

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 24-4130-A RVF-LOT #12 ROOF

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Riverside Roof Truss.

Pages or sheets covered by this seal: I66743289 thru I66743321

My license renewal date for the state of North Carolina is December 31, 2024.

North Carolina COA: C-0844



July 9,2024

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





818 Soundside Road

Edenton, NC 27932



and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #12 ROOF	
						166743290
24-4130-A	HG01	HIP GIRDER	1	2		
				_	Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s Ju	in 13 2024 MiTek Industries, Inc. Tue Jul 9 09:32:28 2024	Page 2
		ID:	tdHS5IWy	Lng?jaR9	E1eBtgyly9 -LefjQJ1cCbylu2tNt A0w58NDH2KEMXe6abb0)Vyzr01

NOTES-

14) Use Simpson Strong-Tie THJA26 (THJA26 on 2 ply, Left Hand Hip) or equivalent at 4-0-6 from the left end to connect truss(es) to front face of bottom chord.

15) Use Simpson Strong-Tie THJA26 (THJA26 on 2 ply, Right Hand Hip) or equivalent at 7-11-10 from the left end to connect truss(es) to front face of bottom chord.

(a) Fill all nail holes where hanger is in contact with lumber.
(17) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-43, 2-3=-43, 3-4=-53, 4-5=-43, 5-6=-43, 7-10=-20

Concentrated Loads (lb)

Vert: 9=-339(F) 8=-339(F) 13=-135(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCEL Building Component Schut Information, purplication for the trust structure Bucking Component Advancement and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 POLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.08 BC 0.03 WB 0.03	DEFL. Vert(LL) -0.0 Vert(CT) -0.0 Horz(CT) -0.0	n (loc) D 6 D 5-6 D 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCLL 0.0 "	Code IBC2019/TDI2014	Motrix MD	()				Woight: 11 lb	ET 200/
BCDL 10.0	Code IRC2016/1FI2014	IVIAUIX-IVIP					weight. This	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 1-11-11 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 6=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 6=67(LC 16) Max Uplift 6=-20(LC 16), 3=-10(LC 13), 4=-14(LC 16)

Max Grav 6=159(LC 21), 3=37(LC 21), 4=36(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 3, 4.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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July 9,2024

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A MiTek



TRENGINEERING BY A MITEK Atfiliate



			4-0-0	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.26 BC 0.16 WB 0.06 Matrix-MP	DEFL. in (loc) l/defl L/d Vert(LL) -0.01 4-5 >999 240 Vert(CT) -0.02 4-5 >999 180 Horz(CT) -0.00 4 n/a n/a Weight: 24 lb FT = 20	0%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-

 TOP CHORD
 Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 4=Mechanical Max Horz 5=97(LC 13) Max Uplift 5=-39(LC 16), 4=-26(LC 13)

Max Grav 5=221(LC 2), 4=155(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 3-10-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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Edenton, NC 27932



ENGINEERING BY RENCO A Mittek Affiliate

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 Coblecture spaced at 2.0.0 ac
- 10) Gable studs spaced at 2-0-0 oc.11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 20.0ps for the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 12, 11.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #12 ROOF	
24-4130-A	T02G		1			166743299
24 4100 //	1020			3	Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s Ju	n 13 2024 MiTek Industries, Inc. Tue Jul 9 09:32:35 2024	Page 2

ID:tdHS5IWyLng?jaR9E1eBtqyIy9_-e_aNui7?ZlqmD7vjnyofiZxVp6KrNS7gjAoTlbyzr?w

NOTES-

13) Use Simpson Strong-Tie LUS26 (4-10d Girder, 3-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 13-5-4 from the left end to 17-5-4 to connect truss(es) to front face of bottom chord.

14) Fill all nail holes where hanger is in contact with lumber.
15) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-43, 5-9=-43, 10-16=-20

Concentrated Loads (lb)

Vert: 13=-246(F) 11=-132(F) 19=-501(F) 20=-501(F) 21=-501(F) 22=-499(F) 24=-686(F) 26=-686(F) 27=-246(F) 29=-246(F) 31=-133(F) 33=-132(F) 34=-132(F) 34=-132(F) 32=-132(F) 32=-1 35=-132(F)

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Max Horz 15=-210(LC 14) Max Uplift 15=-59(LC 16), 13=-78(LC 16), 10=-71(LC 16) Max Grav 15=466(LC 34), 13=1559(LC 28), 10=719(LC 29)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-362/68, 3-5=0/372, 7-8=-746/78, 2-15=-394/108, 8-10=-619/111

BOT CHORD 14-15=-104/461, 13-14=-111/271, 11-13=0/544, 10-11=-66/284

WEBS 5-13=-569/37, 7-13=-823/118, 7-11=0/396, 3-13=-629/143, 8-11=0/303

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=30ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 14-9-0, Exterior(2R) 14-9-0 to 17-9-0, Interior(1) 17-9-0 to 30-5-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

7) All plates are 2x4 MT20 unless otherwise indicated.

8) Gable studs spaced at 2-0-0 oc.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 13, 10.
 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

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11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14 except (jt=lb)

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Job	Truss	Truss Type	Qty	Ply	RVF-LOT #12 ROOF				
					16	6743301			
24-4130-A	103	PIGGYBACK BASE	3	1					
					Job Reference (optional)				
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s Ju	n 13 2024 MiTek Industries, Inc. Tue Jul 9 09:32:37 2024 P	age 2			
		ID:tdHS5lWyLng?jaR9E1eBtqyIy9aMi7JO8F4M4UTR36uNr7o_0mBw_FrGIzAUHapUyzr?u							

NOTES-

12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #12 ROOF	
					166	6743302
24-4130-A	103A	PIGGYBACK BASE	1	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s Ju	in 13 2024 MiTek Industries, Inc. Tue Jul 9 09:32:39 2024 Pa	age 2
			ID:tdHS5I\	NyLng?jaF	R9E1eBtqyIy9Wlqtk4AVczKBikCU0otbtP65ijgjJAFFeomhuNy	/zr?s

NOTES-

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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L	58-6-0										
				58-6-0							
Plate Offsets (Offsets (X,Y) [14:0-3-0,0-4-0], [19:0-4-0,0-4-8], [23:0-3-0,0-4-0], [43:0-4-0,0-4-8], [51:0-4-0,0-4-8]										
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TPl2	2-0-0 CSI. 1.15 TC 1.15 BC YES WB 2014 Matri	0.14 0.05 0.17 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 0.00 0.01	(loc) 1 1 35	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 588 lb	GRIP 244/190 FT = 20%
LUMBER-				BR	ACING-						
TOP CHORD	2x6 SP No.2			TO	P CHORD	Structura	l wood	sheathing	directly a	pplied or 6-0-0 oc purlins,	
BOT CHORD	2x6 SP No.2					except e	nd verti	cals, and	2-0-0 oc pi	urlins (6-0-0 max.): 14-23	
WEBS	2x4 SP No.3			BO	T CHORD	Rigid cei	ling dire	ctly appli	ed or 10-0-	0 oc bracing.	
OTHERS	2x4 SP No.3			WE	BS	1 Row at	midpt		23-45, 2	2-46, 21-47, 20-48, 19-49	9, 18-50,
SLIDER	Left 2x4 SP No	3 1-6-4					·		17-51, 1 24-44, 2	6-52, 15-53, 14-54, 13-5 5-43	5, 12-56,

50

49 48

51

7x8 =

47 46

44 43 42

7x8 =

45

REACTIONS. All bearings 58-6-0. (lb) - Max Horz 2=234(LC 15)

4x5 ||

64 63 62 61 60 59

Max Uplift All uplift 100 lb or less at joint(s) 35, 2, 46, 47, 48, 49, 50, 51, 52, 55, 56, 58, 59, 60, 61, 62, 63, 64, 44, 43, 42, 41, 40, 39, 38, 37, 36

56₅₅

58

57

4x6 =

53 52

54

Max Grav All reactions 250 lb or less at joint(s) 35, 2, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 58, 59, 60, 61, 62, 63, 64, 44, 43, 42, 41, 40, 39, 38, 37, 36

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 11-12=-122/253, 12-13=-125/301, 13-14=-138/339, 14-15=-124/324, 15-16=-124/324, 16-17=-124/324, 17-18=-123/324, 18-19=-123/324, 19-20=-123/324, 20-21=-123/324,

21-22=-123/324, 22-23=-124/323, 23-24=-138/339, 24-25=-124/300, 25-26=-106/252

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=59ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -0-11-0 to 4-11-6, Exterior(2N) 4-11-6 to 20-6-0, Corner(3R) 20-6-0 to 26-6-0, Exterior(2N) 26-6-0 to 38-6-0, Corner(3R) 38-6-0 to 44-6-0, Exterior(2N) 44-6-0 to 58-4-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

37 36 35

3x4 ||

40 39 38

41

Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #12 ROOF			
24 4120 4	TOOOF		1	1	1667	743303		
24-4130-A	TUSGE	PIGGTBACK BASE SUPPO	1	· ·	Job Reference (optional)			
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s Ju	in 13 2024 MiTek Industries, Inc. Tue Jul 9 09:32:41 2024 Pag	je 2		
		lD:tdHS5lWyLng?jaR9E1eBtqyIy9S8ye9mBm8bavx2Mt7Dv3yqBbQXWTnE5Y56FnyFyzr?q						

NOTES-

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 35, 2, 46, 47, 48, 49, 50, 51, 52, 55, 56, 58, 59, 60, 61, 62, 63, 64, 44, 43, 42, 41, 40, 39, 38, 37, 36.

14) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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L	10-4-12	20-6-0	29-6-0	36-6-4	48-10-4	58-7-8	
Γ	10-4-12	10-1-4	9-0-0	7-0-4	12-4-0	9-9-4	1
Plate Offsets	s (X,Y) [6:0-5-4,0-3-0	0], [13:Edge,0-2-4]					
LOADING () TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	psf) 20.0) 16.5/15.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2018/TPI2014	CSI. TC 0.79 BC 0.68 WB 0.94 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d -0.21 14-16 >693 240 -0.31 14-16 >477 180 0.06 16 n/a n/a	PLATES MT20 Weight: 445 lb	GRIP 244/190 FT = 20%
LUMBER.			B				
TOP CHORE BOT CHORE	2x6 SP No.2 2x6 SP No.2		T	OP CHORD	Structural wood sheathing directly an except end verticals, and 2-0-0 oc pu	oplied or 3-11-15 oc purli urlins (6-0-0 max.): 6-9.	ins,
WEBS	2x4 SP No.3 *Excep	t*	В	OT CHORD	Rigid ceiling directly applied or 10-0-	0 oc bracing, Except:	
	7-16: 2x4 SP No.1				6-0-0 oc bracing: 14-16.		
SLIDER	Left 2x4 SP No.3 2-6	6-0	v	VEBS	1 Row at midpt 4-19, 9-	16, 12-14	

2 Rows at 1/3 pts

6-17, 7-16

REACTIONS. All bearings 0-3-8 except (jt=length) 13=Mechanical.

(lb) - Max Horz 2=236(LC 15)

Max Uplift All uplift 100 lb or less at joint(s) 16 except 2=-124(LC 16), 14=-135(LC 16)

Max Grav All reactions 250 lb or less at joint(s) 13 except 2=1623(LC 28), 16=2882(LC 28), 14=1062(LC 49)

- TOP CHORD 2-4=-2368/257, 4-6=-1404/268, 6-7=-517/265, 7-9=0/641, 9-11=-42/489, 11-12=-83/412
- BOT CHORD
 2-21=-150/2245, 19-21=-150/2245, 17-19=0/1253, 16-17=0/546, 14-16=-465/137

 WEBS
 4-21=0/508, 4-19=-1168/190, 6-19=0/998, 6-17=-1214/61, 7-17=0/1209, 7-16=-1980/179, 9-16=-769/135, 9-14=-24/274, 11-14=-714/307, 12-14=-496/170

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=59ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 4-11-6, Interior(1) 4-11-6 to 20-6-0, Exterior(2R) 20-6-0 to 28-9-8, Interior(1) 28-9-8 to 38-6-0, Exterior(2R) 38-6-0 to 46-9-8, Interior(1) 46-9-8 to 58-5-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16 except (jt=lb) 2=124, 14=135.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Edenton, NC 27932

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

L	10-4-12	20-6-0	29-6-0	36-6-4	48-10-4	58-7-8	
	10-4-12	10-1-4	9-0-0	7-0-4	12-4-0	9-9-4	I
Plate Offsets	s (X,Y) [5:0-5-4,0-3-0)], [12:Edge,0-2-4]					
LOADING (F TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	psf) 20.0) 16.5/15.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI. TC 0.79 BC 0.67 WB 0.94 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L -0.21 13-15 >693 2 -0.31 13-15 >477 11 0.06 15 n/a r	L/d PLATES (40 MT20 80 n/a Weight: 443 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORE BOT CHORE	 2x6 SP No.2 2x6 SP No.2 2x4 SP No.3 *Excercition 	ŧ*	E	BRACING- FOP CHORD	Structural wood sheathing di except end verticals, and 2-0	irectly applied or 3-11-15 oc pu 0-0 oc purlins (6-0-0 max.): 5-8.	rlins,
SLIDER	6-15: 2x4 SP No.1 Left 2x4 SP No.3 2-6	с Э-О	۲. ۱	WEBS	6-0-0 oc bracing: 13-15. 1 Row at midpt 3 2 Rows at 1/3 pts 5	3-18, 8-15, 11-13 5-16, 6-15	

REACTIONS. All bearings 0-3-8 except (jt=length) 12=Mechanical.

(lb) - Max Horz 1=229(LC 15)

Max Uplift All uplift 100 lb or less at joint(s) 1, 15 except 13=-135(LC 16)

Max Grav All reactions 250 lb or less at joint(s) 12 except 1=1573(LC 27), 15=2879(LC 27), 13=1063(LC 48)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-3=-2362/267, 3-5=-1407/273, 5-6=-518/269, 6-8=0/638, 8-10=-41/487, 10-11=-83/410

 BOT CHORD
 1-20=-150/2251, 18-20=-150/2251, 16-18=0/1255, 15-16=0/547, 13-15=-462/133

 WEBS
 3-20=0/508, 3-18=-1171/190, 5-18=0/999, 5-16=-1214/61, 6-16=0/1209, 6-15=-1980/179, 8-15=-766/134, 8-13=-24/272, 10-13=-714/307, 11-13=-495/170

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=59ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0 to 5-10-6, Interior(1) 5-10-6 to 20-6-0, Exterior(2R) 20-6-0 to 28-9-8, Interior(1) 28-9-8 to 38-6-0, Exterior(2R) 38-6-0 to 46-9-8, Interior(1) 46-9-8 to 58-5-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

- 4) Unbalanced snow loads have been considered for this design.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 15 except (jt=lb) 13=135.

10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

A MiTek / 818 Soundside Road Edenton, NC 27932

2=130, 14=104, 18=109. 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and Continuing and ANSI/TPI 1.

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818 Soundside Road

Edenton, NC 27932

July 9,2024

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #12 ROOF	
					166	3743306
24-4130-A	T05	PIGGYBACK BASE	2	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s Ju	in 13 2024 MiTek Industries, Inc. Tue Jul 9 09:32:45 2024 Pa	ige 2
		ID:	tdHS5IWy	Lng?jaR9E	E1eBtqyIy9LvB8_7EGCp5LQggeM2_?6gL7Z8jijtM80jD?50yz	r?m_

NOTES-

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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1			59-0-0					1	
			59-0-0					1	
Plate Offsets (2	X,Y) [2:1-10-12	2,0-2-0], [14:0-3-0,0-4-0], [19:0-4-0,0-4-	8], [23:0-3-0,0-4-0], [33	8:0-2-12,0-2-1], [35	:1-8-10,0-2-0)]			
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI. TC 0.05 BC 0.02 WB 0.17 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (lc -0.00 -0.00 0.01	oc) l/defl 35 n/r 35 n/r 35 n/a	L/d 120 120 n/a	PLATES MT20 Weight: 597 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD	2x6 SP No.2 2x6 SP No.2			BRACING- TOP CHORD	Structural w 2-0-0 oc pur	vood sheathi rlins (6-0-0 n	ng directly ap nax.): 14-23.	plied or 6-0-0 oc purlins	, except
OTHERS	2x4 SP No.3			BOT CHORD	Rigid ceiling	directly app	blied or 10-0-0) oc bracing.	4 40 50
SLIDER	Left 2x4 SP No.3	2-6-0, Right 2x4 SP No.3 2-6-0		WEBS	1 Row at mi	lapt	23-47, 22 17-54, 16 24-46, 25	:-48, 21-49, 20-50, 19-5 :-55, 15-56, 14-57, 13-5 :-45	1, 18-52, 8, 12-59,
REACTIONS.	All bearings 59	-0-0.							

(lb) - Max Horz 2=-216(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 2, 48, 49, 50, 51, 52, 54, 55, 58, 59, 61, 62, 63, 64, 65, 66, 67, 46, 45, 43, 42, 41, 40, 39, 38, 37

Max Grav All reactions 250 lb or less at joint(s) 2, 47, 48, 49, 50, 51, 52, 54, 55, 56, 57, 58, 59, 61, 62, 63, 64, 65, 66, 67, 46, 45, 43, 42, 41, 40, 39, 38, 37, 35

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 12-13=-104/261, 13-14=-120/300, 14-15=-112/289, 15-16=-112/289, 16-17=-112/289, 17-18=-112/289, 18-19=-112/289, 19-20=-112/289, 20-21=-112/289, 21-22=-112/289, 22-23=-112/289, 23-24=-120/300, 24-25=-104/261

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=59ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -0-11-0 to 4-11-13, Exterior(2N) 4-11-13 to 20-6-0, Corner(3R) 20-6-0 to 26-6-0, Exterior(2N) 26-6-0 to 38-6-0, Corner(3R) 38-6-0 to 44-6-0, Exterior(2N) 44-6-0 to 59-11-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Continued on page 2

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818 Soundside Road

Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #12 ROOF	
04 4420 A	TOFOE		1	1		166743307
24-4130-A	TUDGE	PIGGTBACK BASE SUPPO	1	1	Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s Ju	n 13 2024 MiTek Industries, Inc. Tue Jul 9 09:32:47 2024	Page 2
		ID:tdF	IS5IWyLng	g?jaR9E1e	Btqyly9HHJvPpGWkRL3gzq1UT0TC5RfLyZGByaRT1i6	Avyzr?k

NOTES-

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 48, 49, 50, 51, 52, 54, 55, 58, 59, 61, 62, 63, 64, 65, 66, 67, 46, 45, 43, 42, 41, 40, 39, 38, 37.

14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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L	10-4-12	20-6-0	28-6-2	36-6-4	48-7-4		59-0-0	
1	10-4-12	10-1-4	8-0-2	8-0-2	12-1-0	I	10-4-12	
Plate Offset	s (X,Y) [6:0-5-4,	0-3-0], [17:0-4-0,0-5-4], [21:0-2-4,0-4	-12]					
LOADING (psf)						BI 4750	
TCLL (roof)	20.0	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	I/defi L/d	PLATES	GRIP
Snow (Pf/Pc	1) 16 5/15 0	Plate Grip DOL 1.15	TC 0.76	Vert(LL)	-0.37 19-20 :	>720 240	MT20	244/190
TCDI	10.0	Lumber DOL 1.15	BC 0.87	Vert(CT)	-0.58 19-20 :	>462 180		
BCU	0.0 *	Rep Stress Incr YES	WB 1.00	Horz(CT)	0.03 21	n/a n/a		
BCDI	10.0	Code IRC2018/TPI2014	Matrix-MS				Weight: 471 lb	FT = 20%
LUMBER-				BRACING-				
TOP CHOR	D 2x6 SP No.2			TOP CHORD	Structural wood sh	neathing directly ap	plied or 4-6-8 oc purlin	s, except
BOT CHOR	D 2x6 SP 2400F 2	2.0E *Except*			2-0-0 oc purlins (6	6-0-0 max.): 6-10.		
	21-23,23-25: 2x	6 SP No.2, 18-20: 2x4 SP No.1		BOT CHORD	Rigid ceiling direct	tly applied or 6-0-0	oc bracing. Except:	
WEBS	2x4 SP No.3 *E	xcept*			6-0-0 oc bracing:	18-20		
	10-21,10-16: 2x	4 SP No.2		WEBS	1 Row at midpt	4-24, 7-2	2	
SLIDER	Left 2x4 SP No.	.3 2-6-0, Right 2x4 SP No.3 2-6-0			2 Rows at 1/3 pts	6-22, 9-2	21, 10-20	
REACTION	 S. (size) 2=0- Max Horz 2=2: Max Uplift 2=-1 Max Grav 2=1: 	-3-8, 14=0-3-8, 21=0-3-8 22(LC 15) !41(LC 16), 14=-87(LC 16) 584(LC 28), 14=923(LC 29), 21=3717	7(LC 28)					
FORCES. TOP CHOR BOT CHOR WEBS	(lb) - Max. Comp./N D 2-4=-2291/29 10-12=-1220/ D 2-26=-146/21 16-17=-257/1 4-26=0/523, 4 9-22=-114/19 16-18=-142/1	Max. Ten All forces 250 (lb) or less (1, 4-6=-1315/299, 6-7=-512/305, 7-9: 310, 12-14=-1048/121 82, 24-26=-146/2182, 22-24=0/1176, 07, 14-16=-2/910, 19-20=-353/0, 18- -24=-1184/183, 6-24=0/970, 6-22=-1 51, 9-21=-1922/204, 20-21=-1645/66 738, 12-16=-689/300, 17-19=-292/0	except when shown. 512/305, 9-10=0/801, 21-22=-775/99, 17-21= 9=-353/0 193/31, 7-22=-661/169, , 10-20=-1418/106, 10-	-257/107, 18=-117/2036,				
NOTES-								
 Unbalance Wind: AS II; Exp B; to 28-10- exposed grip DOL: TCLL: AS DOL=1.1: surcharge Unbalance This truss non-conce Provide a 	ed roof live loads h CE 7-16; Vult=130r Enclosed; MWFRS 2, Interior(1) 28-10-; ; end vertical left an =1.60 GCE 7-16; Pr=20.0 p 5); Is=1.0; Rough C e applied to all expo ted snow loads have a has been designed urrent with other livu idequate drainage to	ave been considered for this design. nph (3-second gust) Vasd=103mph; (directional) and C-C Exterior(2E) -0 2 to 38-6-0, Exterior(2R) 38-6-0 to 46 d right exposed;C-C for members and osf (roof LL: Lum DOL=1.15 Plate DO at B; Partially Exp.; Ce=1.0; Cs=1.00 used surfaces with slopes less than 0. e been considered for this design. d for greater of min roof live load of 12 e loads. o prevent water ponding.	FCDL=6.0psf; BCDL=6. -11-0 to 4-11-13, Interio -10-2, Interior(1) 46-10- d forces & MWFRS for r L=1.15); Pg=15.0 psf; F ; Ct=1.10, Lu=50-0-0; N 500/12 in accordance v 2.0 psf or 1.00 times flat	0psf; h=25ft; B=45ft; r(1) 4-11-13 to 20-6- 2 to 59-11-0 zone; ca eactions shown; Lum Pf=16.5 psf (Lum DOI lin. flat roof snow load vith IBC 1608.3.4.	L=59ft; eave=7ft; C 0, Exterior(2R) 20-6 antilever left and rig ober DOL=1.60 plat L=1.15 Plate d governs. Rain on overhangs	at. 5-0 ht e	SEAL 036322	
7) This truss	s has been designed	d for a 10.0 psf bottom chord live load	nonconcurrent with an	y other live loads.		1	A. GINEF	AN
8) * This true	ss has been design	ed for a live load of 20.0psf on the bo	ttom chord in all areas	where a rectangle 3-6	6-0 tall by 2-0-0 wid	e ''	CAGILB	Fin

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

9) Bearing at joint(s) 21 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

Continued on page 2

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GILBE

July 9,2024

GILDIN

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #12 ROOF	
					16	66743308
24-4130-A	T05S	PIGGYBACK BASE	3	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s Ju	n 13 2024 MiTek Industries, Inc. Tue Jul 9 09:32:48 2024 F	Page 2
			ID:tdHS5I	WyLng?jaF	R9E1eBtqyIy9IUtHd9H9VkTwH7PD2BXikIzf0LhGwCvaihRfi	iLyzr?j

NOTES-

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14 except (jt=lb) 2=141.
11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria and DSE-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #12 ROOF		
24 4120 4	TD01	Diagubask Bass	F	1	166743311		
24-4130-A		Piggyback Base	5		Job Reference (optional)		
Riverside Roof Truss, LLC,	Danville, Va - 24541,	8.730 s Jun 13 2024 MiTek Industries, Inc. Tue Jul 9 09:32:51 2024 Page 2					
		ID:tdHS5IWyLng?jaR9E1eBtqyly9A3YQFBJ1nfrV8b7ojJ5PMxbCAZj06Zi0OfgJJgyzr?g					

NOTES-

10) Provide metal plate or equivalent at bearing(s) 22 to support reaction shown.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 22 except (jt=lb) 31=134.
 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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July 9,2024

July 9,2024

Edenton, NC 27932

A MiTek . 818 Soundside Road

Edenton, NC 27932

A MITEK AT MIL

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 4-7-3, Exterior(2R) 4-7-3 to 7-7-3, Interior(1) 7-7-3 to 8-7-14 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate
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- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

- exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

- 4) Unbalanced snow loads have been considered for this design.
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REACTIONS. (size) 1=4-2-0, 3=4-2-0 Max Horz 1=12(LC 15) Max Uplift 1=-7(LC 16), 3=-7(LC 16) Max Grav 1=120(LC 2), 3=120(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate
- DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
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- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
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