

RE: J0524-3226 Lot 22 Liberty Meadows **Trenco** 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: J0524-3226 Lot/Block: Address: City:

Model: Subdivision: State:

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 42 individual, dated Truss Design Drawings and 0 Additional Drawings.

No. 1 2 3 4 5 6 7 8 9 10 11 12	Seal# I63188656 I63188657 I63188658 I63188659 I63188660 I63188661 I63188662 I63188663 I63188664 I63188665 I63188666 I63188667	Truss Name A1-GE A2 A3 A4 A4-A A5 A6 A7 A8 A9 B1-GE B2	Date 1/23/2024 1/23/2024 1/23/2024 1/23/2024 1/23/2024 1/23/2024 1/23/2024 1/23/2024 1/23/2024 1/23/2024 1/23/2024	No. 21 22 23 24 25 26 27 28 29 30 31 32	Seal# I63188676 I63188677 I63188678 I63188679 I63188680 I63188681 I63188682 I63188683 I63188683 I63188685 I63188686 I63188687	Truss Name G1-GE G2 G3 G5 G6 G7-GE H1 K1 M1 M2 M3 P1	Date 1/23/2024 1/23/2024 1/23/2024 1/23/2024 1/23/2024 1/23/2024 1/23/2024 1/23/2024 1/23/2024 1/23/2024 1/23/2024
-		-					
						-	
13	163188668	B3	1/23/2024	33	163188688	VA1	1/23/2024
14	163188669	B4	1/23/2024	34	163188689	VA2	1/23/2024
15	163188670	C1-GE	1/23/2024	35	163188690	VA3	1/23/2024
16	163188671	C2	1/23/2024	36	163188691	VA4	1/23/2024
17	163188672	C3	1/23/2024	37	163188692	VB1	1/23/2024
18	163188673	D1-GE	1/23/2024	38	163188693	VC1	1/23/2024
19	163188674	D2	1/23/2024	39	163188694	XH1	1/23/2024
20	163188675	D3	1/23/2024	40	163188695	YH1	1/23/2024

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Tony Miller

My license renewal date for the state of North Carolina is December 31, 2024

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Tony Miller

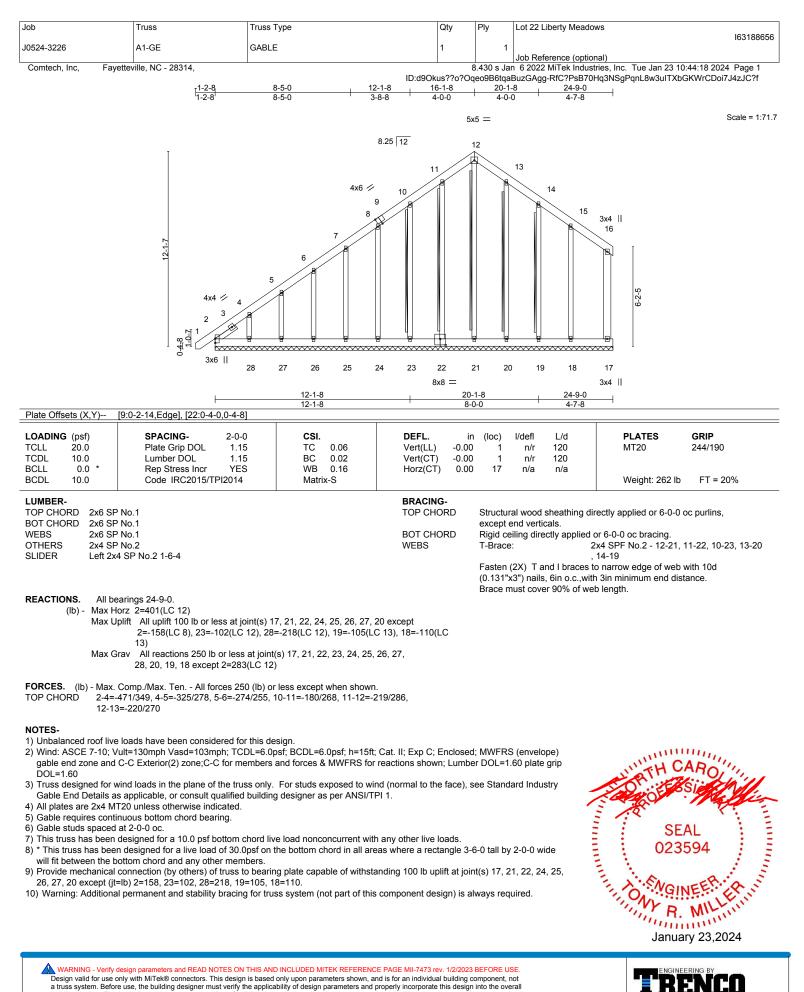


RE: J0524-3226 - Lot 22 Liberty Meadows

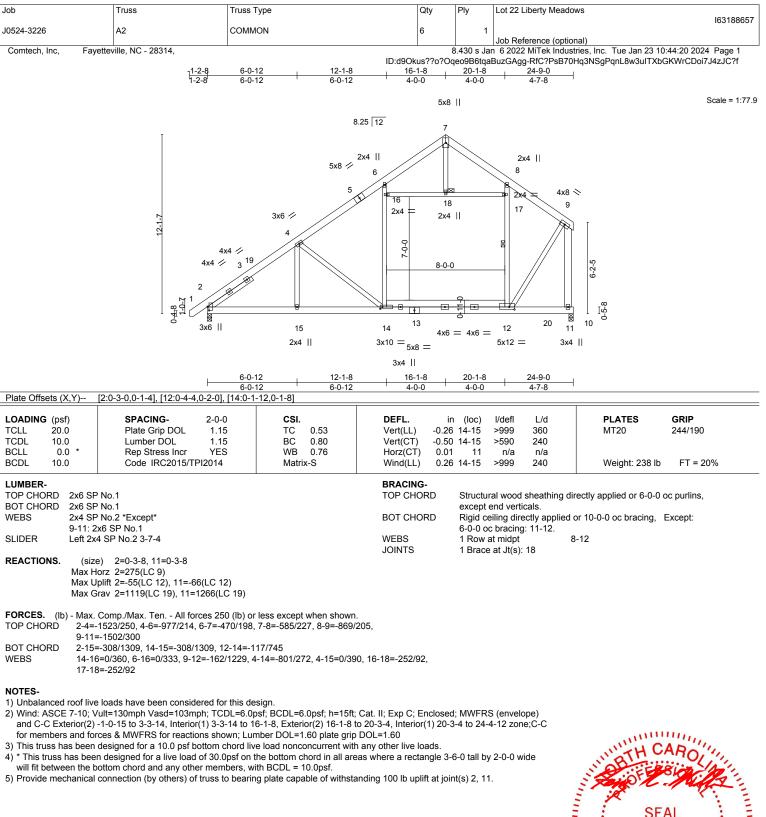
**Trenco** 818 Soundside Rd Edenton, NC 27932

# Site Information:

Proje Lot/B Addre	lock:	Project Name: J0	524-3226	Subdivision:
City,	County:			State:
No. 41 42	Seal# I63188696 I63188697	Truss Name YH2 ZH1	Date 1/23/2024 1/23/2024	

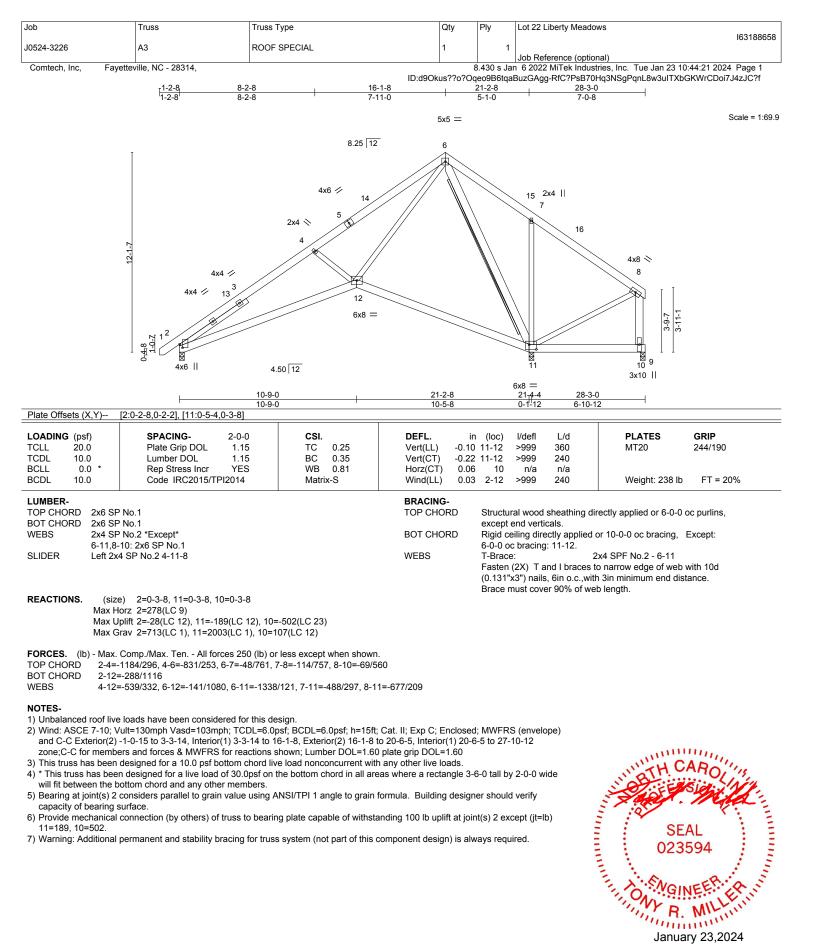


a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPH Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



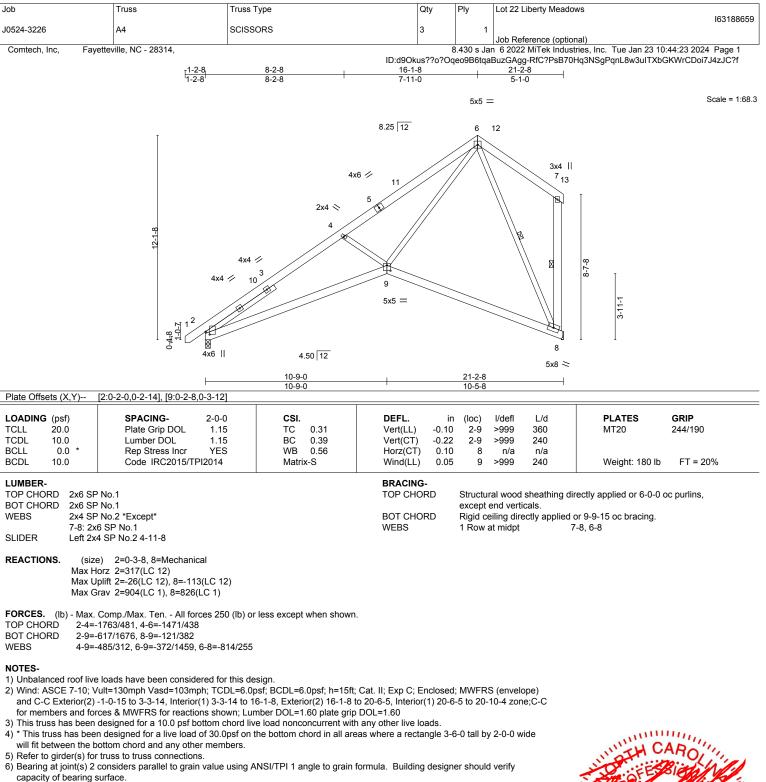


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



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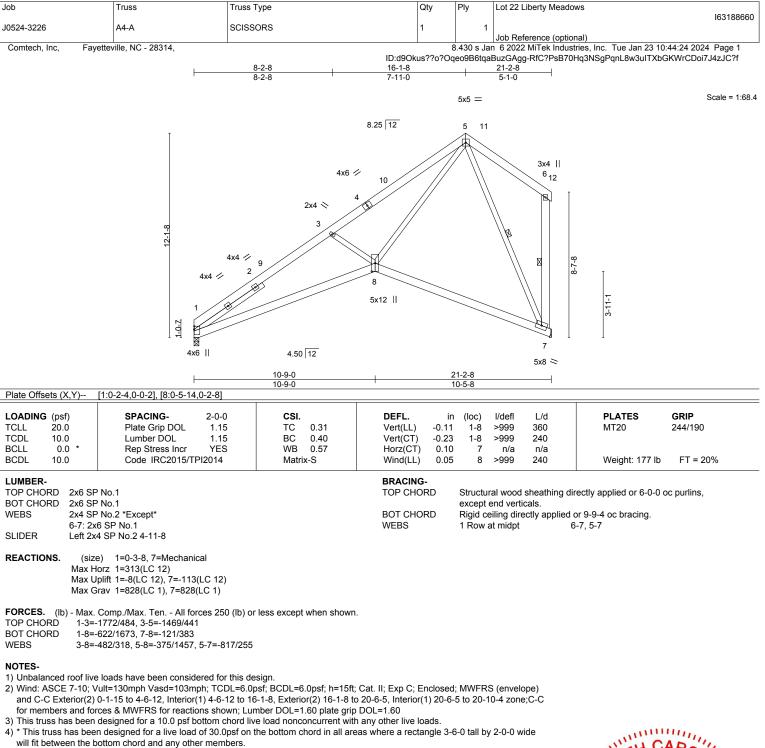




7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 8=113.



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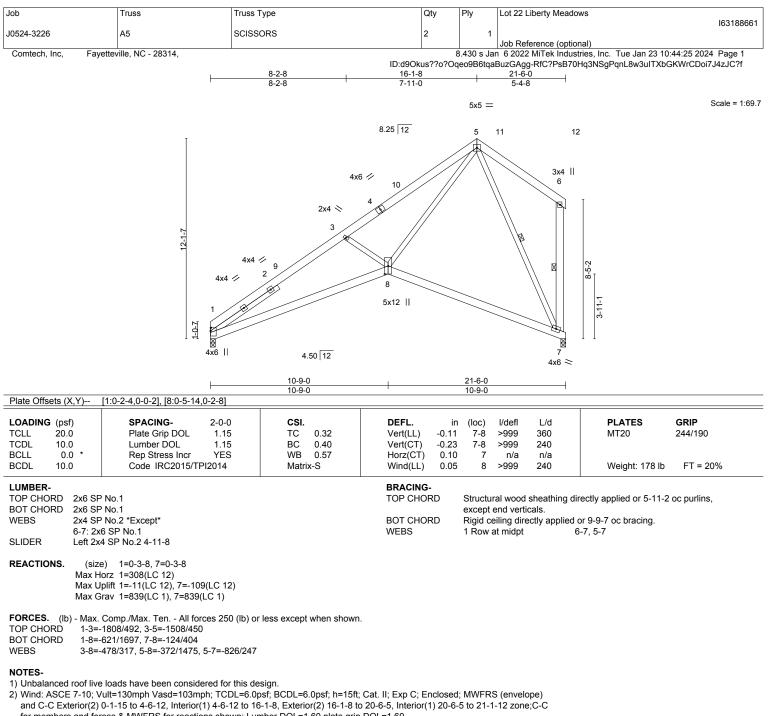
5) Refer to girder(s) for truss to truss connections.

6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 7=113.



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for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

() \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

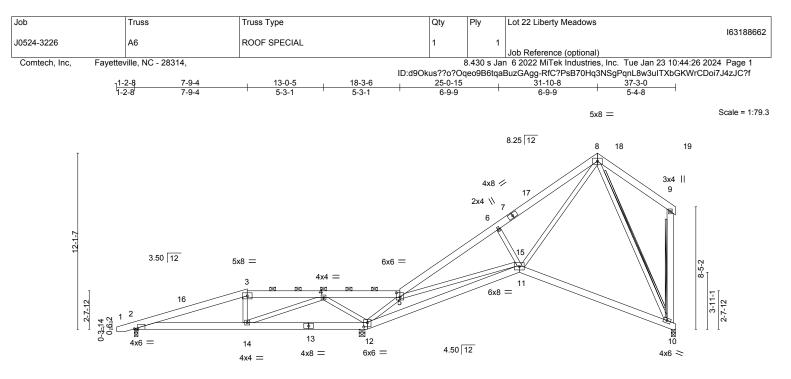
5) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 7=109.



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A MiTek Aff



	7-9-4 7-9-4	<u> </u>	0 <u>-8 26-6-0</u> -13 10-5-8	<u> </u>	
Plate Offsets (X,Y)	[2:0-2-13,Edge], [5:0-3-0,0-3-8], [12	:0-3-0,0-3-8]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.28 BC 0.32 WB 0.62 Matrix-S	DEFL.         in         (loc)           Vert(LL)         -0.11         10-11           Vert(CT)         -0.23         10-11           Horz(CT)         0.10         10           Wind(LL)         0.06         12-14	I/defl         L/d         PLATES           >999         360         MT20           >999         240            n/a         n/a            >999         240         Weight: 28	<b>GRIP</b> 244/190 30 lb FT = 20%

LUMBER-		BRACING-			
TOP CHORD	2x6 SP No.1	TOP CHORD	Structural wood sl	heathing directly applied or 6-0-0 oc purlins,	
BOT CHORD	2x6 SP No.1		except end vertica	als, and 2-0-0 oc purlins (6-0-0 max.): 3-5.	
WEBS	2x4 SP No.2 *Except*	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except:		
	9-10: 2x6 SP No.1		7-9-1 oc bracing: 2-14.		
		WEBS	T-Brace:	2x4 SPF No.2 - 9-10, 8-10	
			Fasten (2X) T and	d I braces to narrow edge of web with 10d	
			(0.131"x3") nails,	6in o.c., with 3in minimum end distance.	

REACTIONS. 12=0-4-13, 2=0-3-0, 10=0-3-8 (size) Max Horz 2=313(LC 12) Max Uplift 12=-142(LC 12), 2=-285(LC 8), 10=-81(LC 12) Max Grav 12=1591(LC 1), 2=634(LC 23), 10=781(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1100/800, 3-4=-1001/795, 4-5=-348/484, 5-6=-1551/316, 6-8=-1359/364 TOP CHORD

BOT CHORD 2-14=-997/994, 12-14=-414/355, 11-12=-181/760, 10-11=-97/366

4-14=-639/696, 4-12=-1008/674, 5-12=-1439/357, 5-15=-266/656, 6-15=-439/273, WFBS 11-15=-421/262, 8-11=-272/1316, 8-10=-736/184

#### NOTES-

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-5 to 3-5-8, Interior(1) 3-5-8 to 31-10-8, Exterior(2) 31-10-8 to 36-3-5, Interior(1) 36-3-5 to 36-10-12 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10 except (jt=lb) 12=142 2=285

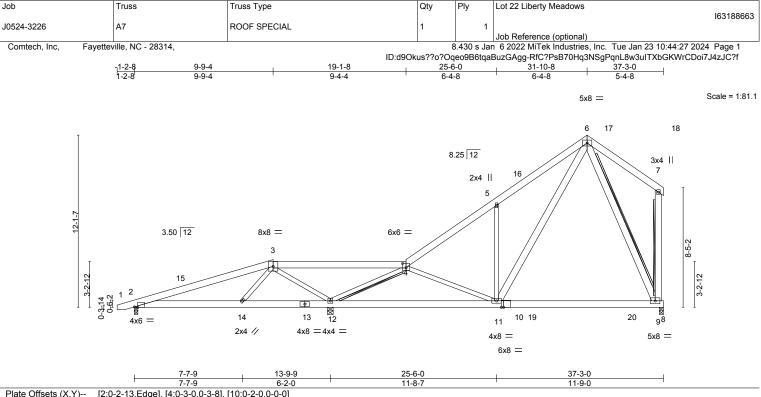
Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



Brace must cover 90% of web length.

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LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.46	Vert(LL)	-0.31	9-11	>894	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.61	Vert(CT)	-0.40	9-11	>687	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.52	Horz(CT)	0.01	9	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-S	Wind(LL)	0.05	2-14	>999	240	Weight: 306 lb	FT = 20%

LUMBER-		BRACING-				
TOP CHORD	2x6 SP No.1	TOP CHORD	Structural wood s	heathing directly applied or 6-0-0 oc purlins,		
BOT CHORD	2x6 SP No.1		except end verticals, and 2-0-0 oc purlins (10-0-0 max.): 3-4			
WEBS	2x4 SP No.2 *Except*	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except:			
	6-11,7-9,6-9: 2x6 SP No.1		6-0-0 oc bracing: 12-14.			
		WEBS	T-Brace:	2x4 SPF No.2 - 4-12, 7-9, 6-9		
			Fasten (2X) T an	d I braces to narrow edge of web with 10d		

Fasten (2X) T and I braces to narrow edge of web with 1( (0.131"x3") nails, 6in o.c.,with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. (size) 2=0-3-0, 12=0-4-13, 9=0-3-8 Max Horz 2=313(LC 12) Max Uplift 2=-251(LC 8), 12=-157(LC 12), 9=-80(LC 12) Max Grav 2=443(LC 23), 12=1744(LC 1), 9=1034(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-321/368, 3-4=-509/906, 4-5=-991/35, 5-6=-1045/250

BOT CHORD 2-14=-553/233, 12-14=-327/97, 11-12=-161/863, 9-11=-56/323

WEBS 3-14=-359/341, 3-12=-1212/788, 4-12=-1841/466, 5-11=-515/308, 6-11=-150/1041,

#### NOTES-

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-5 to 3-5-8, Interior(1) 3-5-8 to 31-10-8, Exterior(2) 31-10-8 to 36-3-5, Interior(1) 36-3-5 to 36-10-12 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

6-9=-693/146

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 2=251, 12=157.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

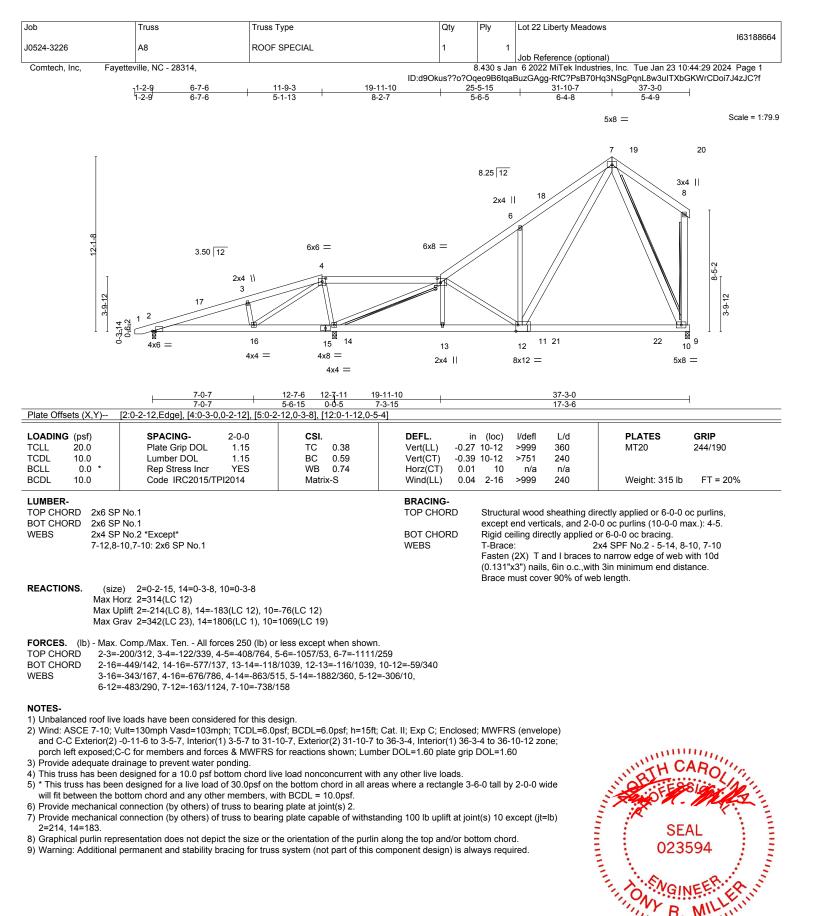
SEAL 023594 VGINEER January 23,2024

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818 Soundside Road

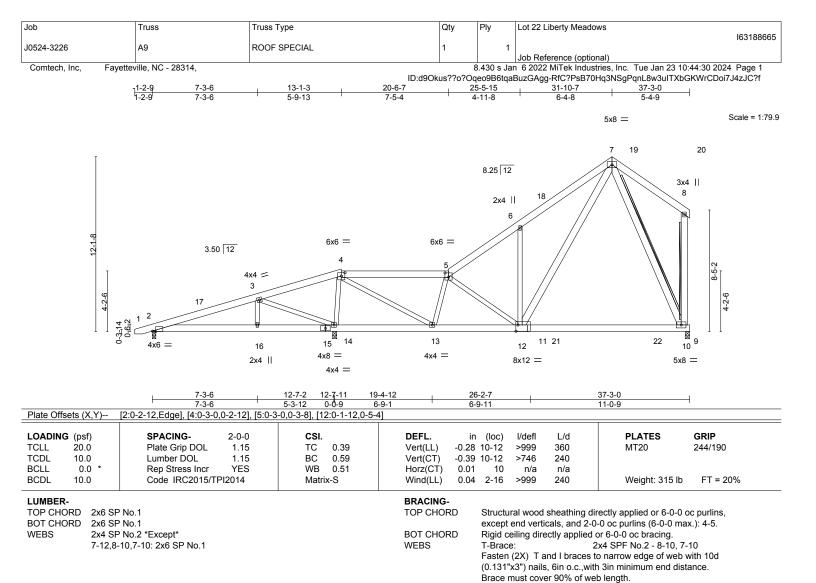
Edenton, NC 27932



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818 Soundside Road Edenton, NC 27932

January 23,2024



REACTIONS. (size) 2=0-2-15, 14=0-3-8, 10=0-3-8 Max Horz 2=313(LC 12) Max Uplift 2=-208(LC 8), 14=-190(LC 12), 10=-74(LC 12) Max Grav 2=335(LC 23), 14=1816(LC 1), 10=1062(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-142/305, 3-4=-449/803, 4-5=-807/0, 5-6=-1043/48, 6-7=-1090/248

- BOT CHORD 2-16=-384/81, 14-16=-384/81, 13-14=-527/186, 12-13=-115/1010, 10-12=-57/336
- WEBS 3-14=-871/686, 4-14=-1340/417, 4-13=-246/1453, 5-13=-743/203, 5-12=-286/16,

6-12=-457/280, 7-12=-150/1097, 7-10=-728/154

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-6 to 3-5-7, Interior(1) 3-5-7 to 31-10-7, Exterior(2) 31-10-7 to 36-3-4, Interior(1) 36-3-4 to 36-10-12 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 2.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10 except (jt=lb) 2=208, 14=190.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

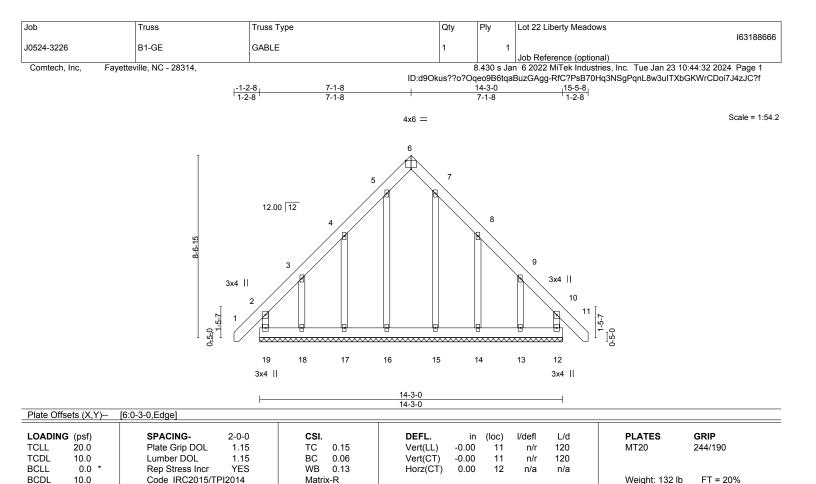


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818 Soundside

Edenton, NC 27932



BRACING-

TOP CHORD	2x6 SP No.1	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x6 SP No.1		except end verticals.
WEBS	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2x4 SP No.2		

REACTIONS. All bearings 14-3-0.

Max Horz 19=-297(LC 10) (lb) -

. . . . .

Max Uplift All uplift 100 lb or less at joint(s) except 19=-159(LC 8), 12=-142(LC 9), 17=-166(LC 12), 18=-244(LC 12), 14=-167(LC 13), 13=-240(LC 13)

All reactions 250 lb or less at joint(s) 17, 14, 13 except 19=266(LC 20), 12=252(LC 19), 16=280(LC Max Grav 22), 18=255(LC 10), 15=276(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 4-5=-211/286, 7-8=-212/286

#### NOTES-

LUMBER-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

7) Gable studs spaced at 2-0-0 oc.

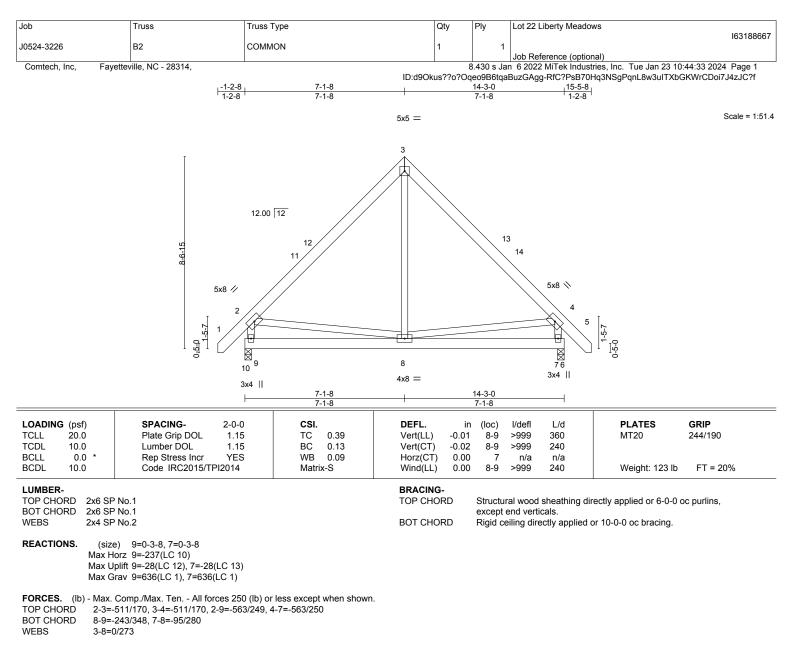
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 159 lb uplift at joint 19, 142 lb uplift at joint 12, 166 lb uplift at joint 17, 244 lb uplift at joint 18, 167 lb uplift at joint 14 and 240 lb uplift at joint 13.



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#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-2 to 3-3-11, Interior(1) 3-3-11 to 7-1-8, Exterior(2) 7-1-8 to 11-6-5, Interior(1) 11-6-5 to 15-4-2 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 9 and 28 lb uplift at joint 7.



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Job	Truss	Truss Type	Qty	Ply		Lot 22 Liberty Mead	lows	
J0524-3226	B3	COMMON	3		1			163188668
				0.400		Job Reference (opt		0.44.04.0004 David
Comtech, Inc, Fayet	eville, NC - 28314,	7-1-8	ID:d9Okus??o?	Oqeo9B 14-			stries, Inc. Tue Jan 23 1 0Hq3NSgPqnL8w3uITXb	
			5x5 =					Scale = 1:51.4
	5 9 9 8 4x8 1 1 1 1		2			12 4x8 % 3	-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1	
		8 <sup>7</sup>	6			54		
		3x4	4x8 =			3x4		
		7-1-8 7-1-8			-3-0 1-8			
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *	SPACING-2-0-Plate Grip DOL1.1Lumber DOL1.1Rep Stress IncrYE	5 TC 0.25 5 BC 0.13 S WB 0.06	Vert(LL) -0.0 Vert(CT) -0.0 Horz(CT) 0.0	)2 6· )0	-7 -7 5	l/defl L/d >999 360 >999 240 n/a n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.0	00	6	>999 240	Weight: 116 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SP N BOT CHORD 2x6 SP N WEBS 2x4 SP N	lo.1	-	BRACING- TOP CHORD BOT CHORD	exce	ept e	nd verticals.	directly applied or 6-0-0 d or 10-0-0 oc bracing.	oc purlins,
Max Hor Max Upl	7=0-3-8, 5=0-3-8 z 7=205(LC 9) ft 7=-20(LC 13), 5=-20(LC 12 v 7=555(LC 1), 5=555(LC 1)							
	5/163, 2-3=-515/163, 1-7=-4 28/298	50 (lb) or less except when shown. 33/174, 3-5=-483/175						

BOT CHORD WEBS 2-6=0/270

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-1 to 4-7-14, Interior(1) 4-7-14 to 7-1-8, Exterior(2) 7-1-8 to 11-6-5, Interior(1) 11-6-5 to 13-11-15 zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 7 and 20 lb uplift at joint 5.



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Job	Truss	Truss Type	Qty	Ply	Lot 22 Liberty Meadow	ws	
J0524-3226	B4	COMMON	1	1			163188669
Comtech, Inc, Fayetter	│ ville, NC - 28314,	ID:d9		qeo9B6tqa 1-0		nal) ries, Inc. Tue Jan 23 10 Iq3NSgPqnL8w3uITXbG	
		5x5 =					Scale = 1:51.4
	8.6.15 2.0.7	4x8 / 1 4x8 / 1 8 7 8 7 3x4    4x8 =	9	10	4x8 × 3 54 3x4		
		4-9-8 4-9-8	11-1 7-1				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	TC 0.27 Vert(I BC 0.11 Vert(0	L) -0.0 <sup>7</sup> CT) -0.03	3 5-6	l/defl L/d >999 360 >999 240 n/a n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S Wind			>999 240	Weight: 107 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SP No BOT CHORD 2x6 SP No WEBS 2x4 SP No 1-7: 2x6 SP	o.1 o.2 *Except*		CHORD	except e	al wood sheathing dir end verticals. iling directly applied o	ectly applied or 6-0-0 c or 10-0-0 oc bracing.	oc purlins,
Max Horz Max Uplift	7=0-3-8, 5=0-3-8 7=-186(LC 8) t 7=-45(LC 13), 5=-4(LC 12) 7=459(LC 1), 5=458(LC 1)						
	mp./Max. Ten All forces 250 3/161, 2-3=-392/115, 1-7=-42	) (lb) or less except when shown. //162, 3-5=-383/141					

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 9-2-5, Interior(1) 9-2-5 to 11-7-15 zone; end vertical right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

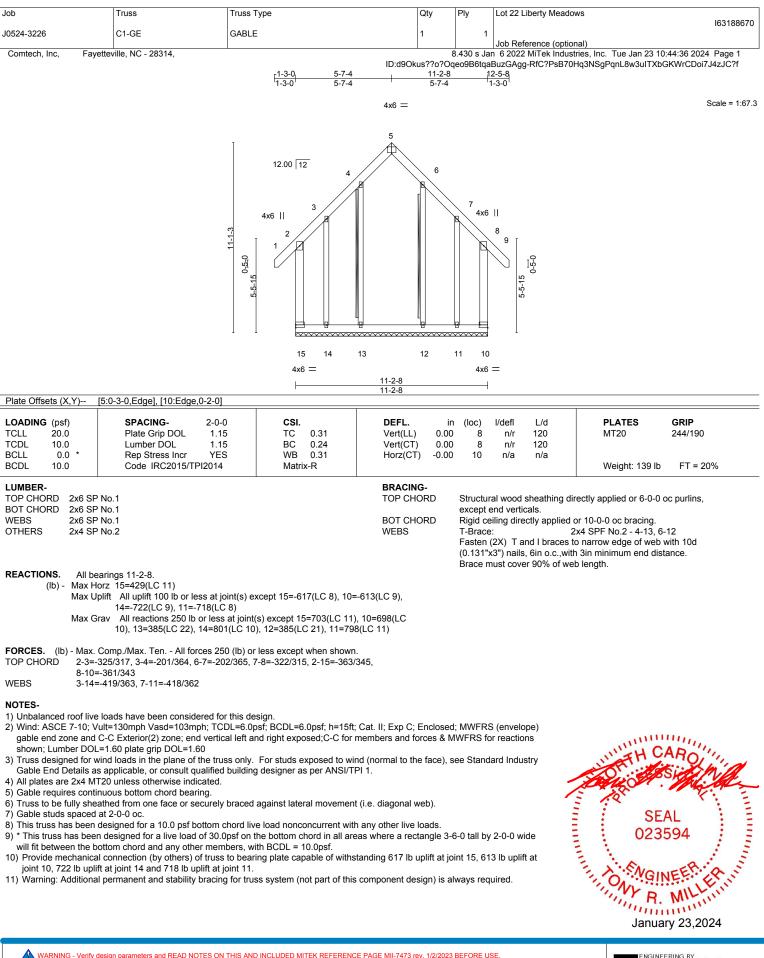
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 7 and 4 lb uplift at joint 5.



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Edenton, NC 27932



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Job	Truss	Truss Type	Qty	Ply	Lot 22 Liberty Meado	ws	10040007
J0524-3226	C2	COMMON	2	1		1)	I63188671
Comtech, Inc, Faye	etteville, NC - 28314,				Job Reference (option an 6 2022 MiTek Indust	ries, Inc. Tue Jan 23 1	
		[1-3-0] 5-7-4 [1-3-0] 5-7-4	11-2-8		aBuzGAgg-RfC?PsB70I <u>12-5-8</u>	Hq3NSgPqnL8w3uITXI	bGKWrCDoi7J4zJC?f
		1-3-0 5-7-4	5-7-4		1-3-0		
			5x5 =				Scale = 1:65.8
			3				
		12.00 12 12.00 12 10 4x8 // 9 0 10 0 10 10 10 10 10 10 10		4x8			
		<sup>™</sup> <sup>™</sup> <sup>8</sup> 3x4	7	3x4    🖗			
			3x10 =				
		5-7-4	11-2-8 5-7-4		1		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.	-0 <b>CSI.</b> 15 TC 0.14 15 BC 0.08 25 WB 0.12	<b>DEFL.</b> ir Vert(LL) -0.00 Vert(CT) -0.01 Horz(CT) -0.00	7-8	l/defl L/d >999 360 >999 240 n/a n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code IRC2015/TPI201	4 Matrix-S	Wind(LL) 0.00	) 7-8	>999 240	Weight: 131 lb	5 FT = 20%
			BRACING- TOP CHORD BOT CHORD WEBS	except Rigid c	ral wood sheathing di end verticals. eiling directly applied o at midpt 3		) oc purlins,
Max He Max U	e) 8=0-3-8, 6=0-3-8 prz 8=-342(LC 10) plift 8=-39(LC 8), 6=-39(LC 9) rav 8=526(LC 20), 6=526(LC	19)					
TOP CHORD 2-3=-3 BOT CHORD 7-8=-3	Comp./Max. Ten All forces 2 300/221, 3-4=-300/221, 2-8=-4 335/369 126/250	50 (lb) or less except when shown. 178/293, 4-6=-478/293					
<ol> <li>Wind: ASCE 7-10; V and C-C Exterior(2) vertical left and right</li> <li>This truss has been 4) * This truss has been will fit between the b</li> </ol>	-1-1-2 to 3-3-11, Interior(1) 3-3 exposed;C-C for members an designed for a 10.0 psf bottom n designed for a live load of 30 ottom chord and any other me	CDL=6.0psf; BCDL=6.0psf; h=15ft; C -11 to 5-7-12, Exterior(2) 5-7-12 to 1 d forces & MWFRS for reactions sho chord live load nonconcurrent with .0psf on the bottom chord in all area	0-0-9, Interior(1) 10-0- own; Lumber DOL=1.60 any other live loads. s where a rectangle 3-	9 to 12-4 ) plate gr 6-0 tall b	-10 zone; end ip DOL=1.60 y 2-0-0 wide	NUMBER OF	ARO
							AL 594



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Job	Truss	Truss Type	Qty	Ply	Lot 22 Liberty Meadows	i	
J0524-3226	СЗ	СОММОЛ	4	1			163188672
Comtech, Inc, Fayette	 wille, NC - 28314,	ID:d9OI 5-7-4 5-7-4		eo9B6tqal	Job Reference (optional 6 2022 MiTek Industrie BuzGAgg-RfC?PsB70Hq 	s, Inc. Tue Jan 23 10:	
		5x5 =					Scale = 1:65.8
		12.00 12 4x8 / 1 4x8 / 1 6 3x4    5	9	4x8 3 3 3x4    <sup>10</sup> / <sub>4</sub>	5-5-15 5		
		3x10 = 	11-2-8 5-7-4				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	5 TC 0.13 Vert(LL 5 BC 0.08 Vert(CT	) -0.01	<b>`</b> 5-6	l/defl L/d >999 360 >999 240 n/a n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S Wind(L			>999 240	Weight: 124 lb	FT = 20%
		BRACII TOP CH BOT CH WEBS	IORD	except e	al wood sheathing direct ind verticals. Iling directly applied or t midpt 2-5	10-0-0 oc bracing.	c purlins,
Max Horz Max Uplif	6=0-3-8, 4=0-3-8 : 6=129(LC 9) t 6=-45(LC 13), 4=-45(LC 12) · 6=433(LC 20), 4=433(LC 15)						
	mp./Max. Ten All forces 25 4/158, 2-3=-284/158, 1-6=-38	0 (Ib) or less except when shown. 6/145, 3-4=-386/145					

NOTES-

1) Unbalanced roof live loads have been considered for this design.

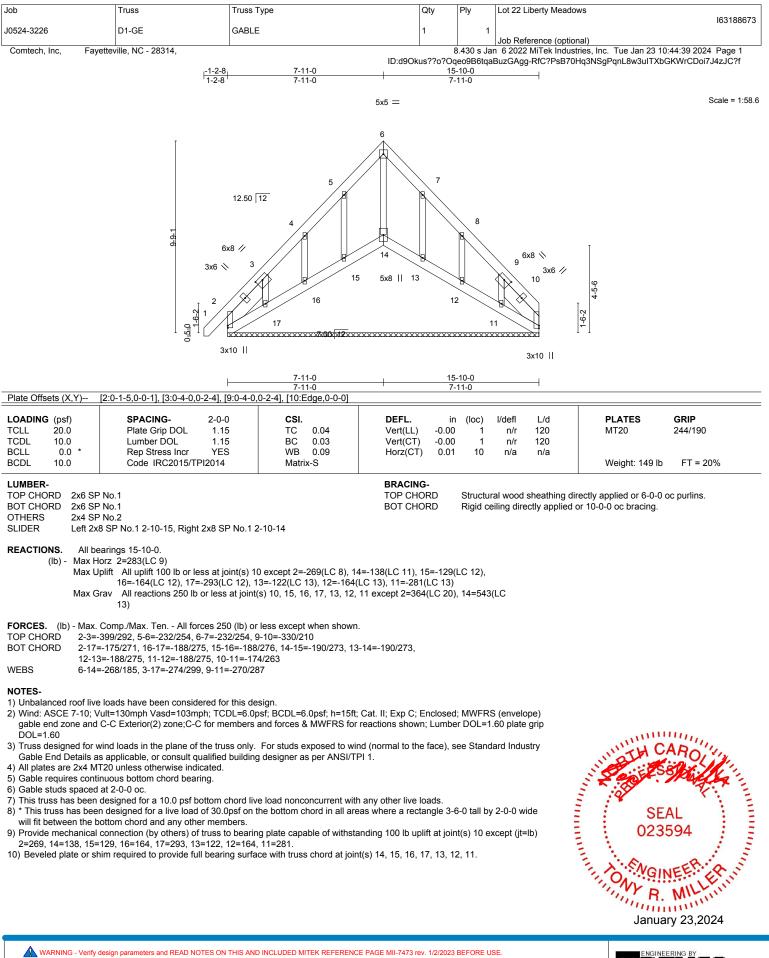
2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-4 to 4-8-1, Interior(1) 4-8-1 to 5-7-12, Exterior(2) 5-7-12 to 10-0-9, Interior(1) 10-0-9 to 11-0-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

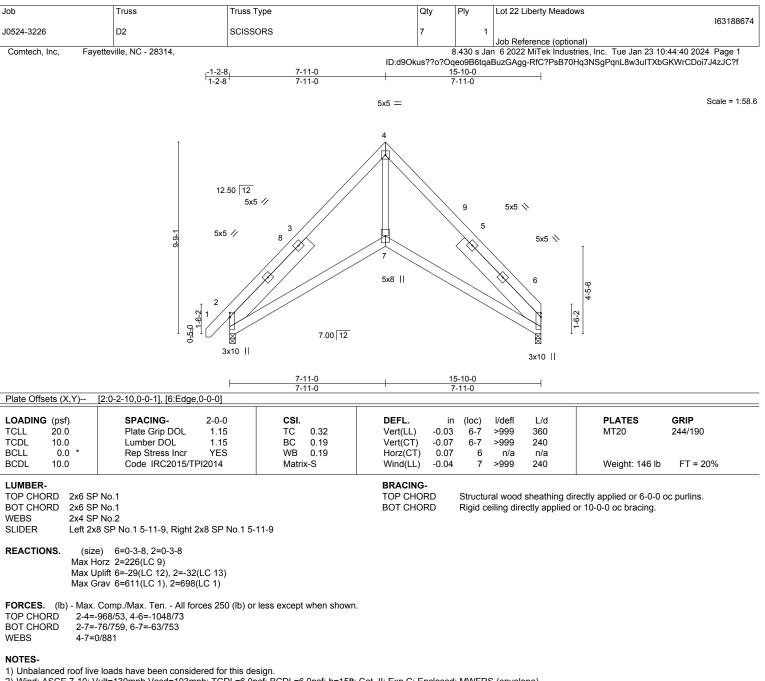
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 6 and 45 lb uplift at joint 4.

# minin Annun ann The second se SEAL 023594 ONY R. MIL R. MILLIN January 23,2024

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2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-1 to 3-3-11, Interior(1) 3-3-11 to 7-11-0, Exterior(2) 7-11-0 to 12-3-13, Interior(1) 12-3-13 to 15-7-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

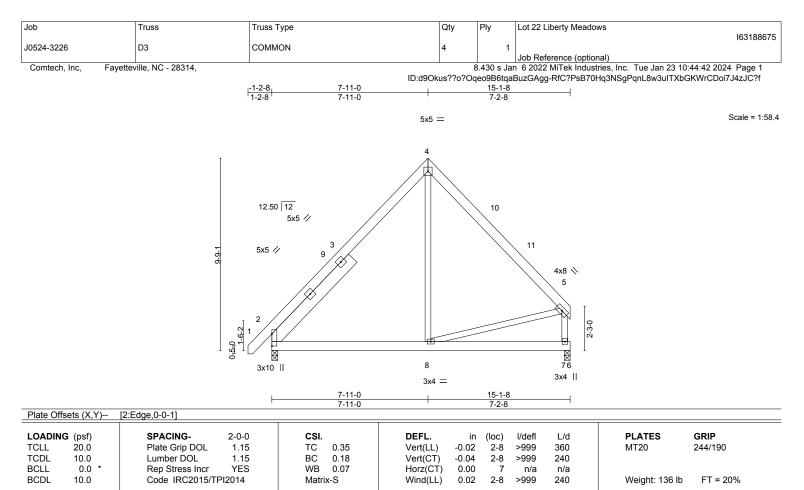
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Bearing at joint(s) 6, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.



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шм	REF	2.

2x6 SP No.1
2x6 SP No.1
2x4 SP No.2
Left 2x8 SP No.1 5-10-11

 BRACING 

 TOP CHORD
 Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 7=0-3-8 Max Horz 2=223(LC 9) Max Uplift 2=-26(LC 13), 7=-33(LC 12) Max Grav 2=662(LC 1), 7=598(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-591/181, 4-5=-533/206, 5-7=-537/211

BOT CHORD 2-8=-21/309

WEBS 4-8=0/296, 5-8=-54/287

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-1 to 3-3-11, Interior(1) 3-3-11 to 7-11-0, Exterior(2) 7-11-0 to 12-3-13, Interior(1) 12-3-13 to 14-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 7.

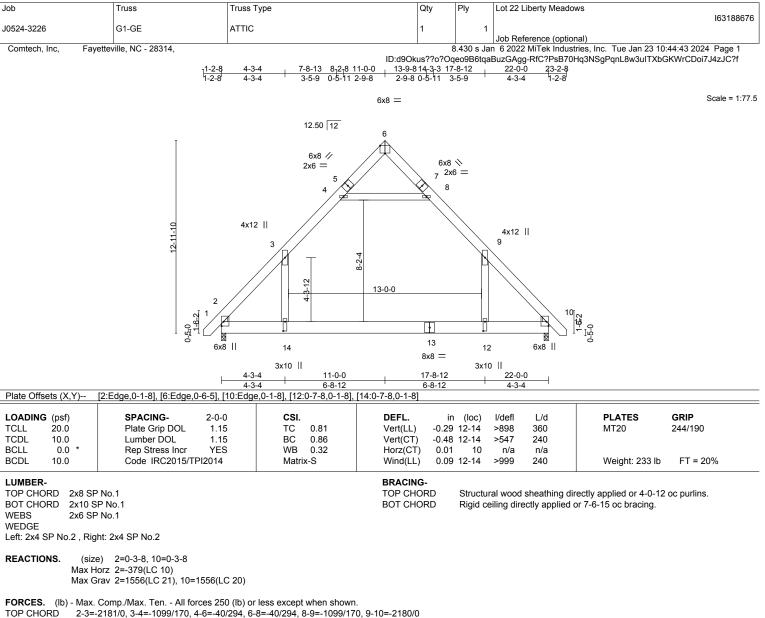


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818 Soundside Road

Edenton, NC 27932



- BOT CHORD 2-14=0/1134, 12-14=0/1134, 10-12=0/1134
- WEBS 9-12=0/1238, 3-14=0/1238, 4-8=-1349/228

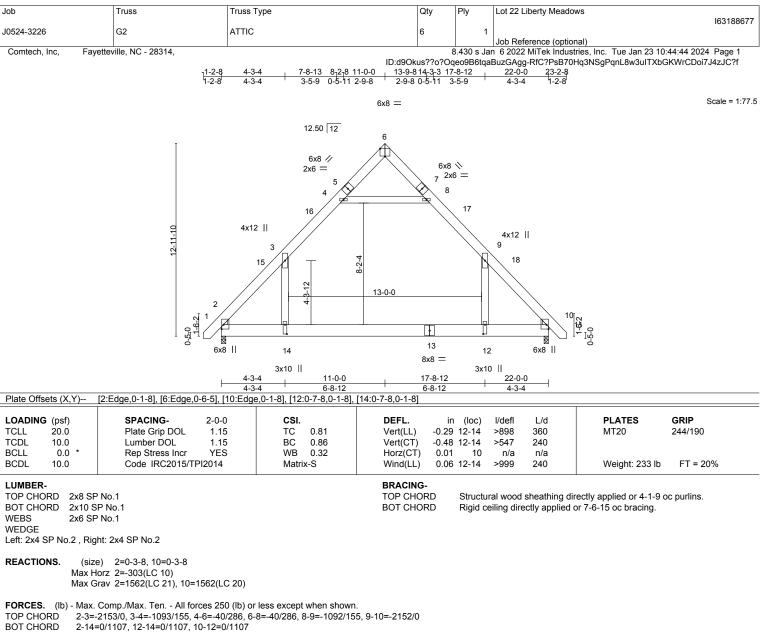
#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Ceiling dead load (10.0 psf) on member(s). 3-4, 8-9, 4-8; Wall dead load (5.0psf) on member(s).9-12, 3-14
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14

7) Attic room checked for L/360 deflection.



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WEBS 9-12=0/1238, 3-14=0/1238, 4-8=-1357/193

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-14 to 3-4-15, Interior(1) 3-4-15 to 11-0-0, Exterior(2) 11-0-0 to 15-4-13, Interior(1) 15-4-13 to 22-11-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

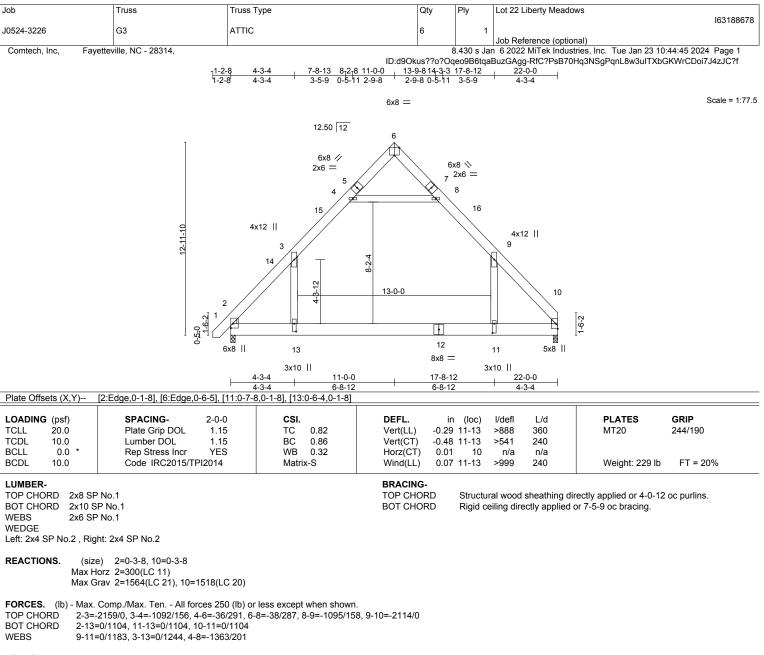
5) Ceiling dead load (10.0 psf) on member(s). 3-4, 8-9, 4-8; Wall dead load (5.0psf) on member(s).9-12, 3-14

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14

7) Attic room checked for L/360 deflection.



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3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

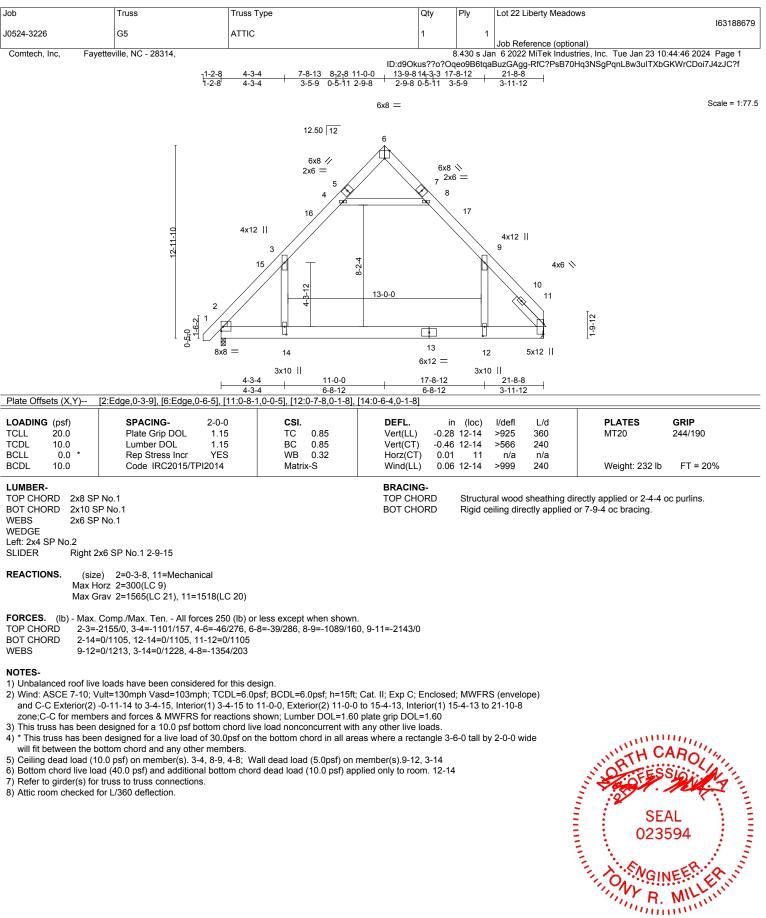
5) Ceiling dead load (10.0 psf) on member(s). 3-4, 8-9, 4-8; Wall dead load (5.0psf) on member(s).9-11, 3-13

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13

7) Attic room checked for L/360 deflection.



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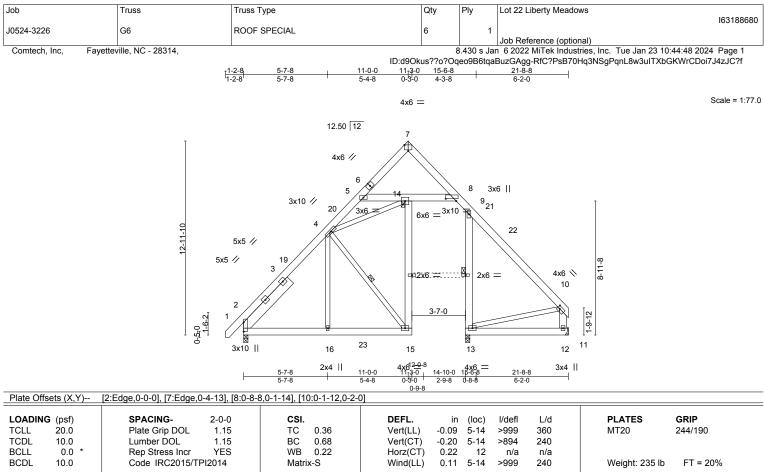


January 23,2024

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A MiTek / 818 Soundside Road

Edenton, NC 27932



LUMBER-		BRACING-	
TOP CHORD	2x6 SP No.1	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x6 SP No.1		except end verticals.
WEBS	2x4 SP No.2 *Except*	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
	10-12: 2x6 SP No.1		6-0-0 oc bracing: 9-13
SLIDER	Left 2x8 SP No.1 4-5-13		8-11-0 oc bracing: 8-14
		WEBS	1 Row at midpt 4-15
		JOINTS	1 Brace at Jt(s): 14
DEADTIONO			

REACTIONS.	(size)	2=0-3-8, 13=0-3-8, 12=Mechanical
	Max Horz	2=302(LC 9)
	Max Uplift	2=-93(LC 13), 12=-144(LC 13)
	Max Grav	2=663(LC 20), 13=1034(LC 19), 12=234(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-4=-678/235, 4-5=-741/76, 5-7=-282/38, 7-8=-332/64, 8-9=-541/273, 9-10=-182/251
- BOT CHORD 2-16=-113/444, 15-16=-113/444, 14-15=-112/598, 8-14=-172/377, 9-13=-970/31
- WEBS 4-15=-671/177, 4-14=-112/547

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-1 to 3-3-11, Interior(1) 3-3-11 to 11-0-0, Exterior(2) 11-0-0 to 15-4-13, Interior(1) 15-4-13 to 21-4-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 12=144.



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A MiTek Affili 818 Soundside Road

Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 22 Liberty Meadows	
J0524-3226	G7-GE	ROOF SPECIAL	1	, 1		163188681
					Job Reference (optional)	Inc. Tue Ion 22 10:14:40 2024 Dage 1
Comtech, Inc, Fay	vetteville, NC - 28314,		ID:d9Okus??o?Oqe		BuzGAgg-RfC?PsB70Hq3N	Inc. Tue Jan 23 10:44:49 2024 Page 1 ISgPqnL8w3uITXbGKWrCDoi7J4zJC?f
		1-2-8 5-7-8 1-2-8 5-7-8	11-0-0 11 <sub>1</sub> 3-0 15-6-8 5-4-8 0-3-0 4-3-8		<u>21-8-8</u> 6-2-0	
			4x6 =			Scale = 1:77.0
		12.50	7			
	Ī	4x6 //	<i>,</i> $\bigwedge$			
		4x0 7				
		5	6	8 3>	(6	
		3x10 //	3x6 = 1	9	I	
	o	4	5x8 =	$\mathbb{N}$		
	2-11-10	5x5 1/	3x10	T  \	$\backslash$	
		5 // // //				φ
		3	€ ==2x6.=	2x6 :	= 4x6 \\	- - - - - - - - - - - - - - - - - - -
					10	
	4		3-7-0	4	2	
	0-5-0 1-6-2			6	1-9-12	
	ó	- ⊠ 3x10    16	19 15	⊠ 13 <sup>2</sup>	0 21 22 <sub>12</sub> 11	
		5-7-8 2x4	4x6 <sup>12<u>-0</u>-8 11-0-0 11<sub>1</sub>3-0 1 14-10-0</sup>	$4x_{15-6-8} =$	5x8 =	
		5-7-8	5-4-8 0-3-0 2-9-8 0-9-8	0-8-8	6-2-0	
Plate Offsets (X,Y)	[2:Edge,0-0-0], [7:Edge,0-4-13]	, [8:0-9-8,0-1-14], [10:0-1-8,0-2-	0], [14:0-4-0,0-3-4]			
LOADING (psf)	SPACING- 2-0-			(loc)	l/defl L/d	PLATES GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.1 Lumber DOL 1.1		Vert(LL) 0.14 Vert(CT) -0.24	5-14 5-14	>999 240 >759 180	MT20 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr N Code IRC2015/TPI2014		Horz(CT) 0.25	12	n/a n/a	Weight: 235 lb FT = 20%
LUMBER- TOP CHORD 2x6 SP	' No.1		BRACING- TOP CHORD	Structur	al wood sheathing directly	applied or 6-0-0 oc purlins,
BOT CHORD 2x6 SP WEBS 2x4 SP	P No.1 P No.2 *Except*		BOT CHORD		end verticals. iling directly applied or 10	-0-0 oc bracing Except:
10-12:	2x6 SP No.1 8 SP No.1 4-4-13			6-0-0 oc	bracing: 9-13	
SLIDER Leit 20	0 SF NU. 1 4-4-15		WEBS	1 Row a		
REACTIONS. (size	e) 2=0-3-8, 13=0-3-8, 12=Me	chanical	JOINTS	1 Brace	at Jt(s): 14	
Max H	lorz 2=374(LC 24)					
	Jplift 2=-209(LC 9), 13=-168(LC Grav 2=669(LC 34), 13=1350(LC					
FORCES. (lb) - Max.	Comp./Max. Ten All forces 2	50 (Ib) or less except when show	n.			
	-718/290, 4-5=-768/163, 5-7=-2 2=-174/305	35/51, 7-8=-346/114, 8-9=-455/2	62, 9-10=-163/309,			
		15=-191/635, 8-14=-172/515, 9-	13=-1002/166,			
	3=-121/347 3=-343/102, 4-15=-717/279, 4-1	4=-172/598				
NOTES-	, ,					
1) Unbalanced roof live	e loads have been considered fo					
	/ult=130mph Vasd=103mph; TC nber DOL=1.60 plate grip DOL=	:DL=6.0psf; BCDL=6.0psf; h=15 :1.60	ft; Cat. II; Exp C; Enclosed;	MWFRS	S (envelope)	
		chord live load nonconcurrent wind the bottom chord in all a		0 toll by	2.0.0 wide	TH CARO
will fit between the b	pottom chord and any other mer		reas where a rectangle 3-0	-0 tali by	2-0-0 wide	O SSOMANI
	r truss to truss connections. connection (by others) of truss	to bearing plate capable of withs	tanding 100 lb uplift at joint	t(s) exce	pt (jt=lb)	and Providence
2=209, 13=168, 12=	=311.	ovided sufficient to support conce		. ,		OFAL STAL
16-8-0, and 289 lb d	lown and 34 lb up at 18-8-0, an	d 248 lb down and 67 lb up at 2			/selection of	SEAL
	vice(s) is the responsibility of otl (S) section, loads applied to the	ners. face of the truss are noted as fro	ont (F) or back (B).		1	023594
,			( ) ( - )·		B lb up at /selection of	S.A. al 3
	dard balanced): Lumber Increase=1.1	5, Plate Increase=1.15				ONGINEE
Uniform Loads (plf) Vert: 1-7=-6	60, 7-10=-60, 2-15=-20, 8-14=-2	0. 11-13=-20				B. MILLIN
voit. 17-0						
Continued on page 2						January 23,2024

## Continued on page 2

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Truss	Truss Type	Qty	Ply	Lot 22 Liberty Meadows
				I63188681
G7-GE	ROOF SPECIAL	1	1	
				Job Reference (optional)
ille, NC - 28314,		8	3.430 s Jar	n 6 2022 MiTek Industries, Inc. Tue Jan 23 10:44:50 2024 Page 2
		G7-GE ROOF SPECIAL	G7-GE ROOF SPECIAL 1	G7-GE ROOF SPECIAL 1 1

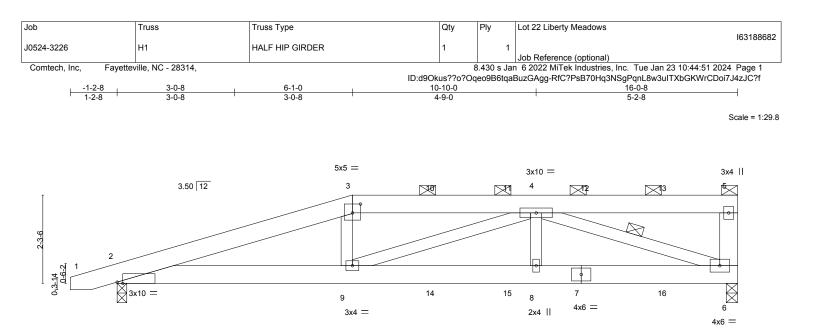
ID:d9Okus??o?Ogeo9B6tgaBuzGAgg-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 20=-283(B) 21=-289(B) 22=-248(B)

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ł	3-0-8 3-0-8	6-1-0 3-0-8	<u>10-10-0</u> 4-9-0	<u>16-0-8</u> 5-2-8	
Plate Offsets (X,Y)	[2:0-1-12,Edge], [3:0-2-8,0-2-12]	5-0-0	4-3-0	3-2-0	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.29 BC 0.45 WB 0.39 Matrix-S	DEFL.         in         (loc)         //def           Vert(LL)         0.11         8-9         >995           Vert(CT)         -0.16         8-9         >995           Horz(CT)         0.04         6         n/a	240 MT20 240	<b>GRIP</b> 244/190 FT = 20%
			except end ve	od sheathing directly applied or 4-3-6 erticals, and 2-0-0 oc purlins (4-5-0 ma directly applied or 7-2-4 oc bracing. pt 4-6	
Max H Max U	e) 6=0-3-8, 2=0-3-0 lorz 2=71(LC 19) Jplift 6=-512(LC 4), 2=-490(LC 4) Grav 6=1219(LC 1), 2=1175(LC 1)				
TOP CHORD 2-3=- BOT CHORD 2-9=-	Comp./Max. Ten All forces 250 (lb) o -3039/1239, 3-4=-2825/1190, 5-6=-258/ -1204/2856, 8-9=-1042/2473, 6-8=-1042 -185/484, 4-6=-2449/1032, 4-8=-86/382	100 2/2473	L		
<ul> <li>porch left and right et</li> <li>2) Provide adequate di</li> <li>3) This truss has been</li> <li>4) * This truss has beee will fit between the b</li> <li>5) Provide mechanical 6=512, 2=490.</li> <li>6) Graphical purlin rep</li> <li>7) Hanger(s) or other of</li> <li>6-1-0, 108 lb down at</li> <li>1b down and 81 lb up</li> <li>down and 50 lb up at</li> <li>The design/selection</li> <li>8) In the LOAD CASE(</li> </ul>	exposed; Lumber DOL=1.60 plate grip E rainage to prevent water ponding. designed for a 10.0 psf bottom chord liv in designed for a live load of 30.0psf on sottom chord and any other members. connection (by others) of truss to bearin resentation does not depict the size or t connection device(s) shall be provided s and 81 lb up at 8-1-12, 108 lb down and p at 14-1-12 on top chord, and 380 lb d at 10-1-12, and 78 lb down and 50 lb up n of such connection device(s) is the res (S) section, loads applied to the face of the section of parts of the section of th	DOL=1.60 we load nonconcurrent with the bottom chord in all are ng plate capable of withsta he orientation of the purlir ufficient to support concer 8 81 lb up at 10-1-12, and own and 234 lb up at 6-1 at 12-1-12, and 78 lb do sponsibility of others.	eas where a rectangle 3-6-0 tall by 2-0-0 anding 100 lb uplift at joint(s) except (jt= n along the top and/or bottom chord. ntrated load(s) 108 lb down and 81 lb up I 108 lb down and 81 lb up at 12-1-12, i -0, 78 lb down and 50 lb up at 8-1-12, 7 wn and 50 lb up at 14-1-12 on bottom o	b) b) b at and 108 '8 lb	
LOAD CASE(S) Stan 1) Dead + Roof Live (b	dard balanced): Lumber Increase=1.15, Plate	Increase=1.15			

anced): Lumber Increase=1.15, Plate Increase=1.15 1) F ROOT LIVE (Da

Uniform Loads (plf)

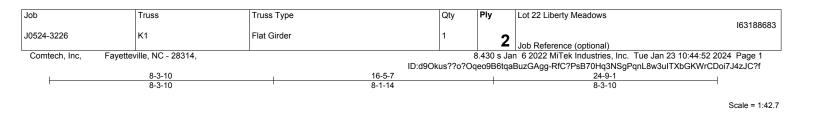
Vert: 1-3=-60, 3-5=-60, 2-6=-20

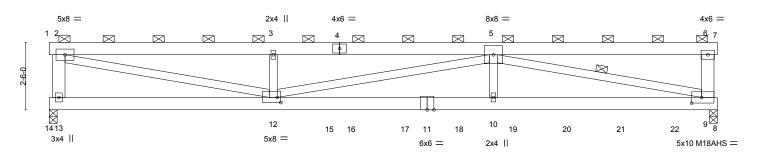
Concentrated Loads (lb)

Vert: 3=-108(B) 7=-39(B) 9=-380(B) 10=-108(B) 11=-108(B) 12=-108(B) 13=-108(B) 14=-39(B) 15=-39(B) 16=-39(B)

innin 1 ONY H. M.Lin R. M January 23,2024

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<b> </b>	8-3-10 8-3-10		5-7 -14			24-9-1 8-3-10	
Plate Offsets (X,Y)	[9:0-4-8,0-2-8], [12:0-1-8,0-2-4]						
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.53 BC 0.93 WB 0.70 Matrix-S	Vert(CT) -0. Horz(CT) 0.	26 10-12 >	/defl L/d 999 360 595 240 n/a n/a 799 240	PLATES MT20 M18AHS Weight: 325 lb	<b>GRIP</b> 244/190 186/179 FT = 20%
			BRACING- TOP CHORD BOT CHORD WEBS		ng directly applied	: 1-7, except end vertic or 10-0-0 oc bracing. 5-9	cals.
Max U	e) 13=0-3-8, 9=0-3-8  plift 13=-597(LC 4), 9=-426(LC 5)  rav 13=1784(LC 1), 9=3064(LC 2)						
TOP CHORD 2-13= BOT CHORD 12-13	Comp./Max. Ten All forces 250 (lb) or 1723/684, 2-3=-5834/2279, 3-5=-5834 3=-62/343, 10-12=-2528/6374, 9-10=-25 2289/5669, 3-12=-441/184, 5-12=-761	/2279, 5-6=-923/79, 6-9=-3 28/6374					
Top chords connect Bottom chords conn Webs connected as 2) All loads are conside ply connections hav 3) Wind: ASCE 7-10; V Lumber DOL=1.60 p 4) Provide adequate dr 5) All plates are MT20 6) This truss has been	rainage to prevent water ponding. plates unless otherwise indicated. designed for a 10.0 psf bottom chord liv	0-9-0 oc. d at 0-6-0 oc. f noted as front (F) or back noted as (F) or (B), unless psf; BCDL=6.0psf; h=15ft; of re load nonconcurrent with	otherwise indicated. Cat. II; Exp C; Enclos any other live loads.	ed; MWFRS (	(envelope);	NUMETH C	AROLA
<ul> <li>7) * This truss has been will fit between the b</li> <li>8) Provide mechanical 13=597, 9=426.</li> <li>9) Graphical purlin reprint (10) Hanger(s) or other 10-4-9, 200 lb down and 164 lb up</li> </ul>	n designed for a live load of 30.0psf on to optiom chord and any other members. connection (by others) of truss to bearing resentation does not depict the size or the connection device(s) shall be provided in and 164 lb up at 11-2-4, 200 lb down p at 17-2-4, 200 lb down and 164 lb up	the bottom chord in all area ng plate capable of withstar ne orientation of the purlin i sufficient to support conce and 164 lb up at 13-2-4, 2 at 19-2-4, and 200 lb dow	as where a rectangle nding 100 lb uplift at j along the top and/or ntrated load(s) 639 lb 200 lb down and 164 n and 164 lb up at 2	oint(s) except bottom chord. down and 33 lb up at 15-2- 1-2-4, and 146	-0-0 wide (jt=lb) 1 lb up at 4, 200 lb 33 lb down	SE 023	
LOAD CASE(S) Stand	m chord. The design/selection of such o dard alanced): Lumber Increase=1.15, Plate		responsibility of othe	rs.		MY R.	MILLER 111

## Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oclapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCEL Building Component Schut Information. Structure Building form the Component Advancibility and properting and property damage. and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road Edenton, NC 27932

January 23,2024

[	Job	Truss	Truss Type	Qty	Ply	Lot 22 Liberty Meadows
						I63188683
	J0524-3226	K1	Flat Girder	1	ົ	
					2	Job Reference (optional)
	Comtech, Inc, Fayettev	ille, NC - 28314,		8	3.430 s Jar	n 6 2022 MiTek Industries, Inc. Tue Jan 23 10:44:52 2024 Page 2

ID:d9Okus??o?Oqeo9B6tqaBuzGAgg-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

LOAD CASE(S) Standard

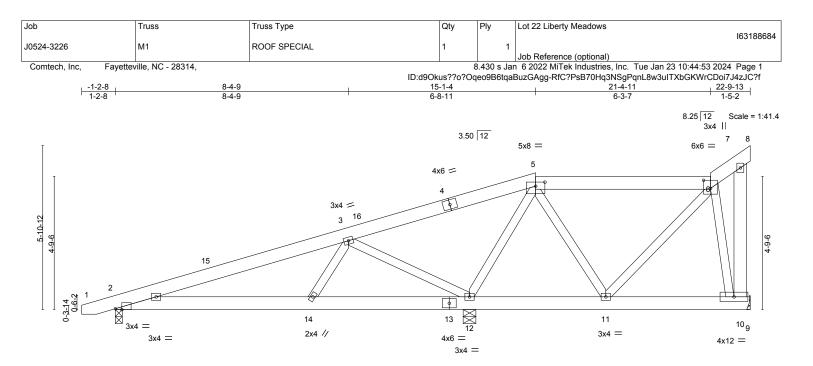
Uniform Loads (plf)

Vert: 1-2=-60, 2-6=-60, 6-7=-60, 8-14=-20 Concentrated Loads (lb)

Vert: 15=-639(F) 16=-177(F) 17=-177(F) 18=-177(F) 19=-177(F) 20=-177(F) 21=-177(F) 22=-1168(F)

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ł	7-1-3 7-1-3		12-8-12 5-7-9	<u>17-7-8</u> 4-10-12	22-9-13 5-2-5
Plate Offsets (X,Y)	[2:0-2-13,Edge], [5:0-4-0,0-1-12], [6:0-3	-0,0-3-8]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.28 BC 0.17 WB 0.39 Matrix-S	<b>DEFL.</b> in Vert(LL) 0.04 Vert(CT) -0.04 Horz(CT) 0.00	2-14 >999 240	PLATES         GRIP           MT20         244/190           Weight: 163 lb         FT = 20%
BOT CHORD 2x6 SI WEBS 2x4 SI	P No.1 P No.1 P No.2 *Except* 2x6 SP No.1		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals, and 2-0- Rigid ceiling directly applied c 6-0-0 oc bracing: 11-12.	
	e) 10=Mechanical, 2=0-3-0, 12=0-5-8 Horz 2=180(LC 12) Jplift 10=-47(LC 12), 2=-172(LC 8), 12=-3	347(LC 8)			

Max Grav 10=268(LC 1), 2=455(LC 1), 12=1156(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-511/339, 3-5=-425/463

BOT CHORD 2-14=-502/429, 12-14=-328/340

WEBS 3-14=-357/324, 3-12=-856/738, 5-12=-705/403, 6-10=-255/81

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-5 to 3-5-8, Interior(1) 3-5-8 to 15-1-4, Exterior(2) 15-1-4 to 21-4-11, Interior(1) 21-4-11 to 22-9-13 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Provide adequate drainage to prevent water ponding.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 4) will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10 except (jt=lb) 2=172, 12=347.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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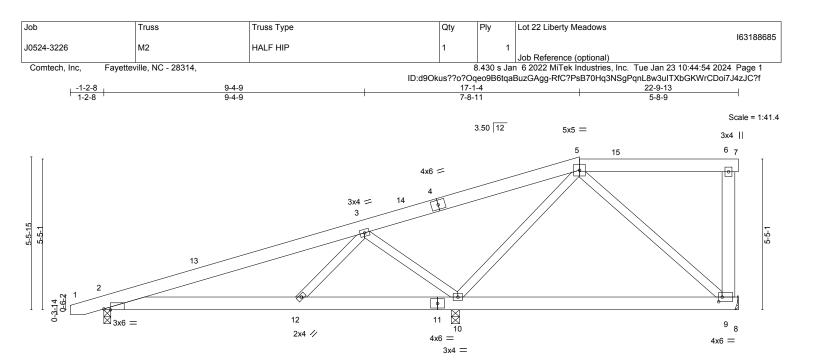


Plate Offsets (X,Y)	7-1-3 7-1-3 [2:0-2-13,Edge], [9:0-1-8,0-2-0]	12-6- 5-4-1		<u>22-9</u> 10-		
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.35 BC 0.21 WB 0.44 Matrix-S	<b>DEFL.</b> in Vert(LL) -0.05 Vert(CT) -0.10 Horz(CT) 0.00 Wind(LL) 0.04	9-10 >999 240 9 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 153 lb         FT = 2	
			BRACING- TOP CHORD BOT CHORD		ectly applied or 6-0-0 oc purlins, 0 oc purlins (6-0-0 max.): 5-7. or 10-0-0 oc bracing, Except:	,

REACTIONS. (size) 9=Mechanical, 2=0-3-0, 10=0-3-8 Max Horz 2=173(LC 8) Max Uplift 9=-14(LC 9), 2=-174(LC 8), 10=-333(LC 8) Max Grav 9=309(LC 24), 2=471(LC 23), 10=1120(LC 23)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-510/370, 3-5=-378/395

BOT CHORD 2-12=-503/421, 10-12=-206/290

WEBS 3-12=-453/319, 3-10=-805/661, 5-10=-576/442

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-5 to 3-5-8, Interior(1) 3-5-8 to 17-1-4, Exterior(2) 17-1-4 to 22-9-13 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

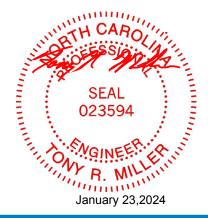
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 2=174, 10=333.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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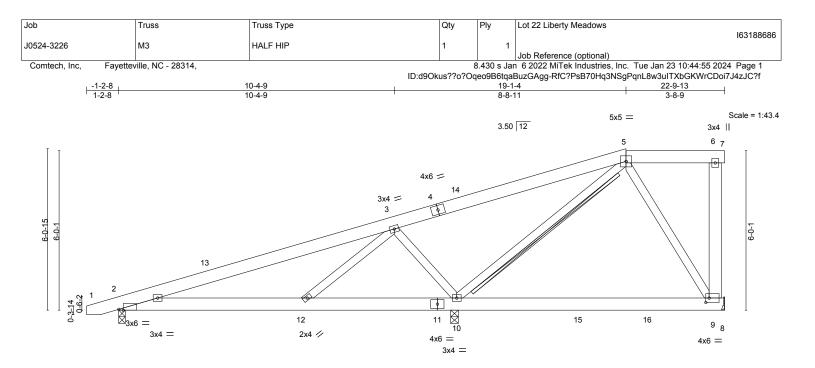


Plate Offsets (X,Y)	7-1-3 7-1-3 [2:0-2-1,Edge], [9:0-1-8,0-2-0]		6-0 12-8 -13 0-2-		<u>22-9-13</u> 10-1-1		
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING-         2-0-0           Plate Grip DOL         1.15           Lumber DOL         1.15	CSI. TC 0.43 BC 0.28	( )	in (loc) -0.07 9-10 -0.12 9-10	l/defl L/d >999 360 >972 240	PLATES MT20	<b>GRIP</b> 244/190
3CLL 0.0 * 3CDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.21 Matrix-S	Horz(CT) Wind(LL)	0.00 9 0.05 2-12	n/a n/a >999 240	Weight: 156 lb	FT = 20%
			BRACING-	) Ctructu	ural wood sheathing dire	with applied on C.O.O.a	a nurlina

 TOP CHORD
 2x6 SP No.1

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x4 SP No.2 \*Except\*

 6-9: 2x6 SP No.1

TOP CHORD BOT CHORD WEBS Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-7. Rigid ceiling directly applied or 10-0-0 oc bracing. T-Brace: 2x4 SPF No.2 - 5-10 Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c.,with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. (size) 9=Mechanical, 2=0-3-0, 10=0-3-8 Max Horz 2=192(LC 8) Max Uplift 9=-18(LC 12), 2=-175(LC 8), 10=-321(LC 8) Max Grav 9=332(LC 2), 2=482(LC 23), 10=1094(LC 23)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-502/368, 3-5=-315/322

BOT CHORD 2-12=-511/407

WEBS 3-12=-521/366, 3-10=-786/593, 5-10=-453/360

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-5 to 3-5-8, Interior(1) 3-5-8 to 19-1-4, Exterior(2) 19-1-4 to 22-9-13 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

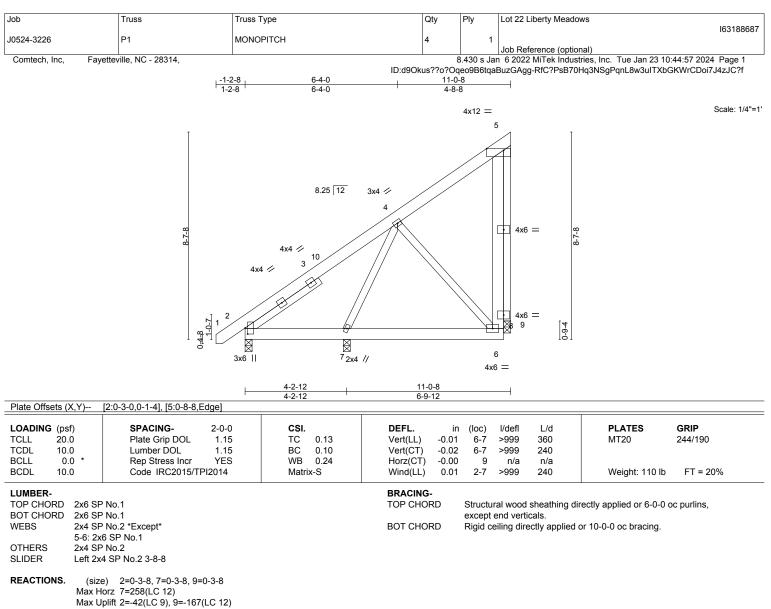
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 2=175, 10=321.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



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Max Grav 2=327(LC 1), 7=335(LC 3), 9=337(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 6-8=-178/332, 5-8=-178/332

BOT CHORD 6-7=-200/265

WEBS 4-6=-367/294, 5-9=-421/229

# NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 10-6-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 9=167.

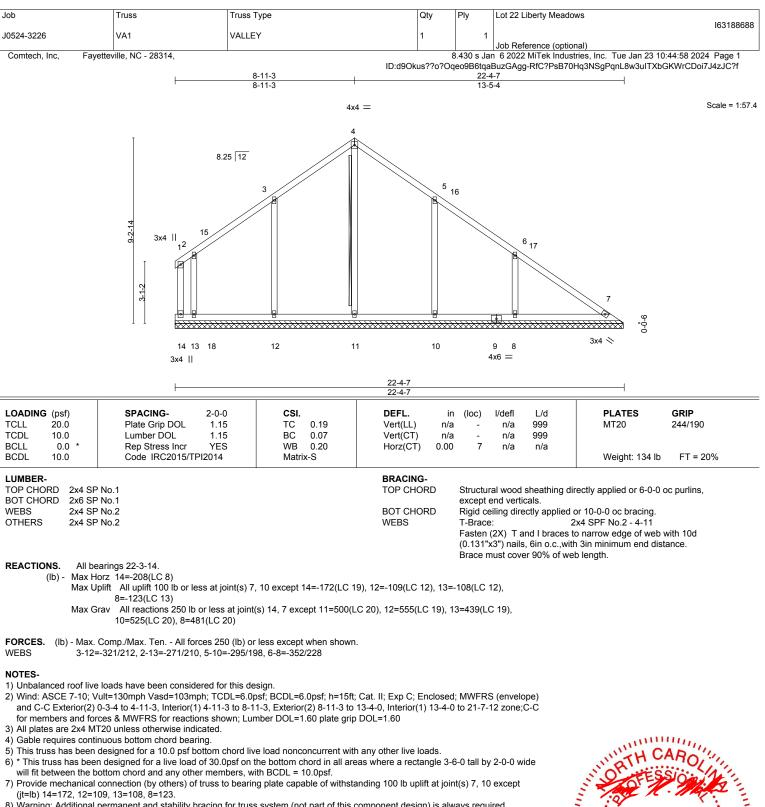


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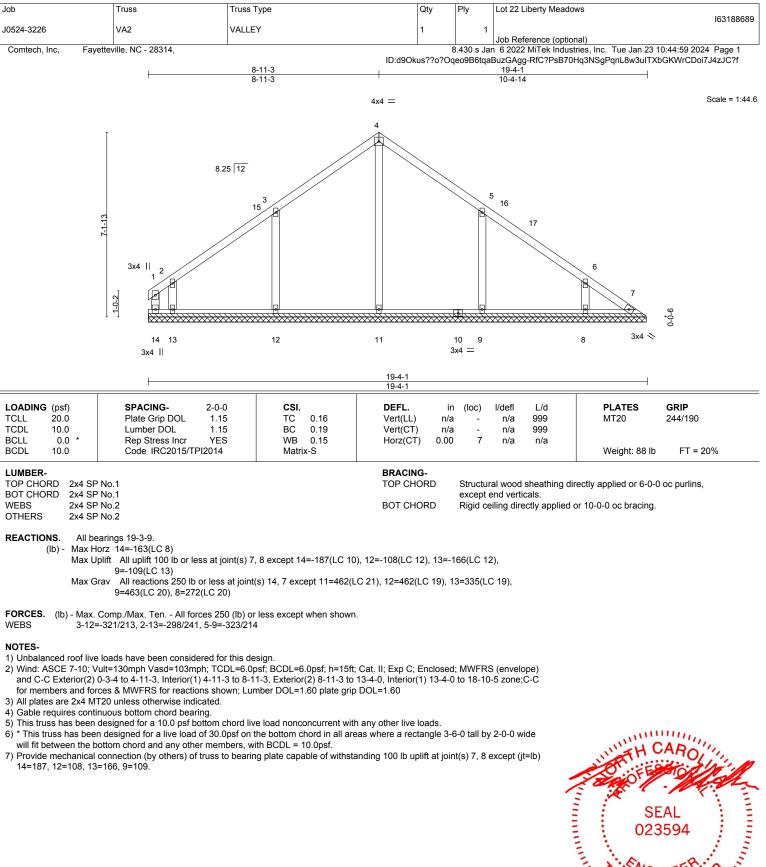
Edenton, NC 27932



8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

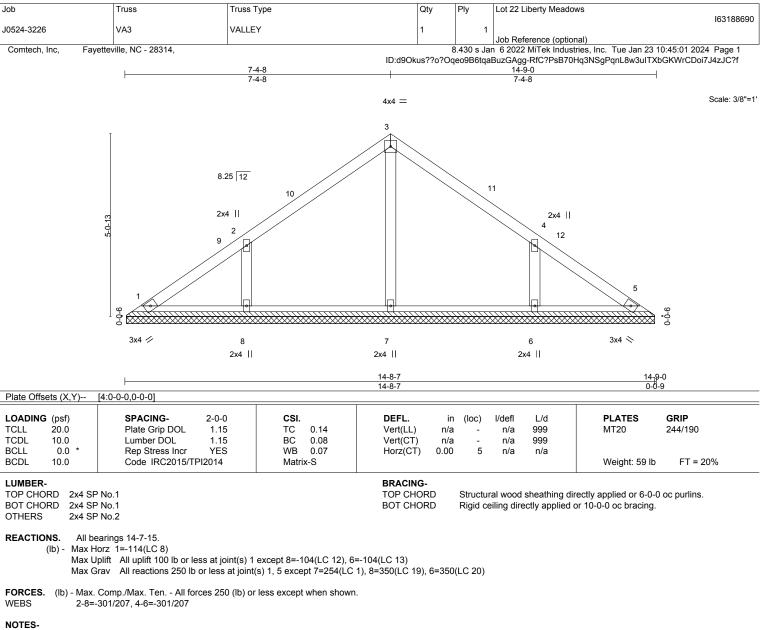


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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-12 to 4-10-9, Interior(1) 4-10-9 to 7-4-8, Exterior(2) 7-4-8 to 11-9-5, Interior(1) 11-9-5 to 14-3-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

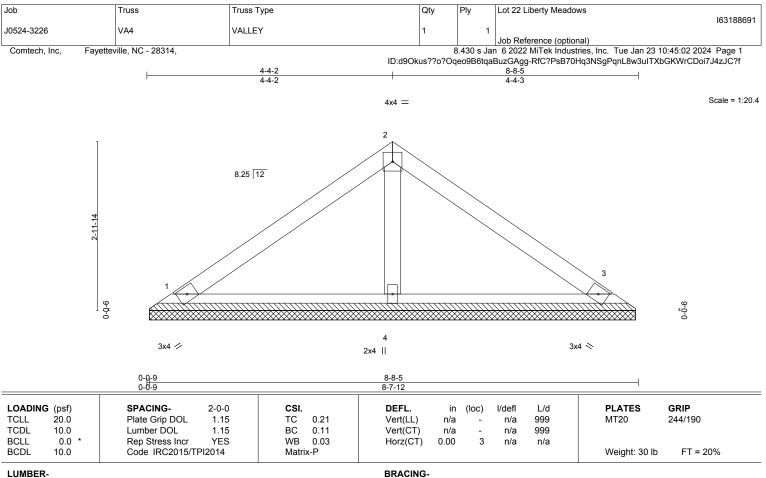
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=104, 6=104.



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818 Soundside Road



TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

OTHERS 2x4 SP No.2

REACTIONS. 1=8-7-3, 3=8-7-3, 4=8-7-3 (size) Max Horz 1=-64(LC 8) Max Uplift 1=-27(LC 12), 3=-33(LC 13) Max Grav 1=169(LC 1), 3=170(LC 1), 4=280(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3. 6) N/A



Structural wood sheathing directly applied or 6-0-0 oc purlins.

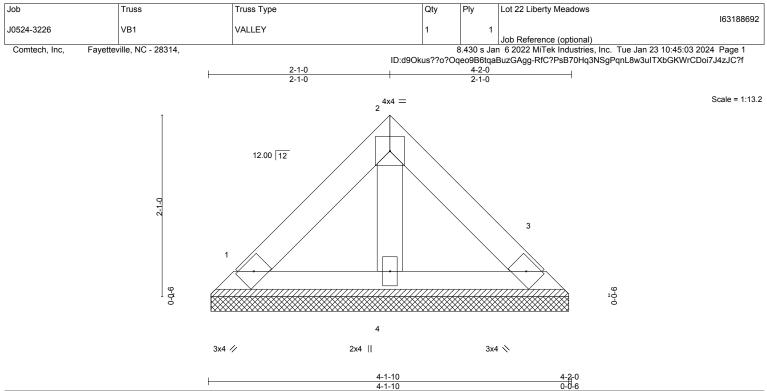
Rigid ceiling directly applied or 10-0-0 oc bracing.

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818 Soundside Road

LUMBER-



				4-1-10			0-0-6	6	
LOADING (psf) TCLL 20.0	SPACING- Plate Grip DOL	2-0-0 1.15	<b>CSI.</b> TC 0.05	DEFL. Vert(LL)	in (lo n/a	oc) l/defl - n/a	L/d 999	-	<b>GRIP</b> 244/190
TCDL 10.0 BCLL 0.0	Lumber DOL Rep Stress Incr	1.15 YES	BC 0.02 WB 0.01	Vert(CT) Horz(CT)	n/a 0.00	- n/a 3 n/a	999 n/a		
BCDL 10.0	Code IRC2015/TF	912014	Matrix-P					Weight: 16 lb	FT = 20%

```
LUMBER-
```

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-2-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=4-1-4, 3=4-1-4, 4=4-1-4 Max Horz 1=-42(LC 8) Max Uplift 1=-15(LC 13), 3=-15(LC 13) Max Grav 1=84(LC 1), 3=84(LC 1), 4=108(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

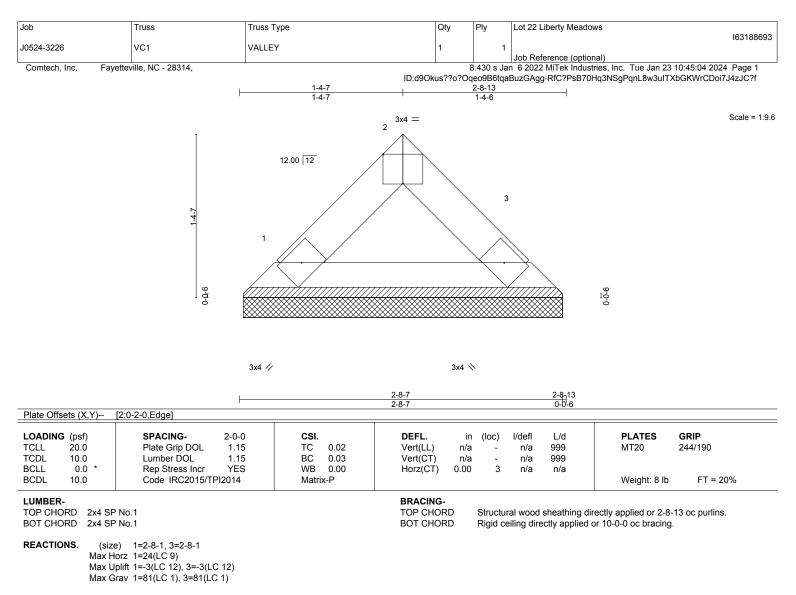
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

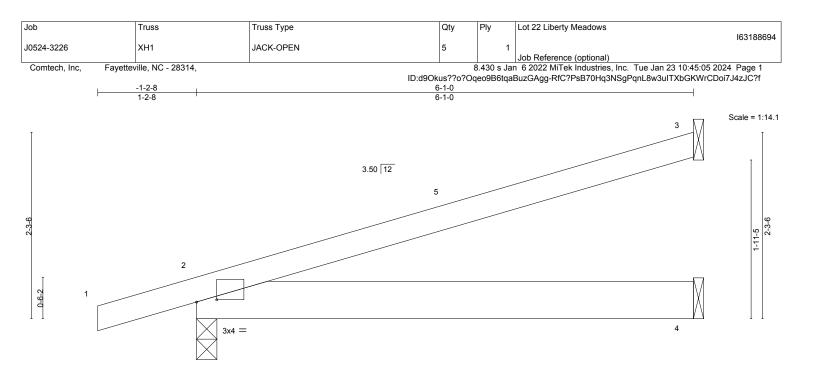
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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	6-1-0 6-1-0										
Plate Offsets (X,Y	- [2:0-2-15,0-0-5]										
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP							
TCLL 20.0	Plate Grip DOL 1.15	TC 0.46	Vert(LL) -0.02 2-4 >999 360	MT20 244/190							
TCDL 10.0	Lumber DOL 1.15	BC 0.14	Vert(CT) -0.03 2-4 >999 240								
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(CT) -0.00 3 n/a n/a								
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.04 2-4 >999 240	Weight: 26 lb FT = 20%							

### LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-0, 4=Mechanical Max Horz 2=72(LC 8) Max Uplift 3=-68(LC 12), 2=-135(LC 8), 4=-30(LC 8)

Max Grav 3=168(LC 1), 2=325(LC 1), 4=118(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 6-0-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

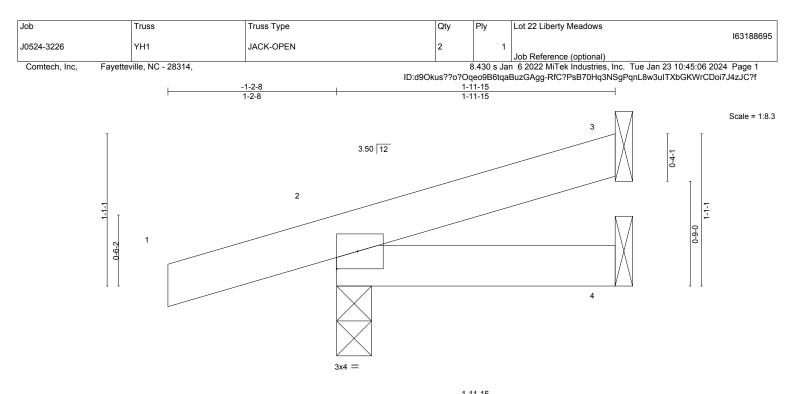
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=135.



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							1-11	-				
LOADING (p	sf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20	0.0	Plate Grip DOL	1.15	тс	0.07	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL 10	0.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	-0.00	2-4	>999	240		
BCLL (	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL 10	0.0	Code IRC2015/TF	PI2014	Matri	ĸ-P	Wind(LL)	0.00	2-4	>999	240	Weight: 8 lb	FT = 20%

# LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1

BRACING-

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 1-11-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-0, 4=Mechanical Max Horz 2=33(LC 8) Max Uplift 3=-20(LC 12), 2=-84(LC 8), 4=-10(LC 8) Max Grav 3=35(LC 1), 2=176(LC 1), 4=39(LC 3)

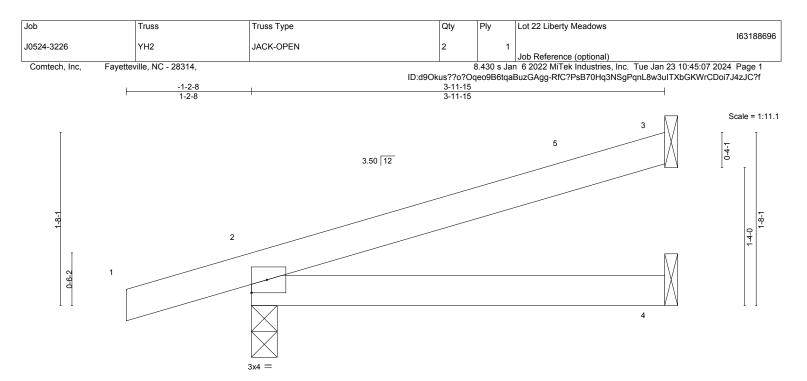
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.



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	3-11-15 3-11-15									
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP							
TCLL 20.0	Plate Grip DOL 1.15	TC 0.15	Vert(LL) -0.01 2-4 >999 360 MT20 244/190							
TCDL 10.0	Lumber DOL 1.15	BC 0.13	Vert(CT) -0.02 2-4 >999 240							
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) -0.00 3 n/a n/a							
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.02 2-4 >999 240 Weight: 14 lb FT = 20%							

# LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-11-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-0, 4=Mechanical Max Horz 2=51(LC 8) Max Uplift 3=-44(LC 12), 2=-108(LC 8), 4=-20(LC 8)

Max Grav 3=100(LC 1), 2=246(LC 1), 4=76(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 3-11-3 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=108.



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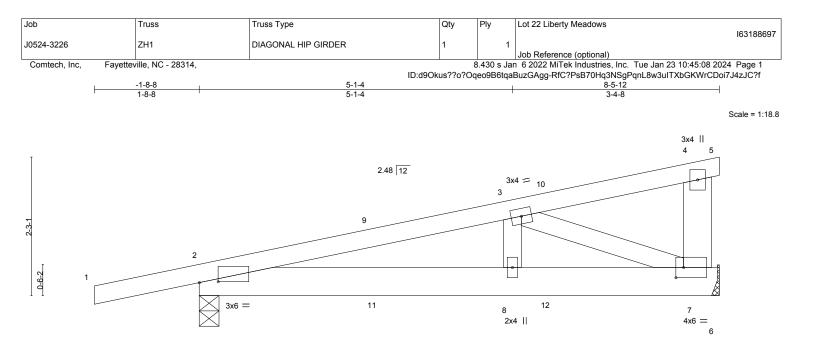


Plate Offsets (X,Y)	[2:0-3-11,0-0-3], [7:0-1-8,0-2-0]	5-1-4 5-1-4				8-5-12 3-4-8	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.20 BC 0.14 WB 0.12 Matrix-P	<b>DEFL.</b> in Vert(LL) 0.02 Vert(CT) -0.02 Horz(CT) 0.00	2-8 >999 2-8 >999		PLATES MT20 Weight: 44 lb	<b>GRIP</b> 244/190 FT = 20%
			BRACING- TOP CHORD BOT CHORD	except end ve	rticals.	lirectly applied or 6-0-0 l or 10-0-0 oc bracing.	) oc purlins,
Max I Max I	ze) 7=Mechanical, 2=0-3-14 Horz 2=71(LC 19) Jplift 7=-150(LC 4), 2=-203(LC 4) Grav 7=370(LC 1), 2=458(LC 1)						
TOP CHORD 2-3= BOT CHORD 2-8=	. Comp./Max. Ten All forces 250 (lb) -639/228 -255/587, 7-8=-255/587 -631/274	or less except when shown.					
<b>NOTES-</b> 1) Wind: ASCE 7-10;	Vult=130mph Vasd=103mph; TCDL=6		Cat. II; Exp C; Enclosed	; MWFRS (env	elope);		

- porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=150, 2=203.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 17 lb down and 17 lb up at 2-10-15, 17 lb down and 17 lb up at 2-10-15, and 40 lb down and 54 lb up at 5-8-14, and 40 lb down and 54 lb up at 5-8-14 on top chord, and 3 lb down and 22 lb up at 2-10-15, 3 lb down and 22 lb up at 2-10-15, and 21 lb down and 22 lb up at 5-8-14, and 21 lb down and 42 lb up at 5-8-14 on top chord, and 42 lb up at 5-8-14 on toptodown and 42 lb up at 5-8-14 on toptodown and 42 lb up at 5-8-14 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
  7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

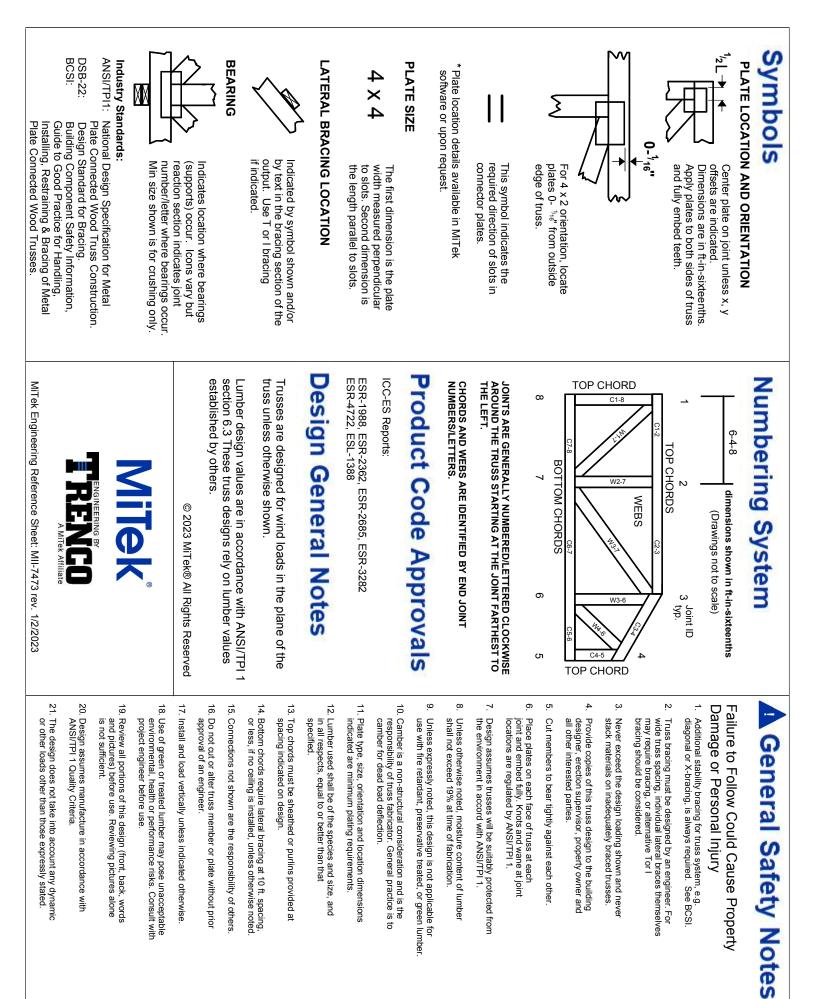
# LOAD CASE(S) Standard

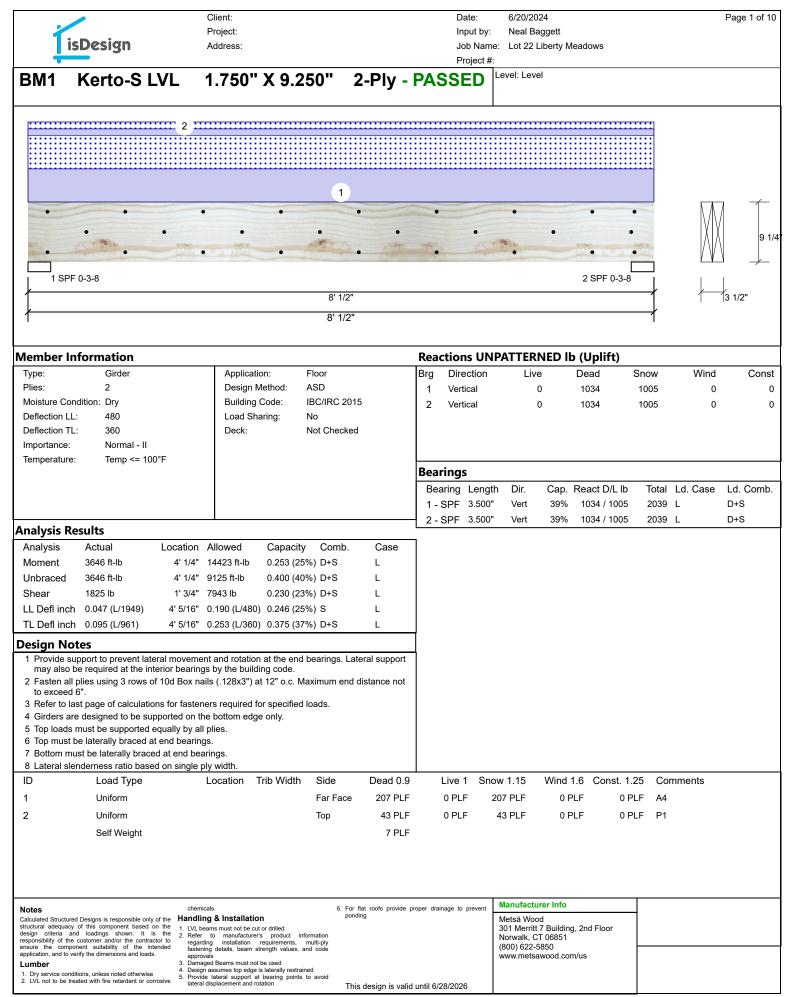
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-4=-60, 4-5=-60, 2-6=-20 Concentrated Loads (lb) Vert: 10=-31(F=-15, B=-15) 12=-20(F=-10, B=-10)

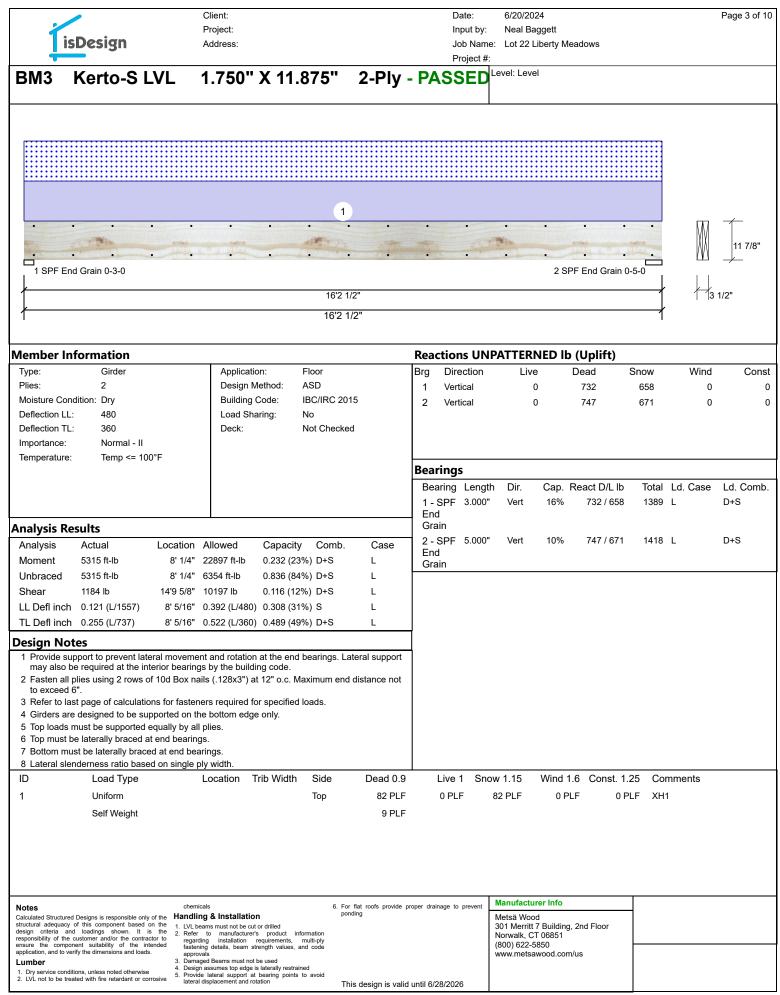


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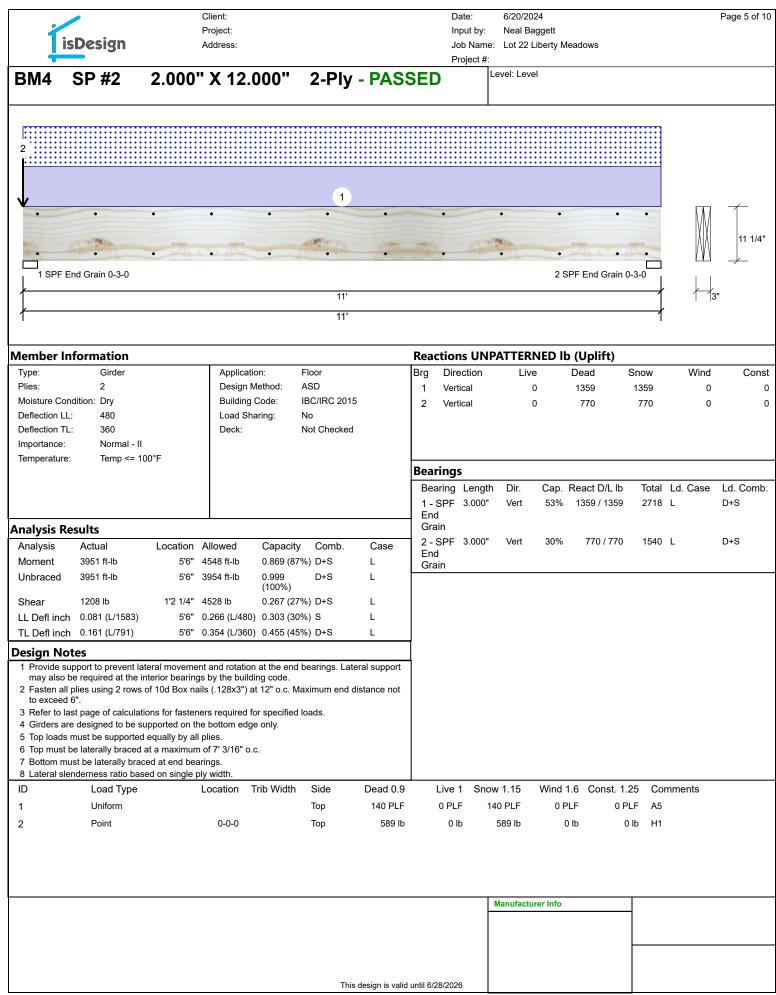


Í	isDesign	Client: Project: Address:			e: Lot 22 Liberty Mea	dows	Page 2 of 10
BM1	Kerto-S LV	L 1.750" X 9.250	' 2-Ply - P	Project #	Level: Level		
•	• •	• •	•	•	•	•••	1/1/2 1/1/2 1/4
	• PF 0-3-8	• •	•	•	•	• • •	
	iff 0-3-0		8' 1/2" 8' 1/2"			2 3FF 0-3-6	3 1/2"
-	y Analysis	of 10d Box nails (.128x3") at 12					
Capacity Load Yield Limit pe GM Yield Mode Edge Distand Min. End Dis Load Combir Duration Fac	2 er Foot 2 er Fastener 9 1 v ce 1 stance 3 nation E	/ 1/2"					
structural adequidesign criteria responsibility of ensure the co- application, and the Lumber 1. Dry service c	stured Designs is responsible only of this component based on the and loadings shown. It is the the customer and/or the contractor tre imponent suitability of the intender to verify the dimensions and loads. conditions, unless noted otherwise e treated with fire retardant or corrosivu	1. IVL beams must not be cut or drilled     2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals     3. Damaged Beams must not be used     4. Design assumes top edge is laterally restrained			Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2 Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/u		

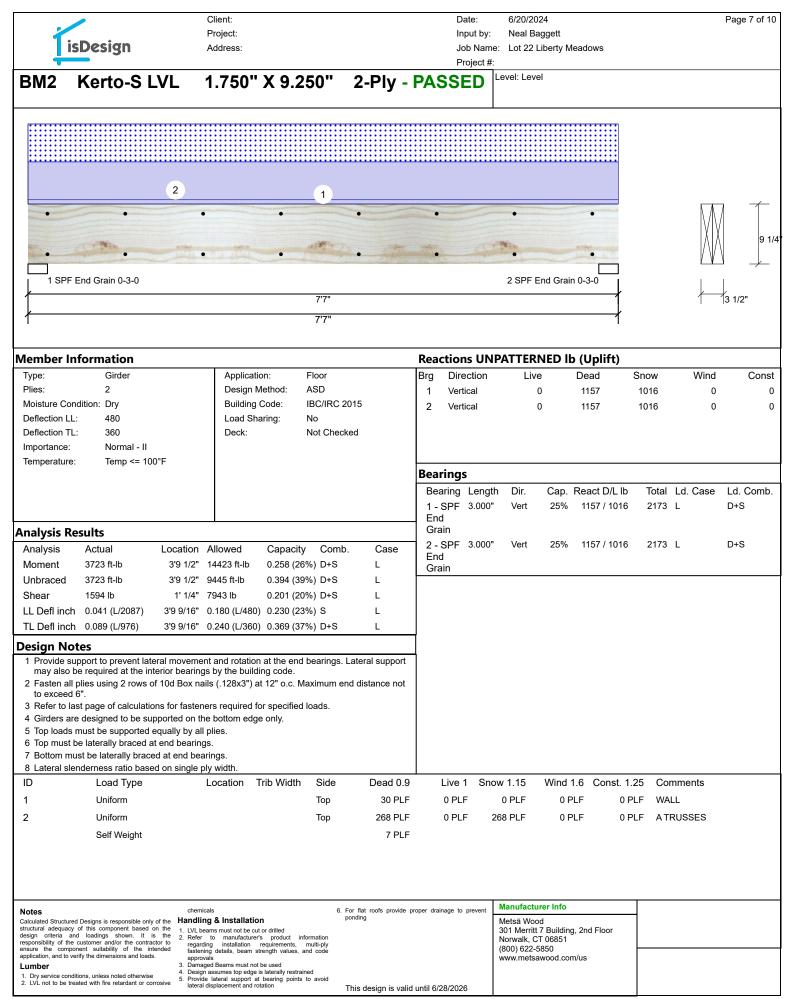


Į	isDesign		Client: Project: Address:				Date: Input by: Job Name: Project #:	6/20/2024 Neal Baggett Lot 22 Liberty Meadows	Page 4 of 10
BM3	Kerto-S	LVL	1.750"	X 11.875	5" 2-F	Ply - PA		evel: Level	
•	• •	•	• •	• •	•	• •	•	×1 1/2"	11 7/8"
1 SPF E	• • End Grain 0-3-0	•	• •	• •	•	<u></u>	•	2 SPF End Grain 0-5-0	
					16'2 1/2" 16'2 1/2"				3 1/2"
	Analysis								
apacity oad ield Limit per ield Limit per ield Mode idge Distance din. End Dista oad Combina uration Facto	r Fastener e ance ation	0.0 % 0.0 PLF 163.7 F 81.9 lb. 1 IV 1 1/2" 3" 1.00	PLF						

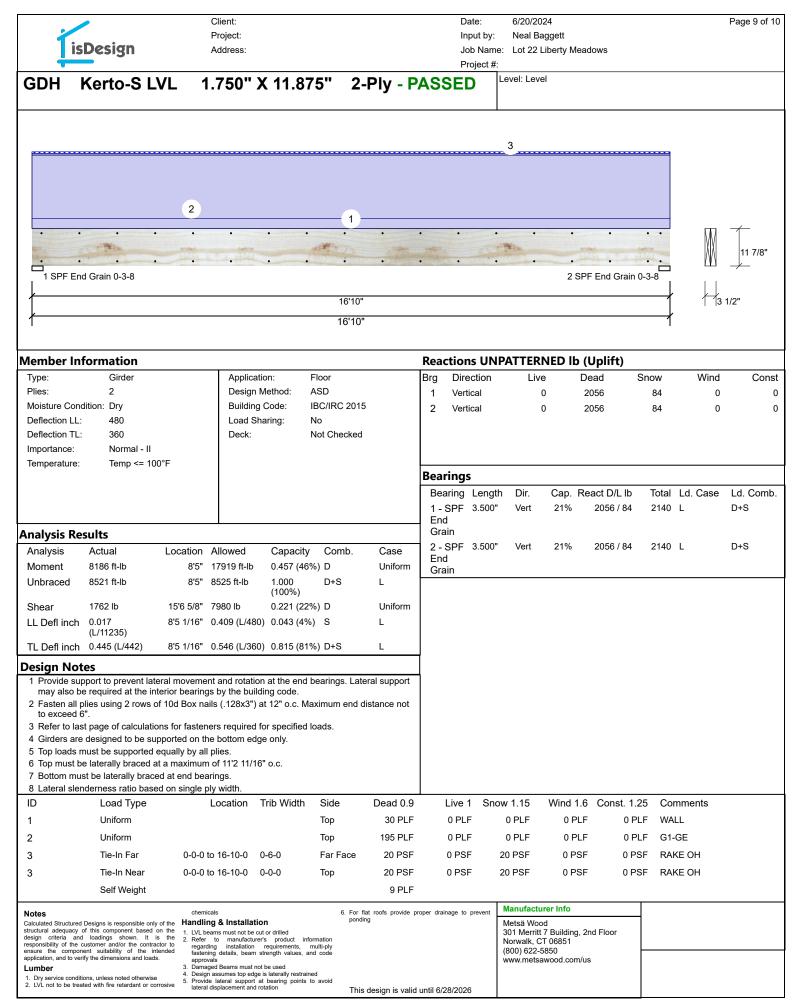
Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. <b>Lumber</b> 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	LVL beams must not be cut or drilled     Refer to manufacturer's product information     regarding installation requirements multi-ply	ponding This design is valid until 6/28/2026	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	



ĺ	isDesign	I	Client: Project: Address:			Date: Input by: Job Name Project #:	6/20/2024 Neal Baggett Lot 22 Liberty Meadows	Page 6 of 10
BM4	SP #2	2.000"	X 12.000	" 2-Ply	- PASSED		evel: Level	
•	•	•	• •	•	•	•	• • •	•
•	•	•	• •	•	•	•	• • •	•
1 SPF	End Grain 0-3-0						2 SPF End Grain (	
				11' 11'				1 13"
 lulti-Ply	/ Analysis							
-	-	ows of 10d B	ox nails (.128x3	8") at 12" o.c N	/laximum end di	istance no	t to exceed 6".	
oad eld Limit pe	er Foot	0.0 PLF 202.6 PLF						
٨	er Fastener	101.3 lb. 1						
eld Mode Ige Distand		IV 1 1/2"						
in. End Dis oad Combir		3"						
uration Fac	tor	1.00						
						Г	Manufacturer Info	
						F		-
				This o	design is valid until 6/28	/2026		1



		Client: Project:			Date: Input by:	6/20/2024 Neal Baggett	F	Page 8 of 10
1	isDesign	Address:				e: Lot 22 Liberty Meadows		
BM2	Kerto-S L	VL 1.750	" X 9.250"	2-Ply		Level: Level		
						I		
	•	•	•		•			
	•	•	•	•	•		<pre>&lt;11/2"</pre>	9 1/
	• PF End Grain 0-3-0	•	•	•	•	• • •		
			7'7" 7'7"					5 1/2"
Fasten al	<b>y Analysis</b> I plies using 2 row		<u>(.128x3") at</u> 12" o	o.c Maximur	n end distance r	ot to exceed 6".		
Capacity Load Yield Limit p	per Foot	0.0 % 0.0 PLF 163.7 PLF						
Yield Limit p См	per Fastener	81.9 lb. 1						
Yield Mode Edge Distan Min. End Dis	nce	IV 1 1/2" 3"						
Load Combi Duration Fac		1.00						
Notes		chemicals		5. For flat roofs provide ponding	proper drainage to prevent	Manufacturer Info	_	
structural adeque design criteria responsibility of	ctured Designs is responsible only juacy of this component based o a and loadings shown. It is f the customer and/or the contrac omponent suitability of the inte	n the 1. LVL beams must not b the 2. Refer to manufac tor to regarding installation	e cut or drilled lurer's product information n requirements, multi-ply			Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850		
application, and Lumber 1. Dry service	to verify the dimensions and loads. conditions, unless noted otherwise	3. Damaged Beams mus 4. Design assumes top e	dge is laterally restrained			www.metsawood.com/us		
2. LVL not to b	be treated with fire retardant or corr	osive 5. Provide lateral support lateral displacement a	rt at bearing points to avoid nd rotation	This design is va	lid until 6/28/2026			



-			Client:			Date:		6/20/2024		Page 10 of
<b>T</b> i	isDesign		Project: Address:			Input b Job Na		Neal Baggett Lot 22 Liberty Meadows		
						Project	t #:			
GDH	Kerto-S L	.VL	1.750"	X 11.875"	2-Ply	PASSED	Le	evel: Level		
•	• •	•	• •	• •	• •	• •	•		· · · ē	$\Pi \neq$
									<112	11 7/8"
Ŀ	•••	•	• •	• •	• •	• •	•	• • •	<u>· ; † ‡</u>	
1 SPF E	End Grain 0-3-8							2 SPF End Grain (	)-3-8 (`	l. l.
1					16'10"					3 1/2"
ſ					16'10"				{	
/lulti-Ply	Analysis									
	plies using 2 rov			(.128x3") at 12"	o.c Maxim	um end distance	not	t to exceed 6".		
apacity oad		5.3 % 10.0 I								
eld Limit per	r Foot	188.3								
eld Limit per	r Fastener	94.1 I	b.							
w eld Mode		1 IV								
dge Distance		1 1/2'	,							
in. End Dista		3" D+S								
uration Facto		1.15								
lotoo			chemicals		6 For flat roofs pro	vide proper drainage to prever	nt N	Manufacturer Info		
Notes Calculated Structu	red Designs is responsible on	y of the Ha	andling & Installa		ponding	vide proper dramage to prever	N	Metsä Wood		
esian criteria	cy of this component based and loadings shown. It he customer and/or the contra	is the 2	LVL beams must not be Refer to manufacture regarding installation	rer's product information			N	301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851		
pplication, and to	he customer and/or the contra ponent suitability of the in verify the dimensions and load	IS.	fastening details, bean approvals	requirements, multi-ply strength values, and code			(	800) 622-5850 www.metsawood.com/us		
Lumber	nditions, unless noted otherwis	3. 4.	Damaged Beams must Design assumes top ed	ge is laterally restrained						
2. LVL not to be 1	nditions, unless noted otherwis treated with fire retardant or o	e r	Provide lateral support lateral displacement and	at bearing points to avoid		valid until 6/28/2026				

This design is valid until 6/28/2026