

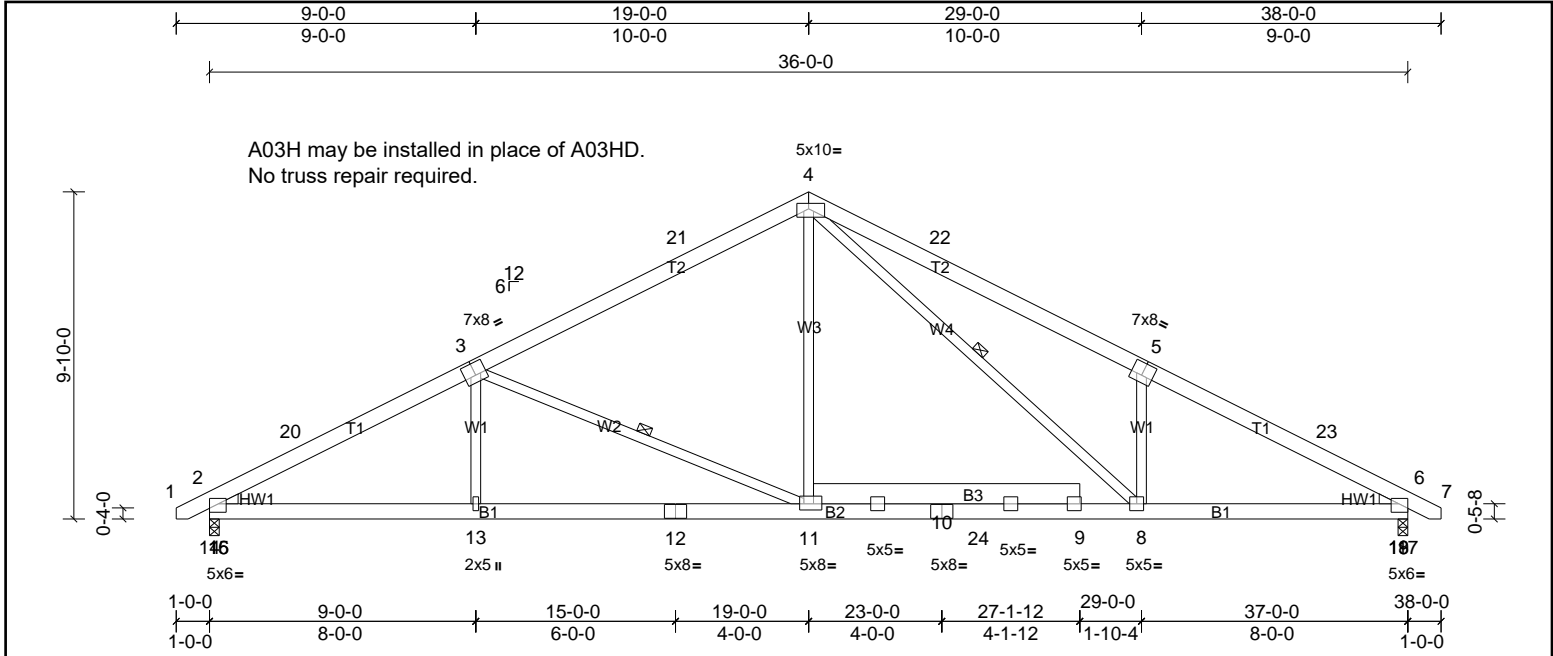
Job 72416327REP1	Truss A03H	Truss Type Truss	Qty 2	Ply 1	SD - COLEMAN BEH RF (NO TRAY) Job Reference (optional)
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Chawn Duty

Run: 8.51 S Oct 22 2021 Print: 8.810 S Sep 13 2024 MiTek Industries, Inc. Thu Nov 14 13:32:00

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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.70	Vert(LL)	-0.11	8-11	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.69	Vert(CT)	-0.22	11-13	>999	180		
BCLL	0.0*	Rep Stress Incr	NO	WB	0.49	Horz(CT)	0.07	6	n/a	n/a		
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MSH							Weight: 270 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x6 SP No.1 *Except* T1:2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied or 3-10-9 oc purlins.
BOT CHORD 2x6 SP No.2 *Except* B3:2x8 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt
WEDGE Left: 2x4 SP No.2 Right: 2x4 SP No.2	
REACTIONS (lb/size) 2=1489/0-3-8, (min. 0-1-12), 6=1489/0-3-8, (min. 0-1-12) Max Horiz 2=128 (LC 10) Max Uplift 2=-71 (LC 10), 6=-71 (LC 11)	

FORCES	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	2-20=-2503/123, 3-20=-2319/151, 3-21=-1760/159, 4-21=-1585/183, 4-22=-2335/292, 5-22=-2509/268, 5-23=-2305/154, 6-23=-2489/127
BOT CHORD	2-13=-168/2148, 12-13=-147/2150, 11-12=-147/2150, 10-11=0/1446, 10-24=0/1452, 9-24=0/1451, 8-9=0/1451, 6-8=-39/2135
WEBS	4-11=0/678, 5-8=-558/275, 4-8=-252/980, 3-13=0/312, 3-11=-812/202

- NOTES (7)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) -0-9-14 to 2-9-6, Interior (1) 2-9-6 to 14-4-13, Exterior (2) 14-4-13 to 21-7-3, Interior (1) 21-7-3 to 33-2-10, Exterior (2) 33-2-10 to 36-9-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 71 lb uplift at joint 2 and 71 lb uplift at joint 6.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - This repair has been prepared based on information and use conditions supplied by client. Designer has made a good faith effort to outline damage and repair conditions as reported by client. When actual field conditions do not approximate those indicated on this drawing, client shall immediately inform the engineer and refrain from applying the repair.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



Job 72416327REP1	Truss A03HD	Truss Type Truss	Qty 1	Ply 1	SD - COLEMAN BEH RF (NO TRAY) Job Reference (optional)
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Chawn Duty

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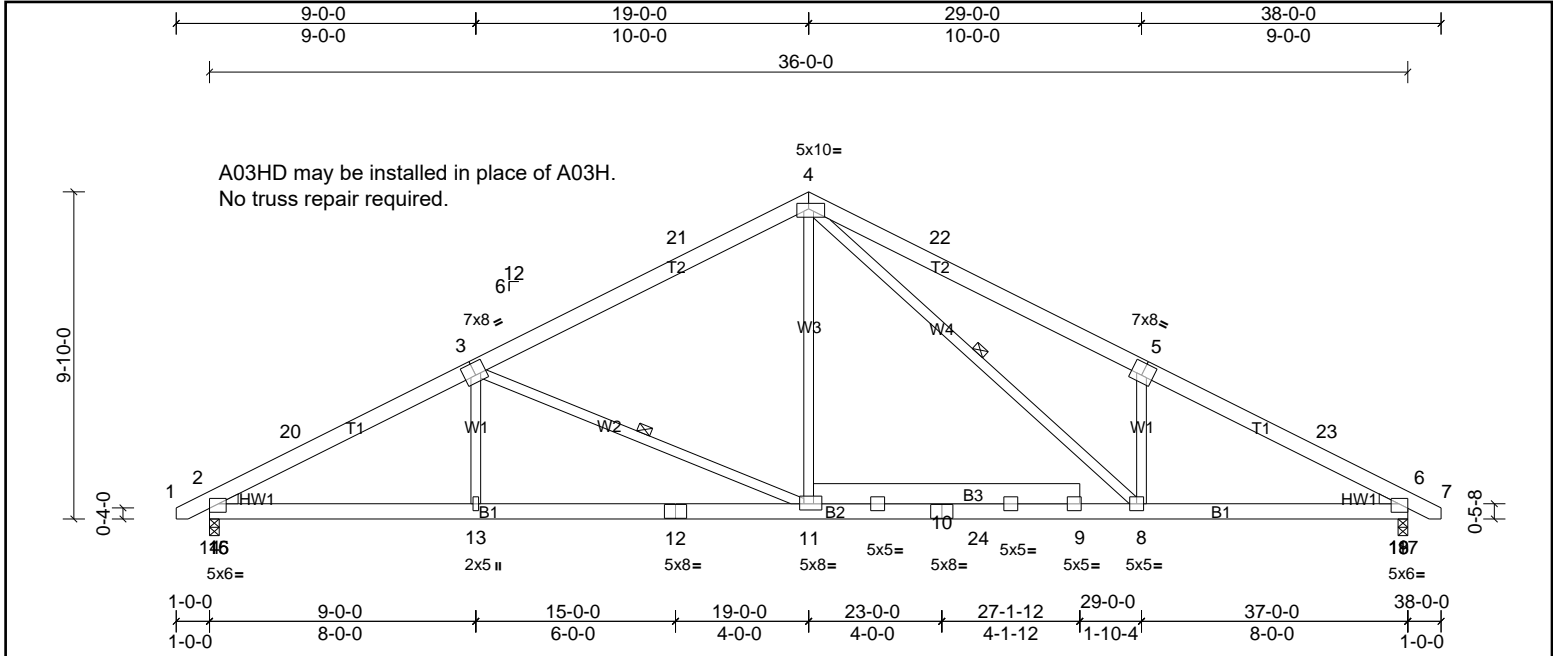


Plate Offsets (X, Y): [2:Edge,0-1-7], [3:0-4-0,0-4-8], [4:0-4-4,0-2-0], [5:0-4-0,0-4-8], [6:Edge,0-1-7], [11:0-1-8,0-2-4]

Loading	(psf)	Spacing	2-3-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.85	Vert(LL)	-0.12	8-11	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.78	Vert(CT)	-0.25	11-13	>999	180		
BCLL	0.0*	Rep Stress Incr	NO	WB	0.55	Horz(CT)	0.08	6	n/a	n/a		
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MSH							Weight: 270 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	2x6 SP No.1 *Except* T1:2x6 SP No.2	TOP CHORD	2-0-0 oc purlins (3-3-12 max.) (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD	2x6 SP No.2 *Except* B3:2x8 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3	WEBS	1 Row at midpt
WEDGE	Left: 2x4 SP No.2 Right: 2x4 SP No.2		4-8, 3-11
REACTIONS			
	(lb/size) 2=1675/0-3-8, (min. 0-2-0), 6=1675/0-3-8, (min. 0-2-0)		
	Max Horiz 2=145 (LC 10)		
	Max Uplift 2=80 (LC 10), 6=80 (LC 11)		
FORCES			
	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.		
TOP CHORD	2-20=-2816/139, 3-20=-2609/170, 3-21=-1980/179, 4-21=-1784/206, 4-22=-2626/329, 5-22=-2823/301, 5-23=-2593/173, 6-23=-2800/142		
BOT CHORD	2-13=-189/2416, 12-13=-165/2419, 11-12=-165/2419, 10-11=0/1627, 10-24=0/1634, 9-24=0/1632, 8-9=0/1633, 6-8=-44/2402		
WEBS	4-11=0/763, 4-8=-283/1103, 5-8=-627/309, 3-13=0/351, 3-11=-914/227		

- NOTES (8)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) -0-9-14 to 2-9-6, Interior (1) 2-9-6 to 14-4-13, Exterior (2) 14-4-13 to 21-7-3, Interior (1) 21-7-3 to 33-2-10, Exterior (2) 33-2-10 to 36-9-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 2 and 80 lb uplift at joint 6.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - This repair has been prepared based on information and use conditions supplied by client. Designer has made a good faith effort to outline damage and repair conditions as reported by client. When actual field conditions do not approximate those indicated on this drawing, client shall immediately inform the engineer and refrain from applying the repair.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.

