

Client:

Tapia Design and Build

Project:

Address: 658 James Norris Road, Angier NC Date: 12/16/2024 Input by:

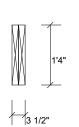
Johnnie Baggett Job Name: 658 James Norris Road

Project #: B1124-6245 evel: Level

Kerto-S LVL 2-Ply - PASSED 1.750" X 16.000" FB1

1 2 SPF End Grain 0-3-0 . 1 SPF End Grain 0-3-0 13'3'

13'3'



Page 1 of 3

Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Snow Wind Brg Direction Live Dead Const 2776 1010 Vertical n 0 0 1 2 Vertical 2776 1010 0 0 0

Analysis Results

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	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	11879 ft-lb	6'7 1/2"	34565 ft-lb	0.344 (34%)	D+L	L
	Unbraced	11879 ft-lb	6'7 1/2"	11901 ft-lb	0.998 (100%)	D+L	L
	Shear	2898 lb	1'7"	11947 lb	0.243 (24%)	D+L	L
	LL Defl inch	0.127 (L/1218)	6'7 1/2"	0.322 (L/480)	0.394 (39%)	L	L
	TL Defl inch	0.173 (L/893)	6'7 1/2"	0.430 (L/360)	0.403 (40%)	D+L	L

Bearings

Bearing Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 3.000" End Grain	Vert	43%	1010 / 2776	3786	L	D+L
2 - SPF 3.000" End Grain	Vert	43%	1010 / 2776	3786	L	D+L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 10'2 7/16" o.c.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	140 PLF	419 PLF	0 PLF	0 PLF	0 PLF	F01

12 PLF Self Weight

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

This design is valid until 6/28/2026



Client:

Tapia Design and Build

Project: Address:

658 James Norris Road, Angier NC

Date: 12/16/2024 Input by:

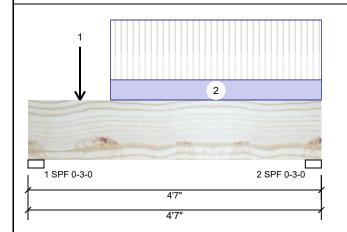
Johnnie Baggett Job Name: 658 James Norris Road

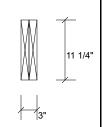
Project #: B1124-6245

2.000" X 12.000" **BBO SP #2**

2-Ply - PASSED

Level: Level





Const

Ld. Comb.

D+L

D+L

Wind

Total Ld. Case

960 L

485 L

Page 2 of 3

Member Information

Type: Plies: Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temp <= 100°F Temperature:

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Brg Direction Snow

Dir.

Vert

Vert

Bearings Bearing Length

1-SPF 3.000"

2 - SPF 3.000"

1	Vertical	720	240	0	0	0
2	Vertical	363	363 122		0	0

Cap. React D/L lb

240 / 720

122 / 363

25%

13%

Analysis	Results
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Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	633 ft-lb	1'7 5/16"	3955 ft-lb	0.160 (16%)	D+L	L
Unbraced	633 ft-lb	1'7 5/16"	3806 ft-lb	0.166 (17%)	D+L	L
Shear	597 lb	1'2 1/4"	3938 lb	0.152 (15%)	D+L	L
LL Defl inch	0.003 (L/16539)	2'1 15/16"	0.105 (L/480)	0.029 (3%)	L	L
TL Defl inch	0.004 (L/12393)	2'1 15/16"	0.140 (L/360)	0.029 (3%)	D+L	L

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- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Point	0-9-12		Тор	227 lb	681 lb	0 lb	0 lb	0 lb	F08-GR	
2	Part. Uniform	1-3-8 to 4-7-0		Тор	41 PLF	122 PLF	0 PLF	0 PLF	0 PLF	F09	

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Manufacturer Info



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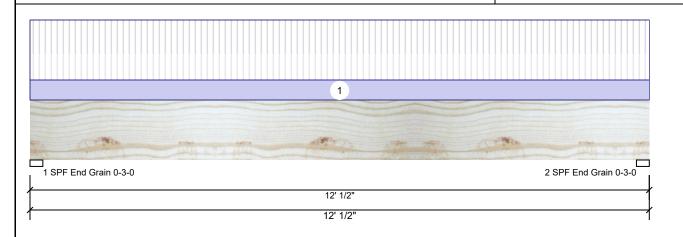
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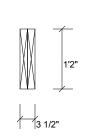
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Project #: B1124-6245

1.750" X 14.000" 2-Ply - PASSED **B1** Kerto-S LVL

Level: Level





Page 3 of 3

Member Information Type: Plies: Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Snow Wind Brg Direction Live Dead Const 5268 1824 Vertical n 0 0 1 2 Vertical 5268 1824 0 0 0

Analysis Results

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Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	20040 ft-lb	6' 1/4"	26999 ft-lb	0.742 (74%)	D+L	L
Unbraced	20040 ft-lb	6' 1/4"	20087 ft-lb	0.998 (100%)	D+L	L
Shear	5436 lb	10'7 1/2"	10453 lb	0.520 (52%)	D+L	L
LL Defl inch	0.263 (L/533)	6' 1/4"	0.292 (L/480)	0.901 (90%)	L	L
TL Defl inch	0.354 (L/396)	6' 1/4"	0.389 (L/360)	0.910 (91%)	D+L	L

Bearings

Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.000"	Vert	80%	1824 / 5268	7092	L	D+L
2 - SPF End Grain	3.000"	Vert	80%	1824 / 5268	7092	L	D+L

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- 5 Top must be laterally braced at a maximum of 4'8 5/16" o.c.
- 6 Bottom must be laterally braced at end bearings.

Self Weight

7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	292 PLF	875 PLF	0 PLF	0 PLF	0 PLF	F04

Notes

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11 PLF

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