

RE: MP31

DRHORTON/WILMINGTON; LOT 31 MCKAY PLACE

Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: MP31 Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.6 Wind Speed: 120 mph Floor Load: N/A psf

This package includes 18 individual, dated Truss Design Drawings and 0 Additional Drawings.

No	Sool#	Truce Name	Data
No.	Seal#	Truss Name	Date
1	165579196	A01AG	5/16/2024
2	165579197	A02	5/16/2024
3	165579198	A02A	5/16/2024
4	165579199	A03	5/16/2024
5	165579200	A04V	5/16/2024
6	165579201	A05AV	5/16/2024
7	165579202	A05V	5/16/2024
8	165579203	A06AVG	5/16/2024
9	165579204	B01G	5/16/2024
10	165579205	B02GR	5/16/2024
11	165579206	P01G	5/16/2024
12	165579207	P02	5/16/2024
13	165579208	V01	5/16/2024
14	165579209	V02	5/16/2024
15	165579210	V03	5/16/2024
16	165579211	V04	5/16/2024
17	165579212	V05	5/16/2024
18	165579213	V06	5/16/2024

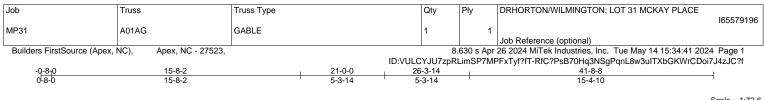
The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource (Apex,NC). Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2024

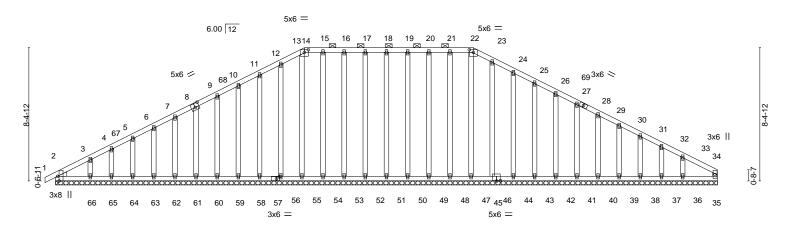
North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.









			41-8-8 41-8-8		I
Plate Offsets (X,Y)	[2:0-3-8,Edge], [8:0-3-0,0-3-0], [14:0-3-0	0,0-2-0], [22:0-3-0,0-2-0], [[45:0-2-12,0-0-4], [57:0-2	2-4,0-1-8]	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO	CSI. TC 0.08 BC 0.07 WB 0.11	DEFL. in Vert(LL) -0.00 Vert(CT) 0.00 Horz(CT) 0.01	1 n/r 120 1 n/r 120	PLATES GRIP MT20 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S			Weight: 361 lb FT = 20%
BOT CHORD 2x4 S WEBS 2x4 S	P No.2 P No.2 P No.3 P No.3		BRACING- TOP CHORD BOT CHORD		directly applied or 6-0-0 oc purlins, -0-0 oc purlins (6-0-0 max.): 14-22. d or 10-0-0 oc bracing.
(lb) - Max I	earings 41-8-8. Horz 2=121(LC 16) Jplift All uplift 100 lb or less at joint(s) 2	, 51, 52, 53, 54, 56, 58, 59	9, 60,		

61, 62, 63, 64, 65, 66, 50, 49, 48, 46, 44, 43, 42, 41, 40, 39, 38, 37, 36 Max Grav All reactions 250 lb or less at joint(s) 35, 2, 51, 52, 53, 54, 55, 56, 58, 59, 60, 61, 62, 63, 64, 65, 66, 50, 49, 48, 47, 46, 44, 43, 42, 41, 40, 39, 38.37.36

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 12-13=-90/259, 22-23=-90/253

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-0 to 4-1-10, Exterior(2) 4-1-10 to 15-8-2, Corner(3) 15-8-2 to 20-5-12, Exterior(2) 20-5-12 to 26-3-14, Corner(3) 26-3-14 to 31-1-8, Exterior(2) 31-1-8 to 41-6-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Provide adequate drainage to prevent water ponding.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 1-4-0 oc.

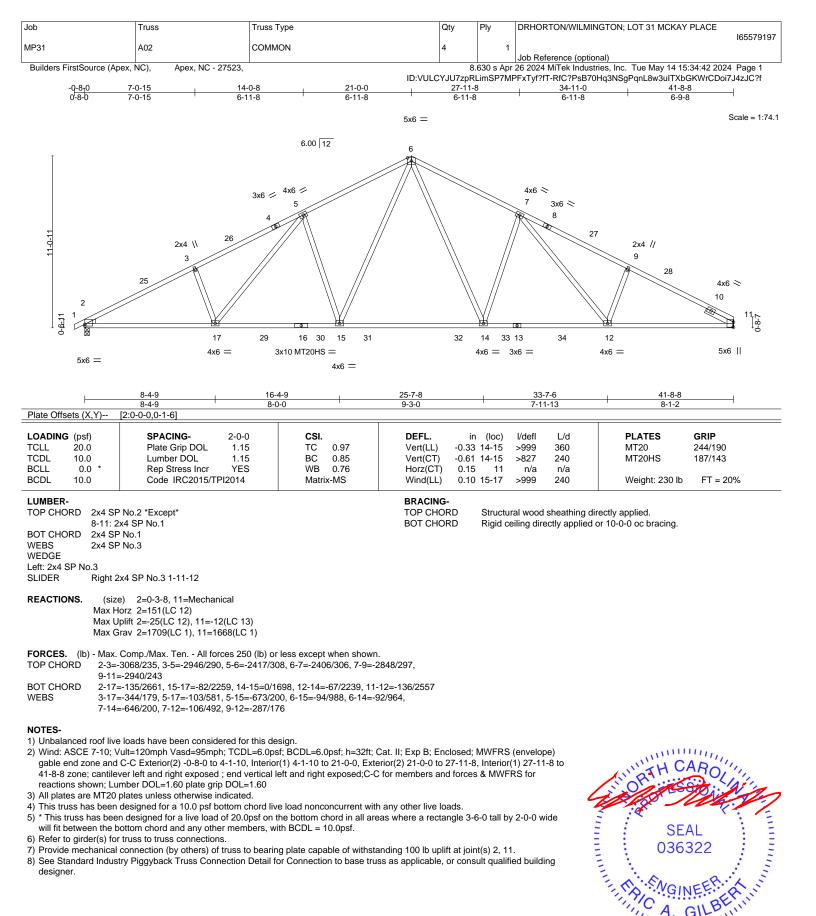
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members. 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 51, 52, 53, 54, 56, 58, 59, 60, 61, 62, 63, 64, 65, 66, 50, 49, 48, 46, 44, 43, 42, 41, 40, 39, 38, 37, 36.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



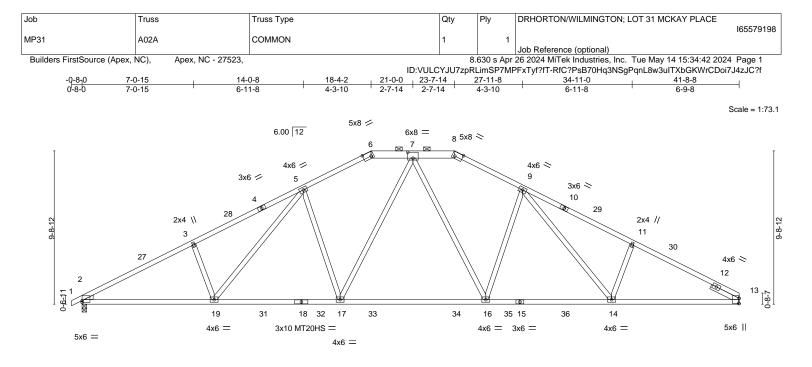
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



May 16,2024

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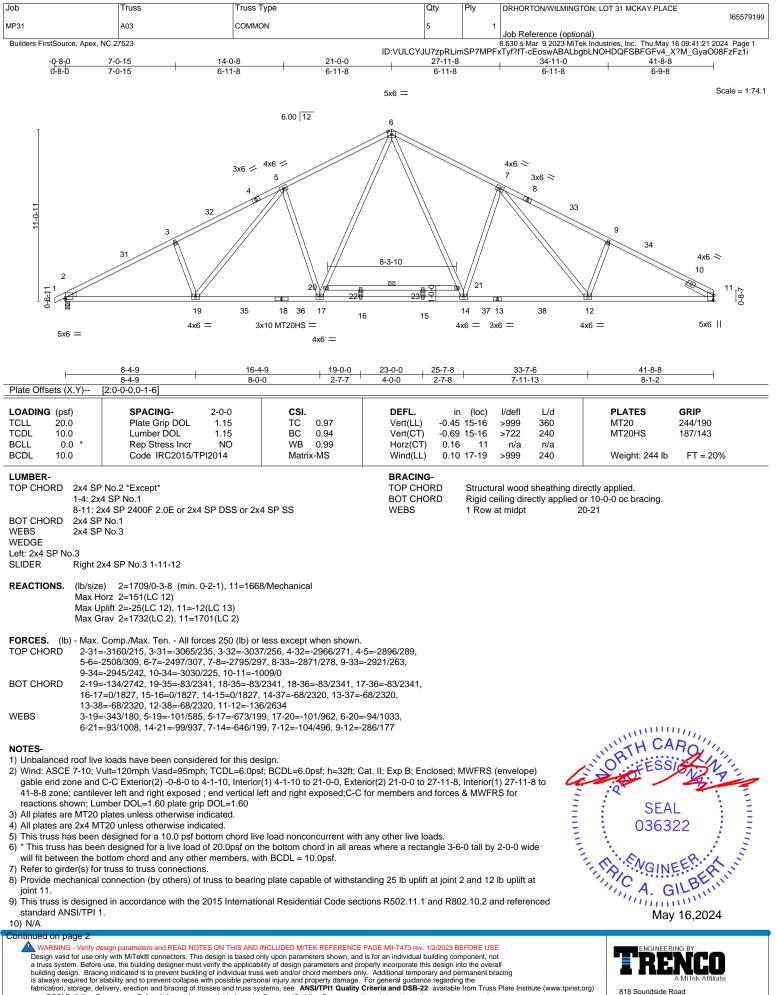
TRENCO A MiTek Affiliate 818 Soundside Road



L		16-4-9	25-7-8	33-7-6	41-8-8
	<u>8-4-9</u> [2:0-0-0,0-1-6], [6:0-5-14,0-3-4], [7:0	8-0-0	9-3-0	7-11-13	8-1-2
Plate Offsets (X,Y)	[2:0-0-0,0-1-6], [6:0-5-14,0-3-4], [7:0	4-0,0-4-8], [8:0-5-14,0-3-4]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.97 BC 0.87 WB 0.60	Vert(LL) -0.40 Vert(CT) -0.77 Horz(CT) 0.15	16-17 >649 240 13 n/a n/a	PLATES GRIP MT20 244/190 MT20HS 187/143
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MS	Wind(LL) 0.14	17 >999 240	Weight: 230 lb FT = 20%
6-8: 2x BOT CHORD 2x4 SF WEBS 2x4 SF WEDGE Left: 2x4 SP No.3				2-0-0 oc purlins (3-2-2 n	ng directly applied, except nax.): 6-8. Jlied or 10-0-0 oc bracing.
Max H Max U Max G FORCES. (Ib) - Max. TOP CHORD 2-3=- 11-13	e) 2=0-3-8, 13=Mechanical orz 2=139(LC 12) plift 2=-16(LC 12), 13=-3(LC 13) rav 2=1709(LC 1), 13=1668(LC 1) Comp./Max. Ten All forces 250 (lb 3064/269, 3-5=-2945/324, 5-6=-235 3=-2935/276, 6-7=-2066/325, 7-8=-24	/339, 8-9=-2346/336, 9-11= 56/322	=-2847/331,		
WEBS 3-19=	=-190/2660, 17-19=-122/2218, 16-17 =-377/186, 5-19=-110/617, 5-17=-534 =-509/186, 9-14=-113/523, 11-14=-31	/187, 7-17=-82/858, 7-16=-			
NOTES-					
 Unbalanced roof live Wind: ASCE 7-10; V gable end zone and 41-8-8 zone; cantile reactions shown; Lu Provide adequate di All plates are MT20 This truss has been * This truss has been * This truss has bee will fit between the b Previde mechanical See Standard Indus designer. 	e loads have been considered for this 'ult=120mph Vasd=95mph; TCDL=6. C-C Exterior(2) -0-8-0 to 4-1-10, Inte ver left and right exposed ; end vertic mber DOL=1.60 plate grip DOL=1.60 rainage to prevent water ponding. plates unless otherwise indicated. designed for a 10.0 psf bottom chord n designed for a live load of 20.0psf ottom chord and any other members truss to truss connections. connection (by others) of truss to be try Piggyback Truss Connection Deta presentation does not depict the size	Dpsf; BCDL=6.0psf; h=32ft; rior(1) 4-1-10 to 18-4-2, Ext al left and right exposed;C-0 live load nonconcurrent wit on the bottom chord in all ar with BCDL = 10.0psf. aring plate capable of withst il for Connection to base tru	terior(2) 18-4-2 to 30-5-5, lu C for members and forces th any other live loads. teas where a rectangle 3-6 tanding 100 lb uplift at joint uss as applicable, or consu	nterior(1) 30-5-5 to & MWFRS for -0 tall by 2-0-0 wide (s) 2, 13. It qualified building	SEAL 036322



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and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

	Truss	Truss Type	Qty	Ply	DRHORTON/WILMINGTON; LOT 31 MCKAY PLACE	165579199
MP31	A03	COMMON	5	1	Job Reference (optional)	10357 9 1 9 9
Builders FirstSource, Apex, N	C 27523				8.630 s Mar 9 2023 MiTek Industries, Inc. Thu May 16 09:41	1:21 2024 Page 2
NOTES-			ID:VULCYJU7ZPRL	mSP/MPF	xTyf?fT-cEoswABALbgbLNOHDQFSBFGFv4_X?M_G	iyaO08FZFZ11
,		connection Detail for Connection to ba	•••	sult qualif	fied building designer.	
12) In the LOAD CASE(S) section, loads appl	ied to the face of the truss are noted	as front (F) or back (B).			
LOAD CASE(S)						
1) Dead + Roof Live (ba	lanced): Lumber Incre	ase=1.15, Plate Increase=1.15				
Uniform Loads (plf)	0, 6-11=-60, 24-27=-2					
		ninhab. Attic Storage: Lumber Increa	se=1.15, Plate Increase=1.15			
Uniform Loads (plf)		· · · · · · · · · · · · · · · · · · ·				
		0, 35-36=-50, 36-37=-20, 37-38=-50, e: Lumber Increase=1.25, Plate Increa				
Uniform Loads (plf)	Auto Minour Storage		436-1.25			
	, 6-11=-20, 24-27=-4					
 Dead + 0.6 C-C Wind Uniform Loads (plf) 	(Pos. Internal) Case	1: Lumber Increase=1.60, Plate Incre	ease=1.60			
u ,	2-31=25. 6-31=14. 6	-7=25, 7-11=14, 24-27=-12				
Horz: 1-2=-5	9, 2-31=-37, 6-31=-26	, 6-7=37, 7-11=26				
,	(Pos. Internal) Case	2: Lumber Increase=1.60, Plate Incre	ease=1.60			
Uniform Loads (plf) Vert: 1-2=9.	2-5=14, 5-6=25, 6-34	=14, 11-34=25, 24-27=-12				
	1, 2-5=-26, 5-6=-37, 6					
,	(Neg. Internal) Case	1: Lumber Increase=1.60, Plate Incre	ease=1.60			
Uniform Loads (plf) Vert: 1-2=-12	2, 2-6=-33, 6-11=-33,	24-27=-20				
	, 2-6=13, 6-11=-13					
,	(Neg. Internal) Case	2: Lumber Increase=1.60, Plate Incre	ease=1.60			
Uniform Loads (plf) Vert: 1-2=-28	, 2-6=-33, 6-11=-33, 1	24-27=-20				
	2-6=13, 6-11=-13					
	Wind (Pos. Internal) L	eft: Lumber Increase=1.60, Plate Incr	rease=1.60			
Uniform Loads (plf) Vert: 1-2=9.	2-6=-2, 6-11=9, 24-27	=-12				
Horz: 1-2=-2	1, 2-6=-10, 6-11=21					
,	Wind (Pos. Internal) R	ight: Lumber Increase=1.60, Plate Inc	crease=1.60			
Uniform Loads (plf) Vert: 1-2=4.	2-6=9, 6-11=-2, 24-27	=-12				
	6, 2-6=-21, 6-11=10					
,	Wind (Neg. Internal)	Left: Lumber Increase=1.60, Plate In	crease=1.60			
Uniform Loads (plf) Vert: 1-2=-	5, 2-6=-20, 6-11=-9,	24-27=-20				
Horz: 1-2=-	5, 2-6=-0, 6-11=11					
,	Wind (Neg. Internal)	Right: Lumber Increase=1.60, Plate I	Increase=1.60			
Uniform Loads (plf) Vert: 1-2=-4	l, 2-6=-9, 6-11=-20, 2	4-27=-20				
Horz: 1-2=-	16, 2-6=-11, 6-11=0					
,	Wind (Pos. Internal)	1st Parallel: Lumber Increase=1.60, F	Plate Increase=1.60			
Uniform Loads (plf) Vert: 1-2=1	7, 2-32=22, 6-32=11,	6-11=3. 24-27=-12				
Horz: 1-2=-	29, 2-32=-34, 6-32=-2	3, 6-11=15				
 Dead + 0.6 MWFRS Uniform Loads (plf) 	Wind (Pos. Internal)	2nd Parallel: Lumber Increase=1.60,	Plate Increase=1.60			
u ,	2, 2-6=3, 6-33=11, 11	33=22. 24-27=-12				
Horz: 1-2=-	10, 2-6=-15, 6-33=23	11-33=34				
14) Dead + 0.6 MWFRS Uniform Loads (plf)	Wind (Pos. Internal)	3rd Parallel: Lumber Increase=1.60, I	Plate Increase=1.60			
	, 2-6=11, 6-11=3, 24-3	27=-12				
	19, 2-6=-23, 6-11=15					
 Dead + 0.6 MWFRS Uniform Loads (plf) 	Wind (Pos. Internal)	4th Parallel: Lumber Increase=1.60, F	Plate Increase=1.60			
	2, 2-6=3, 6-11=11, 24	27=-12				
Horz: 1-2=-	10, 2-6=-15, 6-11=23					
,	Wind (Neg. Internal)	1st Parallel: Lumber Increase=1.60, I	Plate Increase=1.60			
Uniform Loads (plf) Vert: 1-2=9	, 2-32=4, 6-32=-6, 6-1	1=-15. 24-27=-20				
Horz: 1-2=-	29, 2-32=-24, 6-32=-1	4, 6-11=5				
	Wind (Neg. Internal)	2nd Parallel: Lumber Increase=1.60,	Plate Increase=1.60			
Uniform Loads (plf) Vert: 1-2=-	0, 2-6=-15, 6-33=-6,	11-33=4, 24-27=-20				
Horz: 1-2=-	10, 2-6=-5, 6-33=14,	11-33=24				
	le Attic Storage: Luml	per Increase=1.25, Plate Increase=1.2	25			
Uniform Loads (plf) Vert: 1-6=-2	20. 6-11=-20. 24-35=-	20, 35-36=-60, 36-37=-20, 37-38=-60), 27-38=-20, 20-21=-40(F)			
		ab. Attic Storage + 0.75(0.6 MWFRS		Increase	=1.60, Plate	
Increase=1.60		·	· ·			
Uniform Loads (plf) Vert: 1-2=-4	6. 2-6=-50 6-11=-42	24-35=-20, 35-36=-50, 36-37=-20, 3	37-38=-50, 27-38=-20 20-21-	-30(F)		
	, _ 0- 00, 0 11-42	- $ 20, 00 - 00, 00 20, 0$				

ntinued on page 3 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	DRHORTON/WILMINGTON; LOT 31 MCKAY PLACE
MP31	A03	COMMON	5	1	165579199
	7.00		°		Job Reference (optional)

Builders FirstSource, Apex, NC 27523

8.630 s Mar 9 2023 MiTek Industries, Inc. Thu May 16 09:41:21 2024 Page 3 ID:VULCYJU7zpRLimSP7MPFxTyf?fT-cEoswABALbgbLNOHDQFSBFGFv4_X?M_GyaO08FzFz1i

LOAD CASE(S)

20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-38, 2-6=-42, 6-11=-50, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)

Horz: 1-2=-12, 2-6=-8, 6-11=0

21) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-28, 2-32=-32, 6-32=-40, 6-11=-46, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)

Horz: 1-2=-22, 2-32=-18, 6-32=-10, 6-11=4

22) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-43, 2-6=-46, 6-33=-40, 11-33=-32, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)

Horz: 1-2=-7, 2-6=-4, 6-33=10, 11-33=18

23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-6=-60, 6-11=-20, 24-27=-20

24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-20, 6-11=-60, 24-27=-20

25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

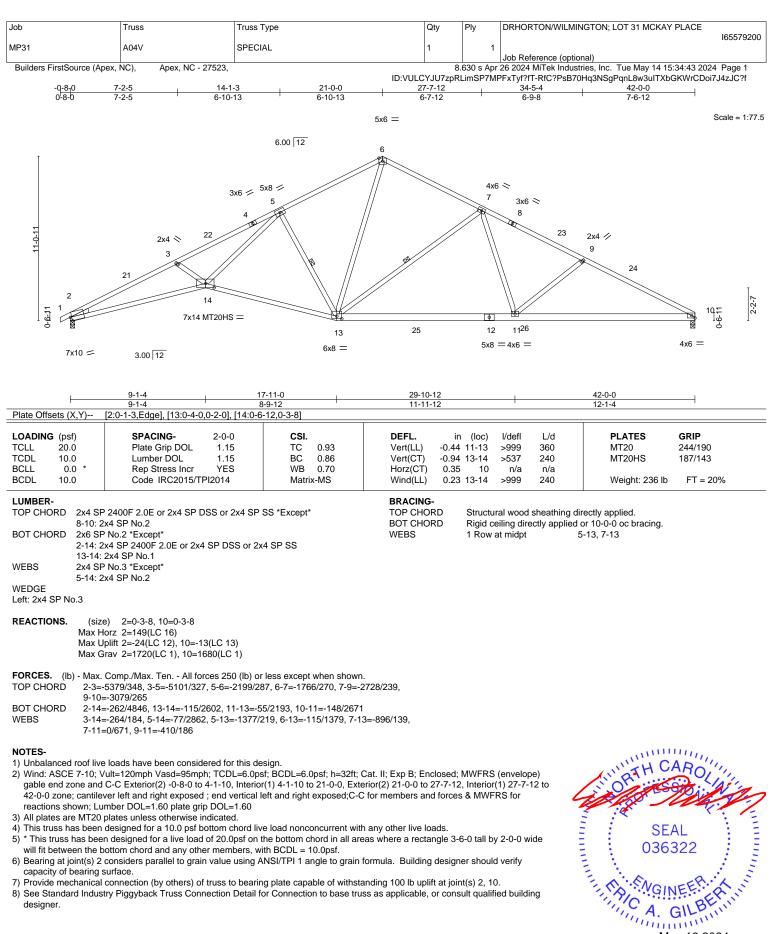
Vert: 1-6=-50, 6-11=-20, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)

26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-20, 6-11=-50, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)

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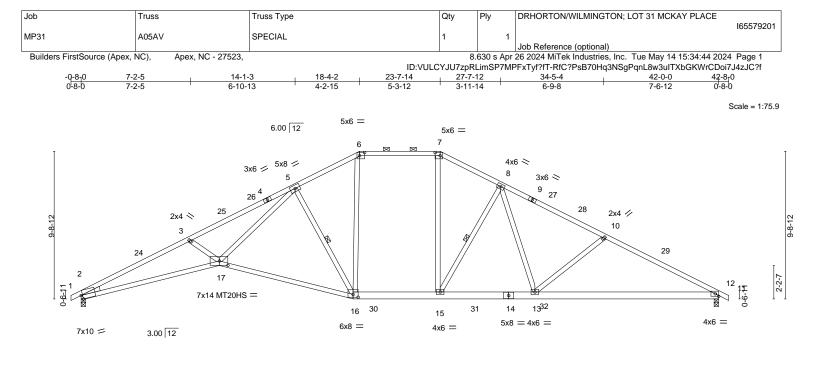




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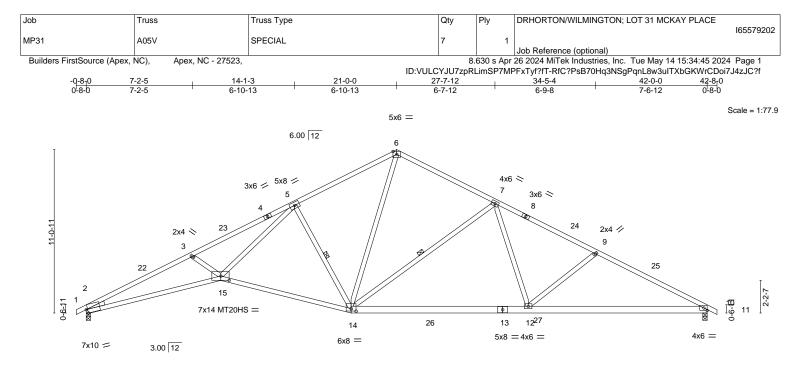
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L	9-1-4		7-11-0	23-7-14	29-10-12			42-0-0	
	9-1-4		-9-12	5-8-14	6-2-14	1		12-1-4	
Plate Offsets (X,Y)-	[2:0-1-3,Edge], [6:0-4-0,0	0-2-8], [7:0-4-0,	0-2-8], [16:0-4-0,0-2-4], [17:0-6-12,0-3-8]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC 1.00 BC 0.95 WB 0.71	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.58 16-17 -1.10 16-17 0.35 11	>458 n/a	L/d 360 240 n/a	PLATES MT20 MT20HS	GRIP 244/190 187/143
BCDL 10.0	Code IRC2015/T	PI2014	Matrix-MS	Wind(LL)	0.31 16-17	>999	240	Weight: 241 lb	FT = 20%
LUMBER- TOP CHORD 2x4 1-4, 6-7: BOT CHORD 2x6 2-1: 16- WEBS 2x4 5-1: WEDGE Left: 2x4 SP No.3 REACTIONS. ((Ma Ma Ma	SP No.1 *Except* 9-12: 2x4 SP 2400F 2.0E or 2x4 SP No.2	2x4 SP DSS or 2x SP DSS or 2x 5(LC 13) 720(LC 1) rcces 250 (lb) or	r 2x4 SP SS 4 SP SS less except when shown	BRACING TOP CHOI BOT CHOI WEBS	- RD Struc 2-0-0 RD Rigid 2-2-0	tural wood oc purlins	sheathing o (3-6-4 max	directly applied, except	
BOT CHORD 2- WEBS 3-	0-3060/21, 50-3162/40 0-11=-3069/299, 6-7=-1857/3 17=-326/4856, 16-17=-140/3 17=-269/190, 5-17=-117/28 13=-41/631, 10-13=-431/186	302 2587, 15-16=-1 75, 5-16=-1312	9/1857, 13-15=-86/2183, /220, 6-16=-50/712, 8-15	, 11-13=-190/2665				2011	110.
 Wind: ASCE 7-10 gable end zone; can reactions shown; Provide adequate All plates are MT This truss has be * This truss has be * This truss has be * This truss has be mill fit between th Bearing at joint(s capacity of bearing Provide mechani See Standard Ind designer. 	live loads have been consid b; Vult=120mph Vasd=95mp ind C-C Exterior(2) -0-8-0 to tilever left and right expose a drainage to prevent water 20 plates unless otherwise i en designed for a 10.0 psf b ween designed for a 10.0 psf been designed for a live load e bottom chord and any oth) 2 considers parallel to grai rg surface. cal connection (by others) of fustry Piggyback Truss Con-	h; TCDL=6.0ps 4-1-10, Interior ; end vertical I p DOL=1.60 bonding. ndicated. ottom chord liv of 20.0psf on t er members, wi n value using A	f; BCDL=6.0psf; h=32ft; (1) 4-1-10 to 18-4-2, Extu- eft and right exposed;C-C e load nonconcurrent with he bottom chord in all are th BCDL = 10.0psf. NSI/TPI 1 angle to grain g plate capable of withsta or Connection to base tru	erior(2) 18-4-2 to 3 C for members and h any other live lo eas where a recta formula. Building anding 100 lb uplit uss as applicable,	30-5-5, Interior d forces & MW ads. ngle 3-6-0 tall designer shou it at joint(s) 2, or consult qua	r(1) 30-5-5 /FRS for by 2-0-0 w uld verify 11. lified buildi	to	SE 036	322 VEER-ALU
Design valid for u a truss system. B building design. is always require	rify design parameters and READ No se only with MiTek® connectors. The fore use, the building designer mu Bracing indicated is to prevent buck d for stability and to prevent collapses and bucken correction and buckens of	nis design is based st verify the applica ling of individual true with possible pers	only upon parameters shown, a bility of design parameters and ss web and/or chord members onal injury and property damag	and is for an individual to properly incorporate the only. Additional tempo ge. For general guidance	ouilding componer is design into the rary and permane be regarding the	nt, not overall nt bracing			ERING BY ENCO A MITEK Attiliate

a truss system: Define use, the bounding designer indix verify the appricationity of design parameters and property incorporate rink design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality** Criteria and **DSE-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



+	<u>9-1-4</u> 9-1-4	17-11-0 8-9-12	29-10- ⁻ 11-11- ⁻		42-0-0 12-1-4	
Plate Offsets (X,	Y) [2:0-1-3,Edge], [14:0-4-0,	0-2-0], [15:0-6-12,0-3-8]	1			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0		-	DEFL. 0.93 Vert(LL) 0.86 Vert(CT) 0.70 Horz(CT)	in (loc) l/defl -0.43 12-14 >999 -0.93 14-15 >542 0.35 10 n/a	360 MT20 240 MT20HS n/a	244/190 5 187/143
BOT CHORD 2	Code IRC2015/TF 2x4 SP 2400F 2.0E or 2x4 SP D 2x6 SP No.2 *Except* 2-15: 2x4 SP 2400F 2.0E or 2x4	SS or 2x4 SP SS	K-MS Wind(LL) BRACING- TOP CHOF BOT CHOF WEBS	D Structural woo	od sheathing directly applied. lirectly applied or 10-0-0 oc bra	
1	14-15: 2x4 SP No.1		WEB0		011,111	

2x4 SP No.3 *Except* WEBS 5-15: 2x4 SP No.2

WEDGE Left: 2x4 SP No.3

REACTIONS.	(size)	2=0-3-8, 10=0-3-8
	Max Horz	2=145(LC 16)
	Max Uplift	2=-24(LC 12), 10=-24(LC 13)
	Max Grav	2=1720(LC 1), 10=1720(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

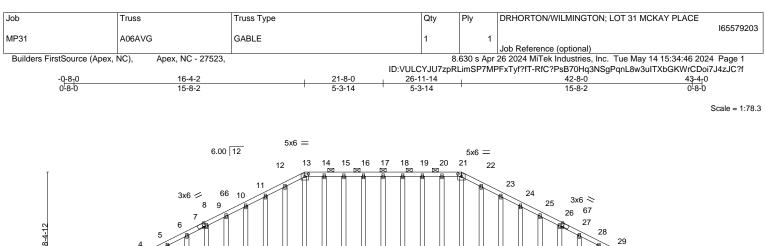
- TOP CHORD 2-3=-5378/342, 3-5=-5100/318, 5-6=-2199/287, 6-7=-1766/268, 7-9=-2726/231, 9-10 = -3077/255
- BOT CHORD 2-15=-228/4845, 14-15=-93/2601, 12-14=-47/2191, 10-12=-122/2670
- WEBS 3-15=-264/185, 5-15=-69/2861, 5-14=-1377/212, 6-14=-113/1377, 7-14=-893/139, 7-12=0/671, 9-12=-412/186

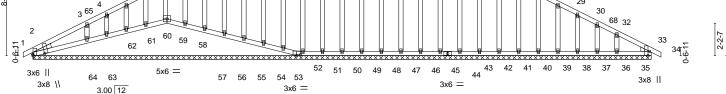
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-0 to 4-1-10, Interior(1) 4-1-10 to 21-0-0, Exterior(2) 21-0-0 to 27-7-12, Interior(1) 27-7-12 to 42-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.
- 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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-0 <mark>-8₁0</mark>	9-9-4	18-7-0					42-8-0			43-4 ₁ 0
0-8-0	9-1-4	8-9-12					24-1-0			0-8-0
Plate Offsets (X,Y)	[2:0-1-8,0-9-8], [2:0-0-14,E	Edge], [7:0-1-13,Edge	, [13:0-3-0,0-2-	-0], [21:0-3-0,0-2-0],	27:0-1	-13,Edg	e], [33:0-	-3-8,Edge], [5	3:0-3-0,0-0-12]	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	1.15 T 1.15 E	SI. C 0.14 C 0.10 /B 0.11	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.01	(loc) 33 34 33	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI	2014 N	latrix-S						Weight: 344 lb	FT = 20%
BOT CHORD 2x4 S	P No.2 P No.2 P No.3			BRACING- TOP CHOR BOT CHOR	D	2-0-0 c Rigid c	c purlins	(6-0-0 max.) ectly applied	rectly applied or 6-0-0 c : 13-21. or 10-0-0 oc bracing, I	

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. All bearings 42-0-0.

(lb) - Max Horz 2=123(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 60, 53, 2, 50, 51, 52, 54, 56, 57, 58, 59, 61, 62, 64, 49, 48, 47, 45, 43, 42, 41, 40, 39, 38, 37, 36, 35

Max Grav All reactions 250 lb or less at joint(s) 60, 53, 2, 33, 50, 51, 52, 54, 55, 56, 57, 58, 59, 61, 62, 63, 49, 48, 47, 46, 45, 43, 42, 41, 40, 39, 38, 37, 36, 35 except 64=306(LC 23)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-0 to 4-1-10, Interior(1) 4-1-10 to 15-8-2, Exterior(2) 15-8-2 to 22-4-0, Interior(1) 22-4-0 to 26-3-14, Exterior(2) 26-3-14 to 33-0-0, Interior(1) 33-0-0 to 42-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Provide adequate drainage to prevent water ponding.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 1-4-0 oc.

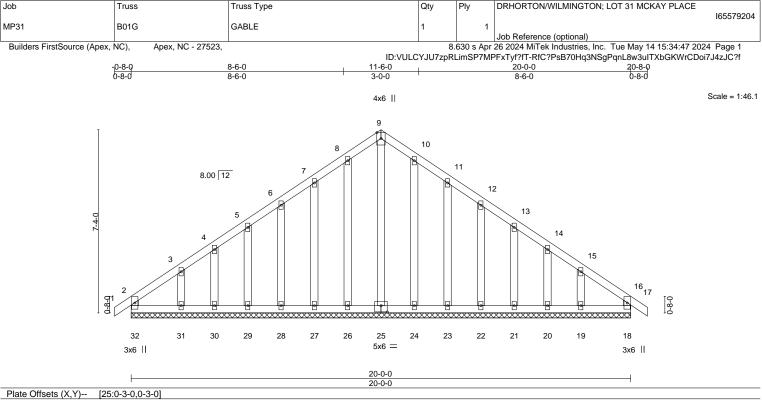
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 60, 53, 2, 50, 51, 52, 54, 56, 57, 58, 59, 61, 62, 64, 49, 48, 47, 45, 43, 42, 41, 40, 39, 38, 37, 36, 35.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



4-4

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	[20:0 0 0,0 0 0]			
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. TC 0.08 BC 0.05	DEFL. in (loc) I/defl L/d Vert(LL) -0.00 16 n/r 120 Vert(CT) -0.00 16 n/r 120	PLATES GRIP MT20 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr NO Code IRC2015/TPI2014	WB 0.13 Matrix-R	Horz(CT) 0.00 18 n/a n/a	Weight: 145 lb FT = 20%
LUMBER-			BRACING-	

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x4 SP No.2		except end verticals.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2x4 SP No.3		

REACTIONS. All bearings 20-0-0.

Max Horz 32=167(LC 11) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 32, 18, 26, 27, 28, 29, 30, 31, 24, 23, 22, 21, 20, 19 All reactions 250 lb or less at joint(s) 32, 18, 25, 26, 27, 28, 29, 30, 31, 24, 23, 22, 21, 20, 19 Max Grav

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

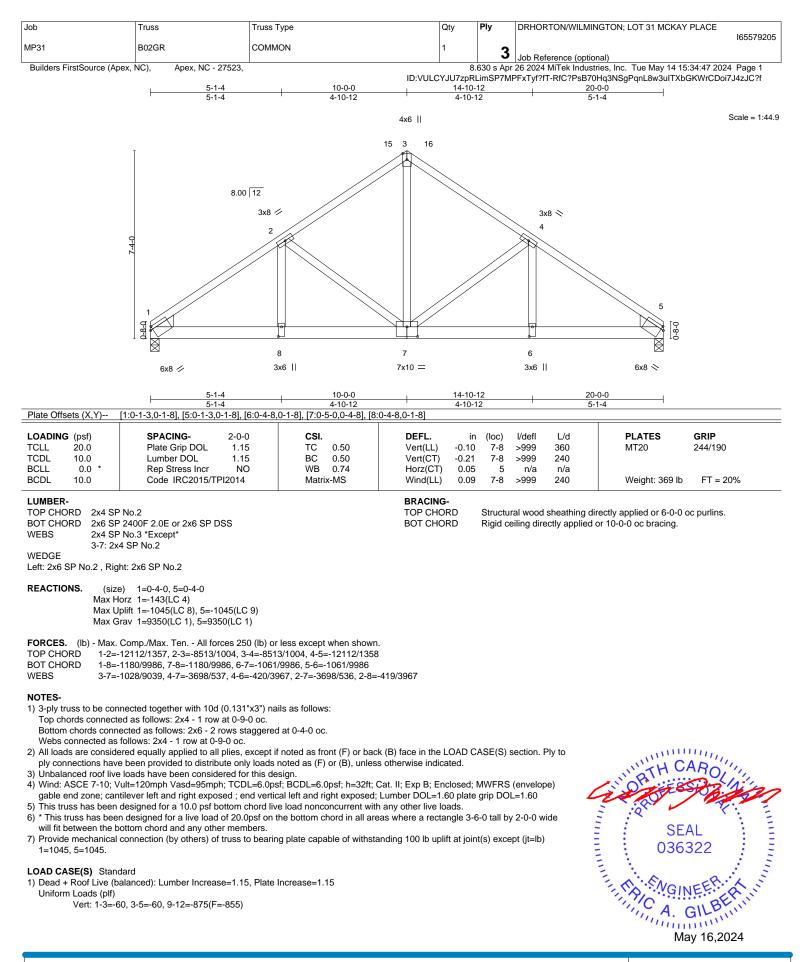
NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-0 to 4-1-10, Exterior(2) 4-1-10 to 10-0-0, Corner(3) 10-0-0 to 14-9-10, Exterior(2) 14-9-10 to 20-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 1-4-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 9) will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 32, 18, 26, 27, 28, 29, 30, 31, 24, 23, 22, 21, 20, 19.

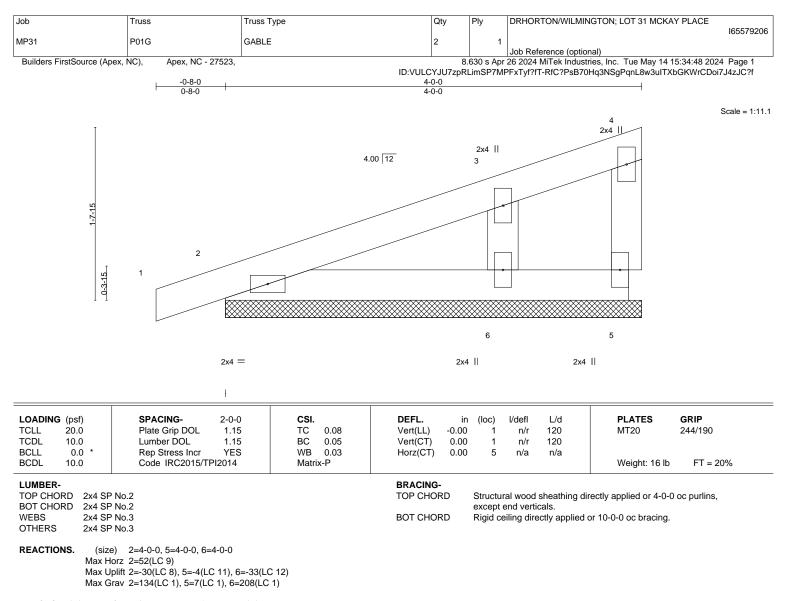


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818 Soundside Road



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult gualified building designer as per ANSI/TPI 1.

3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 1-4-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

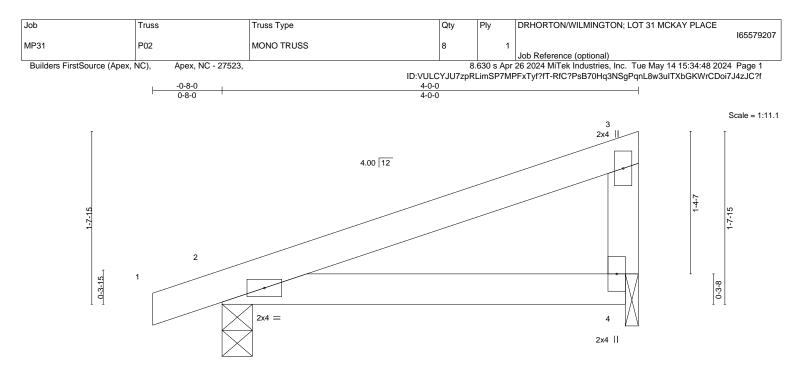
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5, 6.



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A MiTek Affili 818 Soundside Road Edenton, NC 27932



	<u> </u>		4-0-0 4-0-0				
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.19	DEFL. Vert(LL) -0.	in (loc) .01 4-7	l/defl L/d >999 360	PLATES MT20	GRIP 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.17	Vert(CT) -0.	.02 4-7	>999 240	WI 20	244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.00 Matrix-MP	- (-) -	.00 2 .01 4-7	n/a n/a >999 240	Weight: 15 lb	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=52(LC 11) Max Uplift 2=-39(LC 8), 4=-21(LC 12) Max Grav 2=198(LC 1), 4=151(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

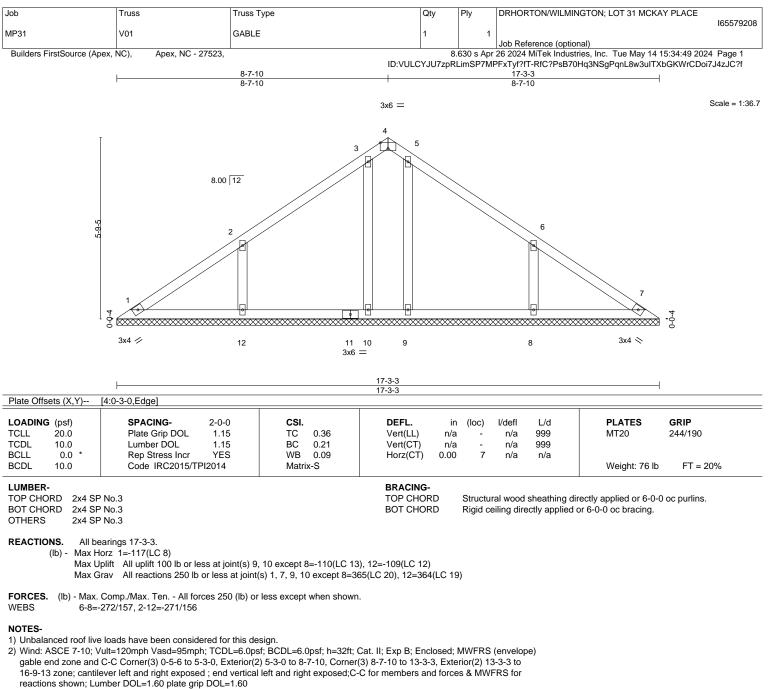
 Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



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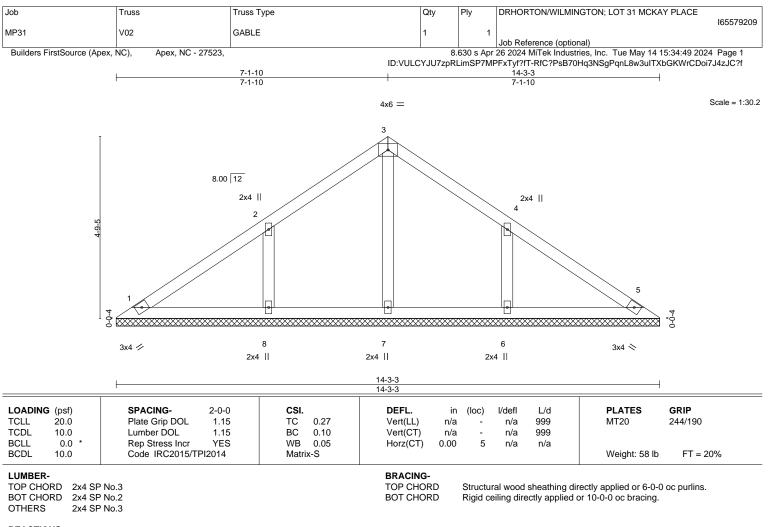


- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 4-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 10 except (jt=lb) 8=110, 12=109.



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REACTIONS. All bearings 14-3-3.

(lb) - Max Horz 1=96(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 6=-101(LC 13), 8=-101(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 6=335(LC 20), 8=335(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-5-6 to 5-3-0, Exterior(2) 5-3-0 to 7-1-10, Corner(3) 7-1-10 to 11-11-3, Exterior(2) 11-11-3 to 13-9-13 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 4-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

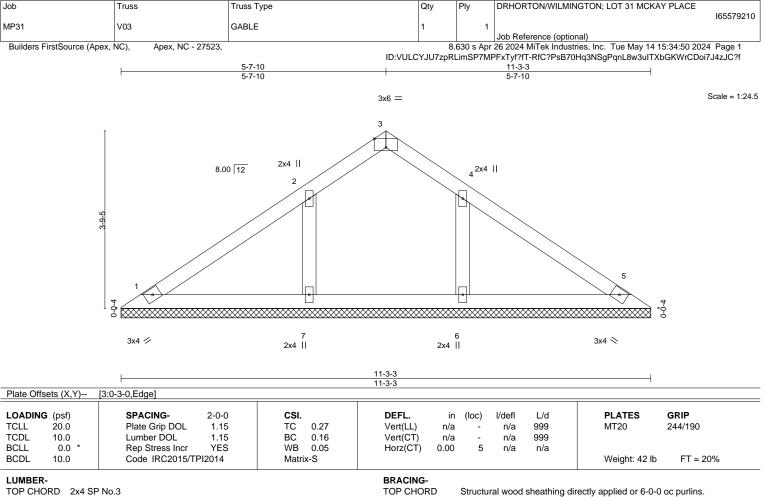
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 6=101, 8=101.



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A MiTek Affi 818 Soundside Road



BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3OTHERS2x4 SP No.3

REACTIONS. All bearings 11-3-3.

(lb) - Max Horz 1=-74(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 6, 7

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 6=303(LC 20), 7=306(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-5-6 to 5-3-0, Exterior(2) 5-3-0 to 5-7-10, Corner(3) 5-7-10 to 10-5-3, Exterior(2) 10-5-3 to 10-9-13 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 4-0-0 oc.

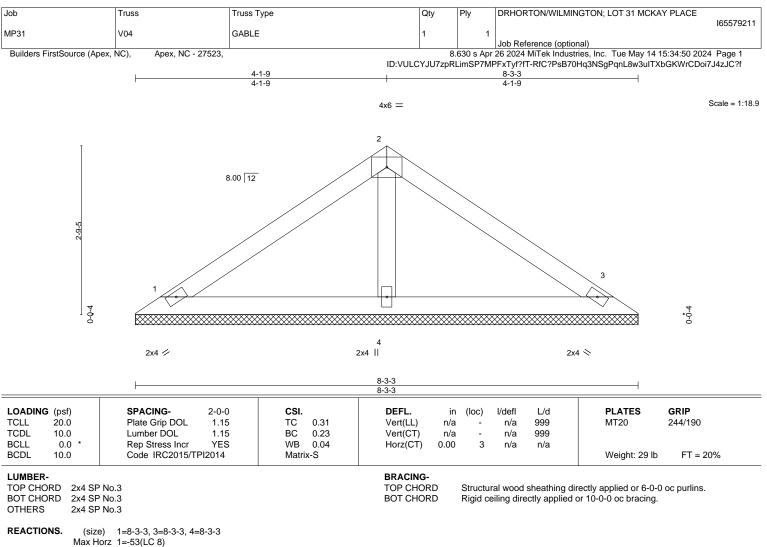
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 7.



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Max Horz 1=-35(LC - 6)Max Uplift 1=-18(LC - 12), 3=-25(LC - 13)Max Grav 1=144(LC - 1), 3=144(LC - 1), 4=302(LC - 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.

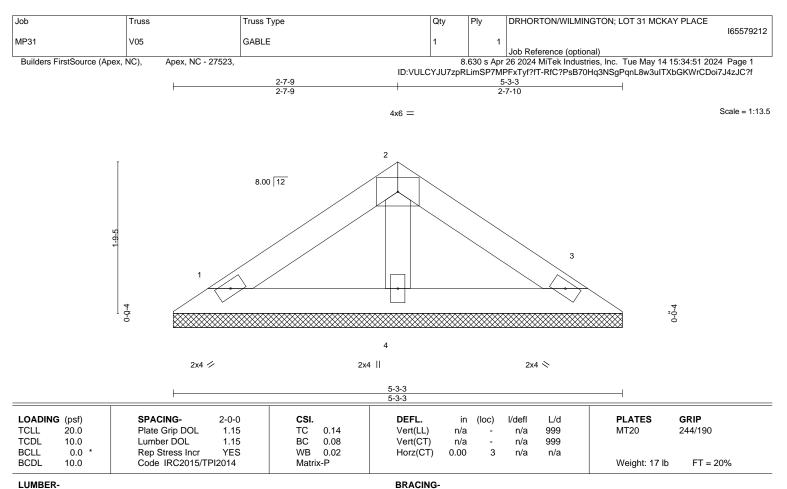
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.3 BOT CHORD OTHERS

2x4 SP No.3 2x4 SP No.3

REACTIONS. (size) 1=5-3-3, 3=5-3-3, 4=5-3-3 Max Horz 1=31(LC 11) Max Uplift 1=-15(LC 12), 3=-19(LC 13) Max Grav 1=93(LC 1), 3=93(LC 1), 4=162(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

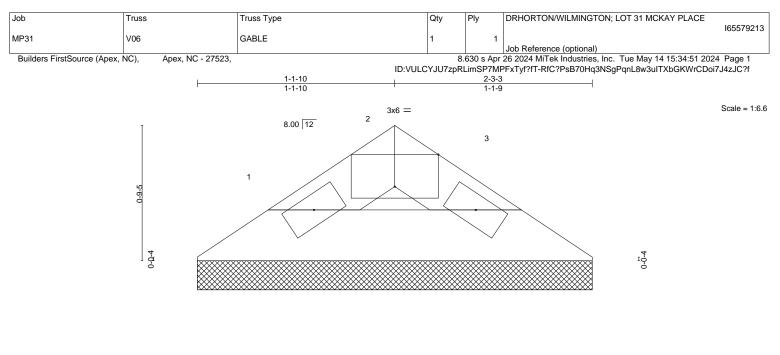


Structural wood sheathing directly applied or 5-3-3 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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2x4 🥢

2x4 📎

BRACING-

TOP CHORD

BOT CHORD

Plate Offsets (X,Y)--[2:0-3-0,Edge] SPACING-PLATES GRIP LOADING (psf) 2-0-0 CSI. DEFL in (loc) l/defl L/d 20.0 Plate Grip DOL 1.15 тс 0.01 Vert(LL) 999 MT20 244/190 n/a n/a 10.0 Lumber DOL 1.15 BC 0.04 Vert(CT) n/a n/a 999 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 3 n/a n/a

LUMBER-

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3

10.0

REACTIONS. (size) 1=2-3-3, 3=2-3-3 Max Horz 1=-10(LC 8) Max Uplift 1=-3(LC 12), 3=-3(LC 13) Max Grav 1=55(LC 1), 3=55(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

Code IRC2015/TPI2014

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-P

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



FT = 20%

Weight: 6 lb

Structural wood sheathing directly applied or 2-3-3 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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