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The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 50509 JOB: 24-5966-R01 JOB NAME: LOT 0.0010 HONEYCUTT HILLS Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. 28 Truss Design(s)

Trusses:

GR01, GR02, R01, R02, R03, R03A, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, R14, R15, R16, SP01, SP02, VT01, VT02, VT03, VT04, VT05, VT06, VT07



Warning !--- Verify design parameters and read notes before use.



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(14)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 4-0-0, Corner(3R) 4-0-0 to 16-0-0, Corner(3E) 16-0-0 to 20-10-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B: Partially Exp.: Ce=1.0: Cs=1.00: Ct=1.10

Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

7) Provide adequate drainage to prevent water ponding.

11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide with the bottom chord and any other members.
13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplife at the true to the

LOAD CASE(S) Standard







<u> </u>									
Plate Offsets (X,Y) [7:0-6-8,0-2-8], [15:0-6-8,0-2-8], [30:0-2-0,0-1-8]									
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.13 BC 0.10 WB 0.15 Matrix-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 0.00	(loc) 21 21 22	l/defl n/r n/r n/a	L/d 180 80 n/a	PLATES MT20 Weight: 200 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No BOT CHORD 2x4 SP No WEBS 2x4 SP No OTHERS 2x4 SP No	.2 .3 .3 .3	В	RACING- OP CHORD OT CHORD	Structo end ve Rigid o MiTe be in Insta	ural wo erticals ceiling ek reco istallec illation	ood shea directly mmenda I during guide.	athing dire applied or s that Stat truss erec	ctly applied or 6-0-0 oc 6-0-0 oc bracing. bilizers and required cro tion, in accordance with	purlins, except ss bracing Stabilizer

REACTIONS. All bearings 31-6-0. (lb) - Max Horz 39=112(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 39, 22, 31, 32, 33, 34, 35, 36, 37, 38, 29, 28, 27, 26, 25, 24, 23

- Max Grav All reactions 250 lb or less at joint(s) 39, 22, 38, 23 except 31=286(LC 44), 32=297(LC 44), 33=300(LC
 - 44), 34=289(LC 52), 35=318(LC 47), 36=276(LC 47), 37=259(LC 39), 29=296(LC 44), 28=300(LC 44), 27=285(LC 52), 26=313(LC 49), 25=276(LC 49), 24=259(LC 39)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.

NOTES-(15)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-9-0, Corner(3R) 3-9-0 to 13-9-0, Exterior(2N) 13-9-0 to 17-9-0, Corner(3R) 17-9-0 to 27-9-0, Corner(3E) 27-9-0 to 32-4-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- PROFESSIO 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 11) Gable studs spaced at 2-0-0 oc.
- 12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

LOAD CASE(S) Standard

MORPHS INTERNAL MORPHS INTERNAL '1/202 'sd and' for Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

NOINEE

7/11/2024







Job	Truss	Truss Type		Qty Ply	LOT 0.0010 H	ONEYCUTT HILLS "	199 SHELBY MEADOW	LANE ANGIER, NC
24-5966-R01	R04	GABLE		1	1	ce (ontional)	# 5	50509
-0-10 ₁ 8 0-10-8	<u>8-0-4</u> 8-0-4	15-9-0 7-8-12	Run: 8 ID:R4 20-10- 5-1-5	8.430 s Feb 12 202 4JAOCFxLoAUF 5 24 3-6	Print: 8.430 s Feb 1 pwqRUTCbAz_Vw 4-5 29- 6-0 5-0 5-0	2 2021 MiTek Industr 6-U98Pu0071hnol -6-6 2-2	ries, Inc. Thu Jul 11 20:: Ix0OsYV1AjKRnG2v 35-0-0 35-1 5-5-10 0-1(38:35 2024 Page 1 y_B_LHluNyz1xo 0 ₁ 8)-8
			5x8 =					Scale = 1:66.1
	546 //	7.00 12 26 12	5	27 4x6 =	5x8 =			
10-2-4 1-0-0 5x5 2-2-8 2-2-8 2-2-4 2-2-2-2 2-2-2-2 2-2-2-2 2-2-2-2 2-2-2-2 2-2-2-2 2-2-2-2 2-2-2-2 2-2-2-2 2-2-2-2 2-2-2-2 2-2-2-2 2-2-2-2 2-2-2-2 2-2-2-2 2-2-2-2 2-2-2-2-2 2-2-2-2-2 2-2-2-2-2 2-2-2-2-2 2-	4 5 = 5x5 = 3 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	W1 W2	B2	W4 W4	STTT STTT 5 W4 8 V	28 829 5T3 75 W6 B3 B3	5x5 = 9 5x5 = ST5 ST6 9 5x5 =	11 10-0 1-2-8
⊠ 8x8	30	31 16 32 5x12 MT20HS=	3	¹³ 15 6x8 =	³⁴ 14 4x4 =		6x8 13 12	11
	10-6-14 10-6-14		20-10-5 10-3-6	24- 3-6	4-5 -0	<u>31-6-0</u> 7-1-11	35-0-0	
Plate Offsets (X,Y) [4:0	<u> -3-0,0-3-4], [7:0-6-0,0-2-4]</u>	, <u>[8:0-1-11,0-1-0], [10:0-</u> ;	<u>-15,0-0-2], [15:0-4-0</u>	0,Edge], [16:0-6	5-0,0-3-0]			
TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/TF	2-0-0 CSI. 1.25 TC 1.25 BC YES WB PI2014 Matr	0.98 \ 0.86 \ 0.58 H x-SH	DEFL. ir /ert(LL) -0.52 /ert(CT) -0.78 Horz(CT) 0.08	n (loc) l/defl 2 15-16 >726 8 15-16 >486 8 10 n/a	L/d 240 180 n/a	PLATESOMT202MT20HS1Weight: 244 lb	GRIP 244/190 187/143 FT = 20%
LUMBER- TOP CHORD 2x4 SP No T2: 2x4 SP BOT CHORD 2x4 SP SP P2: 2x4 SP	0.2 *Except* P SS, T5: 2x4 SP No.1 S *Except* P No.1		BRAC TOP C BOT C WEBS	ING- CHORD Stru CHORD Rigi	uctural wood shea id ceiling directly ow at midpt	athing directly ap applied or 10-0-0 6-15	plied.) oc bracing.	
WEBS 2x4 SP No OTHERS 2x4 SP No SLIDER Left 2x8 S).3).3 P No 2 -° 4-9-1 Right 2v8	SP No 2 -° 3-3-4		Mi be Ins	e installed during t stallation guide.	truss erection, in	and required cross accordance with S	s bracing Stabilizer
REACTIONS. All bearin (Ib) - Max Horz Max Uplift Max Grav	ngs 3-9-8 except (jt=length 2=-192(LC 10) All uplift 100 lb or less at All reactions 250 lb or less) 2=0-3-8. joint(s) except 2=-117(L is at joint(s) 13 except 2=	C 14), 10=-163(LC 1 :1595(LC 24), 10=1;	15), 12=-208(LC 809(LC 41), 13:	C 37) =344(LC 7)			
FORCES. (lb) - Max. Co TOP CHORD 2-3=-230 6-27=-24	mp./Max. Ten All forces)9/190, 3-4=-2197/230, 4-2 \33/339, 6-7=-1990/261, 7-	250 (lb) or less except w 6=-2117/256, 5-26=-201 28=-2055/257, 8-28=-20	hen shown. 6/280, 5-27=-2412/3 56/230, 8-29=-2179	364, /278.				
9-29=-23 BOT CHORD 2-30=-17 15-33=-2	322/276, 9-10=-2496/263 /5/1971, 30-31=-175/1971, 24/1441, 15-34=-81/1715, 4	16-31=-175/1971, 16-32 4-34=-81/1715, 13-14=-	e=-24/1441, 32-33=- 174/1907, 12-13=-1	24/1441, 74/1907,				
10-12=-1 WEBS 4-16=-40 8-14=-32	74/1907)2/227, 5-16=-110/880, 5-1 24/152	5=-212/1416, 6-15=-140	4/246, 7-15=-33/744	1,				
NOTES- (14) 1) Unbalanced roof live ld 2) Wind: ASCE 7-16; Vul (envelope) gable end z Interior(1) 20-6-10 to 2 for members and force 3) Truss designed for win Gable End Details as a 4) TCLL: ASCE 7-16; Pr= Cat B; Partially Exp.; C 5) Unbalanced snow load 6) This truss has been de non-concurrent with ot 7) Provide adequate draii 8) All plates are MT20 pla 9) All plates are 2x4 MT2 10) Gable studs spaced a	bads have been considered t=120mph (3-second gust) zone and C-C Exterior(2E) 0-10-5, Exterior(2R) 20-10 is & MWFRS for reactions and loads in the plane of the applicable, or consult quali :20.0 psf (roof LL: Lum DO ce=1.0; Cs=1.00; Ct=1.10 Is have been considered for signed for greater of min r her live loads. nage to prevent water pond ates unless otherwise indicated t 2-0-0 oc.	d for this design. Vasd=95mph; TCDL=5. -0-10-8 to 3-11-2, Interior(1) 5 to 29-1-14, Interior(1) shown; Lumber DOL=1.4 truss only. For studs ex fied building designer as L=1.25 Plate DOL=1.25 or this design. oof live load of 12.0 psf of ding. ated. ed.	Opsf; BCDL=5.0psf; r(1) 3-11-2 to 10-11 29-1-14 to 31-0-14, 30 plate grip DOL=1 cposed to wind (norr per ANSI/TPI 1. ; Pf=20.0 psf (Lum I pr 2.00 times flat roc	h=23ft; Cat. II; -6, Exterior(2R) Exterior(2E) 31 .60 nal to the face) DOL=1.15 Plate	Exp B; Enclosed) 10-11-6 to 20-6- -0-14 to 35-10-8 , see Standard In e DOL=1.15); Is= osf on overhangs	; MWFRS 10, zone;C-C dustry 1.0; Rough	TH CAROUND ROFESSION SEAL 28147	IIII III IIII IIIIIIIIIIIIIIIIIIIIIIII
11) This truss has been o	lesigned for a 10.0 psf bott	om chord live load nonc	oncurrent with any o	ther live loads.	C	14	7/11/2024	1
warning :verity design Continued on page 2 vertically. Applicability of of individual web members	parameters and read notes l design parameters and proper i only. Additional temporary br	neorporation of component is acting to ensure stability during	sed only upon parameters responsibility of build or construction is the re-	ars snown, and is f ing designer – not sponsibility of the	tor an individual buil t truss designer or tru e erector. Additional	uss engineer. Bracing logramma bracing	ng shown is for lateral	u support re is the

of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0010 HONEYCUTT HILLS 199 SHI	ELBY MEADOW LANE ANGIER, NC
24-5966-R01	R04	GABLE	1	1	Job Reference (optional)	# 50509
		1	Run: 8.430 s Feb 12 D:R4JAOCFxLoAU	2 2021 Prin JFpwqRU	t: 8.430 s Feb 12 2021 MiTek Industries, Inc ITCbAz_Vw6-yMio6M1Io?vfNSVCyZ3k	Thu Jul 11 20:38:36 2024 Page 2 ZOFVBAcHePELD?1sRpyz1xn

NOTES- (14)

12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 2, 163 lb uplift at joint 10 and 208 lb uplift at joint 12.

LOAD CASE(S) Standard

















		34-6	8-8					
34-8-8 Plate Offsets (X,Y) [8:0-6-8,0-2-8], [16:0-6-8,0-2-8]								
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.13 BC 0.10 WB 0.20 Matrix-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loo 0.00 -0.00 0.00 2	c) l/defl 1 n/r 1 n/r 23 n/a	L/d 180 80 n/a	PLATES MT20 Weight: 235 lb	GRIP 244/190 D FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3	2 3 3 3		BRACING- TOP CHORD BOT CHORD	Structura end vertio Rigid ceil MiTek r be insta Installat	I wood shea cals. ling directly ecommend alled during tion guide.	athing direc applied or 6 s that Stabil truss erection	tly applied or 6-0-0 oc 3-0-0 oc bracing. izers and required cro on, in accordance with	purlins, except oss bracing o Stabilizer

REACTIONS. All bearings 34-8-8.

(lb) - Max Horz 43=127(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 43, 23, 33, 34, 36, 37, 38, 39, 40, 41, 32, 30, 29, 27, 26, 25, 24 except 42=-108(LC 14)

Max Grav All reactions 250 lb or less at joint(s) 43, 23, 42, 24 except 33=297(LC 44), 34=296(LC 44), 36=301(LC

44), 37=292(LC 52), 38=314(LC 47), 39=326(LC 47), 40=273(LC 47), 41=263(LC 39), 32=297(LC 44), 30=299(LC

44), 29=294(LC 52), 28=294(LC 49), 27=331(LC 49), 26=273(LC 49), 25=256(LC 39)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(15)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 5-10-2, Corner(3R) 5-10-2 to 15-4-4, Exterior(2N) 15-4-4 to 19-4-4, Corner(3R) 19-4-4 to 29-4-4, Exterior(2N) 29-4-4 to 29-9-2, Corner(3E) 29-9-2 to 34-6-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

 Indicated.
 Indicate.
 Indicate.
 Indicate.
 MORPHINE MOR vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583



REACTIONS. All bearings 16-6-0.

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(13)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) 0-1-12 to 4-3-0, Corner(3R) 4-3-0 to 12-3-0, Corner(3E) 12-3-0 to 16-4-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Provide adequate drainage to prevent water ponding.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- 8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will the test of the bottom chord and any other members, with BCDL = 10.0psf. Provide mechanical connection (by others) of truss to bearing plate copoble of with the test of the bottom. 11)
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 19, 10, 15, 16, 13 except (jt=lb) 18=130, 11=128.

LOAD CASE(S) Standard



⁽lb) - Max Horz 19=-81(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 19, 10, 15, 16, 13 except 18=-130(LC 12), 11=-128(LC 13) Max Grav All reactions 250 lb or less at joint(s) 19, 10, 15, 16, 17, 13, 12, 11 except 18=252(LC 19)

Job	Truss	Truss Type	Qty	Ply	LOT 0.0010 HONEYCUTT HIL	LLS 199 SHELBY MEADOW LANE ANGIER, NC
24-5966-R01	R13	Hip Girder	1	2	Job Reference (optional)	# 50509
			Run: 8.430 s Feb ID:R4JAOCFxLoAU	12 2021 Pri JFpwqRU	int: 8.430 s Feb 12 2021 MiTek li TCbAz Vw6-nW33MQ5WOr	ndustries, Inc. Thu Jul 11 20:38:42 2024 Page 1 gp5NzMlqA8pfVZ8bd422hDbwUAeTyz1xh
	4-	10-2 8-3-0 10-2 3-4-1/	<u>11-7</u>	-14 -14		
		102 041			1102	See = 4.20.2
		5x6 = 2			5x6 =	Scale - 1.50.5
I	[T2			Ţ
				/		
	12.00 12			/		
	/	71	,		pt	
6-3-5 -1-6		/ W3		4		
- 6	5x6 //				w3	
	1					
		V2			W2	
			Ľ			
	⊠ 10 9 4×6 II .	11 8 12 13	7 14	15	6 16 17	5
	HTU26	11026 8x8 = H102 HTU26	6 6x6 = HTU26	HTU26	8x8 = HTU26	4x6
		111020	111020		111020	
	4-6	-12 4-10-2 8-3-0	11-7	-14	11 ₁ 11-4 16-6-0	
Plate Offsets (X.Y) [2:0	4-6 0-3-4.0-2-0], [3:0-3-4.0-2-0]	<u>-12</u> 0-3-63-63-4-14 [6:0-3-8.0-4-12] [7:0-3-0.0-4-8	4 <u>3-4-</u> 1. [8:0-3-8.0-4-12]	-14	0-3-6 4-6-12	
LOADING (psf)	SPACING	200 00		in (loc) l/defl l/d	
TCLL (roof) 20.0 Snow (Pf) 20.0	Plate Grip DOL	1.25 TC 0.82	Vert(LL)	-0.06	5-6 >999 240	MT20 244/190
TCDL 10.0	Lumber DOL Rep Stress Incr	1.25 BC 0.97 NO WB 0.77	Vert(CT) Horz(CT)	-0.10 0.02	5-6 >999 180 5 n/a n/a	
BCLL 0.0 * BCDL 10.0	Code IRC2021/TI	PI2014 Matrix-SH		0.02	0 104 104	Weight: 240 lb FT = 20%
LUMBER-			BRACING-			
TOP CHORD 2x4 SP No BOT CHORD 2x6 SP No	o.2 o.2		TOP CHORD	Structu end ver	ral wood sheathing directl rticals.	y applied or 4-3-10 oc purlins, except
WEBS 2x4 SP No.	o.3 *Except*		BOT CHORD	Rigid co	eiling directly applied or 10	0-0-0 oc bracing.
VV 1. 2X4 C	DF NU.2					
REACTIONS. (Ib/size) Max Horz	9=6956/0-3-8 (min. 0-1-8) 9=92(LC 7)), 5=6171/0-3-8 (min. 0-1-8)				
Max Uplif	$\dot{t}9=-3\dot{3}0(LC^{'}10), 5=-324(LC^{'})$	() () ()				
	/9-/135(EC 4), 5-01/1(EC					
TOP CHORD 1-2=-58	omp./Max. Ten All forces 73/295, 2-3=-4733/279, 3-4	250 (lb) or less except when she =-5820/322, 1-9=-5130/260, 4-5	own. 5=-5091/283			
BOT CHORD 9-10=-13	33/549, 10-11=-133/549, 8- 30/3072 7-14=-213/3030	-11=-133/549, 8-12=-239/3972,	12-13=-239/3972, 39 6-16=-76/495			
16-17=-2	76/495, 5-17=-76/495					
WEBS 2-7=-100 3-6=-14	6/1509, 3-7=-72/1682, 1-8= 1/2452	-211/3/15, 4-6=-228/3/10, 2-8=	-92/2508,			
NOTES- (13)						
1) 2-ply truss to be conne	ected together with 10d (0.	131"x3") nails as follows:				
Top chords connected Bottom chords connected	t as follows: 2x4 - 1 row at cted as follows: 2x6 - 2 row	0-9-0 oc. s staggered at 0-4-0 oc.				
Webs connected as for	ollows: 2x4 - 1 row at 0-9-0	0C.	r back (B) face in the l		SE(S) section Divito niv	
connections have bee	n provided to distribute onl	y loads noted as (F) or (B), unle	ss otherwise indicated			
 Unbalanced roof live live Wind: ASCE 7-16; Vul 	oads have been considere It=120mph (3-second gust)	d for this design. Vasd=95mph; TCDL=5.0psf; B	CDL=5.0psf; h=23ft; C	at. II; Exp	B; Enclosed; MWFRS	
(envelope) gable end	zone; Lumber DOL=1.60 p	late grip DOL=1.60	$0 \operatorname{nof}(\operatorname{lum} \operatorname{DOI} = 1.15)$	E Diata D(MUNITH CARO
Cat B; Partially Exp.; (Ce=1.0; Cs=1.00; Ct=1.10		.0 psi (Luiii DOL-1. it	Fiale D	JL-1.15), IS-1.0, Rouging	POFESSION STILL
 6) Provide adequate drai 7) This truss has been de 	nage to prevent water pone esigned for a 10.0 psf botto	ding. om chord live load nonconcurren	t with any other live lo	ads.	IIIII	CRAL STATISTICS
8) * This truss has been	designed for a live load of	30.0psf on the bottom chord in a	Ill areas where a recta	ngle 3-6-0	0 tall by 1-0-0 wide w∰ fit	28147
9) Bearing at joint(s) 9, 5	considers parallel to grain	value using ANSI/TPI 1 angle to	o grain formula. Buildi	ng desigr	ner should verify capacity	
of bearing surface. 10) Provide mechanical	connection (by others) of tr	uss to bearing plate capable of v	withstanding 100 lb up	lift at ioin	t(s) except (it=lb) 9=330	4 NOINEER S
5=324.		- 11 10dv1 1/2 Truce Single Di	(Cirdor) or or vivolont	, 	t 2.1.9 co mov starting	RAK K. MORMUN
at 0-6-12 from the lef	ft end to 14-6-12 to connec	t truss(es) R08 (1 ply 2x4 SP), F	R09 (1 ply 2x4 SP), R1	0 (1 ply 2	x4 SP) to back face of	7/11/0024
bottom chord.	re hanger is in confact with	WIDE This design to hand 1	upon noremator1	and in f	n individual buildige	//11/2024
Continued on page 2 vertically. Applicability of	design parameters and proper i	ncorporation of component is response	sibility of building designe	and is for a r – not trus	s designer or truss engineer.	Bracing shown is for lateral support
of individual web members	only. Additional temporary br	acing to ensure stability during constr	uction is the responsibility	of the ere	ctor. Additional permanent br	acing of the overall structure is the
Plate Connected Wood Tri	is construction and BCSI 1-0	3 Guide to Good Practice for Handli	ng, Installing & Bracing of	uon and bra of Metal Pla	acing, consult ANSI/TPL1 Na ate Connected Wood Trusses	from Truss Plate Institute, 583
D'Onofrio Drive, Madison	, WI 53719.		_ 0			· -

b

JOD 1105	ss Truss	lype C	Qty	Ply	LOT 0.0010 HONEYCUTT HILLS 199 SHE	LBY MEADOW LANE ANGIER, NC
24-5966-R01 R13	B Hip Girc	ler 1	1	2	Job Reference (optional)	# 50509

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu Jul 11 20:38:42 2024 Page 2 ID:R4JAOCFxLoAUFpwqRUTCbAz_Vw6-nW33MQ5WOrgp5NzMIqA8pfVZ8bd422hDbwUAeTyz1xh

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 3-4=-60, 5-9=-20 Concentrated Loads (lb)

Vert: 8=-1445(B) 10=-1451(B) 11=-1445(B) 13=-1445(B) 14=-1445(B) 15=-1445(B) 16=-1362(B) 17=-1790(B)



Job	Truss	Truss Type	Qty	Ply	LOT 0.0010 HONEYCUTT HILLS 199 SHEL	BY MEADOW LANE ANGIER, NO
24-5966-R01	R14	GABLE	1	1	Job Reference (optional)	# 50509
		Run ID:R4. 5.1	: 8.430 s Feb 1 AOCFxLoAU	2 2021 Prin FpwqRUT	nt: 8.430 s Feb 12 2021 MiTek Industries, Inc. CbAz_Vw6-nW33MQ5WOrgp5NzMlqA8	Thu Jul 11 20:38:42 2024 Page 1 3pfViPbn32E1DbwUAeTyz1xh

5-1-0

Scale = 1:21.8



	0-3- 0-3-	4						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.23 BC 0.33 WB 0.04 Matrix-SH	DEFL. in Vert(LL) -0.02 Vert(CT) -0.04 Horz(CT) 0.00	n (loc) 2 6-7 4 6-7) 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 31 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3		BR/ TOF BOT	ACING- ^o CHORD Stru end r CHORD Rigi Mi be	ctural wo verticals d ceiling Tek reco installec	ood shea directly mmends during t	thing dire applied or that Stat russ erec	ctly applied or 5-1-0 oc 10-0-0 oc bracing. illizers and required cru tion, in accordance with	purlins, except oss bracing n Stabilizer

Installation guide.

REACTIONS. (lb/size) 7=259/0-3-8 (min. 0-1-8), 5=185/0-1-8 (min. 0-1-8) Max Horz 7=86(LC 14) Max Uplift7=-17(LC 14), 5=-50(LC 14) Max Grav 7=351(LC 21), 5=265(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-7=-289/107

NOTES- (12)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

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- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

0-10-8

- between the bottom chord and any other members.
 9) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 10) Provide mechanical connection (by others) of truss to bearing plote at initiation of the structure of the

- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 5.

LOAD CASE(S) Standard

20<u>24</u> Summer Street SEAL 28147 NOINEE K. MORR 7/11/2024



Max Grav 5=351(LC 21), 4=265(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-5=-321/132

NOTES- (10)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 5=137/0-3-8 (min. 0-1-8), 4=27/0-1-8 (min. 0-1-8) Max Horz 5=33(LC 14) Max Uplift5=-14(LC 14), 4=-16(LC 14) Max Grav 5=184(LC 21), 4=33(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.
9) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.
LOAD CASE(S) Standard MORAS 1/2024 rd k 7/11/2024



- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.

- 12) I his truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide with the bottom chord and any other members.
 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 live with 11.
- LOAD CASE(S) Standard

MORPHS INTERNAL T1/202/ 'd and '5 funning and the NOINEE 7/11/2024



7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.

LOAD CASE(S) Standard





REACTIONS. All bearings 16-0-4.

(lb) - Max Horz 1=-93(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 9=451(LC 31), 10=410(LC 19), 8=407(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (9

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-2-2, Exterior(2R) 5-2-2 to 10-10-2, Exterior(2E) 10-10-2 to 15-8-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough
- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Provide adequate drainage to prevent water ponding.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9 except (jt=lb) 10=128, 8=124.





Max Uplift All uplift 100 lb or less at joint(s) 1, 9 except 10=-128(LC 12), 8=-124(LC 13)



REACTIONS. All bearings 13-8-4.

(lb) - Max Horz 1=-93(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 9=369(LC 3), 10=314(LC 23), 8=312(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (9

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-2-2, Exterior(2R) 5-2-2 to 8-6-2, Exterior(2E) 8-6-2 to 13-4-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Provide adequate drainage to prevent water ponding.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7 except (jt=lb) 10=118, 8=117.





Max Uplift All uplift 100 lb or less at joint(s) 1, 7 except 10=-118(LC 12), 8=-117(LC 13)



REACTIONS. All bearings 11-4-4.

(lb) - Max Horz 1=-93(LC 10)

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 9=315(LC 22), 10=308(LC 23), 8=308(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (9

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-2-2, Exterior(2R) 5-2-2 to 6-2-2, Exterior(2E) 6-2-2 to 11-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Provide adequate drainage to prevent water ponding.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7 except (jt=lb) 10=143, 8=142.





Max Uplift All uplift 100 lb or less at joint(s) 1, 7 except 10=-143(LC 12), 8=-142(LC 13)



- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 1, 102 lb uplift at joint 5, 185 lb uplift at joint 8 and 184 lb uplift at joint 6.

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LOAD CASE(S) Standard
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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 1 and 33 lb uplift at joint 3.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 1 and 22 lb uplift at joint 3.

LOAD CASE(S) Standard





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 1 and 12 lb uplift at joint 3.

LOAD CASE(S) Standard

