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The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 49177 JOB: 24-4510-R01 JOB NAME: LOT 1 PROVIDENCE CREEK Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 35 These truss designs comply with IRC 2015 as well as IRC 2018. 20 Truss Design(s)

Trusses:

J01, J03, J04, J05, J06, R01, R02, R02A, R03, R04, R05, R06, R07, R08, R09, SP01, SP02, V01, V02, V03



Warning !--- Verify design parameters and read notes before use.

This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to



LOAD CASE(S) Standard







NOTES- (9)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 3, 68 lb uplift at joint 2 and 11 lb uplift at joint 4.







Max Horz 2=82(LC 11) Max Uplift4=-77(LC 8), 2=-113(LC 8) Max Grav 4=288(LC 19), 2=446(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (11)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads. 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 77 lb uplift at joint 4 and 113 lb uplift at joint

9) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

2

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-60, 4-5=-20 Concentrated Loads (lb) Vert: 8=-43(F) 9=-123(F)





exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads.

LOAD CASE(S) Standard

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 41 lb uplift at joint 4 and 87 lb uplift at joint 2.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum

sheetrock be applied directly to the bottom chord.



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		9-6-14		9-5-2		9-5-2		9-3-6	
Plate Offsets	(X,Y) [2:0-3	-0,0-1-12]				002		000	
LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	sf) 20.0 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/TI	2-0-0 1.15 1.15 YES PI2014	CSI. TC 0.77 BC 0.96 WB 0.71 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.37 14-16 >999 -0.59 14-16 >755 0.13 11 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 212 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SP No.2 2x4 SP No.2 B2: 2x4 SP N 2x4 SP No.3	*Except* No.1			BRACING- TOP CHORD BOT CHORD WEBS	Structural wood she Rigid ceiling directly 1 Row at midpt MiTek recommend be installed during Installation guide	athing directly applied. 5-14, 7- s that Stabilize truss erection	applied, except end 14, 3-17, 9-11 ers and required cros , in accordance with	verticals. s bracing Stabilizer
REACTIONS.	(lb/size) 17 Max Horz 17 Max Uplift17	/=1559/0-3-8 (min. 0-1- /=108(LC 14) /=-207(LC 14), 11=-185	13), 11=1496/ (LC 15)	Mechanical					
FORCES. (IL TOP CHORD BOT CHORD WEBS	 b) - Max. Comp 2-18=-571/ 5-6=-1954/ 9-21=-334/ 16-17=-377 13-23=-222 5-16=-45/4 3-17=-2256 	5./Max. Ten All forces 93, 3-18=-457/105, 3-4= 390, 6-7=-1954/390, 7-2 82, 10-21=-415/70, 2-17 7/2390, 15-16=-225/209 7/2080, 12-13=-222/208 41, 5-14=-712/225, 6-14 5/314, 9-11=-2339/337	250 (lb) or les 2592/388, 4 20=-2402/404 7=-446/148, 1 8, 15-22=-225 0, 11-12=-298 I=-165/1257, 1	ss except when shown -19=-2521/402, 5-19=- , 8-20=-2474/402, 8-9= 0-11=-305/88 i/2098, 14-22=-225/20 i/2312 7-14=-684/222, 7-12=-	n. -2448/404, =-2546/388, 198, 14-23=-222/208 -39/399,	30,			
NOTES- (1 1) Unbalance 2) Wind: ASC (envelope) Interior(1) 2 exposed;C 3) TCLL: ASC Cat B; Part 4) Unbalance 5) This truss I non-concul 6) This truss I 7) * This truss between th 8) Refer to gir 9) Provide me 11=185. 10) This truss sheetrock	1) droof live load E 7-16; Vult=' gable end zor 24-0-15 to 32-4 -C for membe CE 7-16; Pr=20 tially Exp.; Ce- d snow loads has been desi rrent with othe has been desi s has been desi e bottom chor rder(s) for trus echanical conr s design requir c be applied di	ds have been considere 120mph (3-second gust) he and C-C Exterior(2E) 9-2, Exterior(2E) 32-9-2 rs and forces & MWFRS 0.0 psf (roof LL: Lum DC =1.0; Cs=1.00; Ct=1.10 have been considered fi gned for greater of min in r live loads. gned for a 10.0 psf bottot signed for a 10.0 psf bottot signed for a live load of d and any other membe s to truss connections. hection (by others) of tru es that a minimum of 7/ rectly to the bottom choil	d for this desi) Vasd=95mpl -0-10-8 to 3- to 37-6-12 zo 6 for reactions DL=1.15 Plate or this design. roof live load of com chord live 30.0psf on the rs, with BCDL ss to bearing 16" structural rd.	gn. 1; TCDL=5.0psf; BCDI 1-2, Interior(1) 3-11-2 ne; cantilever left and shown; Lumber DOL= DOL=1.15); Pf=20.0 p of 12.0 psf or 2.00 time load nonconcurrent wi e bottom chord in all a . = 10.0psf. plate capable of withs wood sheathing be ap	L=5.0psf; h=35ft; Ca to 13-11-1, Exterio right exposed ; end =1.60 plate grip DO osf (Lum DOL=1.15 es flat roof load of 2 ith any other live loa reas where a rectar tanding 100 lb uplift oplied directly to the	at. II; Exp B; Enclosed r(2R) 13-11-1 to 24-0 l vertical left and right L=1.60 Plate DOL=1.15); Is= 0.0 psf on overhangs ads. ngle 3-6-0 tall by 1-0-0 t at joint(s) except (jt= top chord and 1/2" gy	d; MWFRS -15, -1.0; Rough) wide will fill b) 17=207, ypsum	SEAL 28147	ALL THE REPORT OF THE PARTY OF

LOAD CASE(S) Standard

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of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



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- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 11.

LOAD CASE(S) Standard



vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job Tru	russ	Truss Type	Qty	Ply	LOT 1 PROVIDENCE CREEK 81 PROVIDENCE CREEK DRIVE FUQUAY-V
24-4510-R01 R0	07	Common Girder	1	2	Job Reference (optional) # 49177

8.430~s Feb 12 2021 MiTek Industries, Inc. Thu May 30 07:48:07 2024 Page $\widehat{2}$ ID:zSnl_VDJTy_hu?pmOjJgrKyZRJv-l1Du_nSNfqRotqzoniGVxolA0IJ1_k1vxMcjCRzBOG6

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 11-15=-20

Concentrated Loads (lb)

Vert: 17=-1563(B) 19=-1476(B) 20=-1476(B) 21=-1476(B) 22=-1476(B) 23=-1476(B)



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Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 15, 12, 11 Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(14)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 4-0-0, Corner(3R) 4-0-0 to 8-0-0, Corner(3E) 8-0-0 to 12-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 11.

LOAD CASE(S) Standard

SEAL 28147 9/20/ 9/20/ 1



Job	Truss	Truss Type	Qty	Ply	LOT 1 PROVIDENCE CREEK 81 PROVIDE	ENCE CREEK DRIVE FUQUAY-VARI
24-4510-R01	R09	Common Girder	1	2	Job Reference (optional)	# 49177

8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 30 07:48:09 2024 Page 2 ID:zSnl_VDJTy_hu?pm0jJgrKyZRJv-hQLePSUdARhW786Au7Iz0DqZj6ySSd9BPg5qHKzBOG4

LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 7-11=-20

Concentrated Loads (lb) Vert: 6=-1476(B) 15=-1476(B) 16=-1476(B) 17=-1476(B) 18=-1476(B)







vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



- **NOTES-** (9)
- 1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 1=105/4-1-0 (min. 0-1-8), 3=105/4-1-0 (min. 0-1-8) Max Horz 1=-8(LC 15) Max Uplift1=-13(LC 14), 3=-13(LC 15) Max Grav 1=111(LC 20), 3=111(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (9)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough
- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

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LOAD CASE(S) Standard
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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard

