ABBREVIATIONS	INDEX	
ADDKEVALONS ACCORDINCE AC ARCONTINUE BE BEACH BEACH BE BEACH	MODEL 'GALEN' 0 TITLE SHEET / COVER SHEET I.A.5 BUILDING SECTIONS I.A FRONT ELEVATION 'A' I.I.A.5 BUILDING SECTIONS I.A FRONT ELEVATION 'A' I.I.A.5 BUILDING SECTIONS I.A ROOF PLAN 'A' G.5 IST FLOOR UTILITY PLAN 2.A SIDE AND REAR ELEVATIONS 'A' 7 2ND FLOOR UTILITY PLAN 3.MS.A MONOLITHIC SLAB PLAN 'A' 41 Architectural Sheets 3.MS.A IST FLOOR PLAN 'A' 41 Architectural Sheets 3.MS.B MONOLITHIC SLAB PLAN 'A' 41 Architectural Sheets 3.MS.B MONOLITHIC SLAB PLAN 'A' 41 Architectural Sheets 3.MS.B MONOLITHIC SLAB PLAN 'B' 14 Architectural Sheets 3.MS.B MONOLITHIC SLAB PLAN 'B' 15 16 3.MS.B MONOLITHIC SLAB PLAN 'B' 17 Architectural Sheets 3.MS.B MONOLITHIC SLAB PLAN 'B' 17 17 3.MS.D STEM WALL PLAN 'B' 18 19 3.MS.D STEM WALL PLAN 'B' 19 10 3.GV.C STEM WALL PLAN 'C' 1	EXPRESS 40' St MODEL 'G Mason Ridg Lot 38 Spring Lake
ALL CONSTRUCTION TO COMPLY WITH LOCAL CODES AND ORDINANCES CURRENTLY IN USE WITH THE LOCAL JURISDICTION. APPLICABLE CODES; FOLLOW ALL APPLICABLE STATE AND LOCAL CODES. 2016 NORTH CAROLINA STATE SUPPLEMENTS AND AMENDMENTS	3 MS E MONOLITHIC SLAB PLAN 'E' 3 SW E STEM WALL PLAN 'E' 4 E IST FLOOR PLAN 'E' 5 E 2ND FLOOR PLAN 'E'	
CONTRACTOR AND BUILDER SHALL REVIEW ENTIRE PLAN TO VERIFY CONFORMALCE WITH ALL CURRENT APPLICABLE CODES IN EFFECT AT TIME OF CONSTRUCTION, BY USING THESE DRAWINGS FOR CONSTRUCTION IT IS UNDERSTOOD THAT CONFORMANCE WITH ALL APPLICABLE CODES IS THE RESPONSIBILITY OF THE BUILDER AND CONTRACTOR. PRODUCT: SINGLE FAMILY RESIDENCE	I FFRONT ELEVATION 'F'I.I FROOF PLAN 'F'2 FSIDE AND REAR ELEVATIONS 'F'3 MS FMONOLITHIC SLAB PLAN 'F'3 SW FSTEM WALL PLAN 'F'4 FIST FLOOR PLAN 'F'5 F2ND FLOOR PLAN 'F'	CONSULTANTS:
OCCUPANCY CLASSIFICATION RESIDENTIAL R-3 CONSTRUCTION TYPE: TYPE VB	ALL CONSULTANT DRAWINGS ACCOMPANYING THESE ARCHITECTURAL DRAWINGS HAVE NOT BEEN PREPARED BY OR UNDER THE DIRECTION OF GMD DESIGN GROUP, INC. GMD DESIGN GROUP INC. THEREFORE ASSUMES NO LIABILITY FOR THE COMPLETENESS OR CORRECTNESS OF THESE DRAWINGS.	

GENERAL NOTES DESIGNER NORTH CAROLINA:

THESE DOCUMENTS ARE THE PROPERTY OF THE DESIGNER AND SHALL NOT BE COPIED DUPLICATED, ALTERED, MODIFIED OR REVISED IN ANY WAY WITHOUT THE EXPRESSED WRITTEN APPROVAL OF THE DESIGNER. WRITEN APPROVAL OF THE DESIGNER.

CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE AND ALL INCONSISTENCES SHALL BE BROUGHT TO THE ATTENTION OF THE DEVELOPER AND THE DESIGNER BEFORE PROCEEDING WITH WORK.

ANY ERRORS OR OMISSIONS FOUND IN THESE DRAWINGS SHALL BE BROUGHT TO DEVELOPERS AND DESIGNERS ATTENTION IMMEDIATELY

DO NOT SCALE DRAWINGS, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS.

ALL DIMENSIONS ARE TO FACE OF STUD OR TO FACE OF FRAMING UNLESS

ALL TRUSS DRAWINGS TO BE REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO ISSUANCE OF BUILDING PERMIT.

ALL OR EQUAL SUBSTITUTIONS MUST BE SUBMITTED TO AND APPROVED BY CITY BUILDING OFFICIAL PRIOR TO INSTALLATION.

ALL ANGLED PARTITIONS ARE 45 DEGREES UNLESS OTHERWISE NOTED. PROVIDE FIREBLOCKING. (PER LOCAL CODES.)

ALL ELECTRICAL AND MECHANICAL EQUIPMENT AND METERS ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS, CONTRACTOR TO VERIF

ELASTOMERIC SHEET WATERPROOFING: FURNISH AND INSTALL ALL WATERPROOFING

COMPLETE, A 40 MIL, SELF-ADHENING MEMBRANE OF RUBBERIZED ASPHALT INTEGRALLY BONDED TO POLYETHYLENE SHEETING, OR EQUAL. INSTALL FER MANUFACTURES AND TRADE ASSOCIATIONS PRINTED INSTALLIATION INSTRUCTIONS. 6" MINIMUM LAP AT ALL ADJACENT WALL SURFACES.

to the best of the designer's knowledge these documents are in CONFORMANCE WITH THE REQUIREMENTS OF THE BUILDING AUTHORITIES HAVING JURISDICTION OVER THIS TYPE OF CONSTRUCTION AND OCCUPANCY.

SHOP DRAWING REVIEW AND DISTRIBUSTION, ALONG WITH PRODUCT SUBMITTALS, REQUESTED IN THE CONSTRUCTION DOCUMENTS, SHALL BE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR, UNLESS DIRECTED OTHERWISE INDER A SEPARATE AGREEMENT.

DEVIATIONS FROM THESE DOCUMENTS IN THE CONSTRUCTION PHASE SHALL BE REVIEWED BY THE DESIGNER AND THE OWNER REVIEW TO THE START OF HORK IN QUESTION. ANY DEVIATIONS RROM THESE DOCUMENTS WITHOUT PRIOR REVIEW, SHALL BE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR.

THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL WORK AND MATERIALS REPRESENTED ON THESE DOCUMENTS INCLUDING THE WORK AND MATERIALS FURNISHED BY SUBCONTRACTORS AND VENDORS. THE BUILDER SHALL FURNISH ANY AND ALL REPORTS RECEIVED FROM THE GEOTECHNICAL ENGINEER (SOLES REPORT), ON THE STUDY OF THE FROPOSED SITE, TO THE DESIGNER, STRUCTURAL ENGINEER, AND GENERAL CONTRACTOR. IN THE EVENT THE GEOTECHNICAL REPORTS DO NOT EXIST, THE SOLES CONDITION SHALL DE ASSIMED TO DE A MINIMUM DESIGN SOLL PRESSURE STATED BY THE STRUCTURAL ENGINEER OF RECORD FOR THE REPORE OF STRUCTURAL DESIGN. GENERAL CONTRACTOR SHALL ASSURE THE SOIL CONDITIONS MEET OR EXCEED HE CRITERIA

ALL WORK PERFORMED BY THE GENERAL CONTRACTOR SHALL COMPLY AND CONFORM WITH LOCAL AND STATE BUILDING CODES, ORDINANCES AND REGULATIONS, ALCONG WITH ALL OTHER AUTHORITIES HAVING JURISDICTION. THE GENERAL CONTRCATOR IS RESPONSIBLE TO BE AWARE OF THESE REQUIREMENTS

AND GOVERNING REGULATIONS. PROVIDE AN APPROVED WASHER DRAIN PAN AT SECOND FLOOR ONLY

THAT DRAINS TO EXTERIOR

MINDON SUPPLIER TO VERIFY AT LEAST ONE MINDOW IN ALL BEDROOMS TO HAVE A CLEAR OPENABLE AREA OF 4.0 SQ FT. THE MINIMM NET CLEAR OPENNG HEIGHT SHALL BE 22" AND THE MINIMM NET CLEAR OPENNG WIDTH SHALL BE 20". GLAZING TOTAL AREA OF NOT LESS THAN 5.0 SQ FT IN THE CASE OF A GROUND WINDOW AND NOT LESS THAN 5.1 SQ FT IN THE THAN 30 50 FT IN THE CASE OF A GROUND MINUTA AND NOT LESS THAN 5.1 50 FT CASE OF AN UPPER STORY WINDOW, (PER NORC SECTION R310.1.) ALL HANDRAIL BALLISTERS TO BE SPACED SUCH THAT A 4" SPIERE CANNOT PASS BETWEEN BALLISTERS, (PER LOCAL CODES.)

PROVIDE STAIR HANDRAILS AND GUARDRAILS PER LOCAL CODES

THE SCOPE OF THIS SET OF PLANS IS TO PROVIDE A "BUILDER'S SET" OF CONSTRUCTION DOCUMENTS AND GENERAL NOTES HEREINAFTER REFERRED TO AS "PLANS". THIS SET OF PLANS IS SUFFICIENT TO OBTAIN A BUILDING PERMIT, HOVEVER, ALL MATERIALS AND METHODS OF CONSTRUCTION NECESSARY TO COMPLETE THE PROJECT ARE NOT NECESSARILY DESCRIBED. THE PLANS DELINEATE AND DESCRIBE ONLY LOCATIONS, DIMENSIONS, TYPES OF MATERIALS, AND GENERAL METHODS OF ASSPHULING OR FASTENING. THEY ARE NOT INITENDED TO SPECIFY PARTICULAR PRODUCTS OR OTHER METHODS OF ANY SPECIFIC MATERIALS, PRODUCT OR METHOD. THE INDENSTRUCTION OF THE PLANS DECLIFY OR RECOVERS ALL CONTRACTOR THOROUGHY KNOW EDGEARLE HITH THE APPLICABLE BUILDING CODES ALL ONTRACTOR THOROUGHY KNOW EDGEARLE HITH THE APPLICABLE BUILDING CODES

BUILDER SET:

CONSTRUCTION REQUIREMENTS AND QUALITY: PROVIDE WORK OF THE SPECIFIC QUALITY; CONSTRUCTION REQUIREMENTS AND QUALITY: PROVIDE WORK OF THE SPECIFIC QUALITY: HEREE QUALITY LEVEL IS NOT INDICATED, PROVIDE WORK OF QUALITY CUSTONARY IN SIMILAR TYPES OF WORK, MEREE THE PLANS AND SPECIFICATIONS, CODES, LANS, REGULATIONS, MANUFACTURER'S RECOMPREDIATIONS OR INDUSTRY STANDARDS REQUIRE WORK OF HIGHER QUALITY OR PERFORMANCE, PROVIDE WORK COMPLYING WITH THOSE REQUIRE WORK OF HIGHER QUALITY OR PERFORMANCE, PROVIDE WORK COMPLYING WITH THOSE REQUIRE WORKS OF HIGHER QUALITY OR MORE QUALITY FROVIDES/GONS OF THOSE REQUIRE/MENTS AND QUALITY. MITH THE MOST STRINGENT REQUIRE/MENT, WHERE REQUIRE/MENTS AND EDITEMENT AND ADALTY WHERE THO OR MORE QUALITY FROVIDE/MORK COMPLYING WITH THOSE REQUIRE/MENTS AND LOW WHERE IT IS UNCERTAIN WHICH REQUIRE/MENTS ARE DIFFERENT BUT APPARENTLY EQUAL, AND WHERE IT IS UNCERTAIN WHICH REQUIRE/MENT IS MOST STINIGENT, OBTAIN CLARIFICATION FROM THE GMD DESIGN GROUP BEFORE PROCEEDING.

AND METHODS OF CONSTRUCTION SPECIFIC TO THIS PRODUCT TYPE AND TYPE OF CONSTRUCTION.

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AREA CALCULATIONS:	Express Homes
MODEL'GALEN'SQUAREFOOTAGESAREAELEV 'D'/E'/F'Ist FLOOR982 SF2nd FLOOR1358 SFTOTAL LIVING2340 SFGARAGE416 SF	PROJECT NO: GMD/7049 SHEET TITLE: TITLE SHEET PRINT DATE: March 30, 2023
PORCH 87 SF	SHEET NO:

3 MASONRY FULL STONE AS SELECTED BY DEVELOPER. HEIGHT AS NOTED. 4 8" SOLDIER COURSE. 5 ROWLOCK COURSE 6 N/A TYPICALS: T CORROSION RESISTANT SCREEN LOUVERED VENTS, SIZE AS NOTED. B CODE APPROVED TERMINATION CHIMNEY CAP. CORROSION RESISTANT ROOF TO WALL FLASHING. CODE COMPLIANT FLASHING PER NCRC R905.2.8.3 0 STANDING SEAM METAL ROOF, INSTALL PER MANUFCATURER'S WRITTEN INSTRUCTIONS. II DECORATIVE WROUGHT IRON, SEE DETAILS. SIDING: [2] VINTL SHAKE SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT SHAKE SIDING PER DEVELOPER WI IX4 CORNER TRIM BOARD.) 13 VINYL LAP SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT LAP SIDING PER DEVELOPER W IX4 CORNER TRIM BOARD.) 4 VINYL WAVY SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT WAVY SIDING PER DEVELOPER W IX4 CORNER TRIM BOARD.) 15 VINYL BOARD AND BATT SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT PANEL SIDING W IX3 BATTS AT 12" O.C. PER DEVELOPER W IX4 CORNER TRIM BOARD.) IDER CENTRI FARL SUMME VIAS BALLS AT 12 CC. HER DEVELOPER WIN4 CORRECTION STREAM OF THE CONTRACT OF THE ASS AND THE STREAM OF THE STREAM OF THE STREAM OF THE ASS AND THE ASS A ALL INDOM'S INDOF THE INTER SOUTH AND AN ADVECTING THAT AND AN ADVECTING THE FINISH FLOOR AND INFOSE OPENING IS GREATER THAN T2' ABOVE THE OUTSIDE WALKING SURFACE MUST HAVE INDOM OPENING LIMITING DEVICES COMPLYING WITH THE NCRC SECTION R312.2.1 AND R312.2.2.



NOTES:

GARAGE DOORS: AS SELECTED BY DEVELOPER, RAISED PANEL AS SHOWN. ALL EXTERIOR MATERIALS TO BE INSTALLED PER MANUFACTURER'S WRITTEN INSTRUCTIONS.

ADHERED STONE VENEER AS SELECTED BY DEVELOPER. HEIGHT AS NOTED.

MASONRY FULL BRICK AS SELECTED BY DEVELOPER. HEIGHT AS NOTED.

PROTECTION AGAINST DECAY: (ALL PORTIONS OF A PORCH, SCREEN PORCH OR DECK FROM THE BOTTOM OF THE HEADER DOWN, INCLUDING POST, RAILS, PICKETS, STEPS AND FLOOR STRUCTURE.)

ROOFING: PITCHED SHINGLES PER DEVELOPER.
 WINDOWS: MANUFACTURER PER DEVELOPER. DIVIDED LITES AS SHOWN ON THE EXTERIOR ELEVATIONS

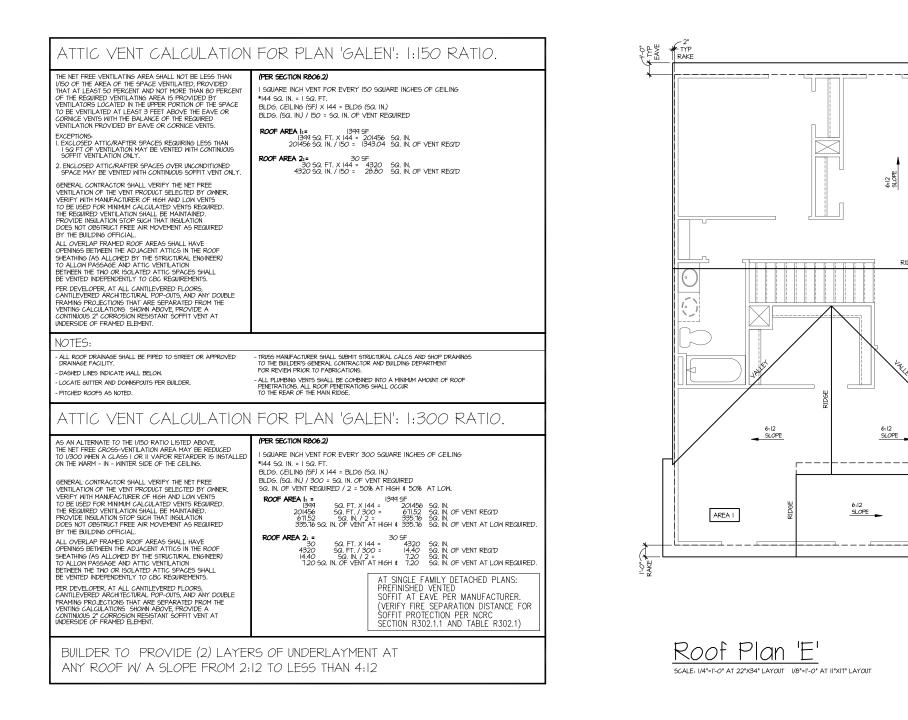
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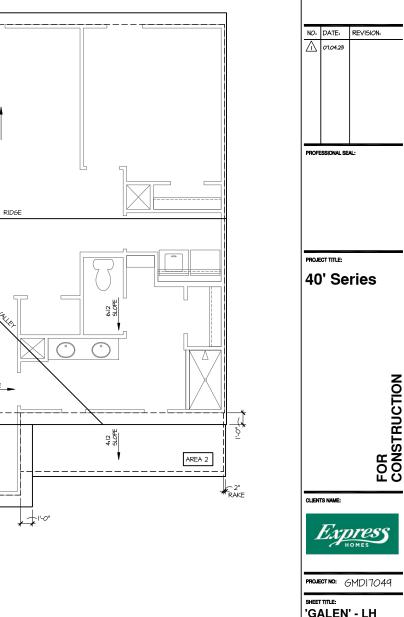
ENTRY DOOR: AS SELECTED BY DEVELOPER.

KEY NOTES: MASONRY:

GRADE CONDITIONS MAY VARY FOR INDIVIDUAL SITE FROM THAT SHOWN, BUILDER SHALL VERIFY AND COORDINATE PER ACTUAL SITE CONDITIONS.
 NINDON HEAD HEIGHTS; IST FLOOR = 6-80° UNO, ON ELEVATIONS, 2ND FLOOR = T-0° UNO, ON ELEVATIONS,

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NOTES:

- GRADE CONDITIONS MAY VARY FOR INDIVIDUAL SITE FROM THAT SHOWN. BUILDER SHALL VERIFY AND COORDINATE PER ACTUAL SITE CONDITIONS.
- MIROW HEAD HEIGHTS: IST FLOOR = 6'-6' UNO, ON ELEVATIONS. 2MD FLOOR = 1'-0' UNO, ON ELEVATIONS.
- ROOFING: PITCHED SHINGLES PER DEVELOPER.
- WINDOWS: MANUFACTURER PER DEVELOPER. DIVIDED LITES AS SHOWN ON THE EXTERIOR ELEVATION
- ENTRY DOOR: AS SELECTED BY DEVELOPER.
- GARAGE DOORS: AS SELECTED BY DEVELOPER, RAISED PANEL AS SHOWN.
- ALL EXTERIOR MATERIALS TO BE INSTALLED PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
 PROTECTION AGAINST DECAY. (ALL PORTIONS OF A PORCH, SCREEN PORCH OR DECK FROM THE BOTTOM OF THE HEADER DOWN, INCLIDING POST, RAILS, PICKETS, STEPS AND FLOOR STRUCTURE.)
- INSULATION. PER TABLE NI/02.1.2. EXTERIOR WALLS: R-15 BATTS MINIMM. VERIFY CELLING WITH ATTIC ABOVE: R-30 BATTS MINIMM. VERIFY FLOOR OVER GARAGE: R-14 BATTS MINIMM. VERIFY ATTIC KNEPKALL: R-14 BATTS MINIMM. VERIFY CRAWL SPACE FLOORING: R-14 BATTS MINIMM. VERIFY
KEY NOTES:
MASONRY:
I ADHERED STONE VENEER AS SELECTED BY DEVELOPER. HEIGHT AS NOTED.
2 MASONRY FULL BRICK AS SELECTED BY DEVELOPER. HEIGHT AS NOTED.
3 MASONRY FULL STONE AS SELECTED BY DEVELOPER. HEIGHT AS NOTED.
4 8" SOLDIER COURSE.
5 ROWLOCK COURSE
6 N/A
TYPICALS:
T CORROSION RESISTANT SCREEN LOUVERED VENTS, SIZE AS NOTED.
8 CODE APPROVED TERMINATION CHIMNEY CAP.
CORROSION RESISTANT ROOF TO WALL FLASHING, CODE COMPLIANT FLASHING PER NCRC R4052.8.3

O STANDING SEAM METAL ROOF, INSTALL PER MANUFCATURER'S WRITTEN INSTRUCTIONS.

SIDING: [2] VIINTL SHAKE SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIDER CEMENT SHAKE SIDING PER DEVELOPER W/ IX4 CORNER TRIM BOARD.)

 III VINTL LAP SIDING PER DEVELOPER WITH VINTL CORNER TRIM PER DEVELOPER.

 (AT SPECIFIED LOCATIONS:

 FIBER CEMENT LAP SIDING PER DEVELOPER WI X4 CORNER TRIM BOARD.)

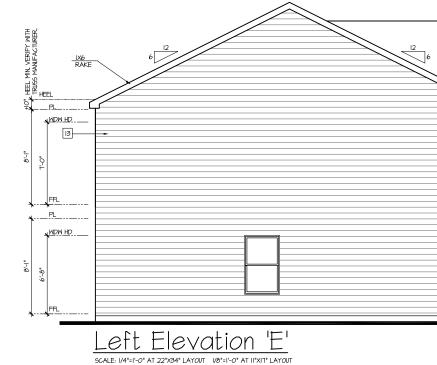
☑ VINTL WAVY SIDING PER DEVELOPER WITH VINTL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT WAVY SIDING PER DEVELOPER W/IX4 CORNER TRIM BOARD.)

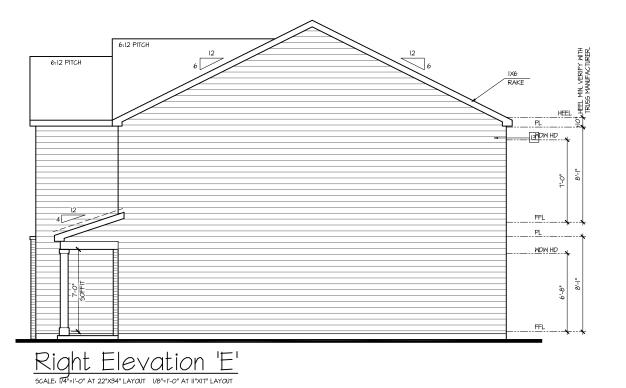
TYPON SHITTERS, TYPE AS SHOWN, SIZE AS NOTED.

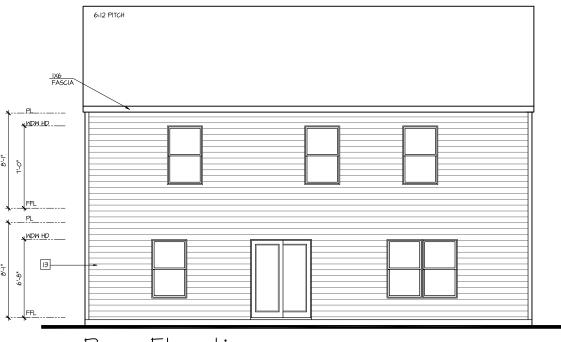
BUINT LOAD AND ANT SIDING FER DEVELOTER VI IN CORRECTION DONADO
 SUNT ANADO AND ANT SIDING PER DEVELOPER WITH VINTL CORRECTION PER DEVELOPER.
 AT SPECIFIED LOCATIONS.
 FIERC ROMENT PAREL SIDING WI XIS BATTS AT 12" O.C. PER DEVELOPER WI XI4 CORRECTION BOARD.
 (AT SPECIFIED LOCATIONS.
 K FIERC CEMENT TRIM OR EQUAL, UND, SIZE AS NOTED
 K FIERC CEMENT TRIM OR EQUAL, UND, SIZE AS NOTED

DECORATIVE WROUGHT IRON. SEE DETAILS.

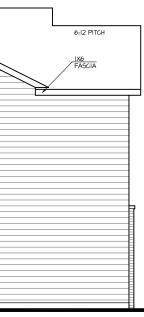
ALL WIRDOWS HADSE OFFENING IS LESS THAN 24' ABOVE THE FINISH FLOOR AND WHOSE OFFENING IS GREATER THA 12' ABOVE THE OUTSIDE WALKING SURFACE MOST HAVE WIRDOW OFFENING LIMITING EDVICES COMPLYING WITH THE NCRC SECTION R3I2.21 AND R3I2.22.



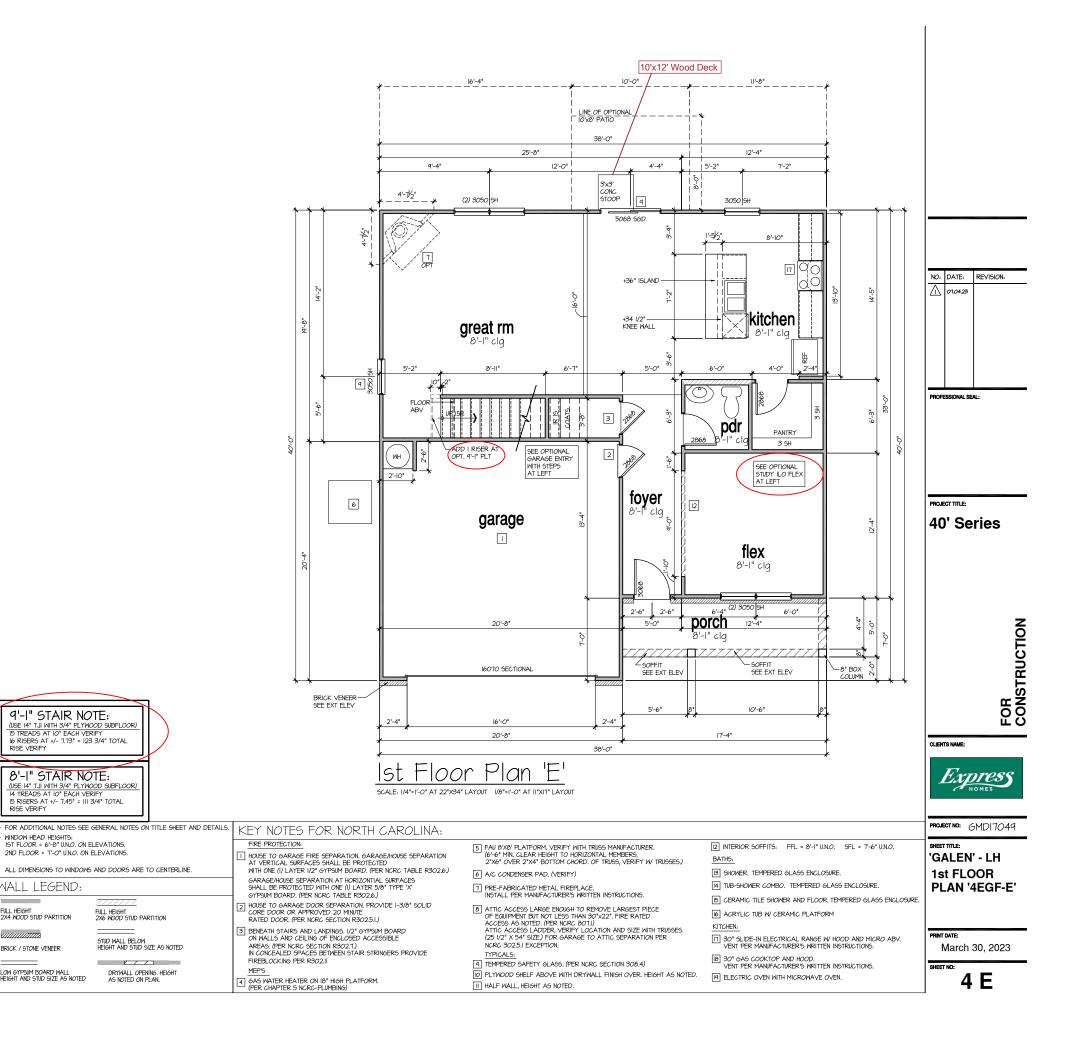


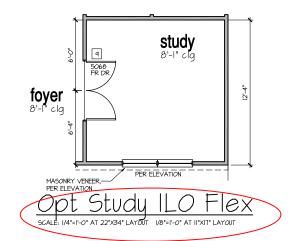


Rear Elevation SCALE: I/4"=I'-0" AT 22"X34" LAYOUT I/8"=I'-0" AT II"XIT" LAYOUT









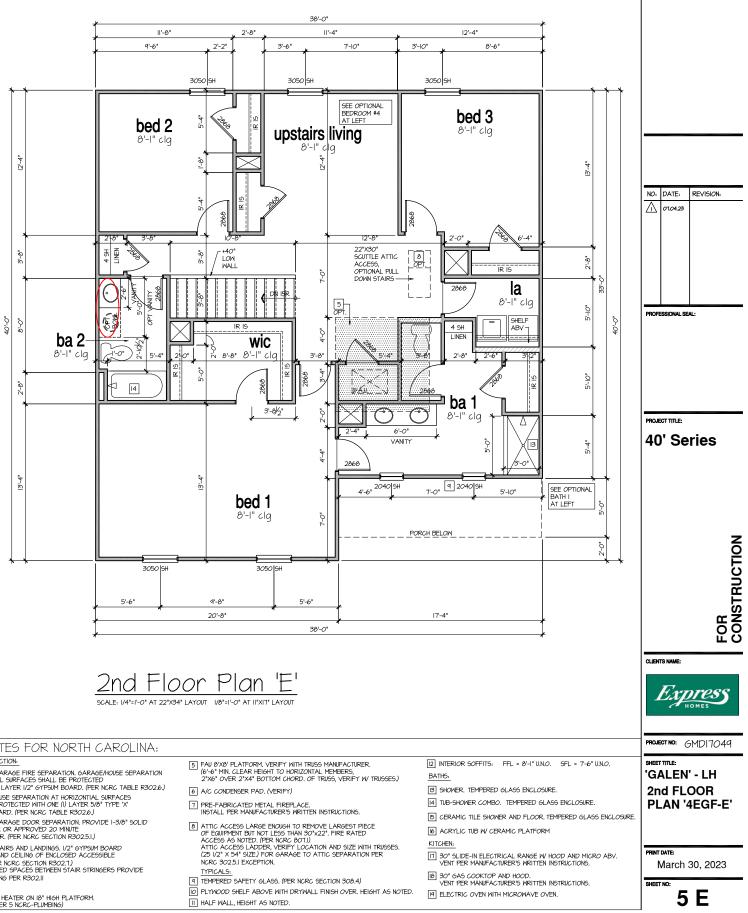
WALL LEGEND:

- --

BRICK / STONE VENEER

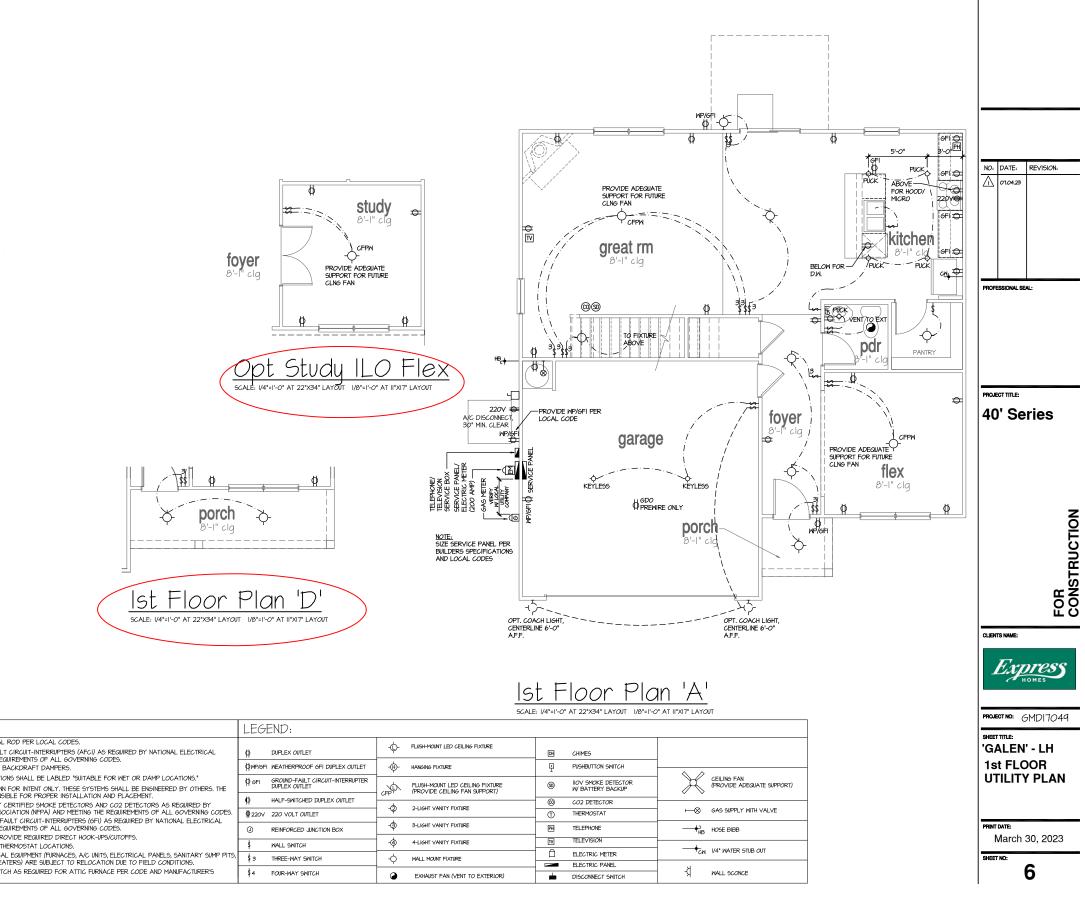
Low Gypsum Board Wall Height and stud Size as Noted

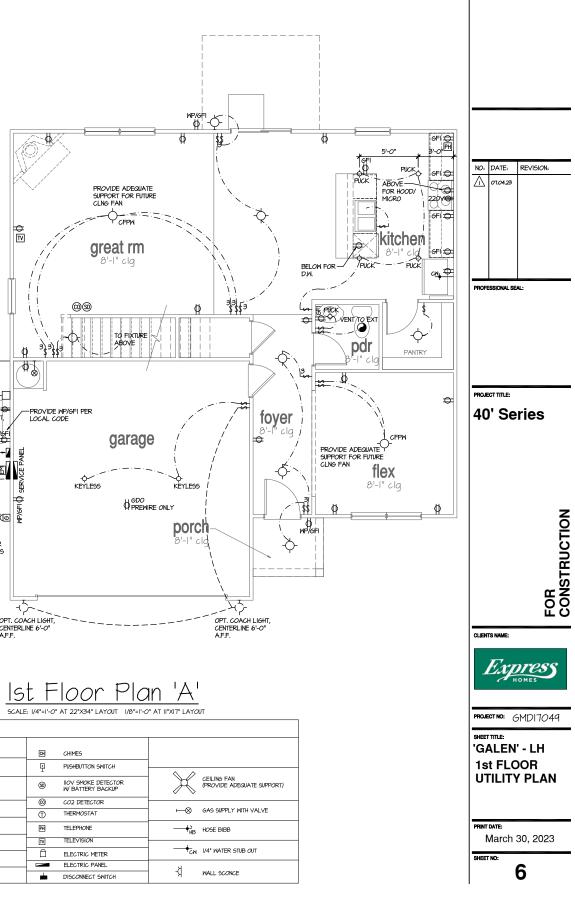
FULL HEIGHT 2X4 WOOD STUD PARTITION



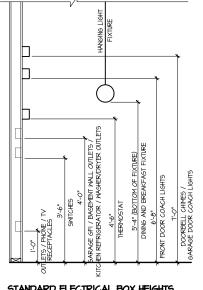
8'-1" STAIR NOTE: (JSE 14" T.JI WITH 3/4" PL YHOOD SUBFLOOR) 14 TREADS AT 10" EACH VERIFY 15 RISERS AT 4/- 7.45" = 111 3/4" TOTAL RISE VERIFY	SCALE: 1/4"=1'-0" AT 22"X34"	ON MIAN E "LAYOUT 1/8"=1'-0" AT II"XIT" LAYOUT
- FOR ADDITIONAL NOTES SEE GENERAL NOTES ON TITLE SHEET AND DETAILS. - MINDOW HEAD HEIGHTS:	KEY NOTES FOR NORTH CAROLINA:	
AILCOM LE-CONTISS 15T FLOOR = 6-69 UNO. ON ELEVATIONS. 2ND FLOOR = 7'-0' UNO. ON ELEVATIONS. ALL DIMENSIONS TO WINDOWS AND DOORS ARE TO CENTERLINE.	FIRE PROTECTION: HOUSE TO GARAGE FIRE SEPARATION, GARAGE/HOUSE SEPARATION AT VERTICAL SURFACES SHALL BE PROTECTED WITH ONE (U LAYER L'S OFFSUM DOARD, (FER NCRC TABLE R302.6.)	 5 FAU 8'X8' PLATFORM. VERIFY WITH TRUSS MANU. (6'-6" MIN. CLEAR HEIGHT TO HORIZONTAL MEME 2"X6" OVER 2"X4" BOTTOM CHORD. OF TRUSS, 6 A/C CONDENSER PAD. (VERIFY)
WALL LEGEND:	GARAGE/HOUSE SEPARATION AT HORIZONTIAL SURFACES SHALL BE PROTECTED WITH ONE (I) LAYER 5/8' TYPE 'X' GYPSIM BOARD. (PER NCRC TABLE R302.6)	PRE-FABRICATED METAL FIREPLACE. INSTALL PER MANUFACTURER'S WRITTEN INSTRUCT
PLL HEIGHT FULL HEIGHT 2X4 WOOD STUD PARTITION 2X6 WOOD STUD PARTITION	HOUSE TO GARAGE DOOR SEPARATION. PROVIDE I-3/8" SOLID CORE DOOR OR APPROVED 20 MINUTE RATED DOOR. (PER NCRC SECTION R302.5.1.)	ATTIC ACCESS LARGE ENOUGH TO REMOVE LAR OF EQUIPMENT BUT NOT LESS THAN 30"x22", FIR ACCESS AS NOTED. (PER NORC 807.1)
STUD WALL BELOW BRICK / STONE VENEER HEIGHT AND STUD SUZE AS NOTED	BENEATH STAIRS AND LANDINGS. I/2" GYPSIM BOARD ON NALLS AND CEILING OF BICLOSED ACCESSIBLE AREAS. (PER NORC SECTION R302.1) IN CONCEALED SPACES BETNEEN STAIR STRINGERS PROVIDE FIREBLOCKING FER R302.11	ATTIC ACCESS LADDER, VERIFY LOCATION AND (25 1/2* X 54* SIZE.) FOR GARAGE TO ATTIC SE NCRC 302.5.1 EXCEPTION. TYPICALS.
LOW GYPSUM BOARD WALL DRYWALL OPENING. HEIGHT HEIGHT AND STUD SIZE AS NOTED AS NOTED ON PLAN.	MEP'S GAS WATER HEATER ON 18" HIGH PLATFORM. (PER CHAPTER 5 NCRC-PLIMBING)	IEMMERED SAFETT GLASS, (MER NORC SECTION ID PLYMOOD SHELF ABOVE WITH DRYMALL FINISH III HALF WALL, HEIGHT AS NOTED.

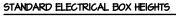
9'-1" STAIR NOTE: (05E 14" TJI WITH 3/4" PLYWOOD SUBFLOOR) IS TREADS AT 10" EACH VERIFY 16 RISER'S AT 1/- T.13" = 123 3/4" TOTAL RISE VERIFY





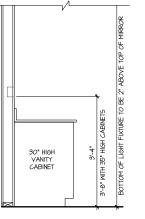
	LEG	END:					
EQUIRED BY NATIONAL ELECTRICAL	¢	DUPLEX OUTLET	-¢-	FLUSH-MOUNT LED CEILING FIXTURE	СН	CHIMES	
E5.	ФиР/6FI	WEATHERPROOF GFI DUPLEX OUTLET	- @ -	HANGING FIXTURE	Ŧ	PUSHBUTTON SWITCH	
R WET OR DAMP LOCATIONS." SHALL BE ENGINEERED BY OTHERS. THE	ф өғі	GROUND-FAULT CIRCUIT-INTERRUPTER DUPLEX OUTLET	\¢\	FLUSH-MOUNT LED CEILING FIXTURE (PROVIDE CEILING FAN SUPPORT)	9	IIOV SMOKE DETECTOR W BATTERY BACKUP	
D PLACEMENT. 202 DETECTORS AS REQUIRED BY	Ø	HALF-SWITCHED DUPLEX OUTLET	cfpT -@	2-LIGHT VANITY FIXTURE	0	CO2 DETECTOR	~ ~
EQUIREMENTS OF ALL GOVERNING CODES. REQUIRED BY NATIONAL ELECTRICAL	₽ 220V	220 VOLT OUTLET	T	2-LIGHT VANITE FIXTURE	T	THERMOSTAT	⊢⊗
E9.	0	REINFORCED JUNCTION BOX	-\$	3-LIGHT VANITY FIXTURE	PH	TELEPHONE	++
JUTOFFS.	\$	WALL SWITCH	-@	4-LIGHT VANITY FIXTURE	N	TELEVISION -	-+
LECTRICAL PANELS, SANITARY SUMP PITS, I DUE TO FIELD CONDITIONS.	\$3	THREE-WAY SWITCH	-\$	WALL MOUNT FIXTURE	6	ELECTRIC METER	
E PER CODE AND MANUFACTURER'S	\$4	FOUR-WAY SWITCH	e	EXHAUST FAN (VENT TO EXTERIOR)		ELECTRIC PANEL DISCONNECT SWITCH	-X
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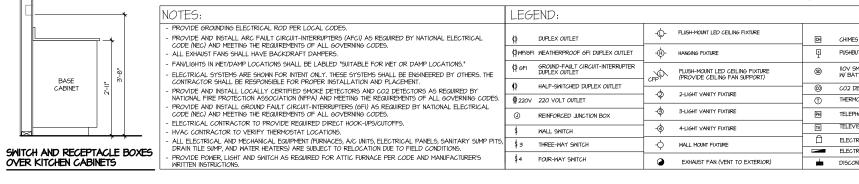


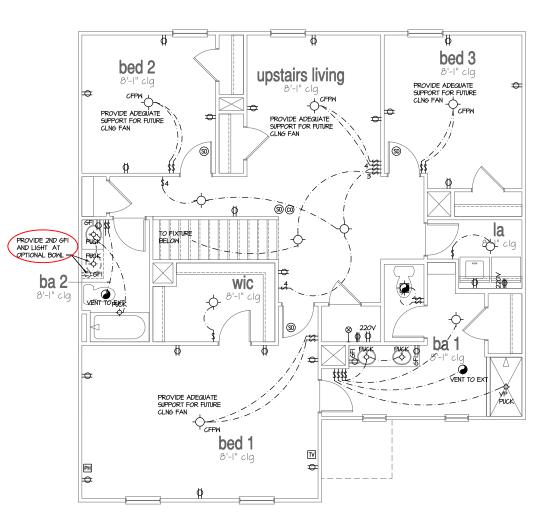
WALL CABINET

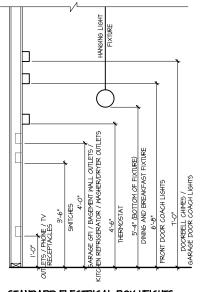
BASE CABINET



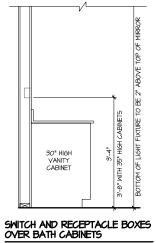


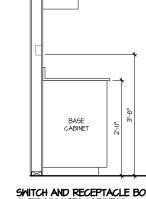






STANDARD ELECTRICAL BOX HEIGHTS



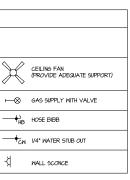


WALL CABINET

SWITCH AND RECEPTACLE BOXES OVER KITCHEN CABINETS

	LEGEND:		
OVIDE GROUNDING ELECTRICAL ROD PER LOCAL CODES. OVIDE AND INSTALL ARC FAILT CIRCUIT-INTERCIPTERS (AFCI) AS REQUIRED BY NATIONAL ELECTRICAL DEFAULT AND METHICS DE CONTRACTOR OF ALL CONTRACTOR OF CONTRACTOR OF A	Ø DUPLEX OUTLET		CHIMES
IDE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. L EXHAUST FANS SHALL HAVE BACKDRAFT DAMPERS.	ØWP/GFI WEATHERPROOF GFI DUPLEX OUTLET		PUSHBUTTON SWITCH
NLIGHTS IN WET/DAMP LOCATIONS SHALL BE LABLED "SUITABLE FOR WET OR DAMP LOCATIONS." ECTRICAL SYSTEMS ARE SHOWN FOR INTENT ONLY. THESE SYSTEMS SHALL BE ENGINEERED BY OTHERS. THE	GFI GROUND-FAULT CIRCUIT-INTERRUPTER DUPLEX CUTLET	FLUSH-MOUNT LED CEILING FIXTURE (PROVIDE CEILING FAN SUPPORT)	SO IIOV SMOKE DETECTOR
NITRACTOR SHALL BE RESPONSIBLE FOR PROPER INSTALLATION AND PLACEMENT. OVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY	HALF-SWITCHED DUPLEX OUTLET		© CO2 DETECTOR
TIONAL FIRE PROTECTION ASSOCIATION (NEPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. OVIDE AND INSTALL GROUND FAULT CIRCUIT-INTERRUPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRICAL	220V 220 VOLT OUTLET	Q 2-LIGHT VANITY FIXTURE	
DE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.	REINFORCED JUNCTION BOX	-③ 3-LIGHT VANITY FIXTURE	PH TELEPHONE
AC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS.	WALL SWITCH	-4 4-LIGHT VANITY FIXTURE	
L ELECTRICAL AND MECHANICAL EQUIPMENT (FURNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUMP PITS, AIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS.	\$ 3 THREE-WAY SWITCH	- WALL MOUNT FIXTURE	
OVIDE POWER, LIGHT AND SWITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S ITTEN INSTRUCTIONS.	\$4 FOUR-WAY SMITCH	EXHAUST FAN (VENT TO EXTERIOR)	DISCONNECT SWITCH

2nd Floor Plan 'A' SCALE: 1/4"=1'-0" AT 22"X34" LAYOUT 1/8"=1'-0" AT 11"X17" LAYOUT





	DESIGN SPECIFICATIONS: Construction Type: Commerical Residential Applicable Building Codes: • 20/8 North Carolina Residential Building Code with All Local • ASCE T-10: Minimum Design Loads for Buildings and Other St Design Loads: 1. Roof Live Loads 11. Conventional 2x 20 PSF 12. Truss 20 PSF 12. Truss 20 PSF 2. Truss 20 PSF 2.1. Conventional 2x 20 PSF 2.2. Truss 20 PSF 2.1. Conventional 2x 20 PSF 2.2. Truss 20 PSF 3. Importance Factor 10 4. Floor Live Loads 10 4. Floor Live Loads 40 PSF 4. Seering Areas 30 PSF 5. Floor Dead Loads 50 PSF 5. Floor Truss 50 PSF 5. Floor Truss 50 PSF 5. Floor Truss 50 PSF 6. Ultimate Design Wind Speed (3 sec. gust) 130 MPH 6. Exposure 631.Vx = 6.31.Vx = 632.Vy = 7. Component and Clading (in PSF)	ructures	ENGINEERING • LAI A Universal Engineeri STRUCTURAL PLA C# ALE PROJECT ADDRESS: TBD DESIGNER: GMD Design Group 1/02 Fountain Brook Circle, Suite C Cary, NC 21511 These drawings are to be coordinated	A with the architectural, mechanical, plumbing, prepared in the responsibility of the prepared in the prepared in the responsibility of the prepared in	Sheet No. CSI Sheet No. CSI Sl@m Sl@m Sl@a Sl@a Sl@a Date Sl@a Sl@a	Description Cover Sheet, Specifications, Revisions Monolithic Slab Foundation Stem Wall Foundation Basement Foundation Basement Flan First Floor Plan Second Floor Plan Roof Framing Plan Project No. T0928 Added Craul Space Foundation	
	HT. UP 10 30 30"-35" 351"-40" 46 ZONE I 16.7.18.0 115.7.18.3 182.7.19.6 18. ZONE I 16.7.18.0 115.7.18.3 182.7.19.6 18. ZONE I 16.7.21.0 115.7.22.1 182.7.27.3 18. ZONE 3 16.7.21.0 115.7.22.1 182.7.27.3 18. ZONE 4 182.7.9.0 192.7.20.0 19.9.7.20.1 120.7.27.3 ZONE 5 182.7.24.0 192.7.50 19.9.7.20.1 120.7.27.3 120.7.20.1 ZONE 5 182.7.24.0 192.7.50 19.9.7.20.1 120.7.27.3 120.7.20.1 120.7.20	0	structural engineering of record (SER) apparent, the contractor shall notify & INC. before construction begins. PLAN ABBREVIATIONS: AB ANCHOR BOLT AFF ABOVE FINISHED FLOOR CJ CEILING JOIST CLR CLEAR DJ DOUBLE JOIST CLR CLEAR DJ DOUBLE STUD POCKET EE EACH END EW EACH WAY NTS NOT TO SCALE OC ON CENTER PSF POUNDS PER SQUARE FOOT PSI POUNDS FER SQUARE INCH Roof truss and floor joist layouts, and were not provided to &UMMIT Enginee prior to the initial design, Therefore, t based on the Information provided by revisions based on roof truss and floor revision list, inclicating the date the la	IMMIT Engineering, Laboratory 4 Testing, PT PRE68uRE TREATED R6 R00F SUPPORT SC STUD COLUMN SJ SINGLE JOIST SPF SPRUCE PINE FIR SST SIMPSCN STRONG-TIE SYP SOUTHERN YELLOW PINE TJ TRIPLE JOIST TSP TRIPLE STUD POCKET TYP TYPICAL UNO UNLESS NOTED OTHERWISE WWE WELDED WIRE FABRIC d their corresponding loading details, ring, Laboratory 4 Testing, NC. (SUMMIT) russ and Joist directions were assumed JD R.Horton, Inc, Subsequent plan or joist laigues shall be noted in the			
 GENERAL STRUCTURAL NOTES. The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUM1T Engineering, Laboratory 4 Testing, INC. (SUM1T) or the SER. For the purposes of these construction documents the SER and SUM1T shall be considered the same entity. The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure. The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of the structure is conform to the contract documents, should any non-conformities occur. Any structural elements or details not fully developed on the construction elements or details not fully developed on the construction of sufficience and professional engineer. These shop drawings shall be subnitted to SUM1T for reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions for accuracy and report any discrepancies to SUM1T before construction failures as it relates to the structural design of the field conditions for accuracy and report any discrepancies to SUM1T before construction all applicable sections of the association to all applicable sections of indication as its structural assemble for any secondary structural elements or non-structural adilements, except for the elements specifically noted on the structural dements or non-structural edimenta, except for the elements or non-structural adilements, except for the elements or non-structural adilements and elements or the structural assemble for any secondary structural elements or non-str	 The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade. Any fill shall be placed under the direction or recommendation of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95% maximum dry density. Excavations of footings shall be lined temporarily with a 6 mill polystivalere membrane if placement of concrete does not occur within 24 hows of excavation. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material. STRUCTURAL STEEL: Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design" latest editions. Structural steel shall neceive one coat of shop applied rust-inhibitive paint. All steel shall nave a minimum yield stress (Fy) of 36 ksi unless otherwise noted. Welding shall have a normal weight aggregate and a minimum compressive strength (Fy) at 28 days of 3000 psi, unless otherwise noted on the plan. Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 38. "Building Code Requirements for Reinforced Concrete for Buildings". All entralise for Structural Concrete for Building Code Requirements for Reinforced Concrete for Buildings. All entralise for Structural Concrete for Buildings. All entralise for Reinforced Concrete for Buildings. All entralise on for Structural Concrete for Buildings. All entralise (in percent) shall be uithin -1% to 42% of target values as follows: 31. Ercotings: 5% No admixtures shall be added to any structural concrete without written	 with ACI 302,R-96: Construction". The concrete slab- subgrade modulus opsT. The SER is no cracking or other fi conditions not in aci conditions not in aci control or sau cut slabs-on-grade at slabs-on-grade at slabs-on-grade at slabs-on-grade at slabs-on-grade at be placed at mid-os supported during t CONCRETE REINFORCEN I. Fibrous concrete re concrete slabs-on- due to shrinkage ar water migration, an abrasion resistance Fiberash relinforcing for us an abrasion resistance Fiberash relinforcing to a minimum of 0,% by ASTM A6/B, grade Steel reinforcing but standard Practice Detailing riar recipies heall con- duct of shrinkage ar water migration, and standard and standard and Standard Practice Detailing riar resistance Detailing riarication of Tibe a minimum of 0,% by ASTM A6/B, grade Detailing riarication and shall have 307 size/spacing as the tension police. Lap reinforcement a for tension or comp 	MENT: einforcement, or fibermesh, specified in -grade may be used for control of cracking und thermal expansion/contraction, lowered Increase in impact capacity, increased e, and residual strength ong to be 100% virgin polypropujene fibers occessed olefin materials and specifically see as concrete secondary reinforcement. emesh per cubic yard of concrete shall equal y volume (15 pounds per cubic yard) nply with ASTM Clifs, any local building code shall meet or exceed the current industry wars shall be new billet steel conforming to	 Where reinforcing dowels are required, the in size and spacing to the vertical reinforce shall extend 48 bar oliameters vertically and into the footing. Where reinforcing steel is required vertical provided unless otherwise noted. WOOD FRAMING: Solid sawn wood framing members shall conf specifications listed in the latest edition or Design Specification for Wood Construction of the old Construction of the second shall have the design values:	ement. The douel d 20 bar diameters ly, douels shall be form to the f the 'National m' (ND5). Unless are designed to be ima-Fir (3FP) 12, the following minimum earth shall be standard C-15. All tited in accordance wise noted. Indard Bi82:1-1981. rdance with ND5 ting framing members be be 2x4 SYP 12 or d. Studs shall be to plated at each Intinuous. tached with one 10d shall be continuous be properly er load transfer. ed with (3) 10d nails @ gether with (2) rous	 WOOD TRUSSES: The wood truss manifacturer/fabricator is respondesign of the wood trusses. Submit sealed show supporting calculations to the SER for review proview. The review by the SER shall have an inimum of five review. The review by the SER shall review for compliance with the design documents. The SER responsibility for the correctness for the struct the wood trusses shall be designed for all requires any specified in the local building code, the AB "Minimum Design Loads for Building and Other (ASCE 1-10), and the localing requirements show specifications. The truss draiwings including but not HVAC equipment, piping, and architectural fixture the trusses. The trusses shall be designed for all requirements show on these draiwings including but not HVAC equipment, piping, and architectural fixture the trusses. The trusses shall be designed, fabricated, and accordance with the latest edition of the "Natif Specification for Medal Plate Connected Wood Trusses" (HIB-9). This be temporary and permanet, shall be shown on the Also, the shop draiwings shall how the required the trusses. Ang chords or truss webs shown on these draiw shown as a reference only. The final design of the per the manifacturer. EXTERIOR WOOD FRAMED DECKS: Decks are to be framed in accordance with the APA. 	p drawings and prior to a (5) days for overall (5) days for ural design for ural design for toylade for tilmited to res attached to (5) erected in onal Design d "Design Trusses." oracing Metal racing, both shop drawings. attachments for gs have been the trusses shall (1) (2) (3) (5) days for (5) d

Manager	Signature	
Operations		
Operations System		
Operations		
Operations Product		
Development		

3.	Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more
4.	Information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise. Roof sheathing shall be APA rated sheathing exposure I or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6°o/ca ta penel edges and at 12°o/c in panel field unless
5.	chrowies noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood cillops or lumber blocking unless otherwise noted. Planel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code. We also floor sheathing shall be APA nated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshark hail at 6'o'c at panel edges and at 12'o'c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall be support by use of 14G bywood or lumber blocking unless otherwise noted. Planel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Buildong the support by use of 14G bywood or lumber blocking unless otherwise noted. Planel end joints shall occur over framing.
6.	Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.
STR	JCTURAL FIBERBOARD PANELS:
1.	Fabrication and placement of structural fiberboard sheathing
	shall be in accordance with the applicable AFA standards.
2.	All structurally required fiberboard sheathing shall bear the mark of the AFA.
3.	Fiberboard wall sheathing shall comply with the requirements of
	local building codes for the appropriate state as indicated on

- local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
 Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.



FOUND ATION NOTES:

- FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AMENDMENTS.
- 2. STRUCTURAL CONCRETE TO BE $F_c = 3000$ PSI, PREPARED AND PLACED IN
- ACCORDANCE WITH ACI STANDARD 318. 3. FOOTINGS TO BE PLACED ON UNDISTURBED EARTH, BEARING A MINIMUM OF 12" BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE
- CODE ENFORCEMENT OFFICIAL. FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF 4, 2000 PSF CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFING THE SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION. FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE 5.
- ELEMENTS. PROVIDE 2" MINIMUM FOOTING PROJECTION FROM THE FACE OF MAGONEY 6.
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS SPECIFIED IN SECTION R404.1 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE.
- ILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL. PROVIDE FOUNDATION WATERPROOFING, AND DRAIN WITH POSITIVE SLOPE TO 8.
- OUTLET AS REQUIRED BY SITE CONDITIONS. PROVIDED PERIMETER INSULATION FOR ALL FOUNDATIONS PER 2016 NORTH 9, CAROLINA RESIDENTIAL BUILDING CODE.
- 10. CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK
- VENEERS. CRAWL SPACE TO BE GRADED LEVEL, AND CLEARED OF ALL DEBRIS. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION R403.16, MINIMUM 1/2" DIA, BOLTS SPACED AT 6'-0" ON CENTER WITH A 1" MINIMUM EMBEDMENT INTO MAGONRY OR CONCRETE. ANCHOR BOLTS SHALL BE IZ" FROM THE END OF EACH PLATE SECTION. MINIMUM (2) ANCHOR BOLTS FER PLATE SECTION. ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE. 9 ABBREVIATIONS:
- DJ = DOUBLE JOIST SJ = SINGLE JOIST GT = GIRDER TRUSS FT = FLOOR TRUSS SC = STUD COLUMN EE = EACH END DR = DOUBLE RAFTER TR = TRIPLE RAFTER TJ = TRIPLE JOIST OC = ON CENTER CL = CENTER LINE PL = POINT LOAD

10. ALL PIERS TO BE 16"X16" MASONRY AND ALL PILASTERS TO BE 8"X16"

- MASONRY, TYPICAL (UNO) WALL FOOTINGS TO BE CONTINUOUS CONCRETE, SIZES PER STRUCTURAL PLAN. A FOUNDATION EXCAVATION OBSERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER OR HIS OUALIFIED REPRESENTATIVE. IF ISOLATED AREAS OF YIELDING MATERIALS AND/OR POTENTIALLY EXPANSIVE SOILS ARE OBSERVED IN THE FOOTING EXCAVATIONS AT THE TIME OF CONSTRUCTION, SUMMIT ENGINEERING, EXCAVATIONS AT THE TIME OF CONSTRUCTION, SUMMIT ENGINEERING, LABORATORY 4 TESTING, INC. MUST BE PROVIDED THE OPPORTUNIT REVIEW THE FOOTING DESIGN PRIOR TO CONCRETE PLACEMENT.
- ALL FOOTINGS & SLABS ARE TO BEAR ON UNDISTURBED SOIL OR 95% COMPACTED FILL, VERIFIED BY ENGINEER OR CODE OFFICIAL.

REFER TO BRACED WALL PLAN FOR PANEL LOCATIONS AND ANY REQUIRED HOLDOWNS ADDITIONAL INFORMATION PER SECTION R602.10.8 AND FIGURES R602.10.6.5 R602.10.1, R602.10.8(1) AND R602.10.8(2) OF THE 2015 IRC

NOTE: ALL EXTERIOR FOUNDATION DIMENSIONS ARE TO FRAMING AND NOT BRICK VENEER, UNO

NOTE: A 4" CRUSHED STONE BASE COURSE IS NOT REQUIRED WHEN SLAB IS INSTALLED ON WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS CLASSIFIED AS GROUP | PER TABLE R405.

REINFORCE GARAGE PORTAL WALLS PER FIGURE R602.10.9 OF THE 2015 IRC.

BEAM POCKETS MAY BE SUBSTITUTED FOR MASONRY PILASTERS AT GIRDER ENDS. BEAM POCKETS SHALL HAVE A MINIMUM 4" SOLID MASONRY BEARING.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL, PLANS PROVIDED BY <u>DR HORION</u> COMPLETED/REVISED ON <u>03/30/23</u>, IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY 4 TESTING, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

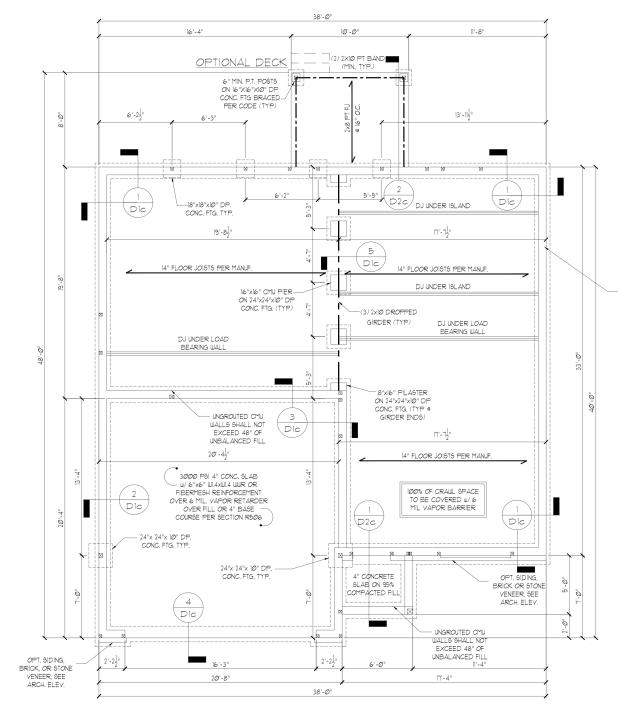
STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, INC. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

CRAWL SPACE FOUNDATION PLAN

SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"



CRAWL SPACE FOUNDATION PLAN - ELEVATION A, B, C

18"x24" MIN. CRAWL SPACE ACCESS DOOR TO BE LOCATED IN FIELD PER BUILDER PROVIDE MIN (2) 2x10 HEADER OVER DOOR W/ MIN. 4" BEARING EACH END. AVOID SHOWN POINT LOADS.

DECK FLOOR JOISTS SHALL BE SPACED AT MAX 12' ON CENTER WHEN DECKING INSTALLED DIAGONALLY



CMU FND WALL ON 16"x10" DP CONT. CONC. FTG. (TYP. BRICK VENEER: 8" CMU FND WALL w/ BRICK VENEER ON 21"x10" DP CONT. CONC. FTG, (TYP.)

SIDING VENEER: 8"

FOUND ATION NOTES

- FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AMENDMENTS.
- 2 STRUCTURAL CONCRETE TO BE EG = 3000 PSI PREPARED AND PLACED IN
- ACCORDACE WITH ACI STANDARD 318. FOOTINGS TO BE PLACED ON UNDISTURBED EARTH, BEARING A MINIMUM OF 3,
- 12" BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE
- CODE ENFORCEMENT OFFICIAL. FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF 4, 2020 PS CONTRACTOR IS SOLELY REPONDED FOR VERYING THE BUITABILITY OF THE STE SOLE CONDITIONS AT THE TIME OF CONSTRUCTION. FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE ELEMENTS, PROVIDE 21 MINIMUM FOOTING PROJECTION FROM THE FACE OF
- 5. MAGONEY
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS SPECIFIED IN SECTION R404.1 OF THE 2018 NORTH CAROLINA RESIDENTIAL 6. BUILDING CODE.
- 8.
- BUILDING CODE. PILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL. PROVIDE FOUNDATION WATERPROFING, AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS, PROVIDED PERIMETER INSULATION FOR ALL FOUNDATIONS PER 2018 NORTH 9,
- CAROLINA RESIDENTIAL BUILDING CODE. 10. CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK
- VENEERS.
- CRAWL SPACE TO BE GRADED LEVEL, AND CLEARED OF ALL DEBRIG. CRAIL 5FACE 10 BE GRADED LEVEL, AND CLEARED OF ALL DEBRIS. FONDATION ANCHORAGE SHALL BE CONSTRUCTED FER THE 200 NORTH CAROLINA RESIDENTIAL CODE SECTION R403.6. MINIMUM 1/2" DIA BOLTS 5PACED AT 6'-0" ON CENTER WITH A 1" MINIMUM EMBEDMENT INTO MASORRY OR CONCRETE, ANCHOR BOLTS SHALL BE 12" FROM THE END OF EACH PLATE SECTION. MINIMUM (12) ANCHOR BOLTS FER PLATE SECTION. ANCHOR BOLTS 5HALL BE LOCATED IN THE CENTER THIRD OF THE PLATE. 8 9 ABBREVIATIONS:

DJ = DOUBLE JOIST	SJ = SINGLE JOIST
GT = GIRDER TRUSS	FT = FLOOR TRUSS
SC = STUD COLUMN	DR = DOUBLE RAFTER
EE = EACH END	TR = TRIPLE RAFTER
TJ = TRIPLE JOIST	OC = ON CENTER
CL = CENTER LINE	PL = POINT LOAD

10. ALL PIERS TO BE 16"X16" MASONRY AND ALL PILASTERS TO BE 8"X16"

- MASONRY, TYPICAL. (UNO) WALL FOOTINGS TO BE CONTINUOUS CONCRETE, SIZES PER STRUCTURAL PLAN. 12. A FOUNDATION EXCAVATION OBSERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER OR HIS QUALIFIED REPRESENTATIVE. IF ISOLATED AREAS OF YIELDING MATERIALS AND/OR POTENTIALLY EXPANSIVE SOILS ARE OBSERVED IN THE FOOTING EXCAVATIONS AT THE TIME OF CANORFRICTION, SUMMIT EXGINEERING, LABORATORY 4 TESTING, INC. MUST BE PROVIDED THE OPPORTUNITY TO REVIEW THE FOOTING DESIGN PRIOR TO CONCRETE PLACEMENT.
- 13. ALL FOOTINGS & SLABS ARE TO BEAR ON UNDISTURBED SOIL OR 95% COMPACTED FILL, VERIFIED BY ENGINEER OR CODE OFFICIAL.

REFER TO BRACED WALL PLAN FOR PANEL LOCATIONS AND ANY REQUIRED HOLDOWNS ADDITIONAL INFORMATION PER SECTION R602.10.8 AND FIGURES R602.10.6.5 R602,10,7, R602,10,8(1) AND R602,10,8(2) OF THE 2015 IRC

NOTE: ALL EXTERIOR FOUNDATION DIMENSIONS ARE TO FRAMING AND NOT BRICK VENEER, UNO

NOTE: A 4" CRUSHED STONE BASE COURSE IS NOT REQUIRED WHEN SLAB IS INSTALLED ON WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS CLASSIFIED AS GROUP | PER TABLE R405.1

REINFORCE GARAGE PORTAL WALLS PER FIGURE R602.10.3 OF THE 2015 IRC.

BEAM POCKETS MAY BE SUBSTITUTED FOR MASONRY

PILASTERS AT GIRDER ENDS. BEAM POCKETS SHALL HAVE A MINIMUM 4" SOLID MASONRY BEARING.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH HELDE FEARLOADE DECISIONED BY DR ACCOMPANDE WITH ARCHITECTURAL, FLANG PROVIDED BY DR HORION COMPLETED/REVIGED ON 03/30/23, IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY 4 TESTING, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMIT ENGINEERING, LABORATORY & TESTING, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

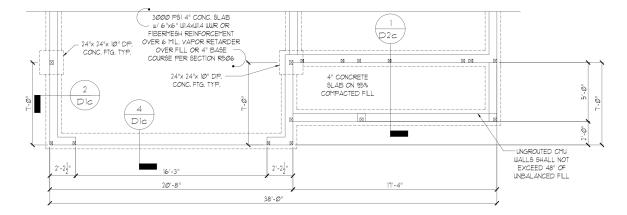
STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, INC. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

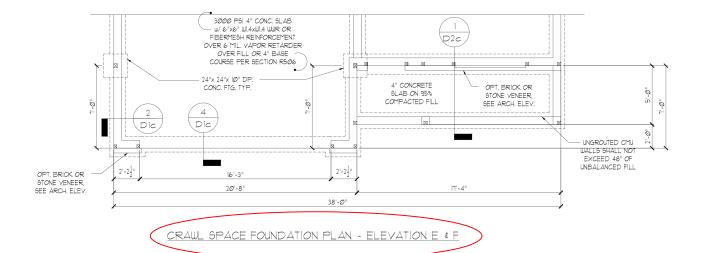
STRUCTURAL ANALYSIS BASED ON 2018 NCRC

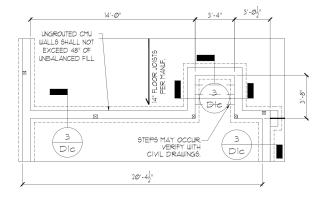
CRAWL SPACE FOUNDATION PLAN

SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"



CRAWL SPACE FOUNDATION PLAN - ELEVATION D





OPT. GARAGE ENTRY STEPS

SUMMIT Charlotte, NC, 28273 Office: 704.504.1717 Fax: 704.504.1125 www.summit-companies. RTH CARO SUMMIT gineering, Labor & Testing, Inc. No. F-1454 CLIENT: DR Horton, Inc. 8001 Arrouridge Blvc Charlotte, NC 28273 6 ndatí П О С Space PROJECT: Galen - LH Crawl NORTH CAROL SEAL 056484 A MOINER THEW S. BED. 25.23 STRUCTURAL MEMBERS ONLY DRAUNG DATE: 10/02/2023 8CALE: 22x34 1/4**1'-@* 1k17 1/8**1'-@* PROJECT 1 528.70928 DRAWN BY: EO CHECKED BY: MSB PROJECT • 10928 DATE Ø4/26/23 REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS SI.Ic

	REQUIRED	BRACED W	ALL PANEL CONNEC	CTIONS
			REQUIRED CONNECTION	
METHOD	MATERIAL	MIN. THICKNESS	@ PANEL EDGES	© INTERMEDIATE SUPPORTS
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS © 6" O.C.	6d COMMON NAILS # 12" O.C.
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS** © 7° O.C.	5d COOLER NAILS** @ 7" O.C.
WGP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS & 6" O.C.	6d COMMON NAILS # 12" O.C.
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R6 <i>0</i> 2.10.6.4	PER FIGURE R602.10.6.4

FIRST FLOOR BRACING (FT)		
CONTINUOUS SHEATHING METHOD		
	REQUIRED	PROVIDED
BWL 1-1	13.9	23.8
BWL 1-2	13.9	14.2
BWL 1-A	9.9	37 <i>.0</i>
BWL I-B	9,9	12.1
BWL 1-C	8.6	33.Ø

GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING
- CODE WITH ALL LOCAL AMEDIMENTS. CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONTRACTOR SHALL COMPLY WITH CONTRACTOR SHALL VERIT ALL DIFIENDING CONTRACTOR SHALL COFFET WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT. ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN. CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED

- REDFONDLE TOR ANT DEVICING THEM THIS THAT TEMPORARY BRACING REQUIRED TO REGIST ALL FORCES ENCOUNTERED DURING ERECTION.
 (ROCPETTES USED IN THE DESIGN ARE AS FOLLOUS: MICROLLAM (LVL), F₀ = 12600 PGI, F₁ = 2260 PGI, F₂ = 2260 PGI, F₂ = 2260 PGI, F₂ = 125100 PGI
 ALL BUOD HEMBERS SHALL BE ''S STP OR ''S SFF UNLESS NOTED ON PLAN, ALL STUD COLUMNS AND JOISTS SHALL BE ''S STP OR ''S SFF UNCESS NOTED ON PLAN, ALL STUD COLUMNS AND JOISTS SHALL BE ''S STP OR ''S SFF UNCESS NOTED ON PLAN, ALL STUD COLUMNS AND JOISTS SHALL BE ''S STP OR ''S SFF UNCESS NOTED ON PLAN, ALL STUD COLUMNS AND JOISTS SHALL BE ''S STP OR ''S SFF UNCESS NOTED ON PLAN, ALL STUD COLUMNS AND JOISTS SHALL BE CRADE 600 BARS CONFORMING TO ASTM A6IS AND SHALL HAVE A MINIMUM COVER OF 3''.
 ('COUNDATION ANCHORAGE SHALL EE CRADE 600 BARS CONFORMING TO ASTM A6IS AND SHALL HAVE A MINIMUM COVER OF 3''.
 ('FOUNDATION ANCHORAGE SHALL EE CRADE 600 BARS CONFORMING TO ASTM A6IS AND SHALL HAVE A MINIMUM COVER OF 3''.
 ('FOUNDATION ANCHORAGE SHALL EE CRADE 600 BARS CONFORMING TO ASTM A6IS AND SHALL ACCODE SECTION REVOLUTION MACOURY OR CONCRETE. WITH A '' MINIMUM ENDEDMENT INTO MASONRY OR CONCRETE. ANCHOR BOLTS SHALL BE I''' ROM THE END OF EACH PLATE SECTION, MINIMUM (2) ANCHOR BOLTS FRE PLATE SECTION, ANCHOR BOLTS SHALL BE CONTROL ANCHOR BOLTS SHALL BE CONTROL TO THE CONCRETE ANCHOR DOLTS SHALL BE CHARD. SECILING JOISTS SPAN
- CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- 10. FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH 1/2" DIA THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/D3f. MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
- AND (22 DOLIS GRALE DE LOCATED TIMIN TO FROT LEADT HE DEAT . ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 STP 72 OR (1) FLAT 2x4 SFF 72, DROPPED. FOR NON-LOAD BEARING HEADERS EXCEEDING 8'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 SYP #2 OR (2) FLAT 2x4 SPF #2 DROPPED. (UNLESS NOTED OTHERWISE)

12.	ABBREVIATIONS:	

DJ = DOUBLE JOIST	SJ = SINGLE JOIST
GT = GIRDER TRUSS	FT = FLOOR TRUSS
SC = STUD COLUMN	DR = DOUBLE RAFTER
EE = EACH END	TR = TRIPLE RAFTER
TJ = TRIPLE JOIST	OC = ON CENTER
CL = CENTER LINE	PL = POINT LOAD

NOTE:

ULL DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE, PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

JOIST & BEAM SIZES SHOWN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

INSTALL ANY REQUIRED HOLDOWNS PER SECTION R602 108 AND FIGURES R602 1065 R602 101 R602.10.8(1) AND R602.10.8(2) OF THE 2015 IRC

NOTE: MEMBER NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

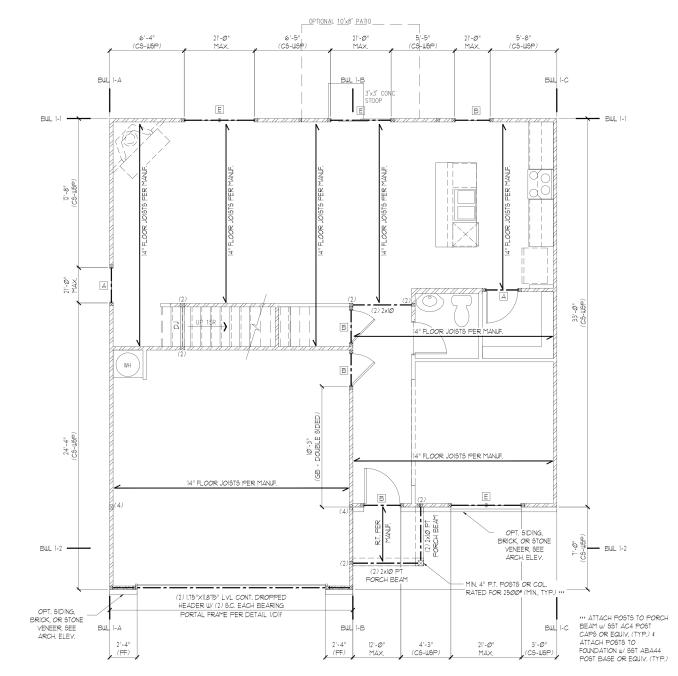
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STRUCTURAL MEMBERS ONLY

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR FRAMING PLAN SCALE: 1/4"=1'-Ø" ON 22"x34" OR 1/8"=1'-Ø" ON 11"x17"



FIRST FLOOR FRAMING PLAN - ELEVATION A, B, C

HEADER SCHEDULE			
TAG	SIZE	JACKS (EACH END)	
А	(2) 2x6	(1)	
в	(2) 2x8	(2)	
С	(2) 2x1Ø	(2)	
D	(2) 2x12	(2)	
E	(2) 9-1/4" LSL/LVL	(3)	
F	(3) 2x6	(1)	
G	(3)2x8	(2)	
н	(3) 2x1Ø	(2)	
	(3) 2xl2	(2)	

HEADER SIZES SHOWN ON PLANS ARE MINIMUMS, GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION. ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE SC NOTED ON PLAN OVERRIDE SC LISTED ABOVE.

LINTEL SCHEDULE			
TAG	SIZE OPENING SI		
	L3x3x1/4"	LESS THAN 6'-Ø"	
2	L5x3x1/4"	6'-0" TO 10'-0"	
3 L5x3-1/2"x5//6" 4 L5x3-1/2"x5//6" ROLLED OR EQUIV		GREATER THAN IØ'-Ø"	
		ALL ARCHED OPENINGS	
SECURE LINTEL TO HEADER w/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED @ 16" O.C. (TYP FOR (3))			
ALL HEADERS WHERE BRICK IS USED, TO BE: () (UNO)			

WALL STUD SCHEDULE

IST & 2ND FLOOR LOAD BEARING STUDS: 2x4 STUDS © 16" O.C. OR 2x6 STUDS © 24" O.C. IST FLOOR LOAD BEARING STUDS w/ WALK-UP ATTIC: 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BASEMENT LOAD BEARING STUDS: 2x4 STUDS © 12" OC. OR 2x6 STUDS © 16" OC. NON-LOAD BEARING STUDS (ALL FLOORS): 2x4 STUDS © 24" OC. IND STORY WALLS: 2x4 STUDS = 12" O.C. OR 2x6 STUDS = 16" O.C. BALLOON FRAMED W/ CROSS BRACING = 6'-0" O.C. VERTICALLY

KI	KING STUD REQUIREMENTS		
(OPENING WIDTH	KINGS (EACH END)	
L	ESS THAN 3'-Ø"	(1)	
	3'-Ø TO 4'-Ø"	(2)	
	4'-0" TO 8'-0"	(3)	
	8'-0" TO 12'-0"	(5)	
1	2'-Ø" TO 16'-Ø"	(6)	
	KING STUD REQUIREMENTS ABOVE DO NOT APPLY TO PORTAL FRAMED OPENINGS		

BRACED WALL NOTES:

- 1) WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2015 INTERNATIONAL RESIDENTIAL CODE AS ALLOWED PER SECTION R602.10 OF THE 2018 NC RESIDENTIAL CODE.
- WALLS ARE DESIGNED FOR SEIGHIC ZONES A-C AND ULTMATE WIND SPEEDS UP TO 130 MPH. REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING
- 2 SI7ES
- 3.
- SIZES. BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH IRC TABLE R602.004. ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED IV RETET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL 4. ENGINEERING CALCULATIONS.
- MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.105. THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF
- INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 12" GTPSUM BOARD (UNO). FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE
- SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFILL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS.
- FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 10 FEET OF
- A DRACED WALL TAKED BALL LINE. EACH END OF A BRACED WALL LINE. THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 20 FEET. 10.
- MAGONRY OR CONCRETE STEM WALLS WITH A LENGTH OF 48" OR LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R60210.9 OF THE 2015 IRC. 11
- 12 BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602/08 13. BRACED WALL PANEL CONNECTIONS TO R007 SHALL BE
- CONSTRUCTED IN ACCORDANCE WITH SECTION R602.0.8.2 AND FIGURES REØ2102(1)4(2)4(3). CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE
- DESIGNED IN ACCORDANCE WITH SECTION R602.10.11 PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.106.4 (UNO) 15.
- 16 ON SCHEMATIC SHADED WALLS INDICATE BRACED WALL PANELS ABBREVIATIONS:

GB = GYPSUM BOARD USP = WOOD STRUCTURAL PANEL CS-XXX = CONT. SHEATHED PF = PORTAL FRAME PF-ENG = ENG. PORTAL FRAME



FIRST FLOOR FRAMING PLAN SCALE: 1/4"=1'-@" ON 22"x34" OR 1/8"=1'-@" ON 11"x17"

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

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STRUCTURAL MEMBERS ONLY

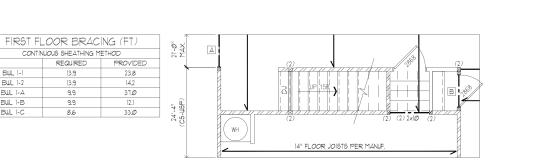
BWL 1-1

BWL 1-2

BWL 1-A

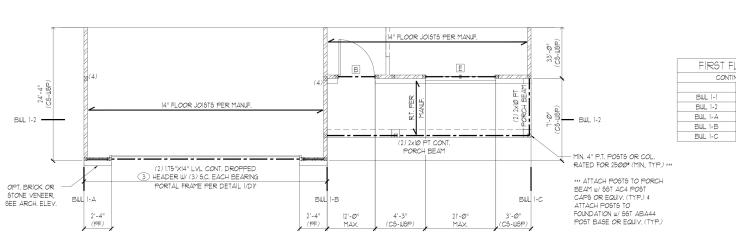
BWL 1-B

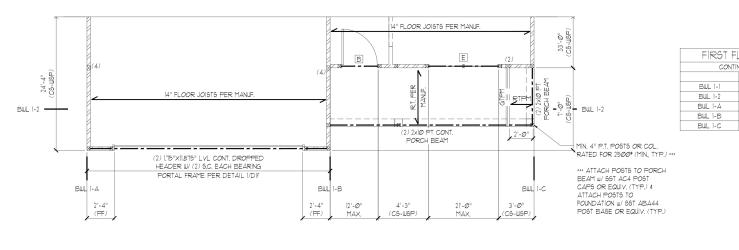
BWL I-C



OPT. GARAGE ENTRY STEPS







FIRST FLOOR FRAMING PLAN - ELEVATION D

OPT. STUDY ILO FLEX

1

4'-3"

(CS-WSP)

OPT. SIDING, BRICK, OR STONE VENEER, SEE

ARCH. ELEV.

I4" FLOOR JOISTS PER MANUF.

F

21'-Ø"

MAX

_OOR BRACING (FT)				
WOUS SHEATHING METHOD				
REQUIRED	PROVIDED			
13.9	23.8			
13.9	14.2			
9.9 37.0				
9,9 12.1				
8.6 33.0				

OOR BRACING (FT)			
WOUS SHEATHING METHOD			
REQUIRED PROVIDED			
13.9	23.8		
13.9	14.2		
9.9 37.0			
9.9 12.1			
8.6 33.0			

FIRST FLOOR BRACING (FT)			
CONTINUOUS SHEATHING METHOD			
REQUIRED	PROVIDED		
13.9	23.8		
13.9	14.2		
9.9	37.0		
9.9	12.1		
8.6	33.Ø		
	14045 SHEATHING M REQUIRED 13.9 13.9 9.9 9.9 9.9		

PORCH DEPENDENT ON ELEV., SEE ARCH.

BWL 1-2

33'-@" CS-IIBF

3'-Ø" C5-W6F

BWL I-C



			1.44
HEADER SCHEDULE			KIN
TAG	SIZE	JACKS (EACH END)	OF
A	(2) 2x6	(1)	LE
В	(2) 2x8	(2)	3
С	(2) 2x1Ø	(2)	4
D	(2) 2xl2	(2)	8
E	(2) 9-1/4" LSL/LVL	(3)	12 '
F	(3)2x6	(1)	KING
G	(3) 2x8	(2)	AP
н	(3) 2x1Ø	(2)	
1	(3) 2x12	(2)	

OPENING WIDTH	KINGS (EACH END)
LESS THAN 3'-0"	(1)
3'-Ø TO 4'-Ø"	(2)
4'-Ø" TO 8'-Ø"	(3)
8'-Ø" TO 12'-Ø"	(5)
12'-Ø" TO 16'-Ø"	(6)

SECOND FLOOR BRACING (FT)			
CONTINUOUS SHEATHING METHOD			
	REQUIRED	PROVIDED	
BWL 1-1	6.8	29.0	
BWL 1-2	6.8	13.3	
BWL 1-A	4.9	40.0	
BWL 1-B	4.9	٦.Ø	
BWL I-C	4.3	33 <i>.</i> Ø	

HEADER SIZES SHOWN ON PLANS ARE MINIMUMS, GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION. ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE. SC NOTED ON PLAN OVERRIDE SC LISTED ABOVE.

LINTEL SCHEDULE TAG SIZE OPENING SIZE

\bigcirc	L3x3x1/4"	LESS THAN 6'-0"		
2	L5x3x1/4"	6'-0" TO 10'-0"		
3	L5x3-1/2"x5/16"	GREATER THAN 10'-0"		
4	L5x3-1/2"x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS		
SECURE LINTEL TO HEADER w/ (2) 1/2" DIAMETER LAG				

SCREWS STAGGERED @ 16" O.C. (TYP FOR 3)

ALL HEADERS WHERE BRICK IS USED, TO BE: () (UNO)

WALL STUD SCHEDULE

IST 4 2ND FLOOR LOAD BEARING STUDS: 2x4 STUDS = 16" O.C. OR 2x6 STUDS = 24" O.C. IST FLOOR LOAD BEARING STUDS = 24" O.C. IST FLOOR LOAD BEARING STUDS = 16" O.C. DASHTENT LOAD BEARING STUDS: 2x4 STUDS = 12" O.C. OR 2x6 STUDS = 16" O.C. NON-LOAD BEARING STUDS: 2x4 STUDS = 12" O.C. OR 2x6 STUDS = 16" O.C. NON-LOAD BEARING STUDS: (ALL FLOORS): 2x4 STUDS = 24" O.C. TUDS STUDS: 1441 JS. 100 STORY WALLS: 2x4 STUDS = 12" O.C. OR 2x6 STUDS = 16" O.C. BALLOON FRAMED W/ CROSS BRACING = 6'-0" O.C. VERTICALLY

INSTALL ANY REQUIRED HOLDOWNS PER SECTION

R602.10.8 AND FIGURES R602.10.6.5, R602.10.1, R602.10.8(1) AND R602.10.8(2) OF THE 2015 IRC

= DESIGNATES JOIST SUPPORTED LOAD BEARING

NOTE:

WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

JOIST & BEAM SIZES SHOWN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION,

NOTE: MEMBER NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED

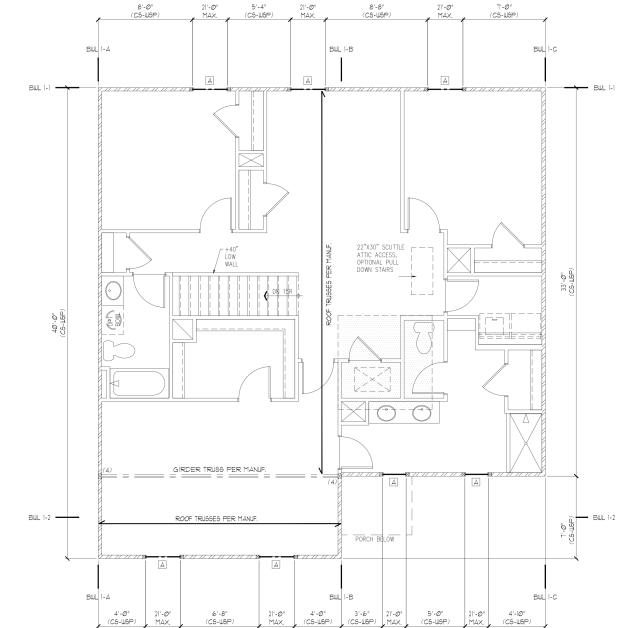
NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS,

TO PREVENT MOISTURE INTRUSION.

ORTED	LOAD	BEARING	WALL.







21'-Ø"

21'-Ø"

SECOND FLOOR FRAMING PLAN - ELEVATION A, B, C

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

SECOND FLOOR FRAMING PLAN

SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"

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NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS,

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JOIST & BEAM SIZES SHOUN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION,

NOTE: = DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE, PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

INSTALL ANY REQUIRED HOLDOUNS PER SECTION R602.10.8 AND FIGURES R602.10.6.5, R602.10.1, R602.10.8(1) AND R602.10.8(2) OF THE 2015 IRC

 IST 4 2ND FLOOR LOAD BEARING STUDS:

 2x4 STUDS = 16" O.C. OR 2x6 STUDS = 24" O.C.

 IST FLOOR LOAD BEARING STUDS = 10" O.C.

 2x4 STUDS = 10" O.C. OR 2x6 STUDS = 16" O.C.

 BASEMENT LOAD BEARING STUDS:

 2x4 STUDS = 10" O.C. OR 2x6 STUDS = 16" O.C.

 NON-LOAD BEARING STUDS:

 2x4 STUDS = 21" O.C. OR 2x6 STUDS = 16" O.C.

 NON-LOAD BEARING STUDS:

 12x4 STUDS = 24" O.C.

 TUDS STUDS:

 12x0 STUDS = 144 15.
 TWO STORY WALLS: 2x4 STUDS © 12" O.C. OR 2x6 STUDS © 16" O.C. BALLOON FRAMED W/ CROSS BRACING © 6'-0" O.C. VERTICALLY

WALL STUD SCHEDULE

ALL HEADERS WHERE BRICK IS USED, TO BE: () (UNO)

SECURE LINTEL TO HEADER w/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED @ 16" O.C. (TYP FOR 3).

| 5x3-1/2"x5/16" ALL ARCHED OPENINGS 4 ROLLED OR EQUIV.

SIZE TAG L3x3x1/4" LESS THAN 6'-0" L5x3x1/4" 6'-0" †0 10'-0" GREATER THAN 3 L5x3-1/2"x5/16" 10'-0"

HEADER SIZES SHOUN ON PLANS ARE MINIMUMS, GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION. ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE. SC NOTED ON PLAN OVERRIDE SC LISTED ABOVE. LINTEL SCHEDULE OPENING SIZE

HEADER SCHEDULE

SI7E

(2) 2x6

(2) 2x8

(2) 2x | 0

(2) 2x|2(2) 9-1/4" LSL/LVL

(3) 2x6

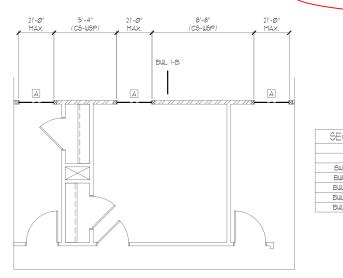
(3)2x8

(3) 2x1Ø

(3) 2xl2

†AG

KING STUD REQUIREMENTS OPENING WIDTH KINGS (EACH END) JACKS (EACH END) LESS THAN 3'-Ø" 3'-Ø TO 4'-Ø" 4'-0" TO 8'-0" 8'-0" TO 12'-0" 12'-Ø" TO 16'-Ø" KING STUD REQUIREMENTS ABOVE DO NOT APPLY TO PORTAL FRAMED OPENINGS

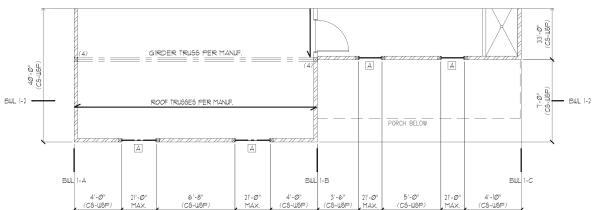


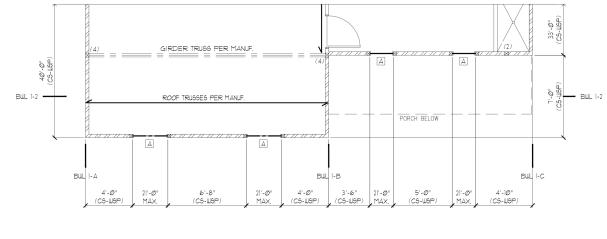
OPT. BEDROOM 4

_			
	SECOND	FLOOR BRA	CING (FT)
	CONTINUOUS SHEATHING METHOD		
		REQUIRED	PROVIDED
	BWL 1-1	6.8	29.0
	BWL 1-2	6.8	13.3
	BWL 1-A	4.9	40.0
	BWL 1-B	4.9	7.0
	BWL 1-C	4.3	33.Ø

		7 - 7 - 7 - 7 - 7 - 7 - 7 - 7 - 7 - 7 -	A	
4'-10" (CS-WSP)	21'-Ø" MAX.	5'-Ø" (CS-WSP)	21'-Ø" MAX.	3'-6" (CS-WSP)







SECOND FLOOR FRAMING PLAN - ELEVATION D

<u>OPT. BATH 1</u>

BWL 1-C

SECOND FLOOR BRACING (FT)			
CONTI	NUOUS SHEATHING M	ETHOD	
	REQUIRED	PROVIDED	
BWL 1-1	6.8	29.0	
BWL 1-2	6.8	13.3	
BWL 1-A	4.9	40.0	
BWL 1-B	4.9	7.Ø	
BWL 1-C	4.3	33.Ø	

- BWL 1-2



ROOF FRAMING PLAN - ELEVATION E & F

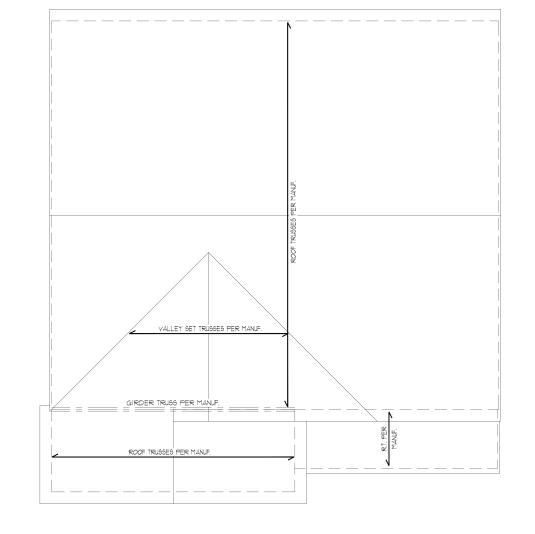
THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>DR. HORTON</u> COMPLETED/REVISED ON <u>25/30/23</u>, IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LADORATORY 4 TESTING, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY 4 TESTING, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, INC. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

NOTE: ROOF TRUSSES SHALL BE SPACE TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

NOTE: 16T PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNO)



ROOF FRAMING PLAN SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"



DESIGN SPECIFICATIONS:

81. Site Class 82. Design Category ... 83. Importance Factor .

Seisnic Use Group . 8.5. Spectral Response Acceleration 85. Seismic Base Shear 861 VX = 8**6**2.Vy =

8.1. Basic Structural System (check one) ⊠ Bearing Wall □ Building Frame □ Moment Frame

8.8. Arch/Mech Components Anchored ... 8.9. Lateral Design Control: Seismic
9. Assumed Soil Bearing Capacity

 Dual w/ Special Moment Frame Dual w/ Intermediate R/C or Special Steel

> Wind 🖂 200005

8 Seismic

Construction Tupe: Commerical 🔲 Residential 🛛

Applicable Building Codes:	
 2018 North Carolina Residential 	

°• 2Ø	le Building Codes: 18 North Carolina Residential Building Code with CE 7-10: Minimum Design Loads for Buildings an	
Design L	oads:	
	Roof Live Loads	
	I.I. Conventional 2x	
	1.2. Truse	20 PSF
	12.1. Attic Truss	_ 60 PSF
2.	Roof Dead Loads	
	2.1. Conventional 2x	
	2.2. Truse	
3.	5now	15 PSF
	3.1. Importance Factor	1.0
4.	Floor Live Loads	
	4.1. Typ. Dwelling	40 PSF
	4.2. Sleeping Areas	
	4.3. Decks	
	4.4. Passenger Garage	
5.	Floor Dead Loads	
	5.1. Conventional 2x	
	52. I-Joist	15 PSF
	5.3. Floor Truss	15 PSF
6.	Ultimate Wind Speed (3 sec. gust)	, PER PL A N
	6.1. Exposure	
	6.2. Importance Factor	
	6.3. Wind Base Shear	
	63.1. Vx =	
	632.Vy =	
٦.	Component and Cladding (in PSF)	

J					
				PSF	
				K I"LAN	
63. VX =					
		in (PSF)			
	UP TO 30'	301"-35'	351"-40	4011-45	
ZONE 1	16.1, - 18.Ø	17.5,-18.9	18.2,-19.6	18.7,-20.2	
ZONE 2	16.7,-21.Ø	17.5,-22.1	18.2,-22.9	18.1,-23.5	
ZONE 3	16.7,-21Ø	17.5,-22.1	18.2, -22.9	18.1,-23.5	
ZONE 4	18.2, - 19.Ø	19.2,-20.0	19.9,-2 0 .1	20.4,-21.3	
ZONE 5	182,-24Ø	192,-252	19.9,-26.1	20.426.9	
	Floor Live LC 41. Typ. Du 42. Sleeping 43. Decks 44. Passen Floor Dead L 51. Conver 53. Floor T Uttimate Und 63. Und 6	Floor Live Loads 41. Typ. Duelling	Floor Live Loads 41. Typ. Duelling	41. Typ. Duelling 40 42. Sleeping Areas 30 43. Decks 40 44. Passenger Garage 50 Floor Dead Loads 50 51. Conventional 2x 10 F 52. I-Jolat 15 F 53. Floor Truss 15 F 61. Exposure 10 F 63. Wind Base Shear 63. Wind Base Shear 63. Wind Base Shear 63. Vy = 63. Wind Ease Shear 63. Vy = Component and Cladding (in PSF) MEAN ROOF MEAN ROOF 115.78.8 182.78.6 ZONE I 16.1.79.00 11522.1 182.72.9 ZONE 3 16.1.200 11522.1 182.72.9	Floor Live Loads 40 PSF 41. Typ. Duelling 40 PSF 42. Sleeping Areas 30 PSF 43. Decks 40 PSF 44. Passenger Garage 50 PSF Floor Dead Loads 10 PSF 51. Conventional 2x 15 PSF 15 PSF 52. I-Joits 15 PSF 15 PSF 53. Floor Truss 16 PSF 61. Exposure 16 63 Ukrd Base 3Peat 63. Ukrd Base 3Peat 63 Ukrd 63 Ukrd 16 63. Ukrd Base 3Peat 632. Vy = 20 20 115-720 120 115-720 120.7235 20 18.7-202 ZONE 1 16.7.180 115-721 182-723 18.7-235 20 18.7-235 20 18.7-235

SUMMIT

SHEET LIST: Description Sheet No. CGI Cover Sheet Specifications Revision Dim Monolithic Slab Foundation Details Dla Stem Wall Foundation Details Dlc Craul Space Foundation Details DЬ Basement Foundation Details DIF Framing Details

STANDARD DETAILS OUNER: DR Horton Carolinas Division

8001 Arrowridge Blvd Charlotte, NC 28213

STRUCTURAL PLANS PREPARED FOR

ARCHITECT/DESIGNER GMD Design Group 1845 Satellite Blvd Duluth GA 3009

PROJECT ADDRESS:

TBD

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory 4 Testing, P.C. before construction begins.

PLAN ABBREVIATIONS:

AВ	ANCHOR BOLT	PŤ	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	SC	STUD COLUMN
CLR	CLEAR	5J	SINGLE JOIST
ÐJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
D5P	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
Ē	EACH END	S YP	SOUTHERN YELLOW PINE
EΨ	EACH WAY	ŤJ	TRIPLE JOIST
NT9	NOT TO SCALE	TSP	TRIPLE STUD POCKET
8	ON CENTER	TYP	TYPICAL
P#F	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
PSI	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by <u>DR Horton. Inc.</u> Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify $\$U^{HHI}$ immediately.

REVISION LIST: **Re**vision Project No. Date Description No. E IIIT Added box bay detail (2/D2f). Added deck options with basement. Revised deck options with stem wall and crawl space foundations 2 7,12,17 Revised stem wall insulation note 3 2.15.18 Revised garage door detail, NC only 4 2.28.18 Added high-wind foundation details 5 12.19.18 Revised per 2018 NCRC 6 2.19.19 Revised per Mecklenburg County Comments Revised stem wall deck attachment and i sheathing on wall sections. 8 3.6.19 Corrected dimensions at perimeter footings 9 3220 Added tall turndown detail 10 3.18.20 Added balloon framing detai Added alternate two-pour detail for slab and 102020 added note for crawl girder above grade 3121 12 Added OX-19 Standard Details 13 5.18.21 Updated OX-15 Standard Details 14 @2.14.23 Added 4/D2m - Tall Slab Detail w/ Siding

GENERAL STRUCTURAL NOTES:

- The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) or the SER. For t purposes of these construction documents the SER and SUMMIT
- shall be considered the same entity. The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction 2 to stabilize the structure.
- The SER is not responsible for construction sequences, methods, or techniques in comection with the construction of this structure. The SER will not be held responsible for the solutions in our on the contract documents, should any non-conformities occur. Any structural elements or details not fully developed on the
- construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions,
- is stop crawings to comprise or or summarized controller, is not the responsibility of the SER or SUMMIT. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins. The SER is not responsible for any secondary structural elements
- or non-structural elements, except for the elements specifically noted on the structural drawings. This structure and all construction shall conform to all
- applicable sections of the international residential code.
- This structure and all construction shall conform to all applicable sections of local building codes.
 All structural assemblies are to meet or exceed to requirements
- of the current local building code

FOUNDATIONS:

The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.

- The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade. 6.
- Any fill shall be placed under the direction or recommendation
- of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95%
- maximum div density. Excavations of footings shall be lined temporarily with a 6 mil polysthylene memorane if placement of concrete does not occur within 24 hours of excavation.
- No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.

- <u>STRUCTURAL STEEL:</u>
 1. Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Fractice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design latest editions.
- Structural steel shall receive one coat of shop applied rust-inhibitive paint. 3. All steel shall have a minimum yield stress (F_{μ}) of 36 ksi unless
- otherwise noted. Welding shall conform to the latest edition of the American
- Weiding shall conform to the latest edition of the American Weiding Society's Structural Weiding Code AWS D.I., Electrodes for shop and field weiding shall be class ETØXX. All weiding shall be performed by a certified weider per the above

CONCRETE:

- NUMBER: Concrete shall have a normal weight aggregate and a minimum compressive strength (Fp) at 28 days of 3000 psi, unless otherwise noted on the plan. Concrete shall be proportioned, mixed, and placed in
- accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- Air entrained concrete must be used for all structural elements exposed to freeze/thau cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of
- target values as follows: 3.1. Footings: 5% 3.2. Exterior Slabs: 5%
- 4. No admixtures shall be added to any structural concrete without written permission of the SER.

- Concrete slabs-on-grade shall be constructed in accordance 5 uith ACI 302.IR-96: "Guide for Concrete Slab and Slab Construction"
- The concrete slab-on-arade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported
- conditions not in accordance with the above assumptions. Control or saw cut joints shall be spaced in interior slabs-on-orade at a maximum of 15'-0" O.C. and in exterior
- slabs-on-grade at a maximum of 10'-0" unless otherwise noted. 8 Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
- Reinforcing steel may not extend through a control joint.
 Reinforcing steel may extend through a control joint.
 Reinforcing steel may extend through a saw cut joint.
 I/I welded uire fabric (UWE) for concrete slabs-on-grade shall be placed at mid-depth of slab. The UWE, shall be securely supported during the concrete pour.

- <u>CONCRETE REINFORCEMENT:</u> I. Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- Thermosh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- Application of fibermesh per cubic yard of concrete shall equal a minimum of 20% by volume (15 pounds per cubic yard) Fibermesh shall comply with ASTM CIII6, any local building code requirements, and shall meet or exceed the current industry 4
- standard. Stæel reinforcing bars shall be new billet steel conforming to
- 6
- ASTM A65, grade 60. ASTM A65, grade 60. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Nanual of Standard Practice for Detailing Concrete Structures" Horizontal footing and wall reinforcement shall be continuous and shall have 30° bends, or comer bars with the same are for acless as the borgent at the class B size/spacing as the horizontal reinforcement with a class B
- tension splice. Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.

- Where reinforcing dowels are required , they shall be equivalent in size and spacing to the vertical reinforcement. The doule shall extend 48 bar diameters vertically and 20 bar diameters
- into the footing. 10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.
- Solid saun wood framing members shall conform to the specifications listed in the latest edition of the "National
- Design Specification for Wood Construction" (NDS), Unless otherwise noted, all wood framing members are designed to be
- Spruce-Yellow-Pine (SYP) 2. LVL or PSL engineered wood shall have the following minimum

- 2.4.Fc = 100 psi
- Wood in contact with concrete, masony, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance
- Nails shall be common wire nails unless otherwise noted.
- specifications. All beams shall have full bearing on supporting framing members
- unless otherwise noted. Exterior and load bearing stud walls are to be 2x4 SYP #2 # 16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- King studs shall be continuous. Individual studs forming a column shall be attached with one l&d nall # 6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all filor levels to ensure proper load transfer.
- Multi-ply beams shall have each ply attached with (3) 10d nails @ 24" O.C.
- 10. Flitch beams, 4-ply beams and 3-ply side loaded beams shall be Inter beams, + py beams and ppg side back to be the bolted together with (2) rous of 12^n diameter through bolts staggered = 16" OC. unless noted otherwise. Min. edge distance shall be 2" and (2) bolts shall be located a min. 6" from each end of the beam

WOOD TRUSSES:

- <u>QOD TRUSES</u>. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overail compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses shall be designed for all required loadings a specification to the designed for all required loadings
- 2 Ine wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 1-05), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to
- Hrve expension, provide a statement of the trusses. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction" (NDS) and "Design Specification for Metal Plate Connected Wood Trusses." 3.
- 4. The truss manufacturer shall provide adequate bracing Instruiss manufacturer sharp provide adequate cracing information in accordance with "Commentary and Recommendations for Handling, installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments fo
- the trusses.
 Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacture

EXTERIOR WOOD FRAMED DECKS:

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

2

- UCOD STRUCTURAL PANELS: I. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA
- All structurally required wood sheathing shall bear the mark of the APA

WOOD FRAMING:

- ign values: 2.1. E = 1,900,000 psi
 - - 2.2. F_b = 2600 psi 2.3. F_v = 285 psi
 - with AWPA standard C-2
 - Lag screws shall conform to ANSI/ASME standard B182.1-1381. Lead holes for lag screws shall be in accordance with NDS

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Development

	DR HORION PROJECT SIGN-OFF:			
I	Manager	Signature		
	Operations			
	Operations System			
I	Operations Product			

SUCCESSION OF A STATE
CLENT: DR Herton Carolina Division 2001 Arrowidge Bivd. Charlotte, NC 20013
PROJECT: Blandard Details (OX-IB) COVERSHEEt
H CARA
ALLE TOAL OF THE TABLE
DRAIN BY, JCEF CHECKED BY, BCP ORIGNAL INFORMATION PROJECT * DATE US/2001 REFER TO COVER SHEET ROR A COMPLETE LIST OF REVISIONS
CSI

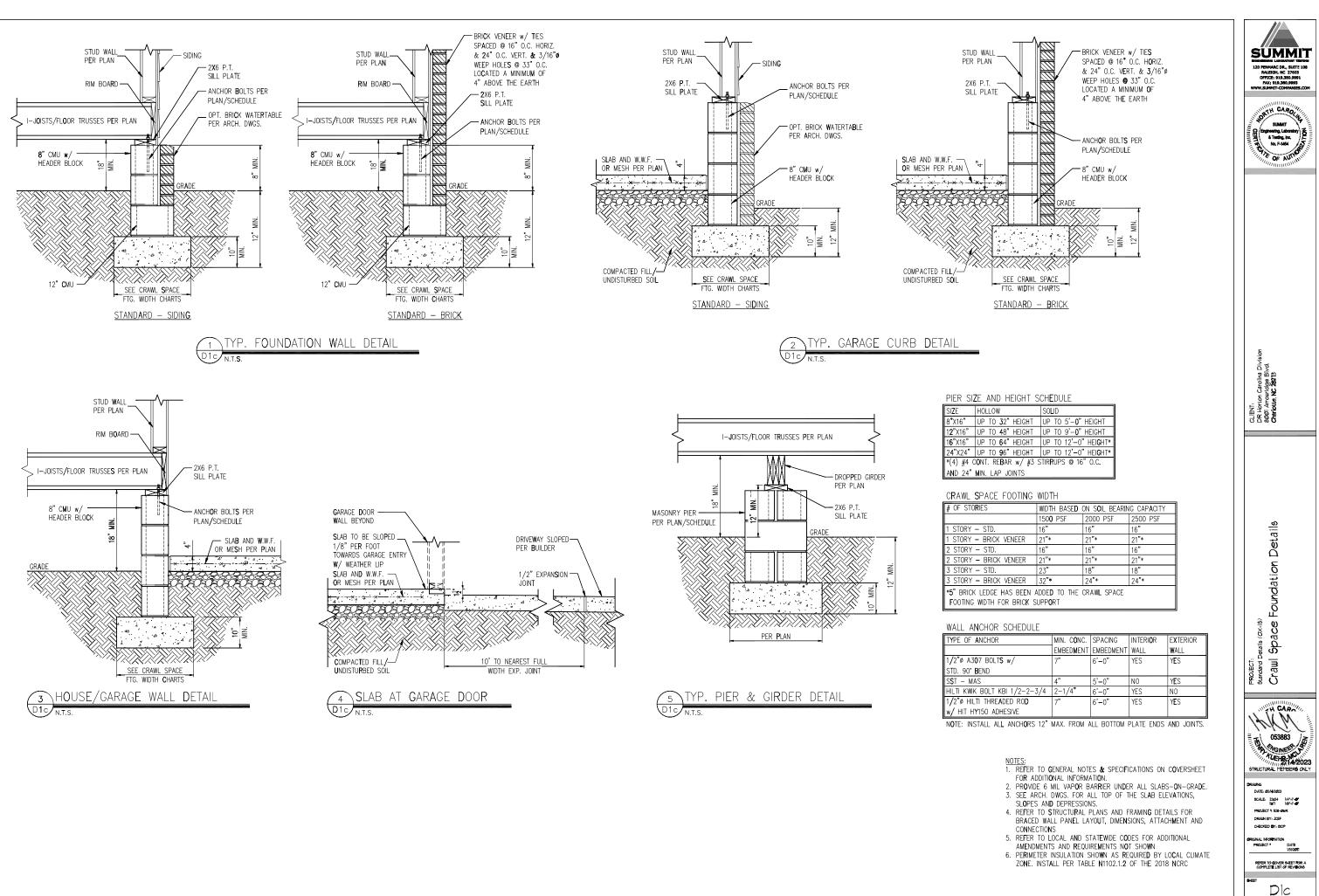
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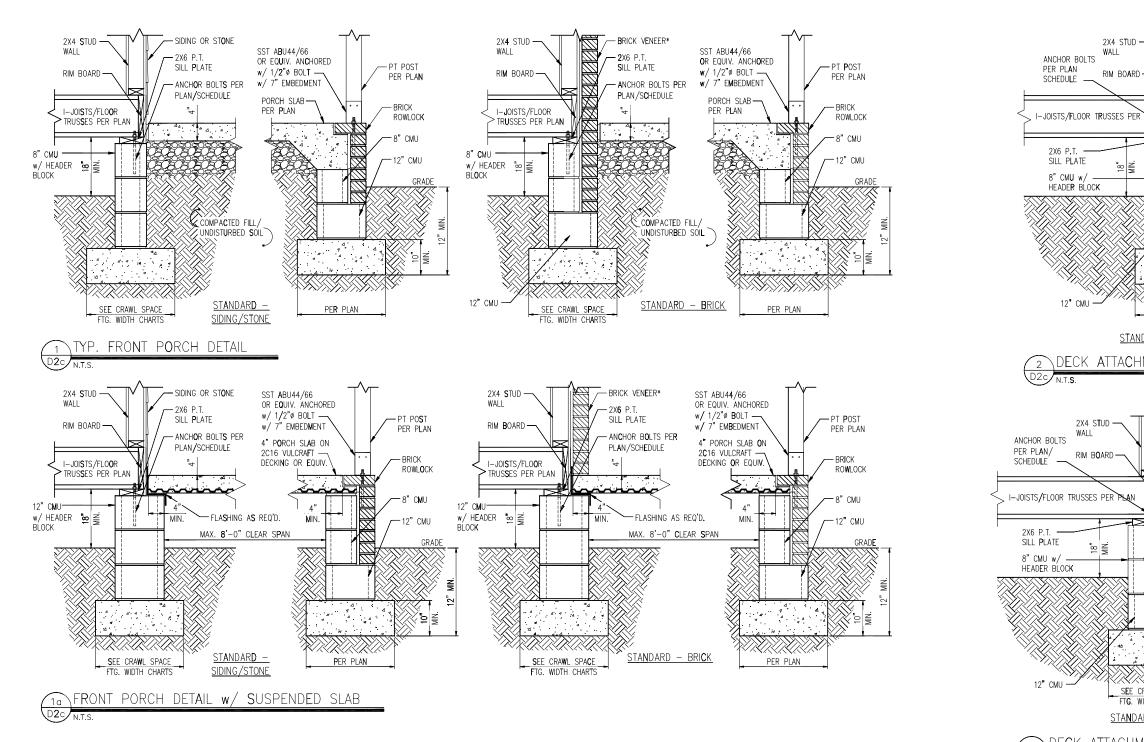
- Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information, theathing shall be applied with the long direction perpendicular to framing, unless noted otherwise. Roof sheathing shall be APA rated sheathing exposure 1 or 2.
- Roof sheathing shall be continuous over two subports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use
- support by use of T4C plywood or lumber blocking unless otherwise noted. Panel and joints shall occur over framing. Apply building paper over the sheathing as required by the
- state Building Code. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

<u>STRUCTURAL FIBERBOARD PANELS:</u> I. Fabrication and placement of structural fiberboard sheathing

- shall be in accordance with the applicable AFA standards All structurally required fiberboard sheathing shall bear the mark of the AFA.
- Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- Sheathing shall have a 1/8" gap at panel ends and edges are

have a span rating consistent with the framing spacing, Use suitable edge support by use of plyucod clips or lumber blocking unless otherwise noted. Panel and joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code. Wood floor sheathing to its supporting framing with (1)-8d CC ringshank nail at 6°/or at panel edges and at 2°/or in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall be applied perpendicular to framing. Sheathing shall be applied perpendicular to framing. Sheathing shall be applied perpendicular to framing sheathing shall be applied perpendicular to framing sheathing shall be applied perpendicular to framing. Sheathing and the edge support buse of TK of bluocod or lumber blocking unless





	DECK ATTACHMENT	SCHEDULE (A	ALL STRUCTURES	FXCEPT BRICK)
--	-----------------	-------------	----------------	--------------	---

MAX. 8'-0" JOIST	MAX. 16'-0" JOIST
SPAN	SPAN
(1) @ 3'-6" 0.C.	(1) @ 1'-8" O.C.
AND	AND
(2) @ 8" 0. C .	(3) @ 6" O.C.
	SPAN (1) @ 3'-6" O.C. AND

a. ATTACHMENT INTERPOLATION BETWEEN 8' AND 16' JOIST SPANS IS ALLOWED.

b. MINIMUM EDGE DISTANCE FOR BOLTS IS 21".

c. NAILS MUST PENETRATE THE SUPPORTING STRUCTURE BAND A MINIMUM OF $1\frac{1}{2}$ "

DECK ATTACHMENT SCHEDULE (BRICK STRUCTURES)

FASTENERS	MAX. 8'-0" JOIST	MAX. 16'-0" JOIST
	SPAN	SPAN
5/8" GALV. BOLTS w/ NUT & WASHER ^b	(1) @ 2'-4" 0.C.	(1) @ 1'-4" O.C.

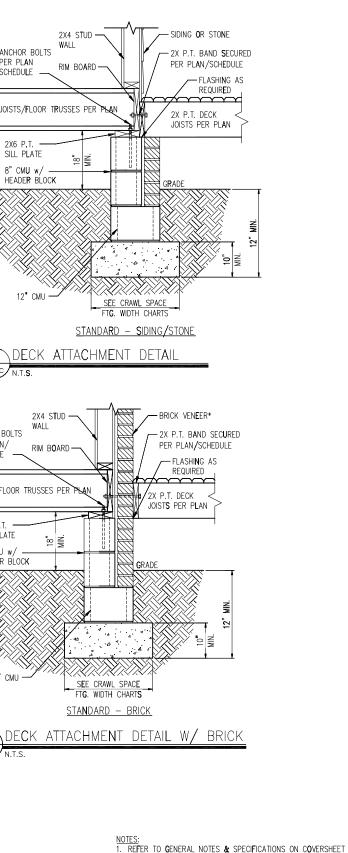
a. ATTACHMENT INTERPOLATION BETWEEN 8' AND 16' JOIST SPANS IS ALLOWED.

b. MINIMUM EDGE DISTANCE FOR BOLTS IS 21".

CRAWL SPACE FOOTING WIDTH

# OF STORIES	WIDTH BASED	ON SOI L BEARIN	ig capa c ity
	1500 PSF	2000 PSF	2500 P S F
1 STORY - STD.	16"	16"	16"
1 STORY - BRICK VENEER	21"*	21"*	21"*
2 STORY - STD.	16"	16"	16"
2 STORY - BRICK VENEER	21"*	21"*	21"*
3 STORY - STD.	23"	18"	18"
3 STORY - BRICK VENEER	32"*	24"*	24"*
*5" BRICK LEDGE HAS BEEN / FOOTING WIDTH FOR BRICK S		CRAWL SPACE	

*BRICK TIES SPACED @ 16" O.C. HORIZ. & 24" O.C. VERT. AND 3/16"Ø WEEP HOLES @ 33" O.C. LOCATED A MINIMUM OF 4" ABOVE THE EARTH

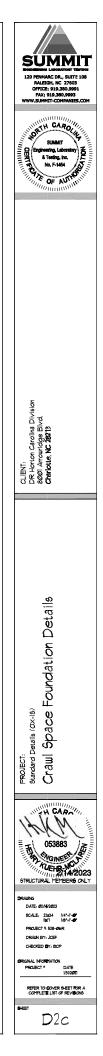


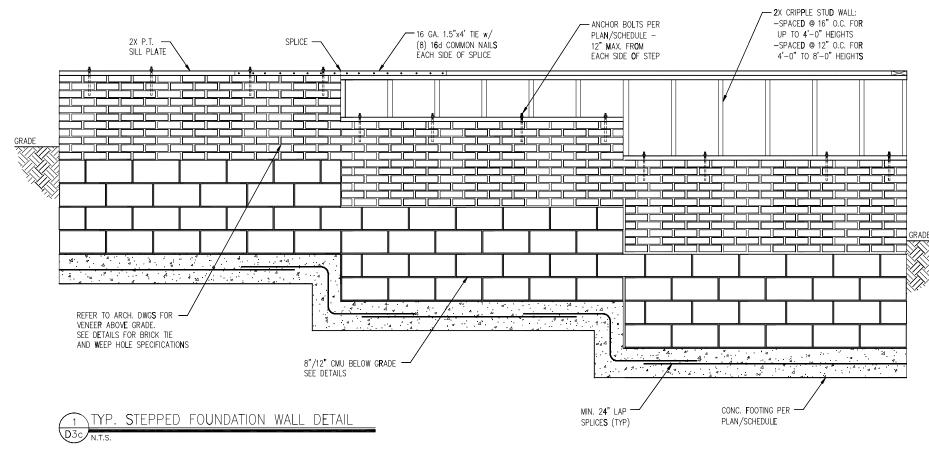
FOR ADDITIONAL INFORMATION.

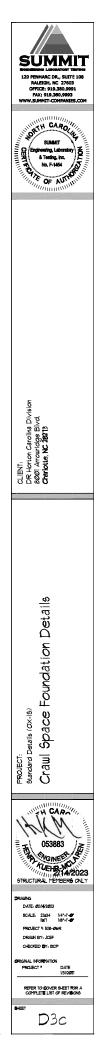
D2c

NTS

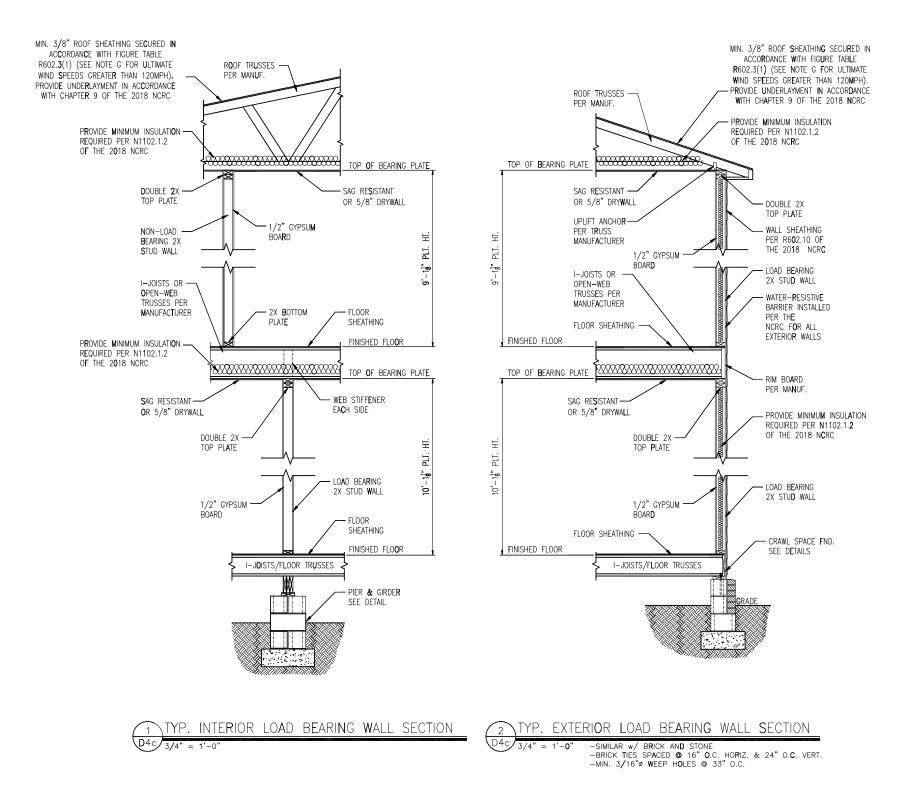
- PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE. . SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.
- 4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND CONNECTIONS
- 5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN
- 6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC

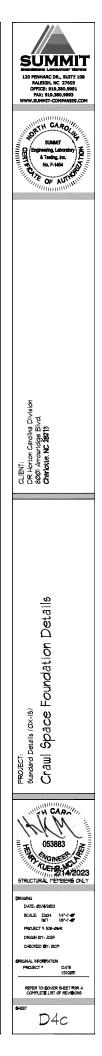




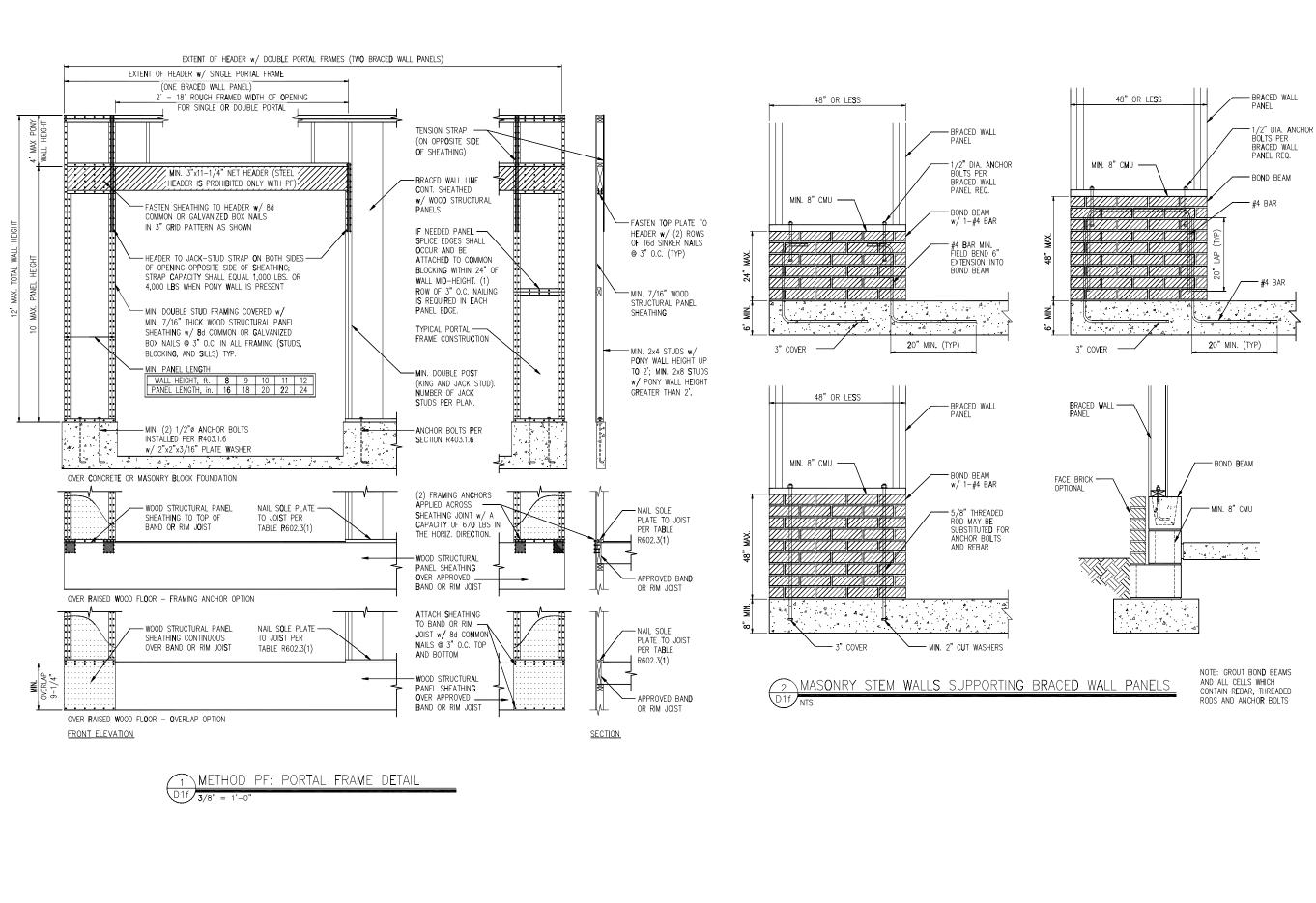


- NOTES: 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET FOR ADDITIONAL INFORMATION.
- PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
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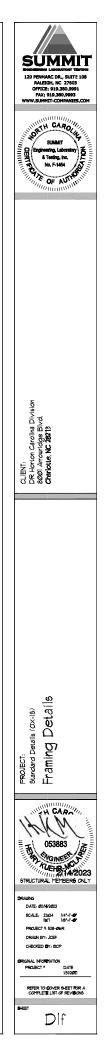


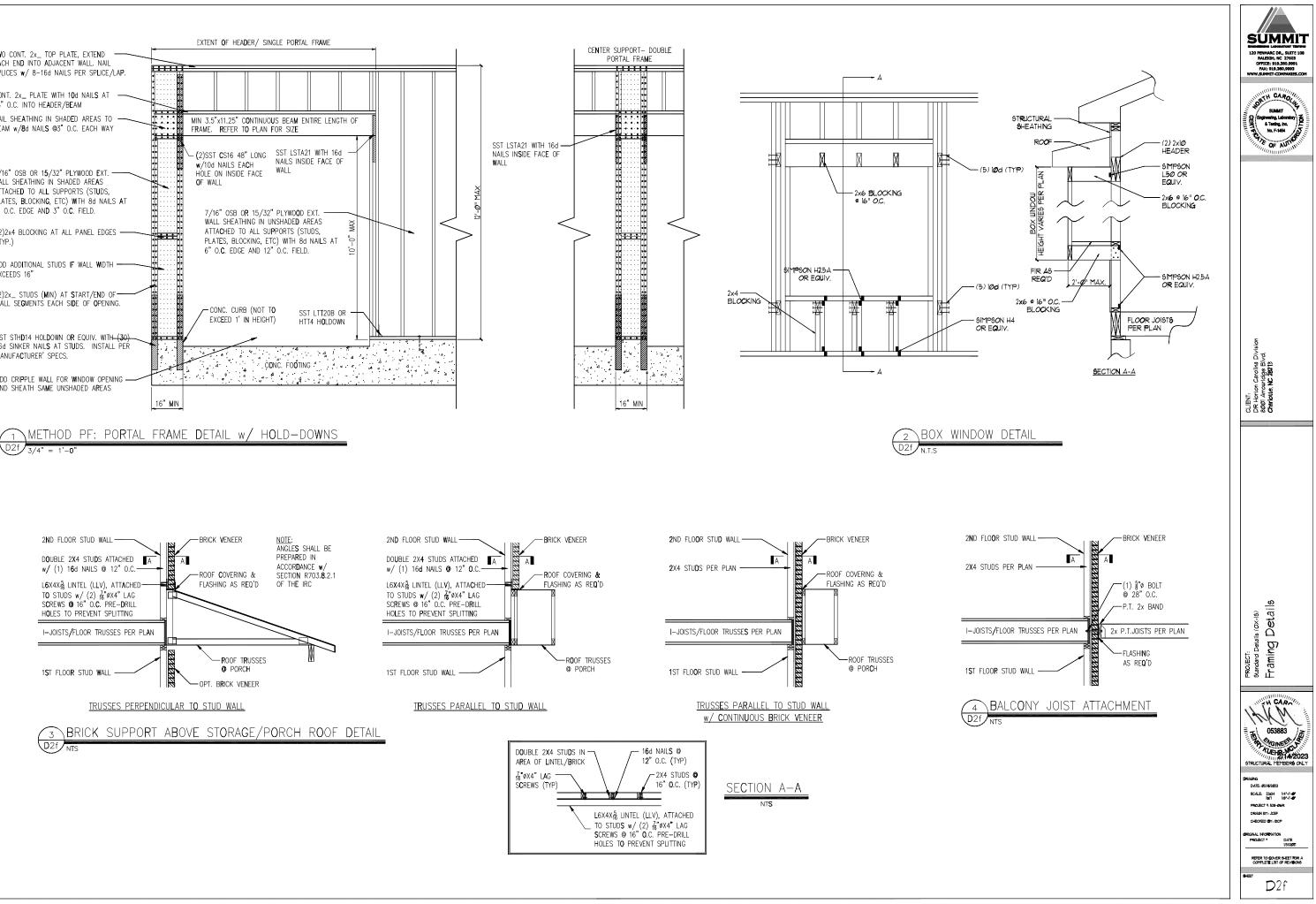


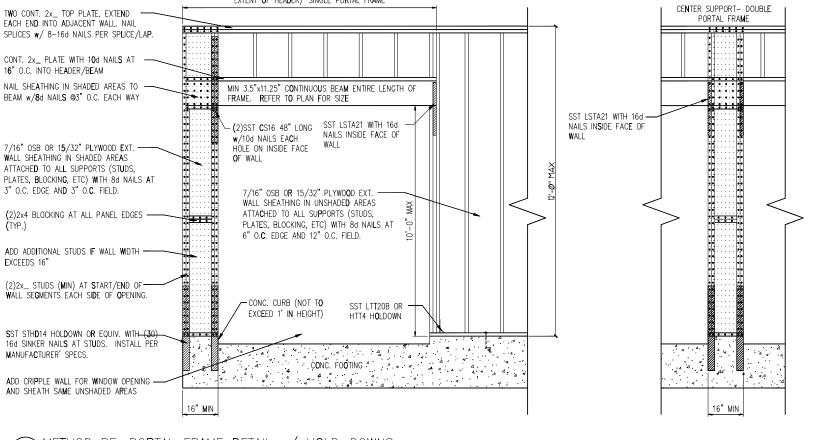
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- CONNECTIONS 5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN
- 6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC

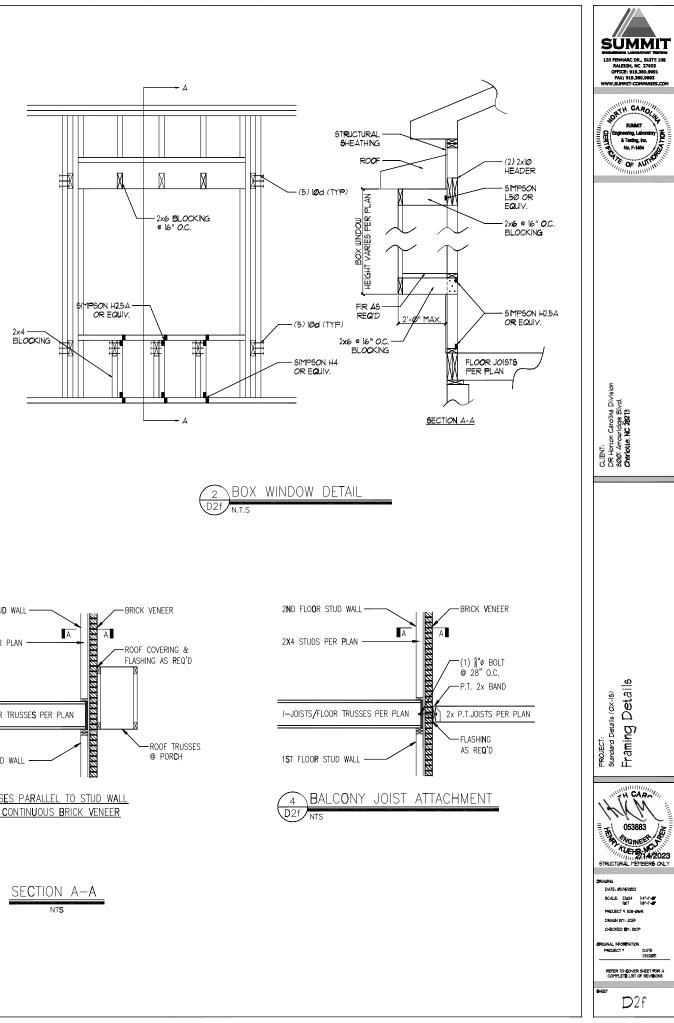


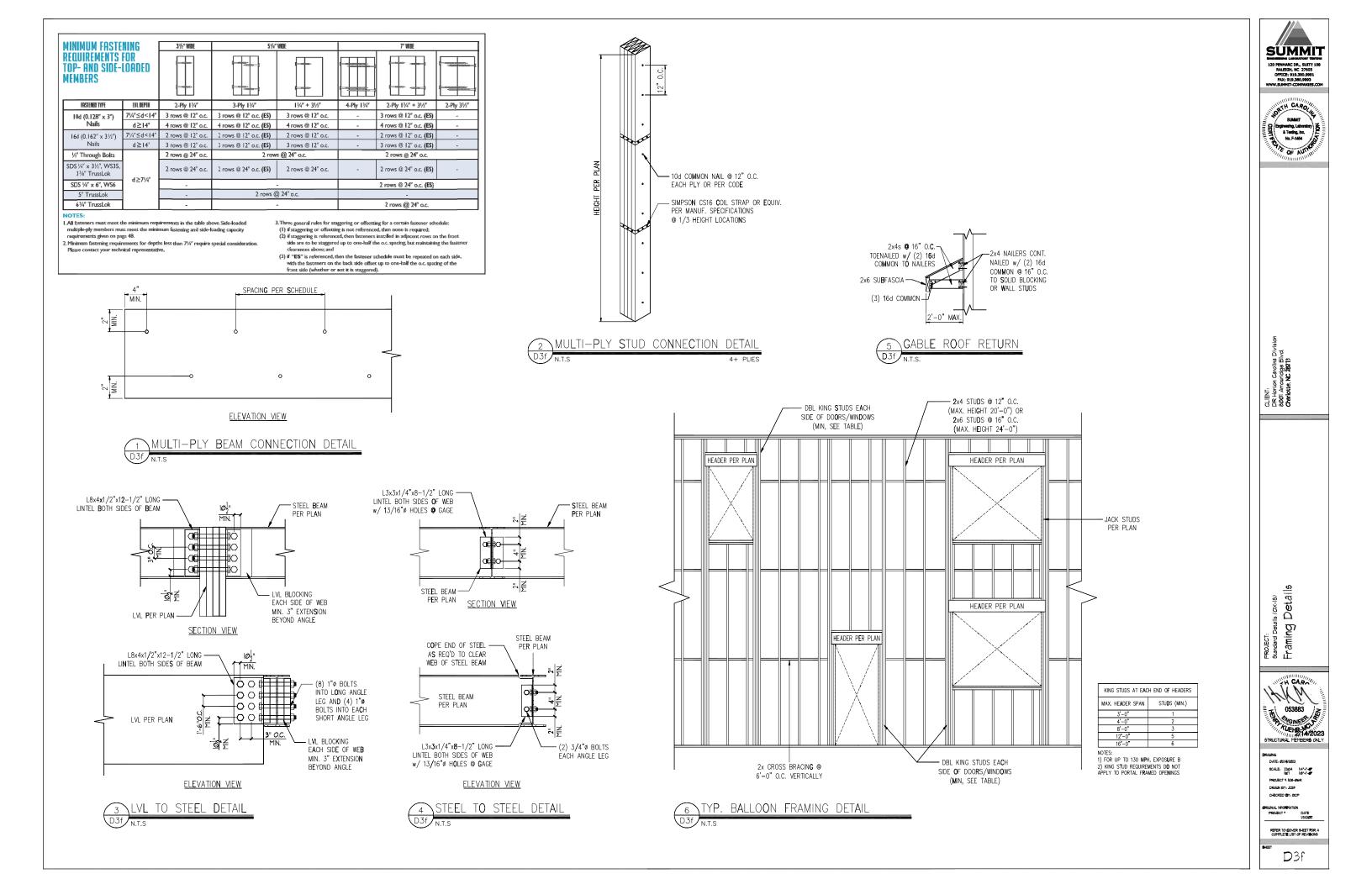


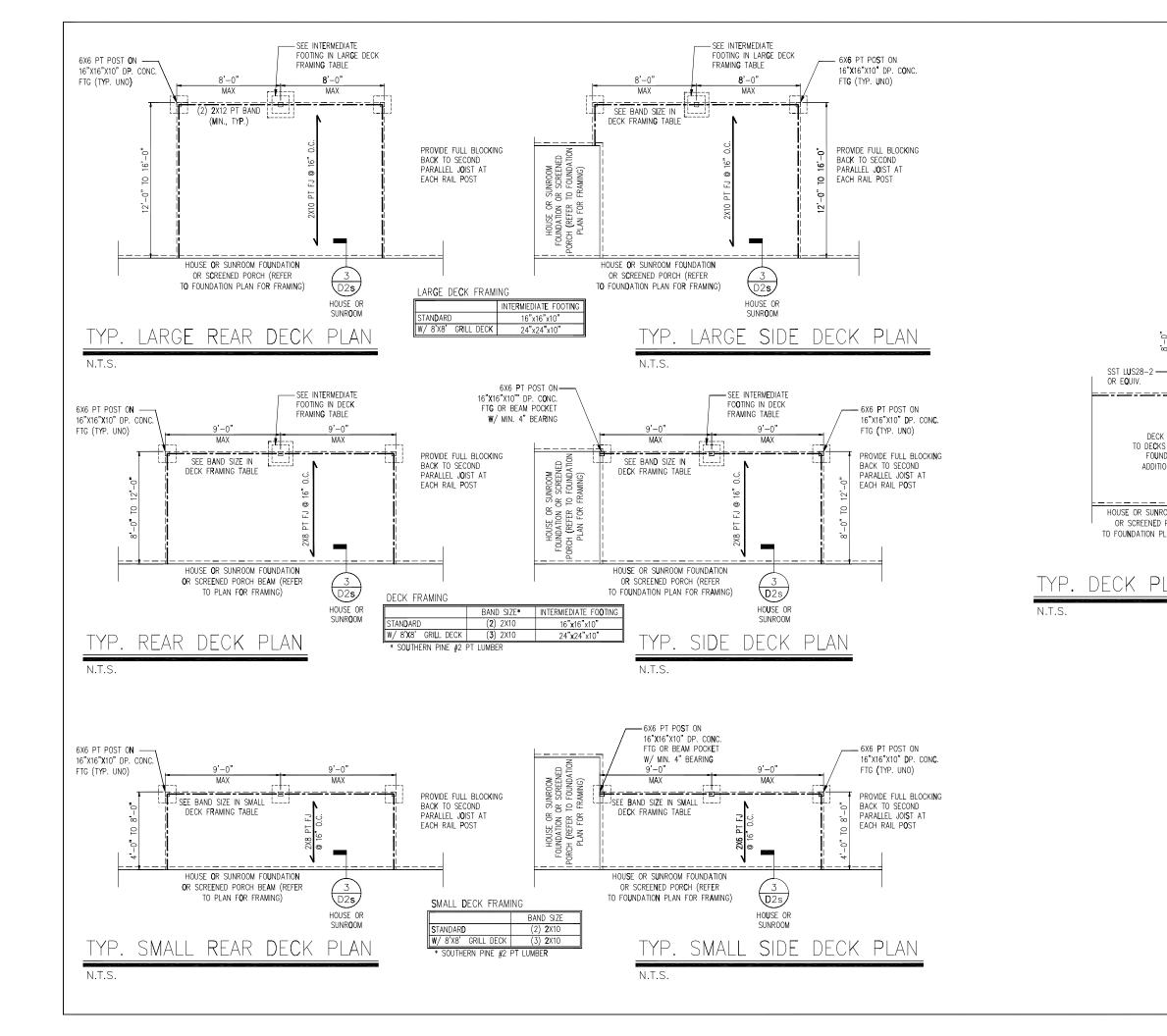


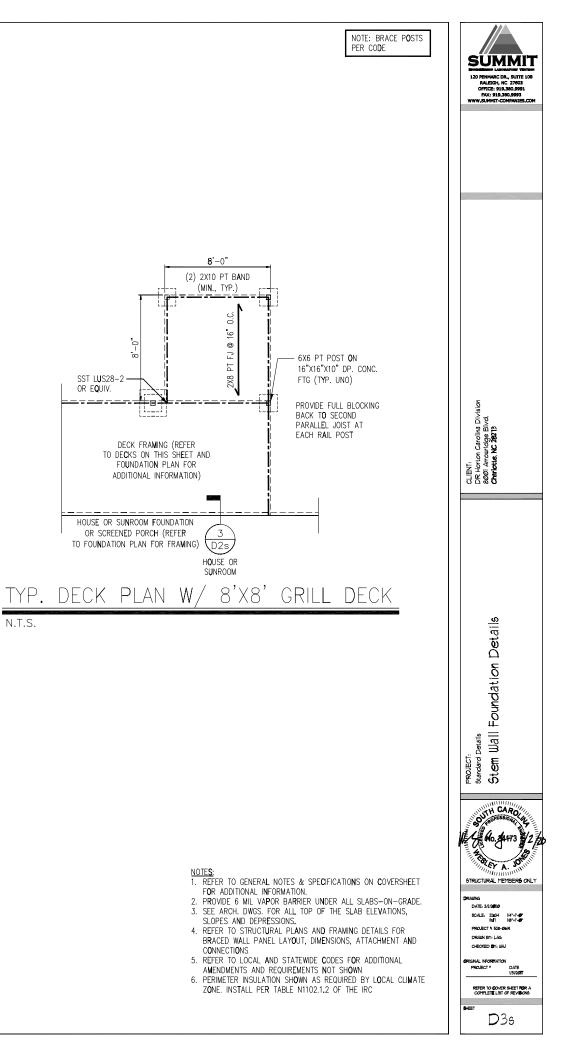




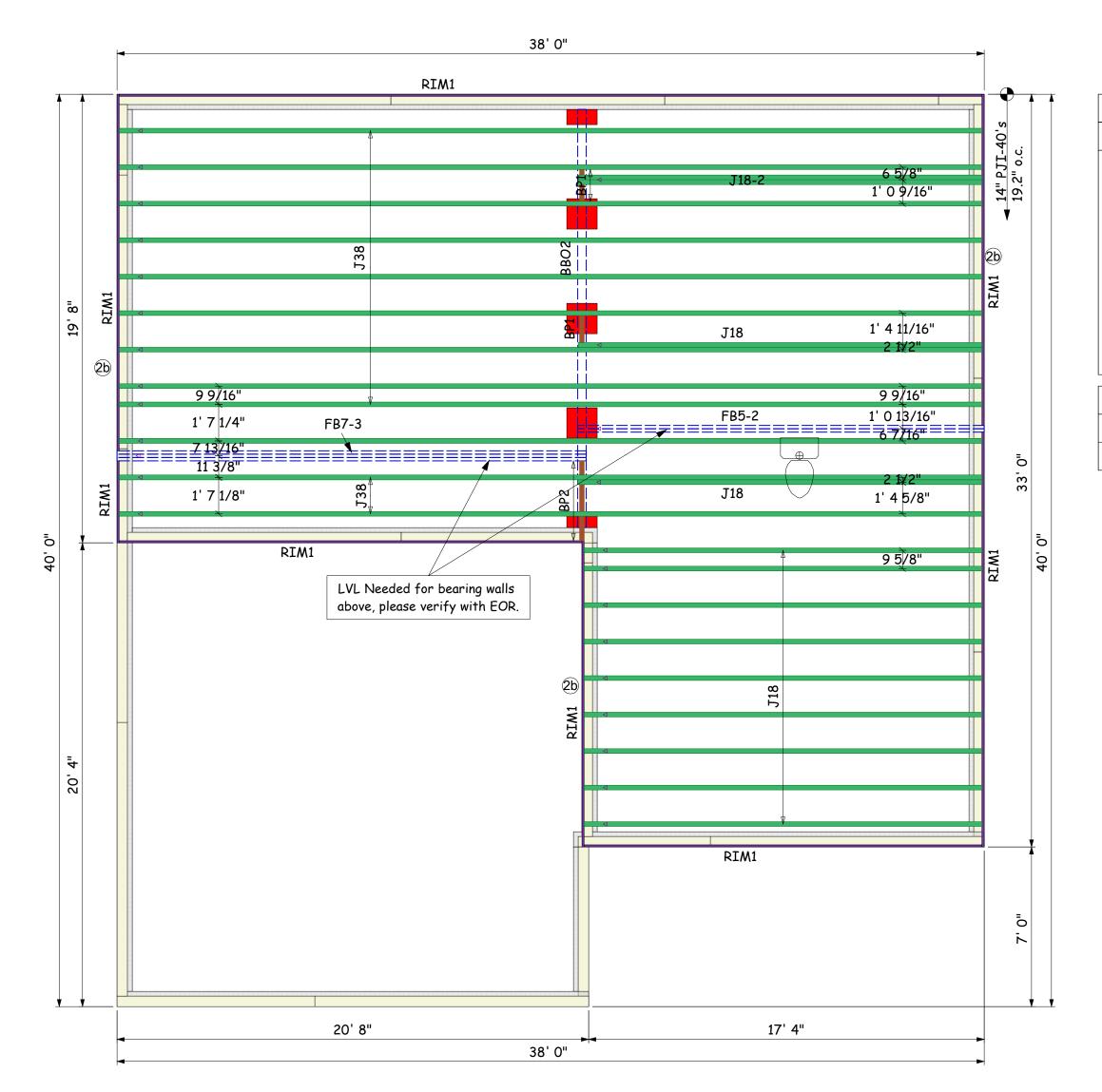












BLOCKING PANELS OF

PJI I-JOIST MAX VERTICAL LOAD TRANSFER 2000PLF

2d

DOUBLE SQUASH BLOCK

LUMBER SQUASH BLOCKS TO BE CUT År" HICHER THAN I-JOIST MINIMUM 1-2x4 SQUASH BLOCK

ELOCKING PANELS ONLY REQUIRED FOR LATERAL STABILITY WHERE FLOOR JOISTS ARE NOT CONTINUOU OVER SUPPORT, REFER TO DETAIL 2G.

.ous Me"

 SOUASH BLOCK
 (In)

 1"x 3 1/2" APA RIM*
 1900

 1"x 5 1/2" APA RIM*
 3000

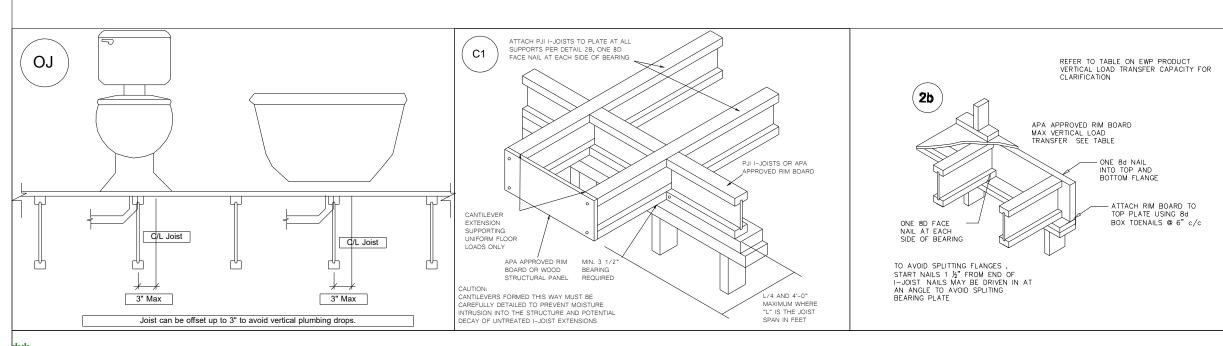
 1 1/8" x 5 1/2" APA RIM*
 2600

 1 1/8" x 5 1/2" APA RIM*
 4000

 1 -2>4
 1900

PROVIDE LATER BRACING AS PER DETAIL 20, 26 & 2c

LUMBER SQUASH BLOCKS TO BE CUT 为6"HIGHER THAN I-JOIST



** PLUMBING DROPS NOTED ARE IN APPROXIMATE LOCATIONS PER PLAN. BUILDER MUST VERY LOCATIONS BEFORE SETTING JOISTS.

		Produ	ucts			
PlotID	Length	Product			Plies	Net Qty
J38	38' 0"	14" PJI-40			1	12
J18	18' 0"	14" PJI-40			1	11
J18-2	18' 0"	14" PJI-40			2	2
FB7-3	22' 0"	2.1 RigidLam SP	LVL 1-3	/4 × 14	3	3
FB5-2	18' 0"	2.1 RigidLam SP	LVL 1-3	/4 × 14	2	2
RIM1	12' 0"	1 1/8" x 14" APA	A Rim Bo	ard	1	12
BP1	2' 0"	14" PJI-40			1	2
BP2	2' 0"	14" PJI-40			1	2
		Accessories				
PlotID	Length	Product	Plies	Net Qt	У	
		3/4" 4x8 OSB	1	31		

TYPICAL PJI I-JOIST MAX VERTICAL LOAD TRANSFER 2000 PLF MAX 18" JOIST DEPTH REFER TO TABLE ON EWP PRODUCT VERTICAL LOAD TRANSFER CAPACITY FOR CLARIFICATION 8d NAILS AT 6"c/c-TO TOP PLATE PJI BLOCKING PANEL PER 20 DETAIL CK/ PJI JOIST ATTACHMENT —/ PER 25 DETAIL INTERIOR SUPPORT BLOCKING REQUIRED OVER ALL INTERIOR SUPPORTS UNDER LOAD BEARING WALLS OR WHERE FLOOR JOISTS ARE NOT CONTINIOUS OVER SUPPORT IN HIGH SEISMIC AREAS (SDC Do, Dr & D₂) THE IRC REQUIRES BLOCKING AT ALL INTERMEDIATE SUPPORTS THE IBC REQUIRES BLOCKING AT ALL SEISMIC DESIGN CATEGORIES

PJI I-JOIST BLOCKING REQUIRED BELOW ALL INTERIOR SUPPORTS

INTERIOR -SUPPORT

2g

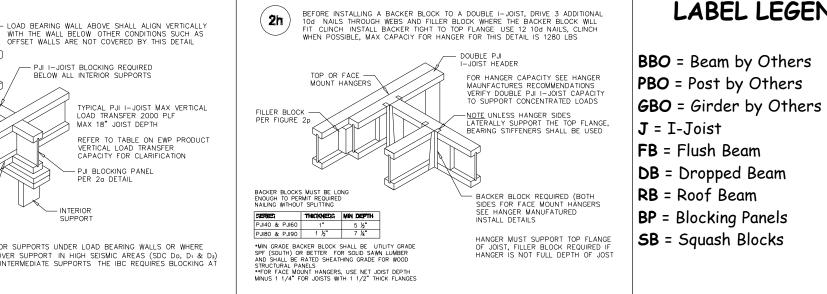
TRANSFER LOAD FROM ABOVE TO BEARING BELOW INSTALL SQUASH BLOCKS PER 2d MATCH BEARING AREA OF BLOCKS IN RIM CAVITY TO POST ABOVE EXAMPLE 3-2x6 POST REQUIRES 3-2x6 SQUASH BLOCKS

** ALL POINT LOADS FROM ABOVE MUST BE TRANSFERRED TO BEARING FROM UNDER SIDE OF SHEATHING.

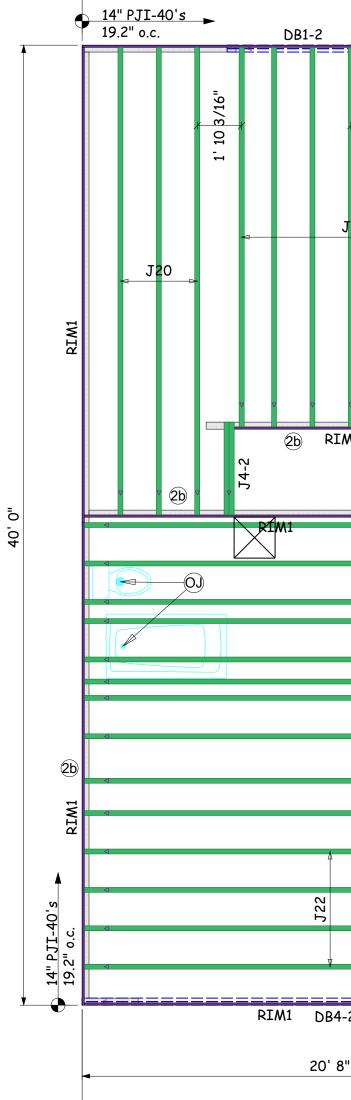
20

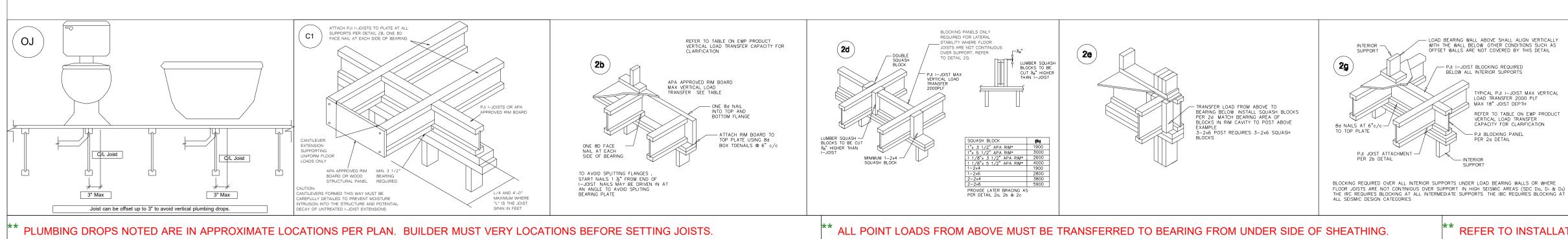
Revisions 00/00/00 Name Its is in any constant in the instant on the state in the instate in the instate in the instant on the state in the in	** DAMAGED FLOOR JOISTS SHOULD NOT BE I	DAMAGED FLOOR JOISTS SHOULD NOT BE INSTALLED UNLESS APPROVED BY COMPONENT PLANT.	** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH.	** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS.	IS.
O0/00/00NameName00/00/00Na					
All	Date: Desig	DR Horton			00/00 00/00 00/00
Algorithm	// 06 ner: ct #: 2	38 Mason Ridge			0/00 0/00 0/00
P 1 0 P 1 0	5/11/ DW 2406	Galen E	A Bit days adding Materials		N N N
	24 0040	FLOOR JOIST LAYOUT	Certer Lumber Company	C	lame lame lame lame

LABEL LEGEND









** PLUMBING DROPS NOTED ARE IN APPROXIMATE LOCATIONS PER PLAN. BUILDER MUST VERY LOCATIONS BEFORE SETTING JOISTS.

DB1-2		RIM1		2b	0)B2-2											.					
					<u>₩</u> =:		==1														oducts	
	= =	3/16"						16"	" 9"									PlotID	Length	Product		
	378	10 3/	¥					10 3/16"	4 3/16"	r								J38	38' 0"	14" PJI-40		
	► 8	1'1						1'1	 									J22	22' 0"	14" PJI-40		
		2																J20	20' 0"	14" PJI-40		
	T16	\geq								т1	۷							J18	18' 0"	14" PJI-40		
	J16	E	*			4				J1	.0							J16	16' 0"	14" PJI-40		
				J20 ⊲ ⊸ ⊳												RIM1		J4-2	4' 0"	14" PJI-40		
																æ		DB1-2	8' 0"	2.1 RigidLam SP		
																		DB3-2	8' 0"	2.1 RigidLam SP	LVL 1-3	3/4 x 9-1
													0.01					DB2-2	6' 0"	2.1 RigidLam SP	LVL 1-3	3/4 x 9-1
							BBO1						BP1 ⊽		E	>		DB4-2	22' 0"	2.1 RigidLam SP	LVL 1-3	3/4 x 11-7
						BP1			>			\sum				5		RIM1	12' 0"	1 1/8" × 14" APA	A Rim Bo	oard
2b RI	M 1		J4-2		4	4				ŝ	Å	1' 4	4 3/16"			33, C		BP1	2' 0"	14" PJI-40		
			J4	, 2b		4				.118	1 2		<u>+</u>							Accessories		
۸1	•••••••••••••••••••••••••••••••••••••••					4					¥	9	9/16"				0	PlotID	Length	Product	Plies	Net Q
										0	J						40'			3/4" 4x8 OSB	1	43
									(\bigcirc								K	EMPSVILLE	BUILDING MATER	IALS IS	NOT RES
			9 5	/8"												2b				OR CALCULATION		
					BP1											-9				TERIAL. ALL ENGIN IS TO BE PROVIDE		
	J38		11 1													RIM1				PPROVED SET OF PL		
			8 1	8"		$\mathbf{\mathbf{X}}$										~				E ENGINEERING RE		
			`	-										$ \uparrow)$						BE PROVIDED BY E		
			1' 10 3	8/16"									(OJ		, ∖					PATTERNS. BUILD		
			1' 4 3	/16"														LE	ENGTHS, Q	UANTITIES, AND S	SIZES PR	RIOR TO C
	¥		¥	-	V												¥					
Å									RI	M1		DB3	-2				Ī					
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M1 DB4	-2																					
20' 8	3"					۹				17	7' 4"											
				38' (כ"																	

38' 0"

** DAMAGED FLOOR JOISTS SHOULD NOT BE INSTALLED UN	INSTALLED UNLESS APPROVED BY COMPONENT PLANT.	** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH.	** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS.	<u>.</u>
Date: Desig	DR Horton		This is an I-Joist Placement Plan Only. All designs of I-Joist follow the IBC/IRC Code Requirements along with Manufacturer's guidelines. This is NOT an engineered	00/0 00/0 00/0 00/0
e: 1/4" // 06/ gner: [ct #: 2 Sheet Nu	38 Mason Ridge		lans ss. It is approve duct	0/00 0/00 0/00
/11/24 DW 40600	Galen E		usage, and quantities. Do not notch or drill holes in beams or flanges on joists without prior approval from the manufacturing Representative unless following hole guidlines in the installation	Nar Nar Nar Nar
	FLOOR JOIST LAYOUT	كسعوسهم تعقيبتها تعتيقه	guide of product. Builder takes full responsibility for doing so and NO Back charge will be accepted.	ne ne ne

	Plies	Net Qty
	1	10
	1	4
	1	5
	1	3
	1	19
	2	4
-1/4	2	2
-1/4	2	2
-1/4	2	2
1-7/8	2	2
	1	16
	1	16

Plies Net Qty

RIALS IS NOT RESPONSIBLE N OF ANY AND ALL I-JOIST AND INEERING AND INFORMATION FOR ED BY THE ENGINEER OF RECORD PLANS. ALL BEAM PLACEMENTS RECEIVED. ALL CONNECTION ENGINEER OF RECORD. REFER ALL MULTI-PLY LVL/ I-JOIST DER TO VERIFY ALL MATERIAL SIZES PRIOR TO ORDERING.

2h

BACKER BLOCKS MUST BE LONG ENOUGH TO PERMIT REQUIRED NAILING WITHOUT SPLITTING

 MINING
 THICKNEDG
 MINIDEPTH

 PJI40 & PJI60
 1"
 5 ½"

PJI80 & PJI90 1 ½" 7 ¼"

*MIN GRADE BACKER BLOCK SHALL BE UTILITY GRADE SPF (SOUTH) OR BETTER FOR SOLID SAWN LUMBER AND SHALL BE RATED SHEATHING GRADE FOR WOOD STRUCTURAL PANELS *FOR FACE MOUNT HANGERS, USE NET JOIST DEPTH MINUS 1 1/4* FOR JOISTS WITH 1 1/2* THICK FLANGES

<u> Adr</u>

2ND FLOOR LAYOUT

LABEL LEGEND

- **BBO** = Beam by Others **PBO** = Post by Others
- **GBO** = Girder by Others
- **J** = I-Joist
- **FB** = Flush Beam
- **DB** = Dropped Beam
- **RB** = Roof Beam
- **BP** = Blocking Panels
- SB = Squash Blocks

** REFER TO INSTALLATION GUIDE FOR PLY TO PLY CONNECTIONS.

BEFORE INSTALLING A BACKER BLOCK TO A DOUBLE I-JOIST, DRIVE 3 ADDITIONAL 10d NAILS THROUGH WEBS AND FILLER BLOCK WHERE THE BACKER BLOCK WILL FIT CLINCH INSTALL BACKER TICHT TO TOP FLANGE USE 12 10d NAILS, CLINCH WHEN POSSIBLE, MAX CAPACIY FOR HANGER FOR THIS DETAIL IS 1280 LBS

- DOUBLE PJI I-JOIST HEADER

FOR HANGER CAPACITY SEE HANGER MAUNFACTURES RECOMMENDATIONS VERIFY DOUBLE PJI I-JOIST CAPACITY TO SUPPORT CONCENTRATED LOADS

<u>NOTE</u> UNLESS HANGER SIDES LATERALLY SUPPORT THE TOP FLANGE, BEARING STIFFENERS SHALL BE USED

BACKER BLOCK REQUIRED (BOTH SIDES FOR FACE MOUNT HANGERS SEE HANGER MANUFATURED INSTALL DETAILS

HANGER MUST SUPPORT TOP FLANGE OF JOIST, FILLER BLOCK REQUIRED IF HANGER IS NOT FULL DEPTH OF JOST