 SLABS ON GRADE, IF MAY, SHALL CONTAIN STRINGT POPROPPRIEDES, FIBER LINEOTH 1 1/2°, DOSC AGRADE AND PROTOTO ENSITY. VAPOR BARRIER MAY BE OMITTED FOR SLABS NOT IN ENCLOSED AREAS. PART 6: REBAR AND WIRE REINFORCEMENT 6.01 REBAR SHALL BE DECLASS B AS DEFINED BY ACI 318, TYP UND. 6.02 LAP SPUCES SHALL BE CLASS B AS DEFINED BY ACI 318, TYP UND. 6.03 LABSON ROSAND WINTS SHALL CONFORM TO ASTM A615 GRADE 60 TYP UND IN ENCLOSED AREAS. PART 6: REBAR BALL BE CLASS B AS DEFINED BY ACI 318, TYP UND. 6.04 LAP SPUCES SHALL BE CLASS B AS DEFINED BY ACI 318, TYP UND. 6.05 LADDER WIFE SHALL BE CLASS B AS DEFINED BY ACI 318, TYP UND. 6.06 LAD SPI MIN. 6.07 CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 AND C55, NORMAL WEIGHT, TWE 1,500 PSI MIN. 6.08 AND SHALL CONFORM TO ASTM C90 AND C55, NORMAL WEIGHT, TWE 1,500 PSI MIN. 6.09 CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM G80-179 CRADE SW. 6.00 AND SHANDON SHALL CONFORM TO ASTM F1667-05 ANALL STAND	NOTES:	LIVE LOAD OF 40 PSF OR A 300 LB. CONCENTRATED LOAD ACTING OVER AN AREA OF 4 SQ. WHICHEVER PRODUCES THE GREATER STRESS. - BUILDER TO VERIFY DEAD LOAD DOES NOT EXCEED 10 PSF WHEN HEAVY FLOOR OR ROOF FINISHES SUCH AS TILE OR SLATE ARE UTILIZED. NOTIFY ENGINEERING UNDER
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AWS CERTIFIED WELDER PART 5: CONCRETE AND SLABS ON GRADE CAST IN PLACE CONCRETE SHALL BE OF NORMAL WEIGHT, 6% AIR ENTRAINMENT, AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS TYP UNIO. ALL CONCRETE, INCLUDING CONCRETE FOR FOOTINGS, IS TO BE CAST IN PLACE, TYP UNIO. 5.02 REINFORCED CAST IN PLACE CONCRETE SHALL BE PROPORTIONED, MIXED AND PLACED I ACCORDANCE WITH THE SPECIFICATIONS OF ACT 318, LATEST EDITION. 5.03 SLABS ON GRADE, IF ANY, SHALL CONTAIN SYNTHETIC POLYPROPYLENE FIBRILLATED MICRO FIBERS, FIBER LENGTH 1 1/2", DOSAGE RATE 1 1/2 LBS/CU YD. SLAB TO BE PLACED ON A 6 MIL WAPOR BARRIER ON 2" MIN GRANULAR FILL ON SOIL WITH 90% MIN STANDARD PROCTOR DENSITY. VAPOR BARRIER MAY BE OMITTED FOR SLABS NOT IN ENCLOSED AREAS PART 6: REBAR AND WIRE REINFORCEMENT 6.01 REBAR SHALL BE DEFORMED STEEL CONFORMING TO ASTM A615 GRADE 60 TYP UNO 6.02 LAP SPLICES SHALL BE CLASS B AS DEFINED BY ACT 318, TYP UNO 6.03 WIRE REINFORCEMENT SHALL BE 9 GA AND SHALL CONFORM TO ASTM A1064. PART 7: MASONRY 7.01 CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 AND C55, NORMAL WEIGHT, I'M = 1,500 PSI MIN 7.02 CLAY MASONRY UNITS SHALL CONFORM TO ASTM C90 AND C55, NORMAL WEIGHT, I'M = 1,500 PSI MIN 7.03 MORTAR SHALL BE TYPE S. MORTAR AND GROUT SHALL CONFORM TO ASTM C476, MIN COMPRESSIVE STRENGTH OF 2000 PSI. 7.04 MASONRY CONSTRUCTION SHALL CONFORM TO THE SPECIFICATIONS OF ACT 530 1.05 LADDER WIRE REINFORCEMENT SHALL CONFORM TO THE SPECIFICATIONS OF ACT 530 2.06 LADDER WIRE REINFORCEMENT SHALL CONFORM TO ASTM A951. 6" MIN LAPS FOR CONTINUOUS WALL APPLICATIONS PART 8: BOLTS AND LAG SCREWS 8.01 BOLTS SHALL CONFORM TO ASTM A307 MINIMUM GRADE TYP UNO. INSTALL STANDARD STEEL WASHERS (ASTM F844-070) FOR THE NUT / BOLT HEAD WHEN BOLTING WOOD MEMBERS 8.02 LAG SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1-1981. PILOT HOLES SHALL BE USED FOR LAG SCREW INSTALLATION AND SHALL BE DORED ACCORDING TO NOS SPECIFICATIONS. INSTALL STANDARD STEEL WASHERS (ASTM F844-070) FOR SCREW HEAD 8.03 ANCHOR RODS AND BOLTS SHALL C		PART 4: WELDING
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DADY 44 ENGINEERS LINES		FOR JOISTS, RAFTERS, GIRDERS, BEAMS, STUDS, ETC.
11.01 LVL OR PSL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS:	1	LVL OR PSL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS: E= 1.9 X 10E6 PSI, Fb = 2600 PSI, Fv = 285 PSI, Fc = 750 PSI LSL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS:

<u>CO</u>	NSTRUCTION	SPE	CIFICATIONS
PART 1: GENERAL CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE NOR CODE, 2018 EDITION. DIMENSIONS SHOWN SHALL GOVERN OVER SCALE ON THESE METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION THE CONTRACTOR, WHO SHALL TAKE ALL NECESSARY PRECAINSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF PART 2: DESIGN LOADS DESIGN LOADS SHALL CONFORM WITH THE TABLE BELOW:	DRAWINGS. ARE THE RESPONSIBILITY OF UTIONS TO MAINTAIN AND	12.01	LVL OR PSL MEMBERS MAY BE RIPPED FROM DEEPER MEMBERS TO MATCH THE MEMBER DEPTH SPECIFIED IN THE PLANS PART 12: PRESSURE TREATED LUMBER LUMBER IN CONTACT WITH THE GROUND, CONCRETE OR MASONRY SHALL BE PRESSURE TREATED IN ACCORDANCE WITH AWPA STANDARD C-15. ALL OTHER EXPOSED LUMBER SHALL BE TREATED IN ACCORDANCE WITH AWPA STANDARD C-2 OR BY ANY METHOD GIVING EQUAL PROTECTION. THE BUILDING CODE OFFICE MAY ALSO APPROVE A NATURAL DECAY RESISTANT WOOD PER SECTION 19-6(A) PART 13: STEEL FLITCH PLATE BEAMS
USE LIVE LOAD BALCONIES, DECKS, ATTICS WITH FIXED STAIR ACCESS, DWELLING UNITS INCLUDING ATTICS WITH FIXED STAIR ACCESS, STAIRS, FIRE ESCAPES	O (PSF) DEAD LOAD (PSF) 40 10	13.01	FLITCH PLATE BEAMS SHALL CONSIST OF A CONTINUOUS STEEL PLATE BOLTED BETWEEN TWO PIECES OF CONTINUOUS LUMBER AS SIZED ON THE PLANS. BOLT PIECES TOGETHER USING 1/2" Ø BOLTS SPACED AT 24" O.C. STAGGERED TOP TO BOTTOM OF THE BEAM. MAINTAIN A 2" EDGE DISTANCE. PLACE TWO BOLTS, ONE ABOVE THE OTHER, 6" ± 2" FROM EACH END OF THE BEAM. PART 14: STUD SUPPORTS FOR BEAMS
GARAGES (PASSENGER CARS ONLY) ATTICS (NO STORAGE, LESS THAN 5' HEADROOM) ATTICS (WITH STORAGE)	50 10 10 20 10	14.01 1–W	STEEL, ENGINEERED LUMBER, AND FLITCH PLATE BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS: HEN THE BEAM IS PERPENDICULAR TO. OR SKEWED RELATIVE TO THE WALL. THE BEAM

1-WHEN THE BEAM IS PERPENDICULAR 10, OR SKEWED RELATIVE TO THE WALL, THE BEAM SHALL BEAR FULL WIDTH ON THE SUPPORTING WALL INDICATED AND SHALL BE SUPPORTED BY A MINIMUM OF THREE GANGED STUDS, OR A GANGED STUD COLUMN WITH A NUMBER OF STUDS SUCH THAT THE STUD COLUMN IS AT LEAST AS WIDE AS THE TRUE WIDTH OF 20 10 (15 FOR VAULTS) READS ARE TO BE DESIGNED FOR THE UNIFORMLY DISTRIBUTED OF OR A 300 LB. CONCENTRATED LOAD ACTING OVER AN AREA OF PRODUCES THE GREATER STRESS.

DEAD LOAD DOES NOT EXCEED 10 PSF WHEN HEAVY FLOOR OR IT AS TILE OR SLATE ARE UTILIZED. NOTIFY ENGINEERING UNDER THE BEAM BEING SUPPORTED, WHICHEVER IS GREATER, TYP UNO. FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLUMN IS CENTERED ON 2-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR A MINIMUM OF 4 1/2" ONTO THE WALL AND BE SUPPORTED BY A TRPL STUD GANGED

COLUMN TYP UNO. 14.02 DIMENSIONAL LUMBER BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS: 1-WHEN THE BEAM IS PERPENDICULAR TO, OR SKEWED RELATIVE TO THE WALL, THE BEAM SHALL BEAR <u>FULL WIDTH</u> ON THE SUPPORTING WALL INDICATED (LESS 1 1/2" TO ALLOW FOR A CONTINUOUS RIM JOIST WHERE APPLICABLE) AND SHALL BE SUPPORTED BY A GANGED STUD COLUMN THE SAME WIDTH AS THE BEAM TYP UNO. (E.G. A TRIPLE 2X10 IS TO BE SUPPORTED BY (3) STUDS). FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLÚMN IS CENTERED ON THE BEAM 2-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR A MINIMUM OF 3" ONTO THE WALL AND BE SUPPORTED BY A DBL STUD GANGED COLUMN

14.03 EXTRA JOISTS BEARING ON A STUD WALL PERPENDICULAR TO OR SKEWED RELATIVE TO

THE BEAM SHALL BE SUPPORTED BY ONE ADDITIONAL STUD. 4.04 STUDS THAT ARE GANGED TO FORM A COLUMN SHALL HAVE ADJACENT STUDS WITHIN THE COLUMN NAILED TOGETHER WITH ONE ROW OF 10d NAILS AT 8" O.C. (TWO ROWS OF 10d NAILS @ 8" O.C., 3" APART, FOR 2X8 OR 2X10 STUDS) ALL COLUMNS SHALL BE CONTINUOUS DOWN TO THE FOUNDATION OR OTHER PROPERLY DESIGNED STRUCTURAL ELEMENT SUCH AS A BEAM. COLUMNS TRANSFERRING LOADS THROUGH FLOOR LEVELS SHALL BE SOLIDLY BLOCKED FOR THE FULL WIDTH OF THE STUD COLUMN WITHOUT THE CAVITY FORMED BY THE

PART 15: NAILING OF MULTI PLY WOOD BEAMS

5.01 SOLID SAWN LUMBER JOISTS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM NAILED TOGETHER WITH THREE ROWS OF 10d NAILS @ 16" O.C. FOR 2X10 OR LARGER, TWO ROWS OF 10d NAILS @ 16" O.C. FOR 2X8, ONE ROW OF 10d NAILS @ 16" O.C. FOR 2X6 OR SMALLER. STAGGER ROWS 5" MIN.

5.02 LVL MEMBERS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM FASTENED TOGETHER PER MANUFACTURERS RECOMMENDATIONS, TYP

PART 16: WALL FRAMING AND BRACING

STUD WALLS SHALL CONSIST OF 2X4 STUDS SPACED AT 16" O.C. UNO. STUDS SHALL BE CONTINUOUS FROM SOLE PLATE AT FLOOR TO DOUBLE TOP PLATE AT THE CEILING OR ROOF. NO INTERMEDIATE BANDS OR PLATES SHALL CAUSE DISCONTINUITIES IN A STUD WALL EXCEPT AS REQUIRED FOR DOOR OR WINDOW OPENINGS. THE KING STUDS FOR SUCH OPENINGS SHALL BE CONTINUOUS. TYP UNO. MAX ALLOWABLE WALL HEIGHTS FOR EXTERIOR STUD WALLS. INCLUSIVE OF SOLF PLATE AND DBL TOP PLATE AND 7/16" OSB EXTERIOR BRACING AND ROW OF 2X4 2X6 PURLINS AT 8' HEIGHT (AND AT 16' HEIGHT FOR TALL WALLS), TYP UNO: 2X4 @ 16" O.C.: 11'-1 1/2" 2X6 @ 16" O.C.: 17'-0" 2X4 @ 12" O.C.: 12'-1 1/2" 2X6 @ 12" O.C.: 18'-8" DBL 2X4 @ 16" O.C.: 13'-4" DBL 2X6 @ 16" O.C.: 21'-0"

16.02 FOR WALL BRACING THE FOLLOWING SHALL APPLY: -BLOCKING AT UNSUPPORTED PANEL EDGES IS REQUIRED TYP UNO. -WALL BRACING IS BY ENGINEERED DESIGN AND NOT PRESCRIPTIVE PER SECTION 602.10 OF THE 2018 NCRC. CONTINUOUS SHEATHING HAS BEEN PROVIDED. ALONG WITH ALTERNATIVE METHODS TO INSURE THE MINIMUM INTENT OF SECTION 602.10 OF THE 2018 NCRC HAS BEEN MET AND EXCEEDED. -BRACED WALL PANELS SHALL BE FASTENED IN ACCORDANCE WITH TABLE 602.3(1) TO

PROVIDE CONTINUOUS PANEL UPLIFT RESISTANCE AND COMPLIANCE WITH NCRBC R602.3.5 AND R802.11 UNLESS NOTED OTHERWISE ON STRUCTURAL PLANS. -MAY SUBSTITUTE WSP FOR GB -SINGLE JOIST, CONTINUOUS RIM JOIST, OR BLOCKING OF EQUAL DEPTH IS REQUIRED ABOVE AND BELOW ALL BRACED WALLS. NAIL BLOCKING ABOVE WALL TO TOP PLATE WITH 16d TOE NAILS @ 6" O.C. NAIL SOLE PLATE OF BRACED WALL TO BLOCKING BELOW WITH (3) 16d NAILS @ 16" O.C. BLOCKING AT HORIZONTAL JOINTS IN BRACED WALL LINES ONLY REQUIRED AT SHADED WALLS, UNO.

PART 17: KING STUDS 7.01 KING STUDS FOR OPENINGS IN EXTERIOR WALLS SHALL BE AS FOLLOWS:

NUMBER OF KING STUDS MAX OPENING WIDTH 5'-0" 9'-0" 13'-0" 17'-0" 21'-0"

PART 18: SUBSTITUTIONS

18.01 MATERIAL OR MEMBER SIZE SUBSTITUTIONS OR PLAN DEVIATIONS REQUIRE THE WRITTEN AUTHORIZATION OF THE DESIGNERS. UNAUTHORIZED DEVIATIONS ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

PART 19: OWNERSHIP OF STRUCTURAL DESIGN

19.01 THE STRUCTURAL DESIGN OF THIS PLAN IS THE PROPERTY OF ENGINEERING TECH ASSOCIATES (ETA). THESE PLANS ARE FOR THE ONE TIME USE AT THE LOCATION INDICATED AND FOR THE CLIENT LISTED. ETA ASSUMES NO LIABILITY FOR THESE PLANS IF THEY ARE REPRODUCED, IN WHOLE OR IN PART, FOR CONSTRUCTION AT ANY OTHER LOCATION WITHOUT WRITTEN PERMISSION FROM ETÁ

THE BUILDER IS RESPONSIBLE FOR REVIEWING PLANS PRIOR TO CONSTRUCTION. THE BUILDER SHALL IMMEDIATELY CONTACT THE ENGINEER OF RECORD (EOR) BEFORE PROCEEDING IF THE FOLLOWING CONDITIONS ARE NOTED BEFORE OR DURING CONSTRUCTION: 1) THE WORKING PLANS DO NOT BEAR THE SEAL OF THE EOR 2) THE PLANS CONTAIN DISCREPANT OR INCOMPLETE INFORMATION

ANY ERRORS DUE TO A FAILURE TO FOLLOW THE ABOVE PROCEDURES SHALL NOT BE THE RESPONSIBILITY OF THE EOR. FURTHERMORE, IT IS THE RESPONSIBILITY OF THE BUILDER TO ENSURE THAN ANY REVISIONS ISSUED BY THE EOR ARE PROMPLY DISTRIBUTED TO THE

THE EOR DOES NOT PERFORM FENESTRATION OR VENTING CALCULATIONS OR ANY OTHER N CALCULATIONS THAT ARE NOT DIRECTLY RELATED TO STRUCTURAL ENGINEERING. ROOF AND FLOOR TRUSSES TO BE DESIGNED BY AN ENGINEER REGISTERED BY THE STATE. FINAL FL PL FLITCH PLATE FUNDER OR PREVIEW. TRUSS DRAWING SHOULD BE SUBMITTED TO THE EOR FOR REVIEW

ABBREVIATIONS ABV ABOVE FND FOUNDATION TJ TRIPLE JOIST R ROTH FTG FOOTING TYP TYPICAL B.E. BOTH ENDS HDG HOT DIPPED TRPL TRIPLE BTWN BETWEEN TSP TRIPLE STUD POCKET | MANUFACTURER DEPTH SERIES MOUNT HGR FLANGE HGR GALVANIZED CIP CAST IN PLACE HGR HANGER UNO UNLESS NOTED LVL LAMINATED VENEER CONC CONCRETE OTHERWISE CS CONTINUOUS SHEATHING LUMBER XJ EXTRA JOIST DIA DIAMETER NTS NOT TO SCALE O.C. ON CENTER DBL DOUBLE DJ DOUBLE JOIST PSL PARALLEL STRAND DSP DBL STUD POCKET LUMBER EQ EQUAL PT PRESSURE TREATED EA EACH QJ QUAD JOIST FLG FLANGE SP STUD POCKET

SQ SQUARE

DECK SPECIFICATIONS

A DECK IS AN EXPOSED EXTERIOR WOOD FLOOR STRUCTURE WHICH MAY BE ATTACHED TO JOIST SPAN DECKING A STRUCTURE OR BE FREE STANDING. ROOFED PORCHES, OPEN OR SCREENED IN, MAY BE CONSTRUCTED USING THESE PROVISIONS. 1" S4S SUPPORT POSTS SHALL BE SUPPORTED BY A FOOTING. 24" O.C. 1 1/4" S4S 2" S4S TREATED WOOD BAND FOR THE LENGTH OF THE DECK, OR CORROSION RESISTANT FLASHING 9. MAXIMUM HEIGHT OF DECK SUPPORT POSTS IS AS FOLLOWS:

WHEN ATTACHED TO A STRUCTURE, THE STRUCTURE TO WHICH ATTACHED SHALL HAVE A SHALL BE USED TO PREVENT MOISTURE FROM COMING IN CONTACT WITH THE UNTREATED FRAMING OF THE STRUCTURE. THE DECK BAND AND THE STRUCTURE BAND SHALL BE CONSTRUCTED IN CONTACT WITH EACH OTHER EXCEPT AT BRICK VENEER AND WHERE PLYWOOD SHEATHING IS REQUIRED AND PROPERLY FLASHED. SIDING SHALL NOT BE INSTALLED BETWEEN THE STRUCTURE AND THE DECK BAND. IF ATTACHED TO A BRICK STRUCTURE, NEITHER FLASHING NOR A TREATED BAND FOR THE BRICK STRUCTURE IS REQUIRED. IN ADDITION, THE TREATED DECK BAND SHALL BE CONSTRUCTED IN CONTACT WITH THE BRICK

WHEN THE DECK IS SUPPORTED AT THE STRUCTURE BY ATTACHING THE DECK TO THE STRUCTURE, THE FOLLOWING ATTACHMENT SCHEDULES SHALL APPLY FOR ATTACHING THE DECK BAND TO THE STRUCTURE:

A. ALL STRUCTURES EXCEPT BRICK STRUCTURES

FASTENERS

	JOIST I	ENGTH
	UP TO 8' MAX.	UP TO 16' MAX.
REQUIRED FASTENERS	(2) ROWS OF 12d NAILS @ 8" O.C. OR	ONE- 5/8" Ø BOLT @ 20" O.C. AND (3) ROWS OF 12d NAILS @ 6" O.C. OR TWO ROWS OF SIMPSON SDWS22400DB @ d = 16" O.C. STAGGERED

	TWO ROWS OF SIMPSON SDWS22400DB @ d = 32" O.C. STAGGERED	TWO ROWS OF SIMPSON SDWS22400DB @ d = 16" O.C. STAGGERED
A . BRICK V	ENEER STRUCTURES	
	JOIST	LENGTH
	UP TO 8' MAX.	UP TO 16' MAX.
DEALIDED		

ONE- 5/8" Ø BOLT @ 28" O.C. ONE- 5/8" Ø BOLT @ 16" O.C.

IF THE DECK BAND IS SUPPORTED BY A 1/2" MINIMUM MASONRY LEDGE ALONG THE FOUNDATION WALL, 5/8" Ø BOLTS SPACED @ 48" O.C. MAY BE USED FOR SUPPORT.

OTHER MEANS OF SUPPORT, SUCH AS JOIST HANGERS, MAY BE USED TO CONNECT DECK JOISTS TO A TREATED STRUCTURE BAND

GIRDERS SHALL BEAR DIRECTLY ON POSTS OR BE BE CONNECTED TO THE SIDES OF POSTS | NOTES: 1) ALL NAILS AND BOLTS ARE TO BE HOT DIPPED GALVANIZED. WITH 2- 5/8" Ø BOLTS

FLOOR DECKING SHALL BE NO. 2 GRADE TREATED SOUTHERN PINE OR EQUIVALENT. THE MINIMUM FLOOR DECKING THICKNESS SHALL BE AS FOLLOWS:

IMUM HEIGHT OF DECK SUPPORT POSTS	o IS AS FULLOWS:
POST SIZE	MAX POST HEIGHT
4X4 6X6 ENGINEERED	8' 20' 20' +

NOTES: 1) THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS. 2) THIS TABLE IS BASED ON A MAXIMUM TRIBUTARY AREA OF 128 SQ. FT. 3) POST HEIGHT IS FROM TOP OF FOOTING TO BOTTOM OF GIRDER.

DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF THE FOLLOWING METHODS:

A. WHEN THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS ATTACHED TO

THE STRUCTURE IN ACCORDANCE WITH SECTION 4, LATERAL BRACING IS NOT REQUIRED. B. 4X4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED

BETWEEN 45° AND 60° FROM THE HORIZONTAL. KNEE BRACES SHALL BE ATTACHED AT THE ENDS TO THE GIRDER AND THE POST WITH ONE - 5/8" BOLT C. FOR FREE STANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN CONCRETE IN ACCORDANCE

WITH THE FOL	LOWING:			
POST SIZE	TRIBUT. AREA	POST HEIGHT	EMB. DEPTH	CONC. DIAM.
4X4 6X6	48 SQ. FT. 120 SQ. FT.	4'-0" 6'-0"	2'-6" 3'-6"	1'-0" 1'-8"

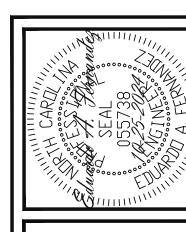
D. 2X6 DIAGONAL VERTICAL CROSS BRACING SHALL BE PROVIDED IN TWO PERPENDICULAR DIRECTIONS FOR FREE STANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE BRACES SHALL BE ATTACHED TO THE POSTS WITH ONE -5/8" ϕ BOLT AT EACH END OF THE BRACE.

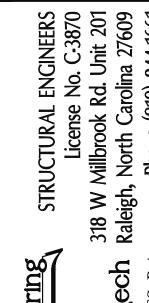
2) MINIMUM EDGE DISTANCE FOR BOLTS IS 2 1/2". 3) NAILS MUST PENETRATE THE SUPPORTING STRUCTURE BAND A MINIMUM OF 1 1/2".

ALLOWABLE I-JOIST SUBSTITUTION NOTE: MAINTAIN JOIST DEPTH, DIRECTION, AND SPACING SPECIFIED ON

SIMPSON FACE SIMPSON TOP

 BLUELINX	 11.875"	BLI 40	IUS2.56/11.88	ITS2.56/11.88
BOISE CASCADE	11.875"		IUS2.06/11.88	ITS2.06/11.88
BOISE CASCADE	11.875"		IUS2.37/11.88	ITS2.37/11.88
NTERNATIONAL	11.875"	IB 400	IUS2.56/11.88	ITS2.56/11.88
BEAMS			,	,
_P CORP	11.875"	LPI 20+	IUS2.56/11.88	ITS2.56/11.88
NORDIC	11.875"	NI 40X	IUS2.56/11.88	ITS2.56/11.88
ROSEBURG	11.875"	RFPI 40s	IUS2.56/11.88	ITS2.56/11.88
WEYERHAEUSER	11.875"	TJI 210	IUS2.06/11.88	ITS2.06/11.88
WEYERHAEUSER	11.875"	EEI-20	IUS2.37/11.88	ITS2.37/11.88
	11			/
BLUELINX	14"	BLI 40	IUS2.56/14	ITS2.56/14
BOISE CASCADE	14"	BCI 5000s	IUS2.06/14	ITS2.06/14
BOISE CASCADE	14"	BCI 6000S	IUS2.37/14	ITS2.37/14
_P CORP	14"	LPI 20+	IUS2.56/14	ITS2.56/14
NORDIC	14"	NI 40X	IUS2.56/14	ITS2.56/14
ROSEBURG	14"	RFPI 40s	IUS2.56/14	ITS2.56/14
WEYERHAEUSER	14"	TJI 210	IUS2.06/14	ITS2.06/14
WEYERHAEUSER	14"	EEI-20	IUS2.37/14	ITS2.73/14
DITIELINIV	14"	DII OA	11107 50 /11	ITC7 EC /1 /
BLUELINX LP CORP	14"	BLI 80	IUS3.56/14	ITS3.56/14
		LPI 42+	IUS3.56/14	ITS3.56/14
NORDIC	14"	NI-80	IUS3.56/14	ITS3.56/14
ROSEBURG	14"	RFPI 80s	IUS3.56/14	ITS3.56/14
WEYERHAEUSER	14"	TJI 360	IUS2.37/14	ITS2.37/14
WEYERHAEUSER	14"	EEI-20	IUS3.56/14	ITS3.56/14
BLUELINX	16"	BLI 40	IUS2.56/16	ITS2.56/16
BLUELINX	16"	BLI 60	IUS2.56/16	ITS2.56/16
BOISE CASCADE	16"	BCI 5000s	IUS2.06/16	ITS2.06/16
BOISE CASCADE	16"	BCI 6000S	IUS2.37/16	ITS2.37/16
NTERNATIONAL	16"	IB 600	IUS2.56/16	ITS2.56/16
BEAMS		.5 000		
P CORP	16"	LPI 20+	IUS2.56/16	ITS2.56/16
NORDIC	16"	NI 40X	IUS2.56/16	ITS2.56/16
ROSEBURG	16"	RFPI 60S	IUS2.56/16	ITS2.56/16
WEYERHAEUSER	16"	TJI 210	IUS2.06/16	ITS2.06/16
BOISE CASCADE	16"	BCI 60s	IUS2.37/16	ITS2.37/16
P CORP	16"		•	
P CORP P CORP	16"	LP 36	IUS2.37/16	ITS2.37/16
		LP 42+	IUS2.56/16	ITS2.56/16
NORDIC	16"	NI 70	IUS2.56/16	ITS2.56/16
ROSEBURG	16"	RFPI 70	IUS2.37/16	ITS2.37/16
WEYERHAEUSER	16"	TJI 360	IUS2.37/16	ITS2.37/16
WEYERHAEUSER	16"	EEI-30	IUS2.37/16	ITS2.73/16
JOISTS NOT LISTEI	O IN THE .	ABOVE TABLE	MAY BE USED	PROVIDED THEY
MEET OR EXCEED	THE PROP	ERTIES OF T	HOSE LISTED. SU	BSTITUTE USP





NEW HOMES INC				
JCTURAL ADDENDUM	NDQN	V		광
CREEK	REV#	REV # REF PROJ # DATE	DATE	
CNLLIN	-	24-66-124	24-66-124 04-02-2024	j
NC.	2	24-66-245	24-66-245 06-26-2024	
				7
				ASSO

DATE: 10-25-2024 PLAN

GUILFORD - FO PROJECT NO. 24-66-336

SHEET NO. **5** of 5

ENG: EAF/RIS