						-
ABBREVI	ATIONS	IND	ΕX			
ABV ABOVE						
A.D. AREA DRAIN	L LENGTH LA LAUNDRY	MODEL	HAYDEN'			
ADJ ADJUSTABLE ALT ALTERNATE ALUM ALUMINUM	LAV LAVATORY LVR LOUVER	0	TITLE SHEET / COVER SHEET	ΙK	FRONT ELEVATION 'K'	
ARCH. ARCHITECTURAL	MAX MAXIMUM MECH MECHANICAL MFR. MANUFACTURER	0.1	QUICK VIEW	1.1 K	ROOF PLAN 'K'	
BA BATHROOM BD BOARD BF BI-FOLD (DOOR)	MIN MINIMUM MISC MISCELLANEOUS	0.2	QUICK VIEW	2K	SIDE AND REAR ELEVATIONS 'K'	
BLDG BUILDING BLK BLOCK (CMUs)	N NORTH N.T.S. NOT TO SCALE O.G.D. OVERHEAD GARAGE DOOR	IA	FRONT ELEVATION 'A'	2.I K	SIDE AND REAR ELEVATIONS 'K'-	
BLW BELOW BM BEAM	OH OVERHEAD	1.1 A	ROOF PLAN 'A'		W CRAWL SPACE	
BP BI-PASS (DOOR) BOT BOTTOM	OPT OPTIONAL PAR PARALLEL P.B. PUSH BUTTON	2 A	SIDE AND REAR ELEVATIONS 'A'	2.2 K	SIDE AND REAR ELEVATIONS 'K'-	
BTWN BETWEEN CAB CABINET	PDR POWDER PED PEDESTAL	2.I A	SIDE AND REAR ELEVATIONS 'A'-		W/ BASEMENT	
CER CERAMIC C.J. CONTROL JOINT OR CONSTRUCTION JOINT	PL PLATE		W CRAWL SPACE	3 MS K	MONOLITHIC SLAB PLAN 'K'	
CL CLOSET OR CENTER LINE CLG CEILING CLR CLEAR	P.T. PRESSURE TREATED WOOD PVC POLITVINYL CHLORIDE PIPE	2.2 A	SIDE AND REAR ELEVATIONS 'A'-	3 SW K	STEM WALL PLAN 'K'	
CMU CONCRETE MASONRY UNIT COL COLUMN	PVMT PAVEMENT P.W. PRE-WIRE PWD PLYWOOD	3 MS A	W/ BASEMENT MONOLITHIC SLAB PLAN 'A'	3 CS K 3 BS K	CRAWL SPACE PLAN 'K' BASEMENT PLAN 'K'	
CONC CONCRETE C CARPET CR CORROSION RESISTANT	R RISER RAG RETURN AIR GRILL	3 SW A	STEM WALL PLAN 'A'	3 65 K 4 K	IST FLOOR PLAN 'K'	
CONT CASEMENT C.T. CERAMIC TILE	REF REFERENCE REFR REFRIGERATOR	3 CS A	CRAWL SPACE PLAN 'A'	4 K 5 K	2ND FLOOR PLAN 'K'	1 1 1
D DRYER DBL DOUBLE	REQ REGUIRED 5 SOUTH 5D SMOKE DETECTOR 5F, SQUARE FEET	3 BS A	BASEMENT PLAN 'A'	<u> </u>	200 ILCONTENT N	
DH DOUBLE HUNG DIM DIMENSION	5.6.D. SLIDING GLASS DOOR	4 A	IST FLOOR PLAN 'A'	IP	FRONT ELEVATION 'P'	
DISP DISPOSAL DN DOWN DR DOOR	SH SINGLE HUNG OR SHELF	5 A	2ND FLOOR PLAN 'A'	I.I P	ROOF PLAN 'P'	
DISP DISPOSIL DIN DOWN DR DOWR DS DOWNSPOUT DW DISH WASHER	SIM SIMILAR SL SLOPE / SLIDING S4P SHELF AND POLE SPEC SPECIEICATIONS			2P	SIDE AND REAR ELEVATIONS 'P'	
DWG DRAWING	SPEC SPECIFICATIONS STD STANDARD STR STRUCTURAL SQ SQUARE	IВ	FRONT ELEVATION 'B'	2.I P	SIDE AND REAR ELEVATIONS 'P'-	
E EAST EA EACH ELEV ELEVATION	SQ SQUARE SYM SYMBOL S4S SMOOTH FOUR SIDES	I.I B	ROOF PLAN 'B'		W/ CRAWL SPACE	
ELEC ELECTRICAL EQ EQUAL	545 SMOOTH FOUR SIDES T TREAD (AT STAIRS) OR TILE T.B. TOWEL BAR	2 B	SIDE AND REAR ELEVATIONS 'B'	2.2 P	SIDE AND REAR ELEVATIONS 'P'-	
EXT EXTERIOR FAU FORCED AIR UNIT	TEMP. TEMPERED (GLASS) T&G TONGUE & GROOVE	2.I B	SIDE AND REAR ELEVATIONS 'B'		W/ BASEMENT	
F.C. FLOOR CHANGE F.D. FLOOR DRAIN FEL FINISH FLOOR LINE	T.O.C. TOP OF CURB TV TELEVISION		W CRAWL SPACE	3 MS P	MONOLITHIC SLAB PLAN 'P'	
FFL FINISH FLOOR LINE F.G. FINISHED GRADE FLR FLOOR(ING)	TYP TYPICAL U.N.O. UNLESS NOTED OTHERWISE	2.2 B	SIDE AND REAR ELEVATIONS 'B'-	3 SW P	STEM WALL PLAN 'P'	
FL FLOURESCENT (LIGHT) FND FOUNDATION	V.B. VAPOR BARRIER VERT VERTICAL		W/ BASEMENT	3 CS P	CRAWL SPACE PLAN 'P'	
F.O.S. FACE OF STUD	V.T.R. VENT THRU ROOF W WASHING MACHINE WD WOOD	3 MS B	MONOLITHIC SLAB PLAN 'B'	3 BS P	BASEMENT PLAN 'P'	
FX FIXED GLASS GALV GALVANIZED GAR GARAGE	WD WOOD WDW WINDOW WH WATER HEATER	3 SW B	STEM WALL PLAN 'B'	4 P	IST FLOOR PLAN 'P'	
G.B. GYPSUM BOARD GD GRADE OR GRADING	WI WROUGHT IRON	3 CS B	CRAWL SPACE PLAN 'B'	5 P	2ND FLOOR PLAN 'P'	
G.D.O. GARAGE DOOR OPENER GFI GROUND FAULT INTERRUPTER	WP WATERPROOF(ING)	3 BS B	BASEMENT PLAN 'B'	1.5		
GL GLASS OR GLAZING GYP BD GYPSUM BOARD	NUM WELDED WIRE MESH R. PROPERTY LINE	4 B 5 B	IST FL <i>OO</i> R PLAN 'B' 2ND FL <i>OO</i> R PLAN 'B'	I R I.I R	FRONT ELEVATION 'R' ROOF PLAN 'R'	
HB HOSE BIBB HD HEAD OR HARD HDR HEADER	Ø ROUND / DIAMETER	50	2ND TEOOR FEAN B	1.1 R 2R	SIDE AND REAR ELEVATIONS 'R'	
HGT HEIGHT HVAC HEATING/VENTILATING/AIR COND.	6. CENTERLINE # POUND / NUMBER	IF	FRONT ELEVATION 'F'	2.I R	SIDE AND REAR ELEVATIONS 'R'-	
HIND HARDWOOD INT INTERIOR JST JOIST JT JOINT		LE	ROOF PLAN 'F'	2.118	W CRAWL SPACE	
JST JOIST JT JOINT KIT KITCHEN		2 F	SIDE AND REAR ELEVATIONS 'F'	2.2 R	SIDE AND REAR ELEVATIONS 'R'-	
		2.I F	SIDE AND REAR ELEVATIONS 'F'-		W/ BASEMENT	
BUILDING CODE	COMPLIANCE /		W CRAWL SPACE	3 MS R	MONOLITHIC SLAB PLAN 'R'	
PROJECT INFORM		2.2 F	SIDE AND REAR ELEVATIONS 'B'-	3 SW R	STEM WALL PLAN 'R'	
			W/ BASEMENT	3 CS R	CRAWL SPACE PLAN 'R'	
ALL CONSTRUCTION TO COMPLY WITH LE CURRENTLY IN USE WITH THE LOCAL JUR	OCAL CODES AND ORDINANCES	3 MS F	MONOLITHIC SLAB PLAN 'F'	3 BS R	BASEMENT PLAN 'R'	
		3 SW F	STEM WALL PLAN 'F'	4 R	IST FLOOR PLAN 'R'	
APPLICABLE CODES: FOLLOW ALL APPLICABLE STATE AND L	OCAL CODES.	3 CS F	CRAWL SPACE PLAN 'F'	5 R	2ND FLOOR PLAN 'R'	
2018 NORTH CAROLINA STATE SUPPLEME		3 BS F	BASEMENT PLAN 'F'			
		4 F 5 F	IST FLOOR PLAN 'F'	IAS	BUILDING SECTIONS	
CONTRACTOR AND BUILDER SHALL REVI CONFORMANCE WITH ALL CURRENT APPL		51	2ND FLOOR PLAN 'F'	I.I A S	BUILDING SECTIONS	
CONSTRUCTION. BY USING THESE DRAWIN	IGS FOR CONSTRUCTION IT IS			1.1.2 A S	BUILDING SECTIONS BUILDING SECTIONS	
UNDERSTOOD THAT CONFORMANCE WITH RESPONSIBILITY OF THE BUILDER AND C	CONTRACTOR.			1.1.5 A S	BUILDING SECTIONS	
PRODUCT:				6	BASEMENT UTILITY PLAN	
SINGLE FAMILY RESIDENCE				7	IST FLOOR UTILITY PLAN	
OCCUPANCY CLASSIFICATION				8	2ND FLOOR UTILITY PLAN	
RESIDENTIAL R-3				76	ARCHITECTURAL SHEETS	
CONSTRUCTION TYPE: TYPE VB			ANT DRAWINGS ACCOMPANYING THESE ARCH			
		PREPARED B	Y OR UNDER THE DIRECTION OF GMD DESIGN ASSUMES NO LIABILITY FOR THE COMPLETENE	GROUP, INC. GM	D DESIGN GROUP INC.	
		TILNLI UNE /	USUNES NO EMPILITETOR THE OUTPLETENE	JUN UURREU		J

# -XYKFC 40'5MODEL - HAYE

# Mason Ridg Lot 45 Spring Lake

2.2 F	W CRAWL SPACE SIDE AND REAR ELEVATIONS 'B'- W/ BASEMENT	3 SW R	STEM WALL PLAN 'R'		PLAN	CHANGES:		
3 MS F 3 SW F 3 CS F 3 BS F	MONOLITHIC SLAB PLAN 'F' STEM WALL PLAN 'F' CRAWL SPACE PLAN 'F' BASEMENT PLAN 'F'	3 BS R 4 R 5 R	BASEMENT PLAN 'R' IST FLOOR PLAN 'R' 2ND FLOOR PLAN 'R'		DATE: 02.22.21 03.10.21 04.14.21 04.15.21	DESCRIPTION: INITIAL PLAN RELEASE CLIENT REVISIONS CLIENT REVISIONS CLIENT REVISIONS		
4 F 5 F	IST FLOOR PLAN 'F' 2ND FLOOR PLAN 'F'				12.03.21 01.26.22 04.25.22 08.08.22			
		6	BASEMENT UTILITY PLAN		CON	ISULTANT	-S:	
		7 8 76	IST FLOOR UTILITY PLAN 2ND FLOOR UTILITY PLAN ARCHITECTURAL SHEETS					
PREPARED B	Y OR UNDER THE DIRECTION OF GMD DESIGN G	BROUP, INC. GME	DESIGN GROUP INC.					
	3 MS F 3 SW F 3 CS F 3 BS F 4 F 5 F ALL CONSULT PREPARED B	2.2 F       SIDE AND REAR ELEVATIONS 'B'-W' BASEMENT         3 MS F       MONOLITHIC SLAB PLAN 'F'         3 SW F       STEM WALL PLAN 'F'         3 CS F       CRAWL SPACE PLAN 'F'         3 BS F       BASEMENT PLAN 'F'         4 F       IST FLOOR PLAN 'F'         5 F       2ND FLOOR PLAN 'F'         A F       IST FLOOR PLAN 'F'         A F       OR UNDER THE DIRECTION OF GMD DESIGN OF GMD DES	2.2 F       SIDE AND REAR ELEVATIONS 'B'- W' BASEMENT       3 GS R         3 MS F       MONOLITHIC SLAB PLAN 'F'       3 BS R         3 SW F       STEM WALL PLAN 'F'       4 R         3 CS F       CRAWL SPACE PLAN 'F'       5 R         3 BS F       BASEMENT PLAN 'F'       1 A S         5 F       2ND FLOOR PLAN 'F'       1 A S         1.1.2 A S       1.1.2 A S       1.1.3 A S         6         76	2.2 F       SIDE AND REAR ELEVATIONS 'B'-W BASEMENT       3 SW R       STEM WALL PLAN 'R'         3 MS F       MONOLITHIC SLAB PLAN 'F'       3 BS R       BASEMENT PLAN 'R'         3 SW F       STEM WALL PLAN 'F'       3 BS R       BASEMENT PLAN 'R'         3 CS F       CRANL SPACE PLAN 'F'       3 BS R       BASEMENT PLAN 'R'         3 BS F       BASEMENT PLAN 'F'       4 R       IST FLOOR PLAN 'R'         3 BS F       BASEMENT PLAN 'F'       5 R       2ND FLOOR PLAN 'R'         4 F       IST FLOOR PLAN 'F'       I A S       BUILDING SECTIONS         5 F       2ND FLOOR PLAN 'F'       I A S       BUILDING SECTIONS         1.1 A S       BUILDING SECTIONS       II.2 A S       BUILDING SECTIONS         1.3 A S       BUILDING SECTIONS       II.3 A S       BUILDING SECTIONS         1.3 A S       BUILDING SECTIONS       II.3 A S       BUILDING SECTIONS         1.3 A S       BUILDING SECTIONS       II.3 A S       BUILDING SECTIONS         8       2ND FLOOR UTILITY PLAN       2ND FLOOR UTILITY PLAN	2.2 F       SIDE AND REAR ELEVATIONS 'B'- W' BAGEMENT       3 SW R       STEM WALL PLAN 'R'         3 MS F       MONOLITHIC SLAB PLAN 'F'       3 BS R       BAGEMENT PLAN 'R'         3 SW F       STEM WALL PLAN 'F'       3 BS R       BAGEMENT PLAN 'R'         3 GS F       GRAWL SPACE PLAN 'F'       3 BS R       BAGEMENT PLAN 'R'         3 BS F       BAGEMENT PLAN 'F'       4 R       IST FLOOR PLAN 'R'         3 BS F       BASEMENT PLAN 'F'       5 R       2ND FLOOR PLAN 'R'         4 F       IST FLOOR PLAN 'F'       1 A S       BUILDING SECTIONS         1.1 A S       BUILDING SECTIONS       1.1.2 A S       BUILDING SECTIONS         1.1.2 A S       BUILDING SECTIONS       1.1.3 A S       BUILDING SECTIONS         1.1.3 A S       BUILDING SECTIONS       1.1.3 A S       BUILDING SECTIONS         1.1.3 A S       BUILDING SECTIONS       1.1.3 A S       BUILDING SECTIONS         1.1.3 A S       BUILDING SECTIONS       1.1.3 A S       2ND FLOOR UTILITY PLAN         7       IST FLOOR UTILITY PLAN       7       3 CND FLOOR UTILITY PLAN         76       ARCHITECTURAL SHEETS       76       ARCHITECTURAL SHEETS	2.2 F       SIDE AND REAR ELEVATIONS 'B'- W BASEMENT       3 SW R       STEM WALL PLAN 'R'         3 MS F       MONOLITHIC SLAB PLAN F'       3 BS R       BASEMENT PLAN 'R'         3 SW F       STEM WALL PLAN F'       3 BS R       BASEMENT PLAN 'R'         3 SW F       STEM WALL PLAN F'       3 BS R       BASEMENT PLAN 'R'         3 SS F       GRANL SPACE PLAN 'F'       3 BS R       2ND FLOOR PLAN 'R'         3 BS F       BASEMENT PLAN 'F'       1 A S       BUILDING SECTIONS         4 F       IST FLOOR PLAN 'F'       1 A S       BUILDING SECTIONS         5 F       2ND FLOOR PLAN 'F'       1.1 A S       BUILDING SECTIONS         1.13 A S       BUILDING SECTIONS       04:32:2         1.33 A S       BUILDING SECTIONS       04:32:2         1.33 A S       BUILDING SECTIONS       04:32:2         1.33 A S       BUILDING SECTIONS       02:22:2         06:08:22       06:08:22       06:08:22         06       BASEMENT UTILITY PLAN       7         7       IST FLOOR UTILITY PLAN       8         8       2ND FLOOR UTILITY PLAN       7         76       ARCHITECTURAL SPREETS       04:52:0	2.2 F       SIDE AND REAR ELEVATIONS 'B'- W/ BASEMENT       3 SW R       STEM WALL PLAN R' 3 CS R       CRANL SPACE PLAN R' 3 BS R       BS CR       CRANL SPACE PLAN R' 3 BS R       DATE       DESCRIPTION.         3 MS F       MONOLITHIC SLAB PLAN F' 3 SW F       3 BS R       BASEMENT PLAN R' 4 R       IST FLOOR PLAN R' 5 R       2ND FLOOR PLAN R' 201021       DATE       DESCRIPTION.         3 BS F       BASEMENT PLAN F' 3 CS F       CRANL SPACE PLAN F' 5 R       2ND FLOOR PLAN R' 2ND FLOOR PLAN R'       INTIAL PLAN RELEASE (LIENT REVISIONS 04.14.21       CLIENT REVISIONS 04.12.2       CL	2.2 F SIDE AND REAR ELEVATIONS 'B'- W BASEMENT '3 GG R GRANL SPACE PLAN 'R' 3 MSF MONOLITIES SLAP PLAN 'F' 3 SM F STEM WALL PLAN 'F' 3 SM F STEM WALL PLAN 'F' 3 SM F STEM WALL PLAN 'F' 3 SSM F STEM WALL PLAN 'F' 3 SSM F STEM WALL PLAN 'F' 3 SS F BASEMENT PLAN 'R' 3 SS F BASEMENT PLAN 'R' 4 F IST FLOOR PLAN 'F' 5 F 2ND FLOOR PLAN 'F' 1 A S BUILDING SECTIONS 11.2 A S BUILDING SECTIONS 11.2 A S BUILDING SECTIONS 11.2 A S BUILDING SECTIONS 11.3 A S BUILDING SECTIONS

## GENERAL NOTES DESIGNER NORTH CAROLINA:

THESE DOCUMENTS ARE THE PROPERTY OF THE DESIGNER AND SHALL NOT BE COPIED, PROVIDE BLOCKING AND/OR BACKING AT ALL TOMEL BAR, TOMEL RING AND/OR DUPLICATED, ALTERED, MODIFIED OR REVISED IN ANY WAY WITHOUT THE EXPRESSED WRITTEN APPROVAL OF THE DESIGNER.

CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE AND ALL INCONSISTENCES SHALL BE BROUGHT TO THE ATTENTION OF THE DEVELOPER AND THE DESIGNER BEFORE PROCEEDING WITH WORK.

ANY ERRORS OR OMISSIONS FOUND IN THESE DRAWINGS SHALL BE BROUGHT TO

DO NOT SCALE DRAWINGS. WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED ALL DIMENSIONS ARE TO FACE OF STUD OR TO FACE OF FRAMING UNLESS

ALL TRUES DRAWINGS TO BE REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO ISSUANCE OF BUILDING PERMIT.

ALL OR EQUAL SUBSTITUTIONS MUST BE SUBMITTED TO AND APPROVED BY CITY BUILDING OFFICIAL PRIOR TO INSTALLATION.

ALL ANGLED PARTITIONS ARE 45 DEGREES UNLESS OTHERWISE NOTED. PROVIDE FIREBLOCKING. (PER LOCAL CODES.)

ALL ELECTRICAL AND MECHANICAL EQUIPMENT AND METERS ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS, CONTRACTOR TO VERIFY.

TOILET PAPER HOLDER LOCATIONS, AS SHOWN PER PLAN. TYPICAL AT ALL BATHROOMS AND POWDER ROOMS. VERIFY LOCATIONS AT FRAMING WALK. ELASTOMERIC SHEET WATERPROOFING: FURNISH AND INSTALL ALL WATERPROOFING

COMPLET: A 40 ML, SELF-ADERING HEMBRANE OF RUBBERIZED ASPHALT INTEGRALLY BONDED TO POLYETHYLENE SHEETING, OR EQUAL. INSTALL FER MANEFACTURES AND TRADE ASSOCIATIONS PRINTED INSTALLIATION INSTRUCTIONS, 6" MINIMUM LAP AT ALL ADJACENT WALL SURFACES.

TO THE BEST OF THE DESIGNER'S KNOMLEDGE THESE DOCUMENTS ARE IN CONFORMANCE WITH THE REQUIREMENTS OF THE BUILDING AUTHORITIES HAVING JURISDICTION OVER THIS TYPE OF CONSTRUCTION AND OCCUPANCY.

SHOP DRAWING REVIEW AND DISTRIBUSTION, ALONG WITH PRODUCT SUBMITTALS, REQUESTED IN THE CONSTRUCTION DOCUMENTS, SHALL BE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR, UNLESS DIRECTED OTHERWISE UNDER A SEPARATE AGREEMENT.

DEVIATIONS FROM THESE DOCUMENTS IN THE CONSTRUCTION PHASE SHALL BE REVIEWED BY THE DESIGNER AND THE OWNER PRIOR TO THE START OF WORK IN QUESTION. ANY DEVIATIONS FROM THESE DOCUMENTS WITHOUT PRIOR REVIEW, SHALL BE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR.

THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL WORK AND MATERIALS REPRESENTED ON THESE DOCUMENTS INCLUDING T MATERIALS FURNISHED BY SUBCONTRACTORS AND VENDORS.

THE BUILDER SHALL FURNISH ANY AND ALL REPORTS RECEIVED FROM THE

GEOTECHNICAL ENGINEER (SOILS REPORT), ON THE STUDY OF THE PROPOSED SITE, TO THE DESIGNER, STRUCTURAL ENGINEER, AND GENERAL CONTRACTOR. IN THE THE DELIGIEST ON A DEPORTS DO NOT EXIST, THE SOLLS CONDITION SHALL BE ASSUMED TO BE A MINIMUM DESIGN SOL DRESSURE STATED BY THE STRUCTURAL ENGINEER OF RECORD FOR THE PURPOSE OF STRUCTURAL DESIGN. GENERAL CONTRACTOR SHALL ASSURE THE SOIL CONDITIONS MEET OR EXCEED

ALL WORK PERFORMED BY THE GENERAL CONTRACTOR SHALL COMPLY AND

CONFORM WITH LOCAL AND STATE BUILDING CODES, ORDINANCES AND REGULATIONS ALONG WITH ALL OTHER AUTHORITIES HAVING JURISDICTION. THE GENERAL CONTRACTOR IS RESPONSIBLE TO BE AVARE OF THESE REQUIREMENTS AND GOVERNING REGULATIONS

PROVIDE AN APPROVED WASHER DRAIN PAN AT SECOND FLOOR ONLY THAT DRAINS TO EXTERIOR.

WINDOW SUPPLIER TO VERIFY AT LEAST ONE WINDOW IN ALL BEDROOMS TO HAVE A CLEAR OPENABLE AREA OF 4.0 SQ FT. THE MINIMM NET CLEAR OPENING HEIGHT SHALL BE 22" AND THE MINIMM NET CLEAR OPENING MIDTH SHALL BE 20", GLAZING TOTAL AREA OF NOT LESS THAN 5.0 SQ FT IN THE CASE OF A GROUDD WINDOW AND NOT LESG THAN 5.7 SQ FT IN THE CASE OF AN UPPER STORY WINDOW. (PER NORC SECTION R310.1.1) ALL HANDRAIL BALLISTERS TO BE SPACED SUCH THAT A 4" SPHERE CANNOT PASS BETWEEN BALLISTERS, (PER LOCAL CODES.) PROVIDE STAIR HANDRAILS AND GUARDRAILS PER LOCAL CODES.

THE SCOPE OF THIS SET OF PLANS IS TO PROVIDE A "BUILDER'S SET" THE SCOPE OF THIS SET OF PLANS IS TO PROVIDE A "BILLDER'S SET" OF CONSTRUCTION DOCUMENTS AND GENERAL. NOTES HEREINAFTER REFERRED TO AS "PLANS". THIS SET OF PLANS IS SUFFICIENT TO OBTAIN A BUILDING PERMIT; HOMEVER, ALL MATERIALS AND METHODS OF CONSTRUCTION NECESSARY TO COMPLETE THE PROJECT ARE NOT NECESSARILY DESCRIBED. THE PLANS DELINEATE AND DESCRIBE ONLY LOCATIONS, DIMENSIONS, TYPES OF MATERIALS, AND GENERAL METHODS OF ASSEMBLING OR FASTENING. THEY ARE NOT INTENDED. THE PLANS DELINEATE AND DESCRIBE ONLY LOCATIONS, SPECIFIC MATERIALS, PROJUCT OR METHOD. THE IMPLEMENTATION OF THE PLANS REQUIRES A CLIENT / CONTRACTOR THOROUGHLY KNOWLEDGEABLE WITT THE APPLICABLE BUILDING CODES AND METHODS OF CONSTRUCTION SPECIFIC TO THIS PRODUCT TYPE AND TYPE OF CONSTRUCTION.

BUILDER SET:

CONSTRUCTION REQUIREMENTS AND QUALITY: PROVIDE WORK OF THE SPECIFIC QUALITY: WHERE QUALITY LEVEL IS NOT INDICATED, PROVIDE WORK OF QUALITY CUSTOMARY IN SIMILAR TYPES OF WORK. WHERE THE PLANS AND SPECIFICATIONS, CODES, LAWS, REGULATIONS,

MANUFACTURER'S RECOMMENDATIONS OR INDUSTRY STANDARDS REQUIRE NORK OF HOHER QUALITY OR PERFORMANCE, PROVIDE WORK COMPLYING WITH THOSE REQUIREMENTS AND QUALITY WHERE TWO OR MORE QUALITY PROVISIONS OF THOSE REQUIREMENTS COMPLICIT WITH THE MOST STRINGENT REQUIREMENT; WHERE REQUIREMENTS ARE DIFFERENT BUT APPARENTLY EQUAL. AND WHERE IT IS UNCERTAIN WHICH REQUIREMENT IS MOST STRINGENT, OBTAIN CLARIFICATION FROM THE GMD DESIGN GROUP BEFORE PROCEEDING

5 HOMES ERIES N 4 BR - RH	NO:         DATE:         REVISION:           ↑         042522
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e, NC 28390	PROFESSIONAL SEAL:
	PROJECT TITLE: 40' Series
	CIENTS NAME:
AREA CALCULATIONS:	HOMES
MODEL     'HAYDEN'     SQUARE     FOOTAGES       AREA     ELEV F', K'     1066 SF       Ist FLOOR     1045 SF       TOTAL. LIVING     2511 SF       6ARAGE     422 SF       PORCH     109 SF       OPT. GOVERED PORCH     80 SF       OPT. BASEMENT     1006 SF	PROJECT NO: GMD17049 SHEET TITLE: TITLE SHEET
**BASEMENT AREA IS TAKEN TO INSIDE OF CONCRETE WALL**	PRINT DATE: January 22, 2021 SHEET NO: <b>0</b>



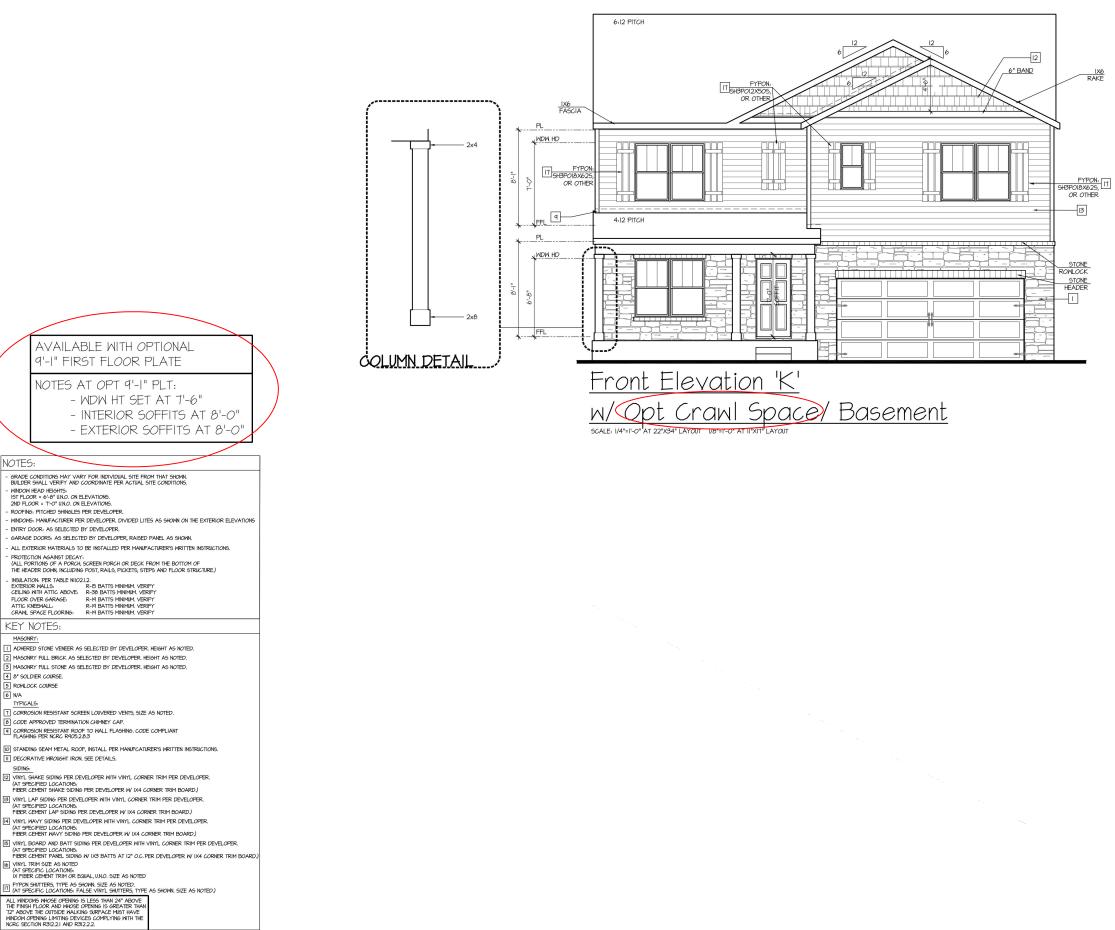


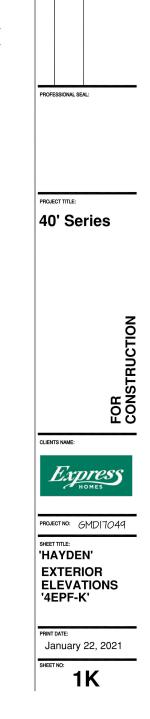


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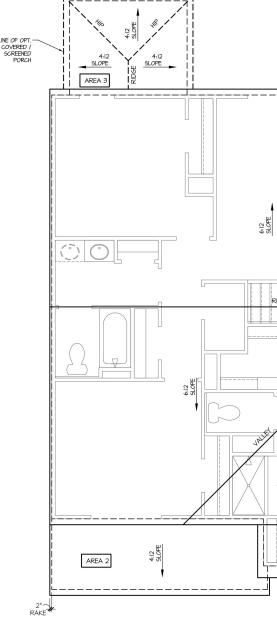




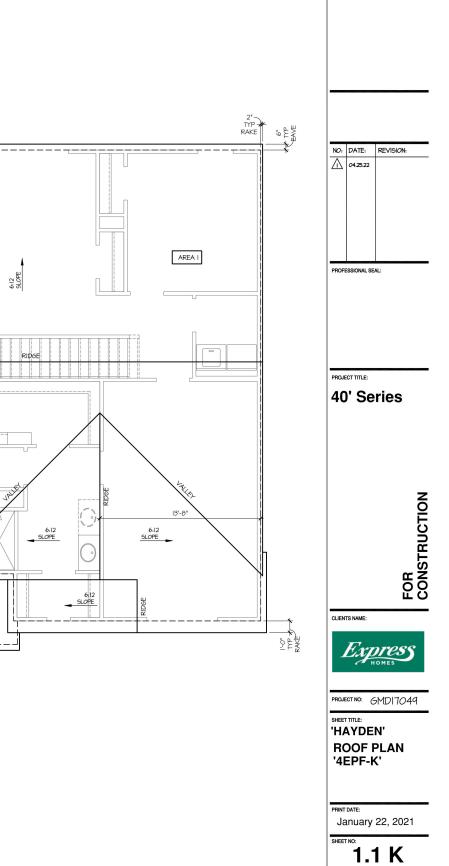
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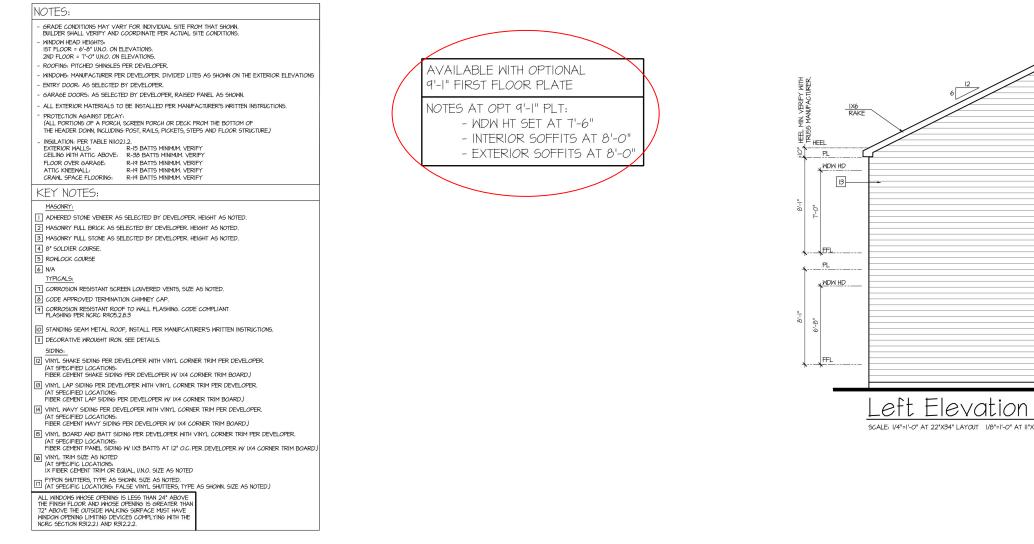
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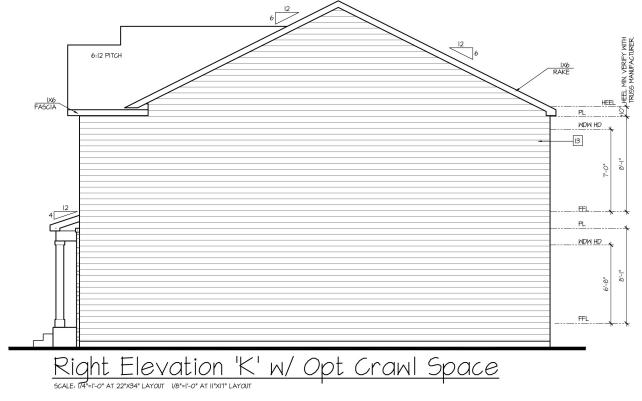
## LINE OF OPT. ATTIC VENT CALCULATION FOR PLAN 'HAYDEN': 1:150 RATIO. (PER SECTION ROOK 2) THE NET FREE VENTILATING AREA SHALL NOT BE LESS THAN THE NET THE AREA OF THE SPACE VENTILATED, PROVIDED THAT AT LEAST SO PERCENT AND NOT MORE THAN 60 PERCENT OF THE REQUIRED VENTILATING AREA IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE SPACE I SQUARE INCH VENT FOR EVERY 150 SQUARE INCHES OF CEILING \*144 SQ. IN. = 1 SQ. FT. BLDG. CEILING (SF) X 144 = BLDG (SQ. IN.) O BE VENTILATED AT LEAST 3 FEET ABOVE THE EAVE OR BLDG. (SQ. IN.) / ISO = SQ. IN. OF VENT REQUIRED CORNICE VENTS WITH THE BALANCE OF THE REQUIRED VENTILATION PROVIDED BY EAVE OR CORNICE VENTS. ROOF AREA I:= 1488 5F 1488 50. FT. X 144 = 214272 50. IN. 214272 50. IN. / I50 = 1428.48 50. IN. OF VENT REQ'D EXCEPTIONS: EXCEPTIONS: . EXCLOSED ATTIC/RAFTER SPACES REQUIRING LESS THAN I SQ FT OF VENTILATION MAY BE VENTED WITH CONTINUOUS SOFFIT VENTILATION ONLY. ROOF AREA 2:= 34 5F 34 5Q, FT, X 144 = 5616 5Q, IN. 5616 5Q, IN. / 150 = 37.44 5Q, IN. OF VENT REQ'D 2. ENCLOSED ATTIC/RAFTER SPACES OVER UNCONDITIONED SPACE MAY BE VENTED WITH CONTINUOUS SOFFIT VENT ONLY GENERAL CONTRACTOR SHALL VERIFY THE NET FREE ROOF AREA 3:= 180 SF 180 SQ, FT, X 144 = 25920 SQ, IN. 25920 SQ, IN. / 150 = 172.80 SQ, IN. OF VENT REQ'D VENTILATION OF THE VENT PRODUCT SELECTED BY OWNER. VENTILATION OF THE VENT PROUDT SELECTED BY OWNER VENTRY WITH MANUFACTURER OF HIGH AND LOW VENTS TO BE USED FOR MINIMUM CALCULATED VENTS REQUIRED. THE REQUIRED VENTILATION SHALL BE MAINTAINED. PROVIDE INSULATION STOP SUCH THAT INSULATION DOES NOT OBSTRUCT RERE AIR MOVEMENT AS REQUIRED BY THE BUILDING OFFICIAL BY THE BUILDING OFFICIAL. ALL OVERLAP FRAMED ROOF AREAS SHALL HAVE OPENINGS BETWEEN THE ADJACENT ATTICS IN THE ROOF SHEATHING (KS ALLOWED BY THE STRUCTURAL ENGINEER) TO ALLOW PASSAGE AND ATTIC VENTILATION BETWEEN THE TWO OR ISOLATED ATTIC SPACES SHALL BE VENTED INDEPENDENTLY TO CBC REQUIREMENTS. PER DEVELOPER, AT ALL CANTILEVERED FLOORS, CANTILEVERED ARCHITECTURAL POP-OUTS, AND ANY DOUBLE CANTILEVERED ARCHITECTURAL POP-CUTS, AND ANT LOUG FRAMING PROJECTIONS THAT ARE SEPARATED FROM THE VENTING CALCULATIONS SHOWN ABOVE, PROVIDE A CONTINUOUS 2° CORROSION RESISTANT SOFFIT VENT AT UNDERSIDE OF FRAMED ELEMENT. NOTES: TRISS MANIFACTURER SHALL SUBMIT STRUCTURAL CALCS AND SHOP DRAWINGS TO THE BUILDER'S GENERAL CONTRACTOR AND BUILDING DEPARTMENT FOR REVIEW IRIOR TO FABRICATIONS. - ALL ROOF DRAINAGE SHALL BE PIPED TO STREET OR APPROVED DRAINAGE FACILITY. DASHED LINES INDICATE WALL BELOW. - ALL PLUMBING VENTS SHALL BE COMBINED INTO A MINIMUM AMOUNT OF ROOF PENETRATIONS. ALL ROOF PENETRATIONS SHALL OCCUR TO THE REAR OF THE MAIN RIDGE. - LOCATE GUTTER AND DOWNSPOUTS PER BUILDER. PITCHED ROOFS AS NOTED. ATTIC VENT CALCULATION FOR PLAN 'HAYDEN': 1:300 RATIO. (PER SECTION R806.2) AS AN ALTERNATE TO THE 1/150 RATIO LISTED ABOVE. THE NET FREE CROSS-VENTILATION AREA MAY BE REDUCED TO 1/300 WHEN A CLASS I OR II VAPOR RETARDER IS INSTALLED ON THE WARM - IN - WINTER SIDE OF THE CEILING. I SQUARE INCH VENT FOR EVERY 300 SQUARE INCHES OF CEILING \*144 SQ IN = 1 SQ FT BLDG. CEILING (SF) X 144 = BLDG (SQ. IN.) BLDG. (SQ. IN.) / 300 = SQ. IN. OF VENT REQUIRED GENERAL CONTRACTOR SHALL VERIFY THE NET FREE ENERAL CONTRACTOR SHALL VERIFY THE NET FREE VENITLATION OF THE VENT PRODUCT SELECTED BY OWNER, VERIFY MITH MANUFACTURER OF HIGH AND LOW VENTS TO BE USED FOR MINIMM CALCULATED VENTS REQUIRED. THE REQUIRED VENTILATION SHALL BE MAINTAINED. PROVIDE INSULATION STOP SUCH THAT INSULATION DOES NOT OBSTRUCT FREE AIR MOVEMENT AS REQUIRED BY THE BUILDING OFFICIAL. SQ. IN. OF VENT REQUIRED / 2 = 50% AT HIGH & 50% AT LOW. ROOF AREA I: = 1488 SF 1488 Sa, FT, X 144 = 142712 Sa, IN, 2142712 Sa, FT, 7300 = 714.24 Sa, IN, OF VENT READ 714.24 Sa, IN, / 2 = 357.12 Sa, IN, OF VENT ALLOW REAURED. 357.12 Sa, IN, OF VENT AT HIGH & 357.12 Sa, IN, OF VENT AL LOW REAURED. ROOF AREA 2: = 34 5F 34 50. FT. X 144 5616 50. IN. 5616 50. FT. X 300 18.72 50. IN. OF VENT READ 18.72 50. IN. / 2 = 436 50. IN. OF VENT AT HIGH & 436 50. IN. OF VENT AT LOW REQUIRED. ALL OVERLAP FRAMED ROOF AREAS SHALL HAVE OPENINGS BETWEEN THE ADJACENT ATTICS IN THE ROOF SHEATHING KG ALLOWED BY THE STRUCTURAL ENGINEER) TO ALLOW PASSAGE AND ATTIC VENTILATION BETWEEN THE TWO OR ISOLATED ATTIC SPACES SHALL BE VENTED INDEPENDENTLY TO CBC REQUIREMENTS. ROOF AREA 3: # 180 SF. 180 Sa. FT. X 144 25420 Sa. IN. 25420 Sa. FT. X 300 86:40 Sa. IN. OF VENT READ 26:40 Sa. IN. / 2 43:20 Sa. IN. OF VENT AT HIGH 4 43:20 43:20 Sa. IN. OF VENT AT HIGH 4 43:20 Sa. IN. OF VENT AT LOW REQUIRED. PER DEVELOPER, AT ALL CANTILEVERED FLOORS, CANTILEVERED ARCHITECTIRAL POP-OUTS, AND ANY DOUBLE FRAMING FRO. BECTIONS THAT ARE SEPARATED FROM THE VITTING CALCULATIONS SHOWN ABOVE, PROVIDE A CONTINUOUS 21 CORROSION RESISTANT SOFFIT VENT AT UNDERSIDE OF FRAMED ELEVENT. BUILDER TO PROVIDE (2) LAYERS OF UNDERLAYMENT AT ANY ROOF W/ A SLOPE FROM 2:12 TO LESS THAN 4:12 AT SINGLE FAMILY DETACHED PLANS: PREFINISHED VENTED SOFFIT AT EAVE PER MANUFACTURER. (VERIFY FIRE SEPARATION DISTANCE FOR SOFFIT PROTECTION PER NCRC SECTION R302.1.1 AND TABLE R302.1)



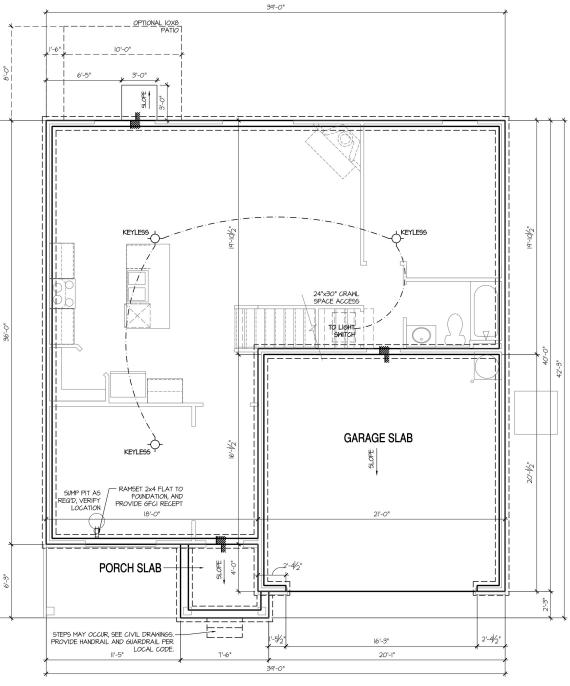




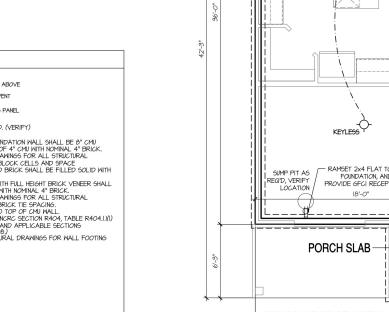






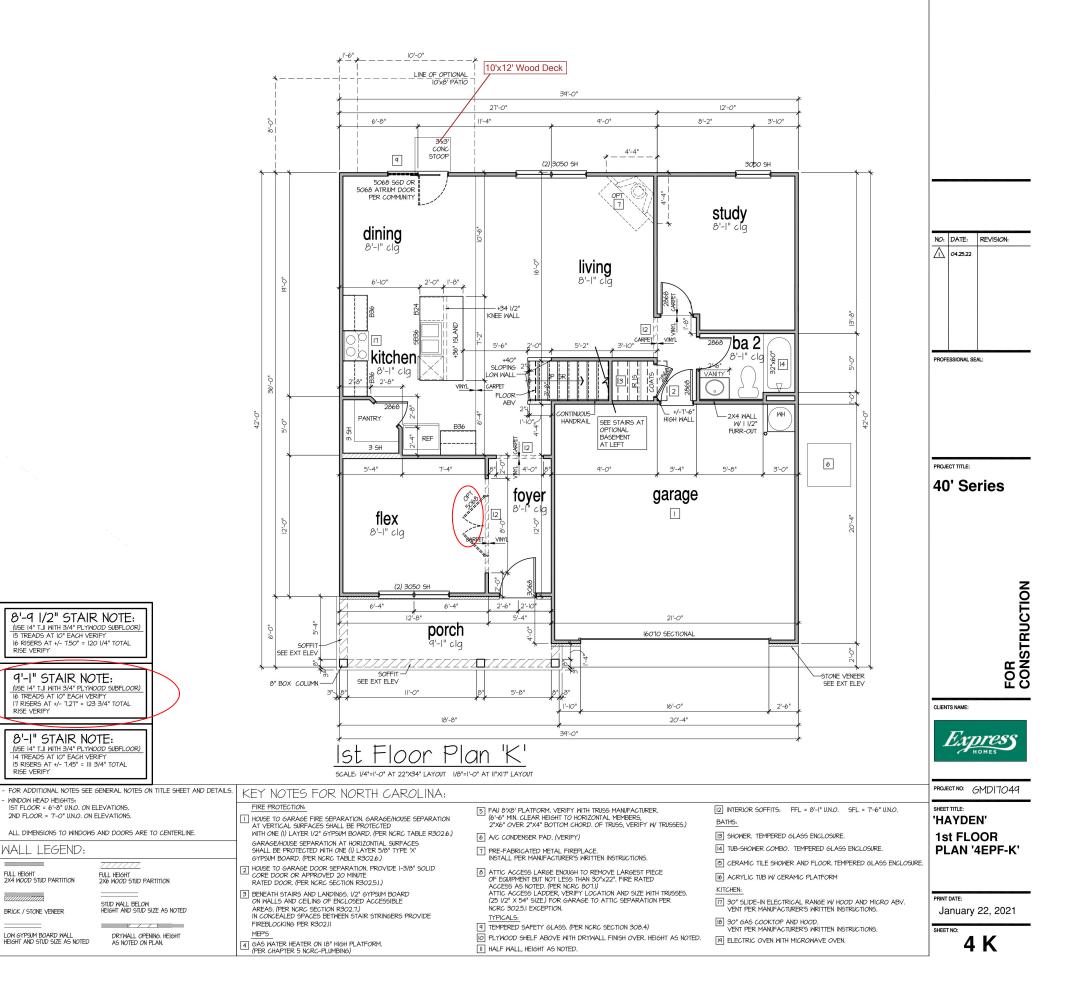


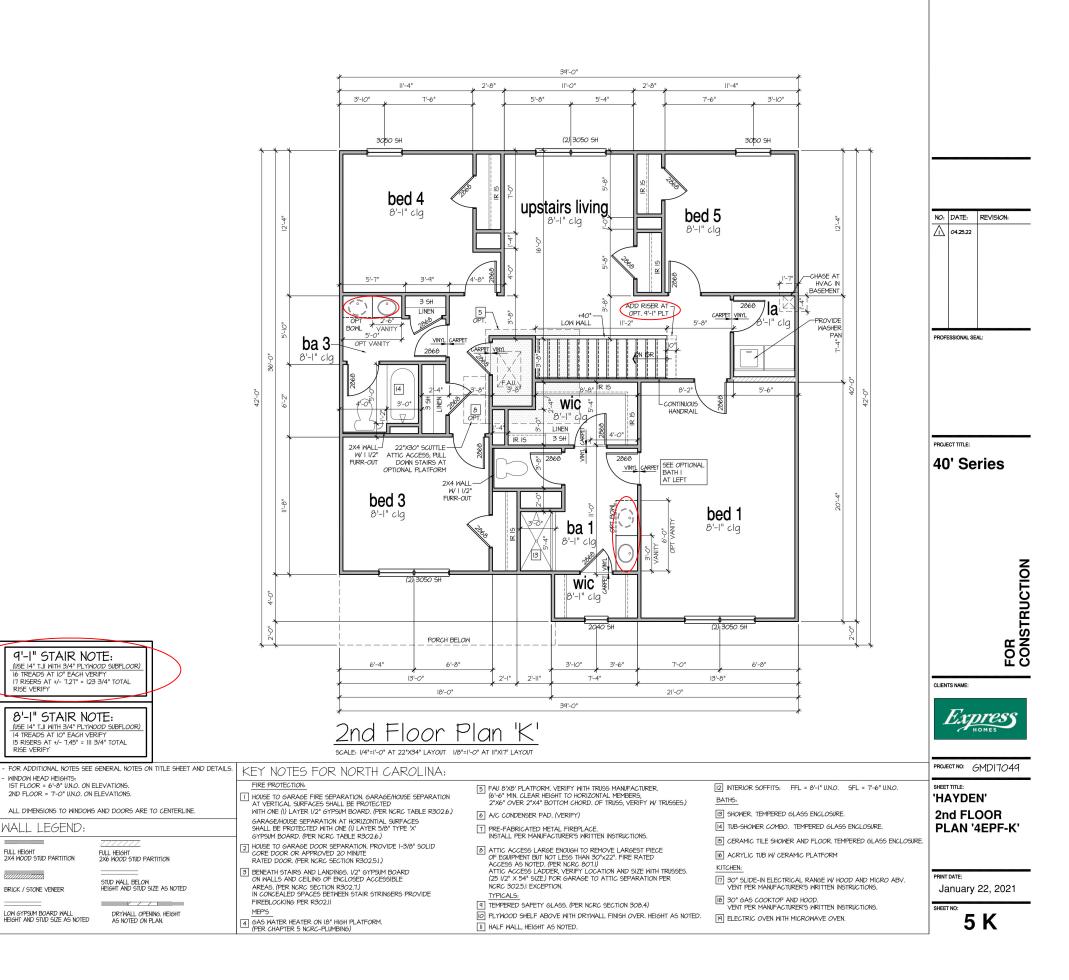
CRAWL SPACE NOTES NORTH CAROLINA:	KEY NOTES:
<ul> <li>REFER TO STRUCTURAL DRAWINGS FOR INFORMATION NOT SHOWN ON THIS PLAN.</li> <li>FOR ADDITIONAL NOTES SEE GENERAL NOTES ON TITLE SHEET AND DETAILS.</li> <li>PROVIDE FIREBLOCKING. (PER LOCAL CODES)</li> <li>ALL ELECTRICAL AND MECHANICAL EQUIPMENT AND METERS ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS, CONTRACTOR TO VERIFY.</li> <li>VERIFY ALL DOOR TIRESHOLD HEIGHTS TO HARD SUFFACES.</li> <li>J 1/4' MAX AT INSVING DOORS, (PER NORG SECTION RSIIJ.)]</li> <li>SLOPE ALL STOOPS AND HARDSCAFE MATERIAL. ANAY FROM BUILDING - TYPICAL.</li> <li>SLOPE GARAGE FLOOR 1/8' PER FOOT TO GARAGE DOOR OPENING.</li> <li>VERIFY CURB CUT BLOCKOLIVITH GARAGE DOOR MANIFACTURER.</li> <li>REFER TO CIVIL DRAWINGS FOR FINISH SUFFACES.</li> <li>TYP STOOP AT INSWINJSLIDER DOORS; 36' DEEP BY THE WIDTH OF THE DOOR SERVED, MINIMM (FER NORG SECTION RSIJ.)]</li> <li>SOILS TREATMENT: DE APPLIED TO FRAMING PER PRODUCT SPECIFICATIONS.</li> <li>TYP STOOP AT INSWINJSLIDER DOORS; 36' DEEP BY THE WIDTH OF THE DOOR SERVED, MINIMM (FER NORG SECTION RSIJ.)]</li> <li>SOILS TREATMENT: DE APPLIED TO FRAMING PER PRODUCT SPECIFICATIONS. (FROVIDE CHENICAL TREATMENT FOR PROTECTION FROM TERMITE INVESTATION ACCORDING TO LOCAL CODES)</li> <li>AT VENTED CRAAL SPACE.</li> <li>APPLY AN APPROVED VAPOR RETARDER OR EQUIVALENT, 6 MIL POLY-VINTL, GROUND COVER OVER FINISH GRADE OR CRANL SPACE FER NORG SECTION 4002.</li> <li>PROVIDE VENTS SPACED ARCING PERVERT TO DE ROMOTE CROSS VENTILATION AT A RATE OF 1 SY VENT FOR EVERT 'SDO SF OF CRAAL FLOOR AREA. ONE VENT MOST BE LOCATED WITH SPACE EVERT 'SDO SF OF CRAAL FLOOR AREA. ONE VENT MOST BE LOCATED WITHIN 3-0'' OF EVERT 'SDO SF OF CRAAL FLOOR AREA. ONE VENT MOST BE LOCATED WITHIN 3-0'' OF EVERT 'SDO SF OF CRAAL FLOOR AREA. ONE VENT MOST BE LOCATED WITH MECHANICAL CONTRACTOR FOR LARGER SIZE REGURAMENTS I MECHANOLAL EXCUPERTION.)</li> <li>PROVIDE VANGES SENTILATION. (FER NERS SECTION AROLL EXCEPTION)</li>     PROVIDE VANGE SECTION HORALL CONTRACTOR F</ul>	<ul> <li>LINE OF SLAB ABOVE</li> <li>LINE OF FRAMED MAIL ABOVE</li> <li>LINE OF FRAMED MAIL ABOVE</li> <li>IGY38* CRANL SPACE VENT</li> <li>GRANL SPACE ACCESS PANEL</li> <li>AC CONDENSER PAD. (VERIFY)</li> <li>TYPICAL CRANL FOUNDATION WALL SHALL BE 8* CMU OR A COMBINITION OF 4* CMU WITH NOMINAL 4* BRICK. SEE STRUCTURAL DRAININGS FOR ALL STRUCTURAL ATTACHMENTS. ALL BLOCK CELLS AND SPACE BETWEEN BLOCK AND BRICK SHALL BE FILLED SOLID WITH CONCRETE.</li> <li>FOUNDATION WALL WITH FULL HEIGHT BRICK VENEER SHALL CONSIST OF 8* CMU WITH NOMINAL 4* BRICK. SEE STRUCTURAL DRAININGS FOR ALL STRUCTURAL ATTACHMENTS AND BRICK TIE SPACING.</li> <li>FILL VOIDS SOLID TO TOP OF CMU WALL.</li> <li>MICT COMPLY NITH NORCE SECTIONS OF REGOR, REOT, REGOL</li> <li>VERIFY WITH STRUCTURAL DRAININGS FOR WALL FOOTING SIZE AND DEPTH.</li> </ul>



Scale: 1/4\*=1-0\* AT 22\*X34\* LAYOUT 1/8\*=1-0\* AT 11\*X17\* LAYOUT

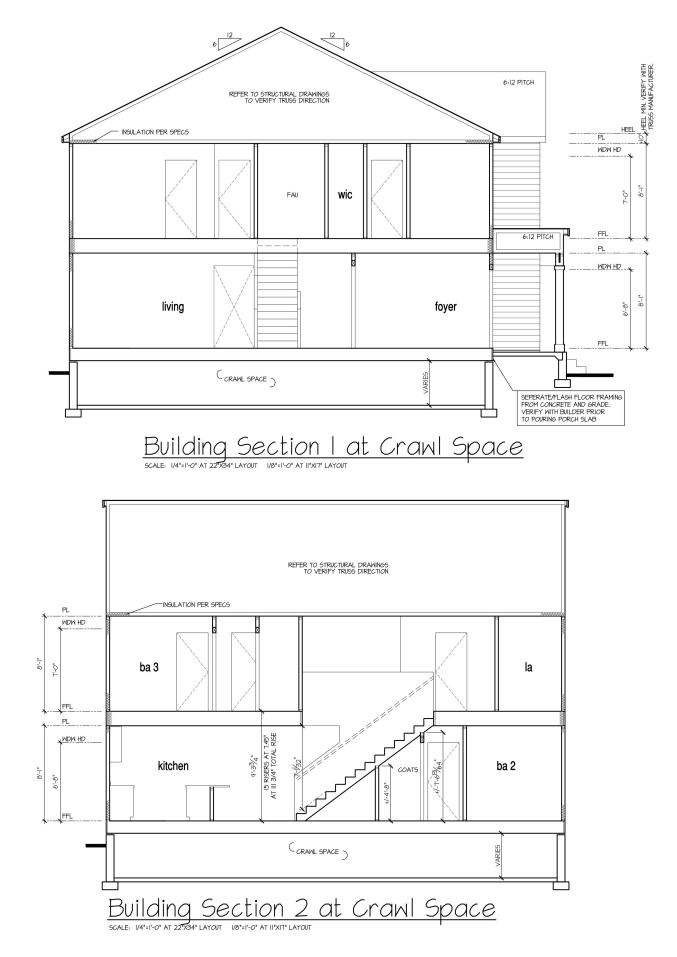
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PROFESSIONAL SEAL:
PROJECT TITLE: 40' Series
NUCTION
Express Homes
PROJECT NO: GMD17049 SHEET TITLE: 'HAYDEN'
CRAWL SPACE PLAN '4EPF-K'
PRINT DATE: January 22, 2021
3 CS K





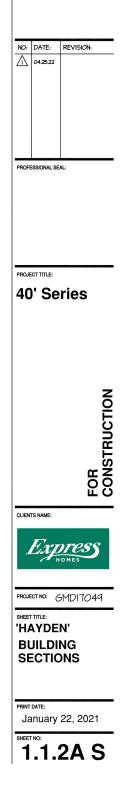
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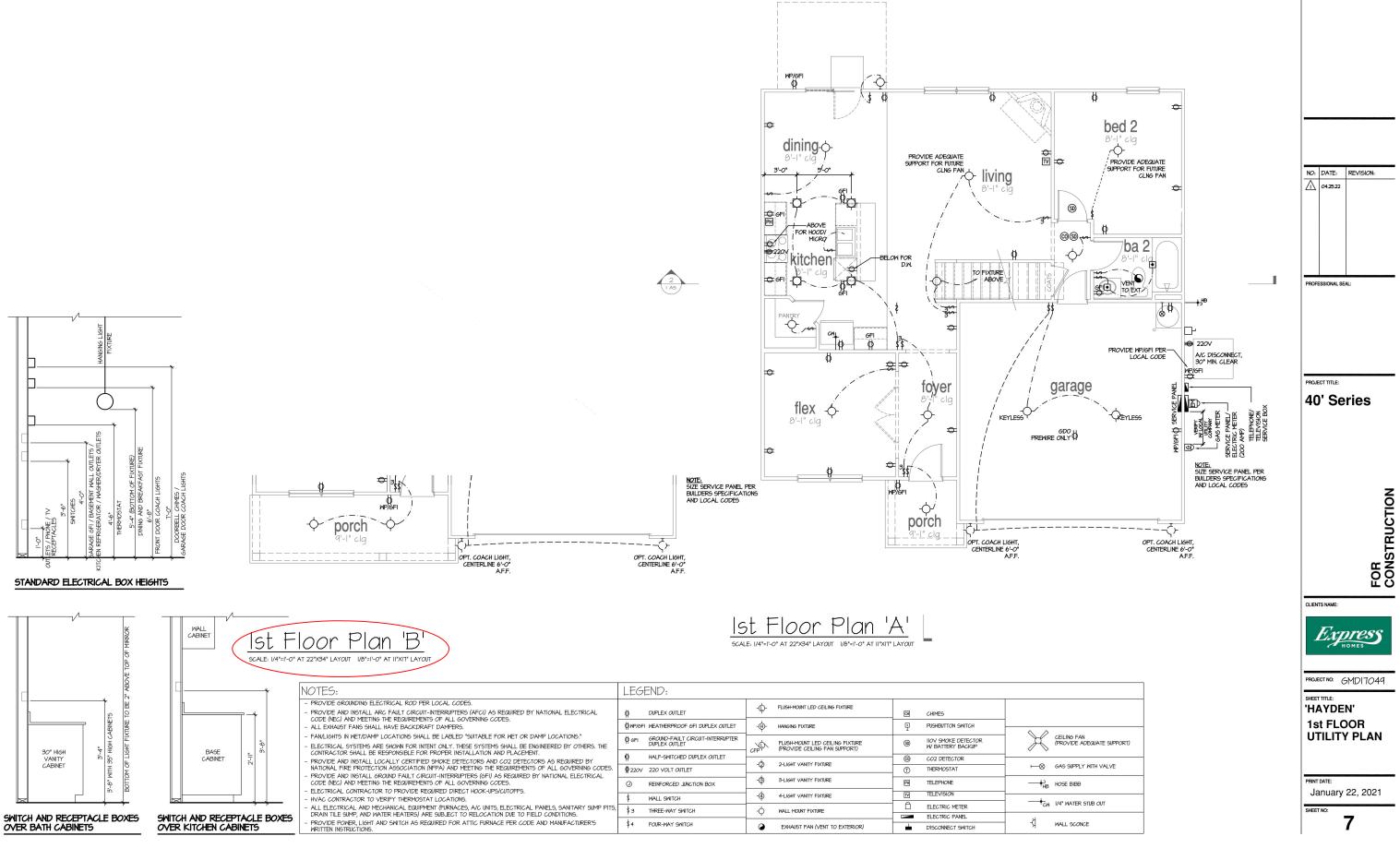
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- REFER TO FLOOR PLAN NOT	TES FOR TYPICAL FIRE PROTECTION NOTES AND LOC	ATIONS.
CONDITIONS. REFER TO MAI	1AY VARY AT ALTERNATE ELEVATION STYLES AND , N FLOOR PLAN AND ALTERNATE FLOOR PLANS FOR	INFORMATION NOT SHOWN HERE.
	HERE DEPICT VOLUMN SPACES WITHIN THE STRUCTUR S, STRUCTURAL DETAILS AND CALCULATIONS BY OTH	
- ROOFING: PITCHED SHINGLE	ROOF. REFER TO ROOF PLAN FOR TYPICALS.	
<ul> <li>WOOD FLOORS: FLOOR SHE REFER TO STRUCTURAL AND</li> </ul>	ATHING OVER FLOOR JOIST. P TRUSS DRAWINGS BY OTHERS.	
<ul> <li>VERIFY STAIRS MINIMUM AN WITH LOCAL CODES.</li> </ul>	D MAXIMUM REQUIREMENTS FOR CONSTRUCTION CLE/	ARANCES
- INSULATION:		
	R-13 BATTS MINIMUM. VERIFY R-15 BATTS MINIMUM. VERIFY	
CEILING WITH ATTIC ABOVE		
	R-38 BATTS MINIMUM. VERIFY	PER STATE RESIDENTIAL COL
CEILING WITH ATTIC ABOVE	UNCOMPRESSED INSULATION (HEELS IN TRUSSES): R-30 BATTS MINIMUM. VERIFY	COMPLIANCE METHOD TO BE DETERMINED BY BUILDER.
FLOOR OVER GARAGE:	R-19 BATTS MINIMUM, VERIFY	
ATTIC KNEEWALL:	R-19 BATTS MINIMUM. VERIFY	
CRAWL SPACE FLOORING:	R-19 BATTS MINIMUM, VERIFY	

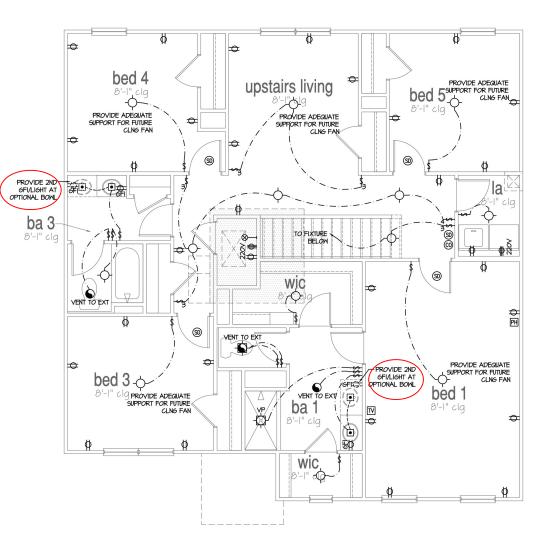


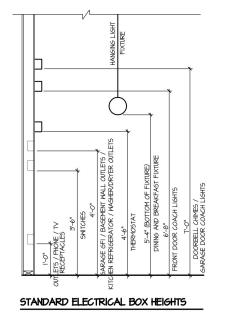


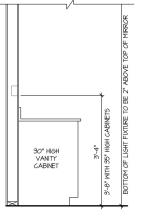
		AFF.
ALL	-	
	+	NOTES:
		- PROVIDE GROUNDING ELECTRICAL ROD PER LOCAL CODES.
	╤╼╅	<ul> <li>PROVIDE AND INSTALL ARC FAULT CIRCUIT-INTERRIPTERS (AFCI) AS REQUIRED BY NATIONAL CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.</li> <li>ALL EXHAUST FANS SHALL HAVE BACKDRAFT DAMPERS.</li> </ul>
		- FAN/LIGHTS IN WET/DAMP LOCATIONS SHALL BE LABLED "SUITABLE FOR WET OR DAMP LOCA
BASE	3-9"	- ELECTRICAL SYSTEMS ARE SHOWN FOR INTENT ONLY. THESE SYSTEMS SHALL BE ENGINEERED CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER INSTALLATION AND PLACEMENT.

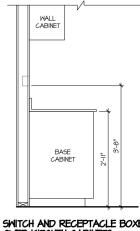
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	LEG	END:						
AL CODES. RUPTERS (AFCI) AS REQUIRED BY NATIONAL ELECTRICAL	ø	DUPLEX OUTLET	-¢-	FLUSH-MOUNT LED CEILING FIXTURE	СН	CHIMES		
ALL GOVERNING CODES. MPERS.	п Фир/он	I WEATHERPROOF GFI DUPLEX OUTLET		HANGING FIXTURE	9	PUSHBUTTON SWITCH		
LABLED "SUITABLE FOR WET OR DAMP LOCATIONS." NLY, THESE SYSTEMS SHALL BE ENGINEERED BY OTHERS, THE	ф ағі	GROUND-FAULT CIRCUIT-INTERRUPTER DUPLEX OUTLET	CFP	FLUSH-MOUNT LED CEILING FIXTURE	9	IIOV SMOKE DETECTOR W BATTERY BACKUP		k î₽
PER INSTALLATION AND PLACEMENT. KE DETECTORS AND CO2 DETECTORS AS REQUIRED BY	ø	HALF-SWITCHED DUPLEX OUTLET		(PROVIDE CEILING FAN SUPPORT)	0	CO2 DETECTOR		~
) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. ITERRUPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRICAL	<b>\$</b> 220√	220 VOLT OUTLET	-\$	2-LIGHT VANITY FIXTURE	Ū	THERMOSTAT		-8 6
ALL GOVERNING CODES. D DIRECT HOOK-UPS/CUTOFES.	J	REINFORCED JUNCTION BOX	-\$	3-LIGHT VANITY FIXTURE	PH	TELEPHONE		+р н
CATIONS. URNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUMP PITS.	\$	WALL SWITCH	-@	4-LIGHT VANITY FIXTURE		TELEVISION ELEGTRIC METER		+CH 1/
BJECT TO RELOCATION DUE TO FIELD CONDITIONS.	33	THREE-WAY SWITCH	- <b>\$</b>	WALL MOUNT FIXTURE		ELECTRIC PANEL	К	
D FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S	<b>\$</b> 4	FOUR-WAY SWITCH	0	EXHAUST FAN (VENT TO EXTERIOR)		DISCONNECT SWITCH	<sup>א</sup>	W







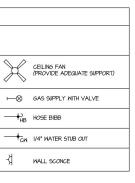


LEGEND:		
	FLUSH-MOUNT LED CEILING FIXTURE	он снімез
ØWP/GFI WEATHERPROOF GFI DUPLEX OUTLET	-(H)- HANGING FIXTURE	PUSHBUTTON SWITCH
GFI GROUND-FAULT CIRCUIT-INTERRUPTER		IIOV SMOKE DETECTOR     W BATTERY BACKUP
O HALF-SWITCHED DUPLEX OUTLET		© CO2 DETECTOR
	-(3) 3-LIGHT VANITY FIXTURE	THERMOSTAT
-		
	-Ó WALL MOUNT FIXTURE	
\$4 FOUR-WAY SWITCH	EXHAUST FAN (VENT TO EXTERIOR)	ELECTRIC PANEL
ò,	Image: Stress of the stress	Image: Second

2nd Floor	Plan 'A'
SCALE: 1/4"=1'-0" AT 22"X34" LAYOUT	1/8"=1'-0" AT 11"X17" LAYOUT

## SWITCH AND RECEPTACLE BOXES OVER BATH CABINETS SWITCH AND RECEPTACLE BOXES

SCALE:	I/4"=I'-O"	AT 22"X34"	' LAYOUT	1/8"=1'-0"	AT   "X 7"



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	DESIGN SPECIFICATIONS:				SHEET LIS	<u>5T:</u>	
	Construction Type: Commerical 🗌 Residential 🛛				Sheet		Description
	Applicable Building Codes:				C3		Cover Sheet, Specifications, Revisions Monolithic Slab Foundation
	<ul> <li>2018 North Carolina Residential Building Code with All Loca</li> <li>ASCE 7-10: Minimum Design Loads for Buildings and Other S</li> </ul>				61.e		Stem Wall Foundation Crawl Space Foundation
	Design Loads:				51.Ø	6	Basement Foundation
	I. Roof Live Loads		CIIK	ΙΜΙΤ	52. 53.	-	Basement Framing Plan First Floor Framing Plan
	12. Truss		<b>3</b> 0r		54.	0	Second Floor Framing Plan
	2. Roof Dead Loads		ENGINEERING LA	BORATORY TESTING			Roof Framing Plan Basement Bracing Plan
	2.2. Truss				67.	0	First Floor Bracing Plan
	3. 9now 15 P9F 3.1. Importance Factor 1.0				58.	0	Second Floor Bracing Plan
	4. Floor Live Loads 4.1. Typ. Dwelling		HAYE	DEN RH			
	42. Sleeping Areas		PROJECT ADDRESS:	OUNER:	DEVICIO		
	<ul><li>4.4. Passenger Garage</li></ul>	:	TBD	DR Horton, Inc.	REVISION	LI51:	
	5.1. Conventional 2x 10 PSF 5.2. I-Joist 15 PGF			8001 Arrowridge Blvd. Charlotte, NC 28273	Revision No.	Date	Project Description
	5.3. Floor Truss	н	DESIGNER:		1	4.19.21	TØITT Updated elevation names
	6.1. Exposure B 62. Importance Factor		GMD Design Group 102 Fountain Brook Circle				Added Stem Wall, Crawlspace, and Basement Foundations
	6.3. Wind Base Shear 6.3. VX =		Suite C		2	6.14.21	TØITT Added OX-16 option and Updated OX-16 table framina
	6.3.2.Vy = 1. Component and Cladding (in PSF)		Cary, NC 27511		3	11.23.21	TØITT Updated the engineering in the first floor framing
	MEAN ROOF UP TO 201 2011 251 25111 401 4	Ø' "-45'	electrical, and civil drawings. This co	d with the architectural, mechanical, plumbing, ordination is not the responsibility of the			
	HI,	38,-202	structural engineering of record (SER apparent, the contractor shall notify S	1). Should any discrepancies become MMMIT Engineering, Laboratory & Testing,			
	ZONE 2 16.7,-21.0 17.6,-22.1 18.3,-22.9 18	3.8,-23.6	P.C. before construction begins.				
		8.8,-23.6 Ø.4,-21.3	PLAN ABBREVIATIONS:				
	ZONE 5 182,-24,0 192,-25.2 19.9,-26.2 20	0.4,-26.9	AB ANCHOR BOLT	PT PRESSURE TREATED			
	8. Seismic		AFF ABOVE FINISHED FLOOR	RS ROOF SUPPORT SC STUD COLUMN			
	8.1. Site Class [ 82. Design Category (		CLR CLEAR	SJ SINGLE JOIST			
	8.4. Seismic Use Group	Ø	DJ DOUBLE JOIST DSP DOUBLE STUD POCKET	SPF SPRUCE PINE FIR SST SIMPSON STRONG-TIE			
	85. Spectral Response Acceleration 8.5.1. Sms = %g		EE EACH END	SYP SOUTHERN YELLOW PINE			
	8.5.2. Gml = %g 8.6. Seismic Base Ghear		EW EACH WAY NTS NOT TO SCALE	TJ TRIPLE JOIGT TSP TRIPLE STUD POCKET			
	86.1. VX = 862.VU =		OC ON CENTER				
	8.1. Basic Structural System (check one) ⊠ Bearing Wall		PSF POUNDS PER SQUARE FOOT PSI POUNDS PER SQUARE INCH	UNO UNLESS NOTED OTHERWISE WUF WELDED WIRE FABRIC			
	□ Building Frame □ Moment Frame						
	□ Dual w/ Special Moment Frame □ Dual w/ Intermediate R/C or Special Ste	d	were not provided to SUMMIT Engine	d their corresponding loading details, ering, Laboratory & Testing, P.C. (SUMMIT)			
	Inverted Pendulum 8.8. Arch/Mech Components Anchored1		prior to the initial design. Therefore, based on the information provided b	truss and joist directions were assumed by DR Horton, Inc. Subsequent plan			
	8.9. Lateral Design Control: Seismic 🗆 🛛 Wind 🖂		revisions based on roof truss and flo revision list, indicating the date the l	por joist layouts shall be noted in the			
	9. Assumed Soil Bearing Capacity	0000051		contractor shall notify SUMMIT immediately.			
GENERAL STRUCTURAL NOTES: 1. The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The	<ol> <li>The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.</li> </ol>	with ACI 302.IR-96 Construction".	n-grade shall be constructed in accordance 5: "Guide for Concrete Slab and Slab	<ol> <li>Where reinforcing dowels are required , the in size and spacing to the vertical reinforc shall extend 48 bar diameters vertically ar</li> </ol>	cement. The dow	el	UCOD TRUSSES: I. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and
<ul> <li>SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering. Laboratory 4 Tessing, P.C. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entity.</li> <li>The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.</li> <li>The SER is not responsible for construction of this structure. The SER will not be held responsible for the contractor of the structure.</li> <li>Any structural elements or details not fully developed on the construction chail any orders shall be completed for the direction of allcensed professional shall be completed on the direction of the structure.</li> </ul>	<ol> <li>The resulting soil shall be compacted to a minimum of 95% maximum dry density.</li> <li>Excavations of footings shall be lined temporarily with a 6 mil polyethylene memorane if placement of concrete does not occur within 24 hours of excavation.</li> <li>No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.</li> <li><u>STRUCTURAL STELL</u>:         <ol> <li>Structural steel shall be fabricated and erected in accordance with the American institute of Steel Construction "Code of Standard Practice for Steel Buildings and Eridges" and the manual of Steel Construction "Load Resistance Factor Design" latest editions.</li> <li>Structural steel shall receive one coat of shop applied rust-inhibitive paint.</li> </ol> </li> </ol>	subgrade modulus psf. The SER is no cracking or other conditions not in . Control or sale ucu slabs-on-grade a slabs-on-grade a 8. Control or sale ucu process within 4 t 9. Reinforcing steel Reinforcing steel Io. All weided wire fo be placed at mid supported during <u>CONCRETE REINFORCE</u> 1. Fibrous concrete	reinforcement, or fibermesh, specified in	<ul> <li>Into the footing.</li> <li>Uher enintroning steel is required vertica provided unless otherwise noted.</li> <li><u>WOOP FRAMING:</u> <ol> <li>Solid saw wood framing members shall conspecifications listed in the latest edition of Design Specification for Wood Construction otherwise noted, all wood framing members Southern-Teilou-Pine (STP) 2;</li> <li>LVL or PSL engineered wood shall have tidesign values:</li></ol></li></ul>	form to the of the "National of "(NDS). Unlet are designed t he following min earth shall be standard C-I5.	is o be mum All	<ul> <li>supporting calculations to the SER for eview prior to - flabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 1-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.</li> <li>The trusses will be designed, fabricated, and erected in accordance will the latest edition of the "National Design Specification for Wood Construction" (NDS) and "Design</li> </ul>
<ul> <li>audmitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER of SUMMIT.</li> <li>Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.</li> <li>The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.</li> <li>This structure and all construction shall conform to all applicable sections of local building codes.</li> <li>All structural asymboliang code.</li> <li>EOUNDATIONS:</li> <li>The structural engineer has not performed a subsurface investigation, verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be condited by adverse soil condition be SER must be contracted before proceeding.</li> </ul>	standards. <u>CONCRETE:</u>	<ul> <li>due to shrikkage water migration, ai abrasion resistant.</li> <li>Fibermesh reinforc containing no report manufactured for</li> <li>Application of file a minimum of 0.% if a minimum of 0.% if A Fibermesh shall cc requirements, and standard.</li> <li>Steel reinforcing ASTM AbIS gradd be in accordances Standard Practica 1. Horizontal footing and shall have 928 size/spacing as th tension splice.</li> <li>Lap reinforcement for tension or com</li> </ul>	-grade may be used for control of cracking and themal expansion/contraction, lowered in increase in impact capacity, increased is, and residual strength. Increases in impact capacity, increased use as concrete secondary reinforcement, bermesh per cubic yard of concrete shall equal by volume (15 pounds per cubic yard) moby with ASTM Cliffs, any local building code shall meet or exceed the current industry bars shall be new billet steel conforming to s 60. Lion, and placement of reinforcing steel shall with the latest edition of ACI 315. "Manual of s for Detailing Concrete Structures" g and wall reinforcement shall be continuous " bends, or corner bars with the same te horizontal reinforcement with a class B . as required, a minimum of 40 bar diameters pression unless otherwise noted. Splices in minimum of 48 bar diameters.	<ul> <li>other molsture exposed wood shall be tre with AUPA standard C-2</li> <li>Nails shall be common wire nails unless other Lag screws shall conform to ANS/IASME st. Lead holes for lag screws shall be in accc specifications.</li> <li>All beams shall have full bearing on suppor unless otherwise noted.</li> <li>Exterior and load bearing stud walls are t OC, unless otherwise noted. Studs shall be sole plate to the double top plate. Stud discontinuous at headers for window/door of one king stud shall be placed at each King studs shall be comming a column shall be a holidual studs forming a column shall be locked at all floor levels to ensure prop 9. Multi-ply beams shall have each ply attach 24" OC.</li> <li>Four and Tive ply beams shall be bolted to of 1/2" diameter through bolts staggered noted otherwise.</li> </ul>	rwise noted. andard Bi82.1-19 ordance with ND rting framing me o be 2x4 SYP * continuous from s shall only be openings. A mi end of the head attached with or shall be contin. I be properly er load transfe ed with (3) IØd ogether with (2)	81. 6 2 e 16" 1 the nimum ter. ne 10d tous nails e rows	<ul> <li>Specification for Metal Plate Connected Wood Trusses."</li> <li>The truss manufacture shall provide adequate bracing information in accordance with "Commentary and Recommendations for Hardling, Installing, and Bracing Netal Plate Connected Wood Trusses" (HIB-9). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.</li> <li>Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.</li> <li>EXTERIOR WOOD FRAMED DECKS:</li> <li>Decks are to be framed in accordance with local building codes and as referenced on the structural plane, either through code references or construction details.</li> <li>WOOD STRUCTURAL PANELS:</li> <li>Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA standards.</li> <li>All structurally required wood sheathing shall bear the mark of the APA.</li> </ul>

- Lap reinforcement as required, a minimum of 40 bar plameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.
- noted otherwise.
- All structurally required wood sheathing shall bear the mark of the APA.

Manager	
	Signature
Operations	
Operations System	
Operations	

	3.	Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more
	4.	information, sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise. Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d C nail at 6'o/c at panel edges and at 12'o/c in panel field unless
		cheruise noted on the plans, Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plyucod clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
	5.	Wood floor sheathing shall be APA rated sheathing exposure I or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshark nail at 6'o/c at panel edges and at 12'o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing, sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of 14G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
	6.	Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.
	STR	UCTURAL FIBERBOARD PANELS:
	l.	Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.
	2.	All structurally required fiberboard sheathing shall bear the mark of the AFA.
	3.	Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
L	1	Sheathing shall have a 1/8" gap at panel ends and edges are

Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.



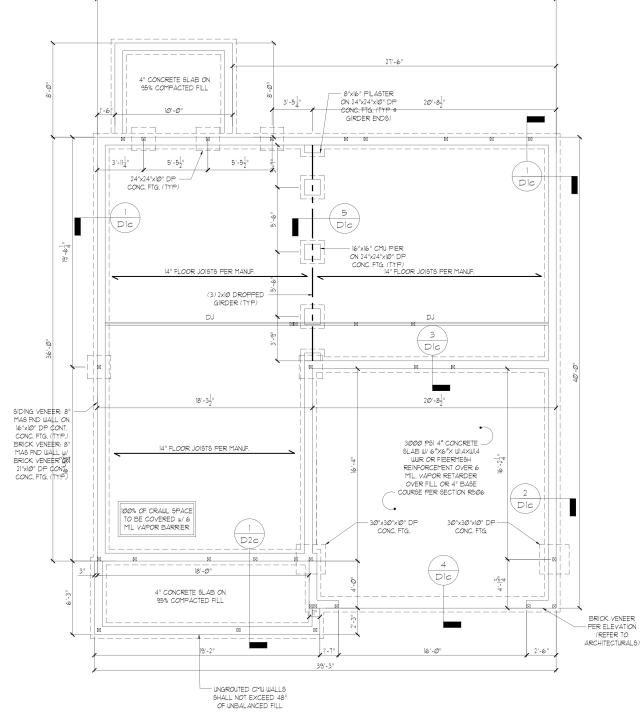
STRUCTURAL MEMBERS ONLY

ENGLOCEMENTAL MEMOLING OTHER ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

CRAWL SPACE FOUNDATION PLAN SCALE: 1/4"=1'-@" ON 22"x34" OR 1/8"=1'-@" ON 11"x17"

<u>ELEVATION B.F.K.</u>



39'-Ø"

SUMMIT ENGINEERING LABORATORY TESTING 3070 HAMMOND BUISINESS PLACE, SUITE 171 RALEIGH, INC 27603 OFFICE: 919.380.9991 FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM NUMBER OF STRANT SUMMIT Engineering, Laborat & Testing, Inc. No. F-1454 DR Horton, Inc. 8001 Arrowidge Bivd. Charlotte, NC 28273 Space Foundation Hayden RH Crawl ( TH CARC NEER HER T 81/24/21 STRUCTURAL MEMBERS ONLY DATE: 1/23/2021 8CALE: 22x34 1/4\*\*1'-Ø\* 1x17 1/8\*\*1'-Ø\* PROJECT 1 528-TØITT DRAWN BY: JOEF CHECKED BY: CTB 5/15/19 22869 REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS SI.Ic

	REQUIRED	BRACED W	ALL PANEL CONNEC	CTIONS
			REQUIRED (	CONNECTION
METHOD	MATERIAL	MIN, THICKNESS	© PANEL EDGES	© INTERMEDIATE SUPPORTS
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.
GB	GYPSUM BOARD	1/2=	5d COOLER NAILS** @ 1ª O.C.	5d COOLER NAILS** @ 7" O.C.
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS ଛ 6" O.C.	6d COMMON NAILS @ 12" O.C.
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.6.4	PER FIGURE R602.10.6.4
		HOP FOUND EN	T PER TABLE R10235	

#### BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE WITH ALL LOCAL AND STATE AMENDMENTS. WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND MAXIMUM WIND SPEEDS UP TO
- 130 MPH REFER TO ARCHITECTURAL PLAN FOR DOOR/JUNDOULOPENING SIZES 3
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R602.10.1
- ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED 04 FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS. 5.
- MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.10.1
- THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD (UNO). FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON
- 8 ALL SHEATHABLE SUFFACTING THEO, EACHONG WALLD SHEALL BEACED WALL ALL SHEATHABLE SUFFACES INCLUDING INFILL AREAS BETUEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS. FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION
- 9. OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS. 10. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 12 FEET OF EACH END OF A
- BRACED WALL LINE. THE MAXIMUM EDGE DIGTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 21 FEET.
- 12. MASONRY OR CONCRETE STEM WALLS W/ A LENGTH OF 48" OR LESS SUPPORTING A
- MASCHRY OK CONCRETE 5 IEM WALLS W/A LENGTH OF 48" OK LESS 50FPORTING A BRACED WALL PAREL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.4.3 OF THE 2018 NCRC.
   BRACED WALL PAREL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.4
   BRACED WALL PAREL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.5

- CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R60210.4.6
- PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.00.1 (UNO) 17. ABBREVIATIONS:
  - WSP = WOOD STRUCTURAL PANEL GB = GYPSUM BOARD 
     GB = GYP5UM BOARD
     WDF = WOOD STAND STAND

     CS-XXX = CONT. SHEATHED
     ENG = ENGINEERED SOLUTION

     FF = PORTAL FRAME
     PF-ENG = ENG. PORTAL FRAME

#### GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL
- BUILDING CODE WITH ALL LOCAL AND STATE AMENDMENTS. CONTRACTOR SHALL VERITY ALL DIMENSIONS. CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.
- 3 CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO REGIST ALL FORCES ENCOUNTERED DURING ERECTION. PROPERTIES USED IN THE DESIGN ARE AS FOLLOUS:
- PROPERTIES USED IN THE DESIGN ARE AS FOLLOUG: MICROLLAM (LVL): F<sub>p</sub> = 12600 PS), F<sub>2</sub> = 255 PS), E = 1921/0<sup>6</sup> PSI PARALLAM (PSL): F<sub>2</sub> = 12600 PS), F<sub>2</sub> = 230 PS], E = 1251/0<sup>6</sup> PSI ALL WOOD MEMBERS SHALL BE <sup>9</sup> SYP UNLESS NOTED ON PLAN. ALL STUD COLUMNS AND JOISTS SHALL BE <sup>9</sup> SYP (UNO). ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 <sup>9</sup> SYP STUD COLUMN AT EACH END UNLESS NOTED OTHERUISE. ALL REINFORCING STEEL SHALL BE GRADE 6/0 BARS CONFORMING TO ASTM AGE AND GUILUI UNGE A NUMBER COURD CO 3<sup>11</sup>

- ASTM A615 AND SHALL HAVE A MINIMUM COVER OF 3". 8. CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- FERFENDICULAR TO RAFIERS. FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH 1/2" DIA. THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/D3f. 9. MIN EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
   ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP 2;
- ALE INVESTIGATION DE LA SUBJECTION DE LA CONTRACTION DE LA CONTRACTICA DE LA CONTRACTION DE LA CONTRACTICA DE LA CONTRAC

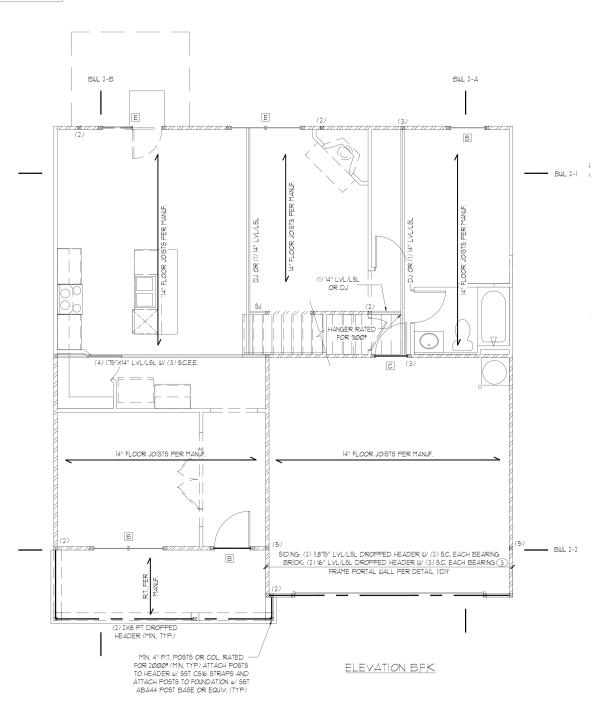
THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL, PLANG PROVIDED BY <u>DR HORTON</u> COMPLETED/REVISED ON <u>4/15/21</u>, IT 16 THE RESPONSIBILITY OF THE CLIENT TO NOTIEY SUMMIT ENGINEERING LABORATORY & TESTING P.C. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY 4 TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR FRAMING PLAN SCALE: 1/4"=1'-Ø" ON 22"x34" OR 1/8"=1'-Ø" ON 11"x17"



FIRST FLOOR BRACING (FT)						
CONTINUOUS SHEATHING METHOD						
	REQUIRED	PROVIDED				
BWL 1-1	11.6	24.8				
BWL 1-2	11.6	15.0				
BWL I-A	11.3	40.0				
BWL 1-B	11.3	36.0				

HEADER SCHEDULE					
TAG	SIZE	JACKS (EACH END)			
Д	(2) 2x6	(1)			
в	(2) 2x8	(2)			
С	(2) 2x1Ø	(2)			
D	(2) 2x12	(2)			
E	(2) 9-1/4" LGL/LVL	(3)			
F	(3) 2x6	(1)			
G	(3) 2x8	(2)			
H	(3) 2x1Ø	(2)			
1	(3) 2x12	(2)			

I. HEADER SIZES SHOWN ON PLANS ARE MINIMUMS. GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION. 2. ALL HEADERS TO BE DROPPED (UN.O.). 3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD COLUMNS LISTED ABOVE (UN.O.)

SCHEDULE
MINIMUM KING STUDS E.E.
(1)
(2)
(2)
(3)
(3)
(3)
(4)
(4)

WALL STUD SCHEDULE (10 FT HEIGHT) ATUD AITE ATUD ARACING (OC

STUD SIZE		STUD SPA	STUD SPACING (O.C.)			
	ROOF ONLY	R00F & 1 FL00R	ROOF & 2 FLOORS	NON-LOAD BEARING		
2×4	24"	16"	12"	24"		
2x6	24"	24"	6"	24"		
NOTES:						

1. BRACED WALLS STUDS SHALL BE A MAX. OF 16" O.C. 2. STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX OF 16" OC

3. TWO STORY WALLS SHALL BE FRAMED W/ 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED W/ HORIZ. BLOCKING @ 6'-Ø" O.C. VERTICALLY.

LINTEL SCHEDULE						
TAG:	SIZE	OPENING SIZE				
	L3x3x1/4"	LESS THAN 6'-0"				
2	L5x3x1/4"	6'-0" TO 10'-0" GREATER THAN 10'-0"				
3	L5x3-1/2x5/16"					
4	L5x3-1/2x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS				
SECURE LINTEL TO HEADER w/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED @ 16" O.C. (TYP FOR )						
ALL HEADERS WHERE BRICK IS USED, TO BE: (UNO)						

SHADED WALLS INDICATED LOAD BEARING WALLS

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

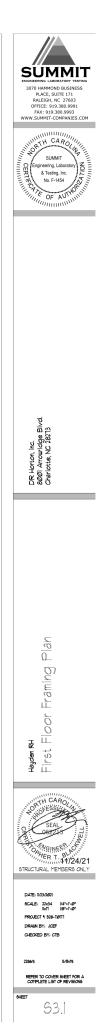
JOIST & BEAM SIZES SHOLN ARE MINIMUMS BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: \_\_\_\_ DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

NOTE: MEMBERS NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDE! THE ENTIRETY OF THE MEMBER 15 WRAPPED TO PREVENT MOISTURE INTRUSION.

INSTALL HOLD-DOWNS FOR BRACED WALL END CONDITIONS PER SECTION R602.0.8 # FIG. R602.0.1 OF THE 2018 NCRC.

NOTE: WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R602.3.5 OF THE 2018 NCRC.



	REQUIRED BRACED WALL PANEL CONNECTIONS				
			REQUIRED	CONNECTION	
METHOD	MATERIAL	MIN. THICKNESS	@ PANEL EDGES	© INTERMEDIATE SUPPORTS	
CS-USP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS ଛ 6" O.C.	6d COMMON NAILS @ 12" O.C.	
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS** ⊛ 1≣ O.C.	5d COOLER NAILS** @ 7" O.C.	
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS ≋ 6" O.C.	6d COMMON NAILS ≋ I2" O.C.	
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R6/02.10.6.4	PER FIGURE R602.10.6.4	
	"OR EQUIVALENT PER TABLE R10235				

#### BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE WITH ALL LOCAL AND STATE AMENDMENTS WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND MAXIMUM WIND SPEEDS UP TO
- 130 MPH REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING SIZES 3
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R6@2.10.1
- 5. ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED WE FEET FOR ISOLATED PANEL METHOD AND IS FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- MINIMUM PANEL I ENGTH SHALL BE PER TABLE R602101
- THE INTERCE LEATE OFFICE DE LET LET AND BOLL ROBAND. THE INTERCE SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD (UNO). FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON
- 8 ALL SHEATHABLE SUFFACES NCLLDNG INTERIOR WALLS SHEATHED ON ALL SHEATHABLE SUFFACES NCLLDNG INFILL AREAS BETLERB BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS.
   FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION
- OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS. 10. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 12 FEET OF EACH END OF A
- BRACED WALL LINE. THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 21 FEET.
- 12. MASONRY OR CONCRETE STEM WALLS W/ A LENGTH OF 48" OR LESS SUPPORTING A
- MASCHRY OK CONCRETE SIEM WALLS W/A LENGTH OF 48° CK LESS SUPPORTING A BRACED WALL PAREL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.4.3 OF THE 2018 NCRC.
   BRACED WALL PAREL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.4
   BRACED WALL PAREL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.5
- CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R60210.4.6
- 16. PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.1 (UNO) 17. ABBREVIATIONS:

GB = GYPSUM BOARD	WSP = WOOD STRUCTURAL PANEL
CS-XXX = CONT. SHEATHED	ENG = ENGINEERED SOLUTION
PF = PORTAL FRAME	PF-ENG = ENG. PORTAL FRAME

GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AND STATE AMENDMENTS. CONTRACTOR SHALL VERITY ALL DIMENSIONS. CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC
- PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM HIS PLAN
- 3 CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO REGIST ALL FORCES ENCOUNTERED DURING ERECTION. PROPERTIES USED IN THE DESIGN ARE AS FOLLOUS:
- PROPERTIES USED IN THE DESIGN ARE AS FOLLOUG: MICROLLAM (LVL), F<sub>0</sub> = 26000 PS), F<sub>2</sub> = 285 PS), E = 1926/<sup>6</sup> PSI PARALLAM (PSL), F<sub>2</sub> = 3200 PSI, F<sub>2</sub> = 230 PSI, F<sub>2</sub> = 125/10<sup>6</sup> PSI ALL WOOD MEMBERS SHALL BE \*2 SYP UNLESS NOTED ON PLAN, ALL STUD COLUMNS AND JOISTS SHALL BE \*2 SYP (INO). ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 \*2 SYP STUD COLUMN AT EACH END UNLESS NOTED OTHERUISE. ALL REINFORCING STELL SHALL BE GRADE 6/0 BARS CONFORMING TO ATT ACH END BUILD UNCE A SIMULAY COLUMN COLOR OF 31

- ASTM AGIS AND SHALL HAVE A MINIMUM COVER OF 3". 8. CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- PERFENDICULAR TO RAFIERS. 9. FLITCH BEAMS, 4-PLY LIVLS AND 3-PLY SIDE LOADED LIVLS SHALL BE BOLTED TOSETHER WITH 1/2" DIA. THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/D3f. MINE DEGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM. 10. ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP <sup>12</sup>,
- DROPPED. FOR NON-LOAD BEARING HEADERS OLECEDING 3'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 SYP 12, DROPPED. (UNLESS NOTED OTHERWISE)

## THESE PLANS ARE DESIGNED IN ACCORDANCE WITH

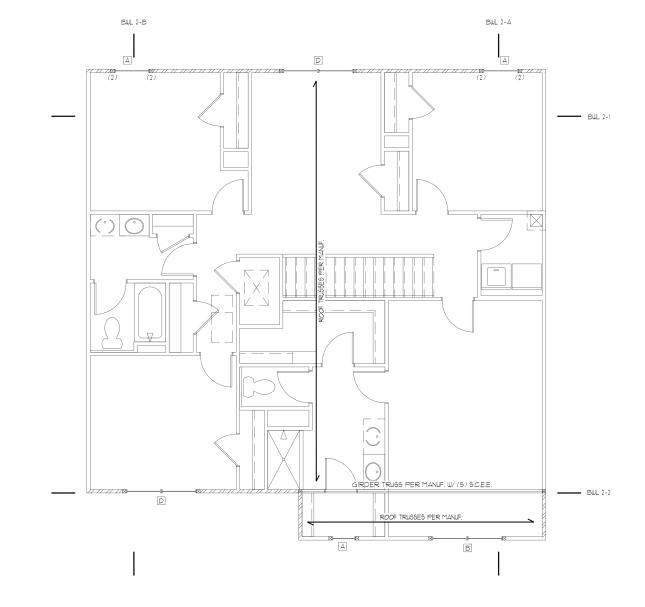
ARCHITECTURAL, PLANG PROVIDED BY <u>DR HORTON</u> COMPLETED/REVISED ON <u>4/15/21</u>, IT 16 THE RESPONSIBILITY OF THE CLIENT TO NOTIEY SUMMIT ENGINEERING LABORATORY & TESTING PC, IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY 4 TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

SECOND FLOOR FRAMING PLAN SCALE: 1/4"=1'-Ø" ON 22"x34" OR 1/8"=1'-Ø" ON 11"x17"



ELEVATION B.F.K.

SECOND FLOOR BRACING (FT)				
CONTIN	WOUS SHEATHING M	ETHOD		
	REQUIRED	PROVIDED		
BWL 2-1	6.0	27.0		
BWL 2-2	6.0	25.Ø		
BWL 2-A	5.8	40.0		
BWL 2-B	5.8	36.0		

TAG	SIZE	JACKS (EACH END)
Д	(2) 2x6	(1)
в	(2) 2x8	(2)
С	(2) 2x1Ø	(2)
D	(2) 2x12	(2)
E	(2) 9-1/4" L6L/LVL	(3)
F	(3) 2x6	(1)
G	(3) 2x8	(2)
Н	(3) 2x1Ø	(2)
1	(3) 2xl2	(2)

2. ALL HEADERS TO BE DROPPED (UN.O.). 3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD

COLUMNS LISTED ABOVE (UN.O.)

KING STUD SCHEDULE			
MAXIMUM HEADER SPAN	MINIMUM KING STUDS E.E.		
4'-Ø"	(D		
6'-0"	(2)		
8'-Ø"	(2)		
10'-0"	(3)		
12'-Ø"	(3)		
14'-Ø"	(3)		
16'-Ø"	(4)		
18'-Ø"	(4)		

WALL STUD SCHEDULE (10 FT HEIGHT) ATUD ADJANK (OC

STUD SIZE	STUD SPACING (O.C.)			
	ROOF ONLY	R00F & 1 FL00R	ROOF \$ 2 FLOORS	NON-LOAD BEARING
2×4	24"	16"	12"	24"
2x6	24"	24"	16 "	24"
NOTES				

I. BRACED WALLS STUDS SHALL BE A MAX. OF 16" O.C. 2. STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX OF 16" OC

3. TWO STORY WALLS SHALL BE FRAMED W/ 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED W/ HORIZ. BLOCKING @ 6'-0" O.C. VERTICALLY,

LINTEL SCHEDULE				
TAG	SIZE	OPENING SIZE		
	L3x3x1/4"	LESS THAN 6'-Ø"		
2	L5x3x1/4"	6'-0" TO 10'-0"		
3	L5x3-1/2x5/16"	GREATER THAN 10'-0"		
(4) L5x3-1/2x5/16" ALL ARCHED ROLLED OR EQUIV. OPENINGS				
SECURE LINTEL TO HEADER #/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED @ 16" O.C. (TYP FOR 3) )				
ALL HEADERS WHERE BRICK IS USED, TO BE: (UNO)				

SHADED WALLS INDICATED LOAD BEARING WALLS

JOIST & BEAM SIZES SHOWN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: MEMBERS NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDE THE ENTIRETY OF THE MEMBER 15 WRAPPED TO PREVENT MOISTURE INTRUSION.

INSTALL HOLD-DOUNS FOR BRACED WALL END CONDITIONS PER SECTION R602.10.8 4 FIG. R602.10.7 OF THE 2018 NCRC.

NOTE: WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R60235 OF THE 2018 NCRC.



STRUCTURAL MEMBERS ONLY

ENGLOCEMENTAL MEMOLING OTHER ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

ROOF FRAMING PLAN SCALE: 1/4"=1'-@" ON 22"x34" OR 1/8"=1'-@" ON 11"x17"

ELEVATION B.F.K.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>DR.HORTON</u> COMPLETED/REVISED ON <u>4/15/21</u> IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY 4 TESTING, PC, IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY 4 TESTING, PC, CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

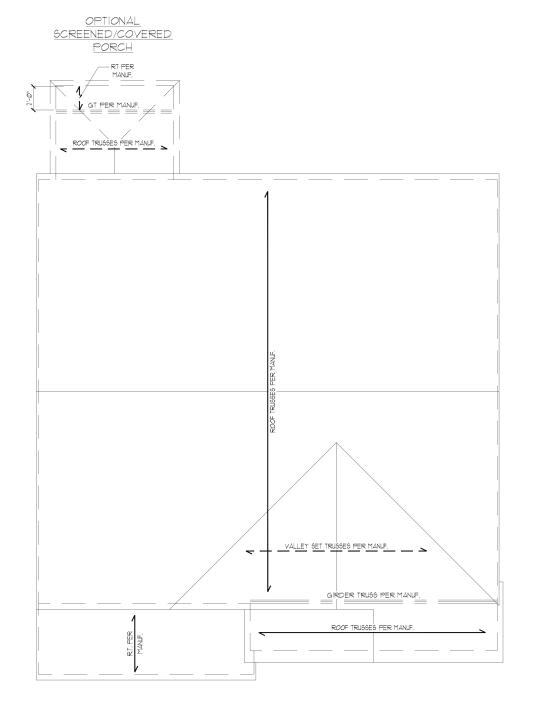
NOTE: TRUSS UPLIFT LOADS SHALL BE DETERMINED PER TRUSS MANUFACTURER IN ACCORDANCE WITH SECTION REGULUI, WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION REG0135 OF THE 2018 NORC, REFER TO BRACED WALL PLANS FOR SHEATHING AND FASTENER REQUIREMENTS.

REFER TO DETAIL 5/D3F FOR EYEBROW, RETURN OR SHED ROOF FRAMING REQUIREMENTS. (TYP FOR ROOFS PROTRUDING MAXIMUM 24" FROM STRUCTURE)

NOTE: ROOF TRUSSES SHALL BE SPACED TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

NOTE: 16T PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNO)

TRUS	TRUSS UPLIFT CONNECTOR SCHEDULE				
MAX, UPLIFT	ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO FND		
6 <i>00</i> LBS	H2.5A	PER WALL SHEATHING & FASTENERS			
12 <i>00</i> LBS	(2) H2.5A	CSI6 (END = 11")	DTT2Z		
1450 LBS	HTS2Ø	CSI6 (END = 11")	DTT2Z		
2 <i>000</i> LBS	(2) MT52Ø	(2) CSI6 (END =   ")	DTT2Z		
2900 LBS	(2) HTS2Ø	(2) CSI6 (END =   ")	HTT4		
3685 LBS	3685 LBS LGT3-5D52.5 M6TC52 HTT4				
2620 EDS LEGIS-50.03.9 195.02.9 191.02.0 191.4 L ALL PRODUCTS LISTED ARE SIMPSON STROKS-TIE. EQUIVALENT PRODUCTS MAY BE USED PER MANIFACTURER'S SPECIFICATIONS. 2. UPLIFT VALUES LISTED ARE FOR SYP <sup>17</sup> GRADE MEMBERS. 3. REFER TO TRUSS LOYOUT PER MANUF. FOR UPLIT VALUES AND TRUSS TO TRUSS CONNECTIONS. CONNECTORS SPECIFIED BY TRUSS MANUFACTURER OVERRIDE THOSE LISTED ABOVE. 4. CONTACT SUMMIT FOR REQUIRED CONNECTORS WHEN LOADS EXCEED THOSE LISTED ABOVE.					





#### DESIGN SPECIFICATIONS:

81. Site Class ...... 82. Design Category ... 83. Importance Factor .

Seisnic Use Group . 8.5. Spectral Response Acceleration 85. Seismic Base Shear 861 VX = 8**6**2.Vy =

8.1. Basic Structural System (check one) ⊠ Bearing Wall □ Building Frame □ Moment Frame

8.8. Arch/Mech Components Anchored ... 8.9. Lateral Design Control: Seismic 
9. Assumed Soil Bearing Capacity

 Dual w/ Special Moment Frame Dual w/ Intermediate R/C or Special Steel

> Wind 🖂 200005

8 Seismic

Construction Tupe: Commerical 🔲 Residential 🛛

Applicable Building Codes:	
<ul> <li>2018 North Carolina Residential</li> </ul>	

°• 2Ø	le Building Codes: 18 North Carolina Residential Building Code with CE 7-10: Minimum Design Loads for Buildings an	
Design L	oads:	
	Roof Live Loads	
	I.I. Conventional 2x	
	1.2. Truse	20 PSF
	12.1. Attic Truss	_ 60 PSF
2.	Roof Dead Loads	
	2.1. Conventional 2x	
	2.2. Truse	
3.	5now	15 PSF
	3.1. Importance Factor	1.0
4.	Floor Live Loads	
	4.1. Typ. Dwelling	40 PSF
	4.2. Sleeping Areas	
	4.3. Decks	
	4.4. Passenger Garage	
5.	Floor Dead Loads	
	5.1. Conventional 2x	
	52. I-Joist	15 PSF
	5.3. Floor Truss	15 PSF
6.	Ultimate Wind Speed (3 sec. gust)	, PER PL <b>A</b> N
	6.1. Exposure	
	6.2. Importance Factor	
	6.3. Wind Base Shear	
	63.1. Vx =	
	632.Vy =	
٦.	Component and Cladding (in PSF)	

J					
				PSF	
				K I"LAN	
		in (PSF)			
	~				
	UP TO 30'	301"-35'	351"-40	4011-45	
ZONE 1	16.1, <b>-</b> 18.Ø	17.5,-18.9	18.2,-19.6	18.7,-20.2	
ZONE 2	16.7,-21.Ø	17.5,-22.1	18.2,-22.9	18.1,-23.5	
ZONE 3	16.7,-21Ø	17.5,-22.1	18.2, -22.9	18.1,-23.5	
ZONE 4	18.2, <b>-</b> 19.Ø	19.2,-20.0	19.9,-2 <b>0</b> .1	20.4,-21.3	
ZONE 5	182,-24Ø	192,-252	19.9,-26.1	20.426.9	
	Floor Live LC 41. Typ. Du 42. Sleepin 43. Decks 44. Passen Floor Dead L 51. Conver 53. Floor T Uttimate Und 63. Und 63	Floor         Live         Loads           41.         Typ. Duelling	Floor         Live         Loads           41.         Typ. Duelling	41. Typ. Duelling       40         42. Sleeping Areas       30         43. Decks       40         44. Passenger Garage       50         Floor Dead Loads       50         51. Conventional 2x       10 F         52. I-Jolat       15 F         53. Floor Truss       15 F         61. Exposure       10 F         63. Wind Base Shear       63. Wind Base Shear         63. Wind Base Shear       63. Vy =         63. Wind Ease Shear       63. Vy =         Component and Cladding (in PSF)       MEAN ROOF         MEAN ROOF       115.78.8       182.78.6         ZONE I       16.1.79.00       11522.1       182.72.9         ZONE 3       16.1.200       11522.1       182.72.9	Floor         Live         Loads         40         PSF           41.         Typ.         Duelling         40         PSF           42.         Sleeping         Areas         30         PSF           43.         Decks         40         PSF           44.         Passenger Garage         50         PSF           Floor         Dead         Loads         10         PSF           51.         Conventional 2x         15         PSF         15         PSF           52.         I-Joits         15         PSF         15         PSF           53.         Irlog Speed (3 sec. gust)         PER PLAN         61         Exposure         10           63.         Ump Tables & Preat         631. Vx =         632. Vy =         632. Vy =         200         115-721         182-723         181,7-202           ZONE 1         I6.1,-180         115-721         182-724         181,7-202         ZONE 1         161,7-210           ZONE 2         16.1,-210         115-721         182,-223         181,-235         ZONE 3         161,7-210         115-721         182,-223         181,-235

SUMMIT

SHEET LIST: Description Sheet No. CGI Cover Sheet Specifications Revision Dim Monolithic Slab Foundation Details Dla Stem Wall Foundation Details Dic Craul Space Foundation Details DЬ Basement Foundation Details DIF Framing Details

STANDARD DETAILS OUNER: DR Horton Carolinas Division

8001 Arrowridge Blvd Charlotte, NC 28213

STRUCTURAL PLANS PREPARED FOR

ARCHITECT/DESIGNER GMD Design Group 1845 Satellite Blvd Duluth GA 3009

PROJECT ADDRESS:

TBD

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory 4 Testing, P.C. before construction begins.

### PLAN ABBREVIATIONS:

AВ	ANCHOR BOLT	PŤ	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	SC	STUD COLUMN
CLR	CLEAR	5J	SINGLE JOIST
ÐJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
D5P	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
Ē	EACH END	<b>S</b> YP	SOUTHERN YELLOW PINE
EΨ	EACH WAY	ŤJ	TRIPLE JOIST
NT9	NOT TO SCALE	TSP	TRIPLE STUD POCKET
8	ON CENTER	TYP	TYPICAL
P#F	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
PSI	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by <u>DR Horton. Inc.</u> Subsequent plan revisions based on roof truss and floor Joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify \$UHHI immediately.

#### REVISION LIST: **Re**vision Project No. Date Description No. E IIIT Added box bay detail (2/D2f). Added deck options with basement. Revised deck options with stem wall and crawl space foundations 2 7,12,17 Revised stem wall insulation note 3 2.15.18 Revised garage door detail, NC only 4 2.28.18 Added high-wind foundation details 5 12.19.18 Revised per 2018 NCRC 6 2.19.19 Revised per Mecklenburg County Comments Revised stem wall deck attachment and i sheathing on wall sections. 8 3.6.19 Corrected dimensions at perimeter footings 9 3220 Added tall turndown detail 10 3.18.20 Added balloon framing detai Added alternate two-pour detail for slab and 102020 added note for crawl girder above grade 3121 12 Added OX-19 Standard Details 13 5.18.21 Updated OX-15 Standard Details 14 @2.14.23 Added 4/D2m - Tall Slab Detail w/ Siding

#### GENERAL STRUCTURAL NOTES:

- The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) or the SER. For t purposes of these construction documents the SER and SUMMIT
- shall be considered the same entity. The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction 2 to stabilize the structure.
- The SER is not responsible for construction sequences, methods, or techniques in comection with the construction of this structure. The SER will not be held responsible for the subcate in call with the benefit to the contract documents, should any non-conformities occur. Any structural elements or details not fully developed on the
- construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions,
- is stop crawings to comprise or or summarized controller, is not the responsibility of the SER or SUMMIT. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins. The SER is not responsible for any secondary structural elements
- or non-structural elements, except for the elements specifically noted on the structural drawings. This structure and all construction shall conform to all
- applicable sections of the international residential code.
- This structure and all construction shall conform to all applicable sections of local building codes.
   All structural assemblies are to meet or exceed to requirements
- of the current local building code

#### FOUNDATIONS:

The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.

- The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade. 6.
- Any fill shall be placed under the direction or recommendation
- of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95%
- maximum div density. Excavations of footings shall be lined temporarily with a 6 mil polysthylene memorane if placement of concrete does not occur within 24 hours of excavation.
- No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.

- <u>STRUCTURAL STEEL:</u>
  1. Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Fractice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design latest editions.
- Structural steel shall receive one coat of shop applied rust-inhibitive paint. 3. All steel shall have a minimum yield stress ( $F_{\mu}$ ) of 36 ksi unless
- otherwise noted. Welding shall conform to the latest edition of the American
- Weiding shall conform to the latest edition of the American Weiding Society's Structural Weiding Code AWS D.I., Electrodes for shop and field weiding shall be class ETØXX. All weiding shall be performed by a certified weider per the above

### CONCRETE:

- NUMBER: Concrete shall have a normal weight aggregate and a minimum compressive strength (Fp) at 28 days of 3000 psi, unless otherwise noted on the plan. Concrete shall be proportioned, mixed, and placed in
- accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- Air entrained concrete must be used for all structural elements exposed to freeze/thau cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of
- target values as follows: 3.1. Footings: 5% 3.2. Exterior Slabs: 5%
- 4. No admixtures shall be added to any structural concrete without written permission of the SER.

- Concrete slabs-on-grade shall be constructed in accordance 5 uith ACI 302.IR-96: "Guide for Concrete Slab and Slab Construction"
- The concrete slab-on-arade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported
- conditions not in accordance with the above assumptions. Control or saw cut joints shall be spaced in interior slabs-on-orade at a maximum of 15'-0" O.C. and in exterior
- slabs-on-grade at a maximum of 10'-0" unless otherwise noted. 8 Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
- process winin + to 1/ nous after the slap has been initiated.
   Reinforcing steel may not extend through a control joint.
   Reinforcing steel may extend through a saw cut joint.
   I/I welded wire fabric (UWE) for concrete slabs-on-grade shall be placed at mid-depth of slab. The UWE, shall be securely supported during the concrete pour.

- <u>CONCRETE REINFORCEMENT:</u> I. Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- Thermosh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- Application of fibermesh per cubic yard of concrete shall equal a minimum of 20% by volume (15 pounds per cubic yard) Fibermesh shall comply with ASTM CIII6, any local building code requirements, and shall meet or exceed the current industry 4
- standard. Stæel reinforcing bars shall be new billet steel conforming to
- 6
- ASTM A65, grade 60. ASTM A65, grade 60. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Nanual of Standard Practice for Detailing Concrete Structures" Horizontal footing and wall reinforcement shall be continuous and shall have 30° bends, or comer bars with the same are for acless as the borgent at the class B size/spacing as the horizontal reinforcement with a class B
- tension splice. Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.

- Where reinforcing dowels are required , they shall be equivalent in size and spacing to the vertical reinforcement. The doule shall extend 48 bar diameters vertically and 20 bar diameters
- into the footing. 10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.
- Solid saun wood framing members shall conform to the specifications listed in the latest edition of the "National
- Design Specification for Wood Construction" (NDS), Unless otherwise noted, all wood framing members are designed to be
- Spruce-Yellow-Pine (SYP) 2. LVL or PSL engineered wood shall have the following minimum

- 2.4.Fc = 100 psi
- Wood in contact with concrete, masony, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance
- Nails shall be common wire nails unless otherwise noted.
- specifications. All beams shall have full bearing on supporting framing members
- unless otherwise noted. Exterior and load bearing stud walls are to be 2x4 SYP #2 # 16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- King studs shall be continuous. Individual studs forming a column shall be attached with one l&d nall # 6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all filor levels to ensure proper load transfer.
- Multi-ply beams shall have each ply attached with (3) 10d nails @ 24" O.C.
- 10. Flitch beams, 4-ply beams and 3-ply side loaded beams shall be Inter beams, + py beams and ppg side back to be the bolted together with (2) rous of  $12^n$  diameter through bolts staggered = 16" OC. unless noted otherwise. Min. edge distance shall be 2" and (2) bolts shall be located a min. 6" from each end of the beam

#### WOOD TRUSSES:

- <u>QOD TRUSES</u>. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overail compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses shall be designed for all required loadings a specification to the designed for all required loadings
- 2 Ine wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 1-05), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to
- Hrve expension, provide a statement of the trusses. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction" (NDS) and "Design Specification for Metal Plate Connected Wood Trusses." 3.
- 4. The truss manufacturer shall provide adequate bracing Instruiss manufacturer sharp provide adequate cracing information in accordance with "Commentary and Recommendations for Handling, installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments fo
- the trusses.
   Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacture

### EXTERIOR WOOD FRAMED DECKS:

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

2

- UCOD STRUCTURAL PANELS: I. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA
- All structurally required wood sheathing shall bear the mark of the APA

## WOOD FRAMING:

- ign values: 2.1. E = 1,900,000 psi
  - - 2.2. F<sub>b</sub> = 2600 psi 2.3. F<sub>v</sub> = 285 psi
  - with AWPA standard C-2
  - Lag screws shall conform to ANSI/ASME standard B182.1-1381. Lead holes for lag screws shall be in accordance with NDS

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Development

	DR HORTON PROJECT SIGN-OFF.		
I	Manager	Signature	
	Operations		
	Operations System		
I	Operations Product		

SUCCESSION OF A STATE
culent: Dr. Hercin Carolina Division 2009 Arramidge Bivd. <b>Charolus, NC 2001</b> 3
FROJECT: Standard Details (OX-15) Coversheet
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DRUING DATE OWADDS DATE OWADDS DATE OWADDS DATE OWADDS DATE OWADDS DATE OWADDS DATE OWADDS DATE OWADDS
CHECKED BY: BOP ORIGINAL INFORMATION PROJECT * DATE V3/VOT REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS
CSI

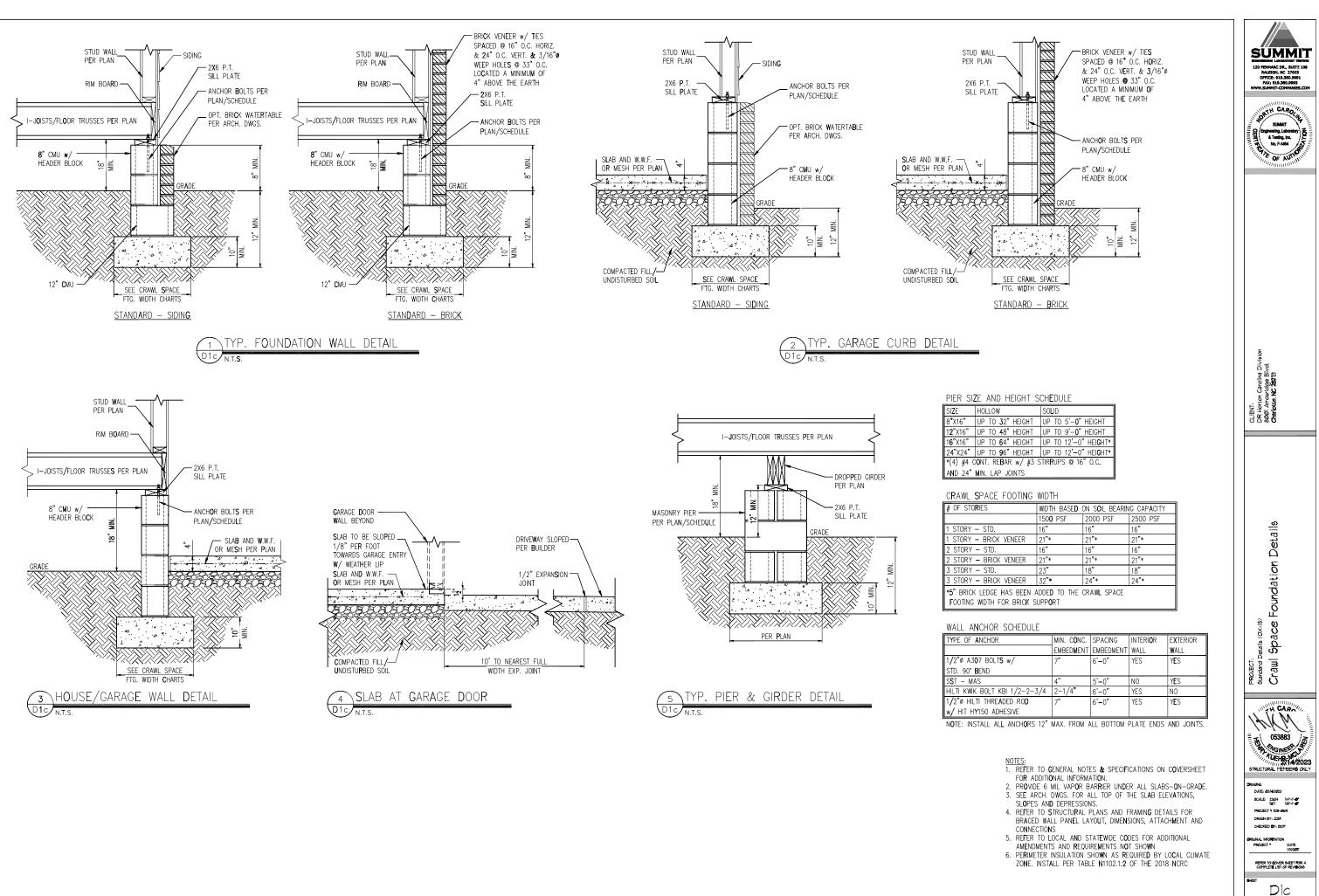
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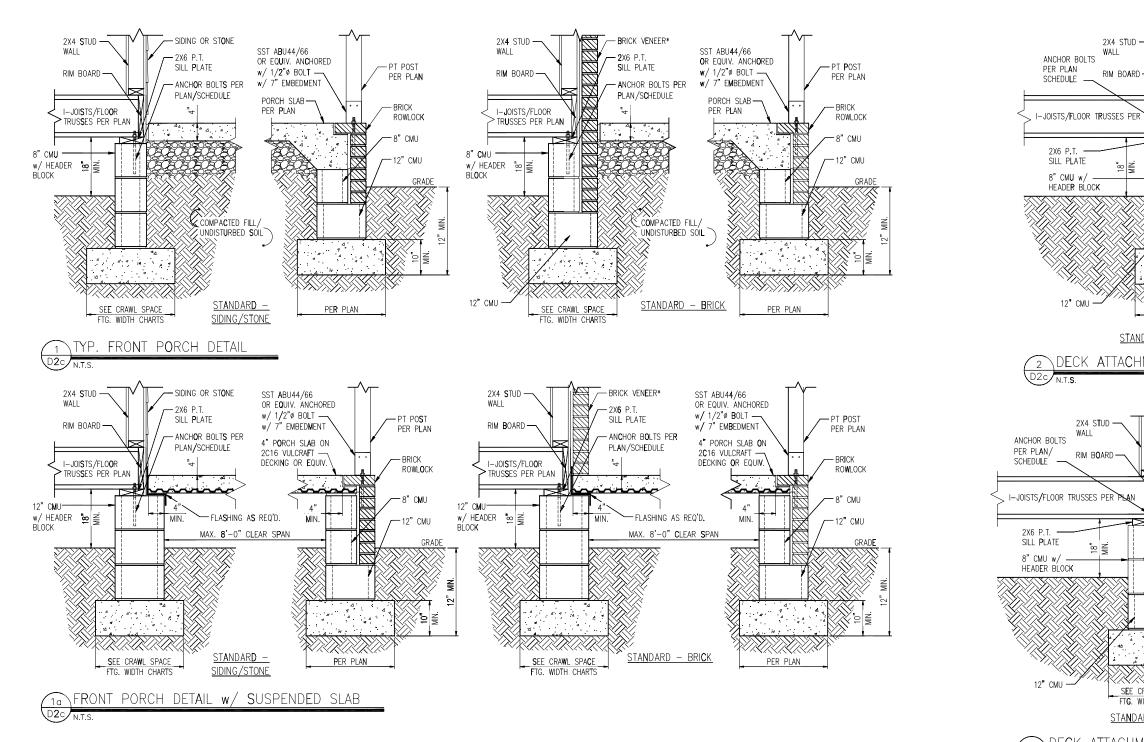
- Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information, theathing shall be applied with the long direction perpendicular to framing, unless noted otherwise. Roof sheathing shall be APA rated sheathing exposure 1 or 2.
- Roof sheathing shall be continuous over two subports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use
- support by use of T4C plywood or lumber blocking unless otherwise noted. Panel and joints shall occur over framing. Apply building paper over the sheathing as required by the
- She building Code. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

<u>STRUCTURAL FIBERBOARD PANELS:</u> I. Fabrication and placement of structural fiberboard sheathing

- shall be in accordance with the applicable AFA standards All structurally required fiberboard sheathing shall bear the mark of the AFA.
- Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- Sheathing shall have a 1/8" gap at panel ends and edges are

have a span rating consistent with the framing spacing, Use suitable edge support by use of plyucod clips or lumber blocking unless otherwise noted. Panel and joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code. Wood floor sheathing to its supporting framing with (1)-8d CC ringshank nail at 6°/oC at panel edges and at 2°/oC in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall be applied perpendicular to framing. Sheathing shall be applied perpendicular to framing. Sheathing shall be applied perpendicular to framing sheathing shall be applied perpendicular to framing sheathing shall be applied perpendicular to framing. Sheathing and the edge support buse of TKG plucod or lumbor blocking unless





	DECK ATTACHMENT	SCHEDULE (A	ALL STRUCTURES	FXCEPT BRICK)	
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MAX. 8'-0" JOIST	MAX. 16'-0" JOIST
SPAN	SPAN
(1) <b>@</b> 3'-6" 0.C.	(1) @ 1'-8" O.C.
AND	AND
(2) @ 8" 0. <b>C</b> .	(3) @ 6" O.C.
	SPAN (1) @ 3'-6" O.C. AND

a. ATTACHMENT INTERPOLATION BETWEEN 8' AND 16' JOIST SPANS IS ALLOWED.

b. MINIMUM EDGE DISTANCE FOR BOLTS IS 21".

c. NAILS MUST PENETRATE THE SUPPORTING STRUCTURE BAND A MINIMUM OF  $1\frac{1}{2}$ "

DECK ATTACHMENT SCHEDULE (BRICK STRUCTURES)

FASTENERS	MAX. 8'-0" JOIST	MAX. 16'-0" JOIST
	SPAN	SPAN
5/8" GALV. BOLTS w/ NUT & WASHER <sup>D</sup>	(1) <b>@</b> 2'-4" 0.C.	(1) @ 1'-4" O.C.

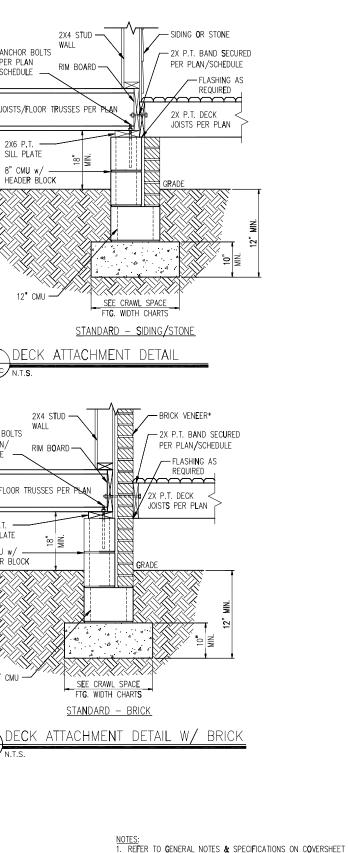
a. ATTACHMENT INTERPOLATION BETWEEN 8' AND 16' JOIST SPANS IS ALLOWED.

b. MINIMUM EDGE DISTANCE FOR BOLTS IS 21".

### CRAWL SPACE FOOTING WIDTH

# OF STORIES	WIDTH BASED	ON SOI <b>L</b> BEARIN	ig capa <b>c</b> ity
	1500 PSF	2000 PSF	2500 P <b>S</b> F
1 STORY - STD.	16"	16"	16"
1 STORY - BRICK VENEER	21"*	21"*	21"*
2 STORY - STD.	16"	16"	16"
2 STORY - BRICK VENEER	21"*	21"*	21"*
3 STORY - STD.	23"	18"	18"
3 STORY - BRICK VENEER	32"*	24"*	24"*
*5" BRICK LEDGE HAS BEEN / FOOTING WIDTH FOR BRICK S		CRAWL <b>S</b> PACE	

\*BRICK TIES SPACED @ 16" O.C. HORIZ. & 24" O.C. VERT. AND 3/16"Ø WEEP HOLES @ 33" O.C. LOCATED A MINIMUM OF 4" ABOVE THE EARTH

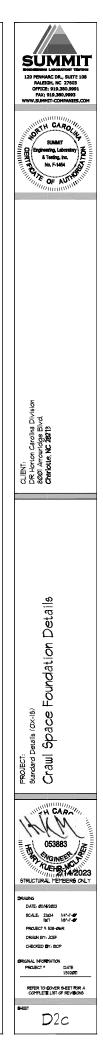


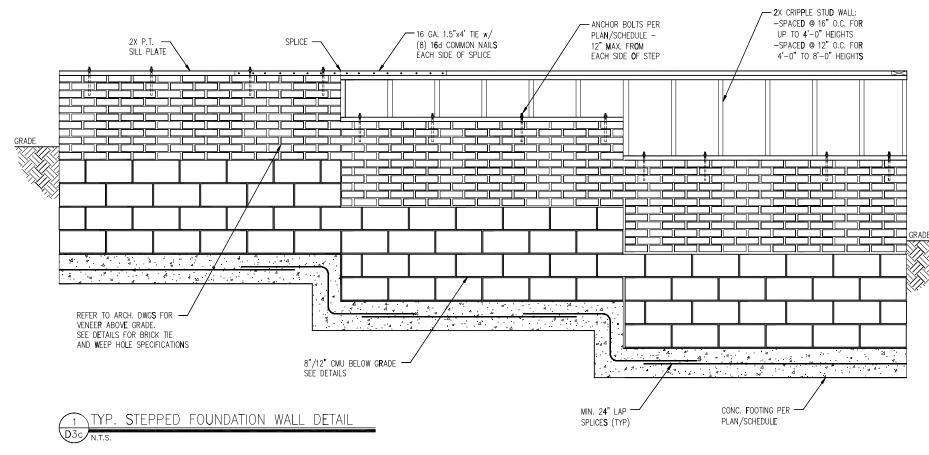
FOR ADDITIONAL INFORMATION.

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NTS

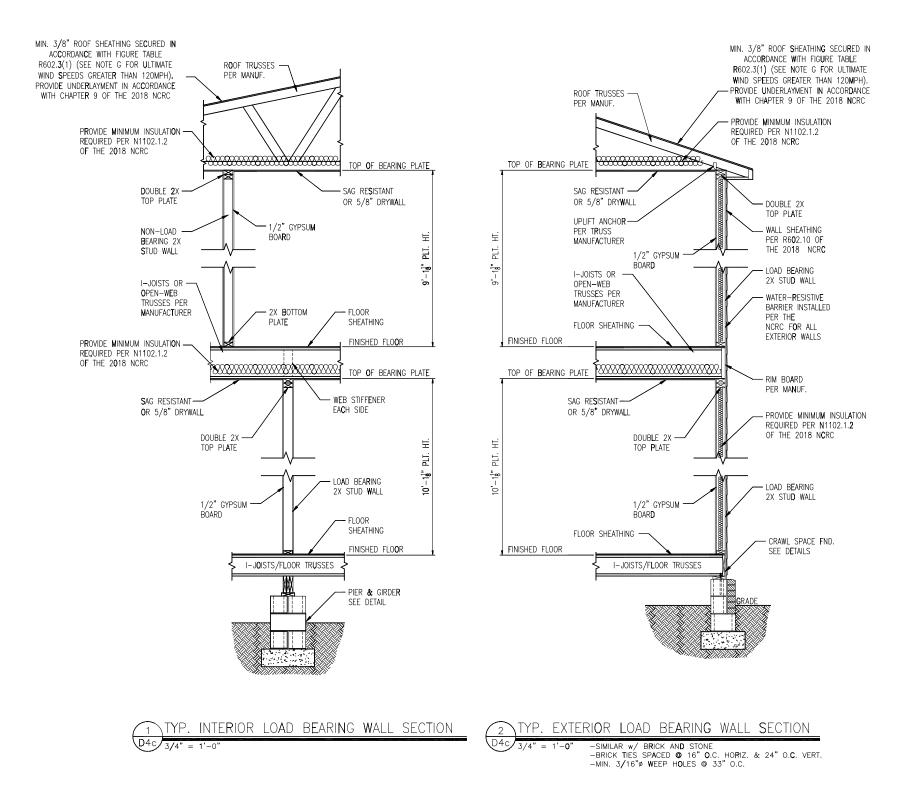
- PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE. . SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.
- 4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND CONNECTIONS
- 5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN
- 6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC

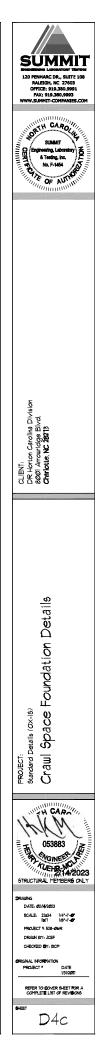




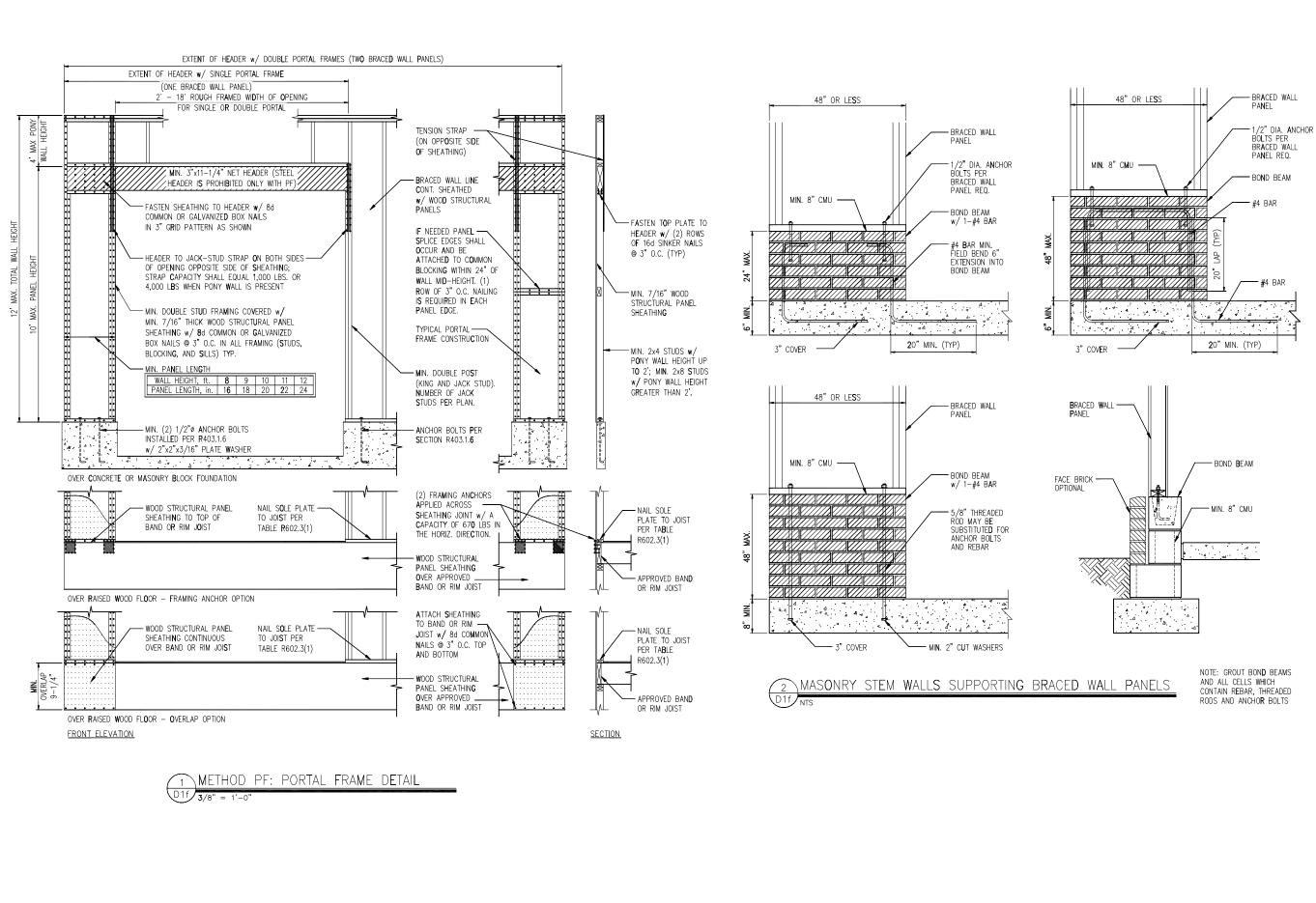


- NOTES: 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET FOR ADDITIONAL INFORMATION.
- PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
   SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.
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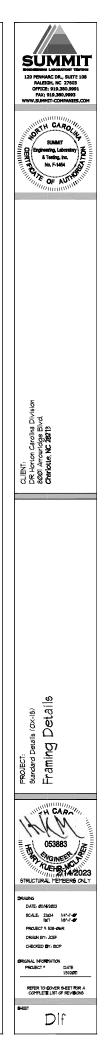


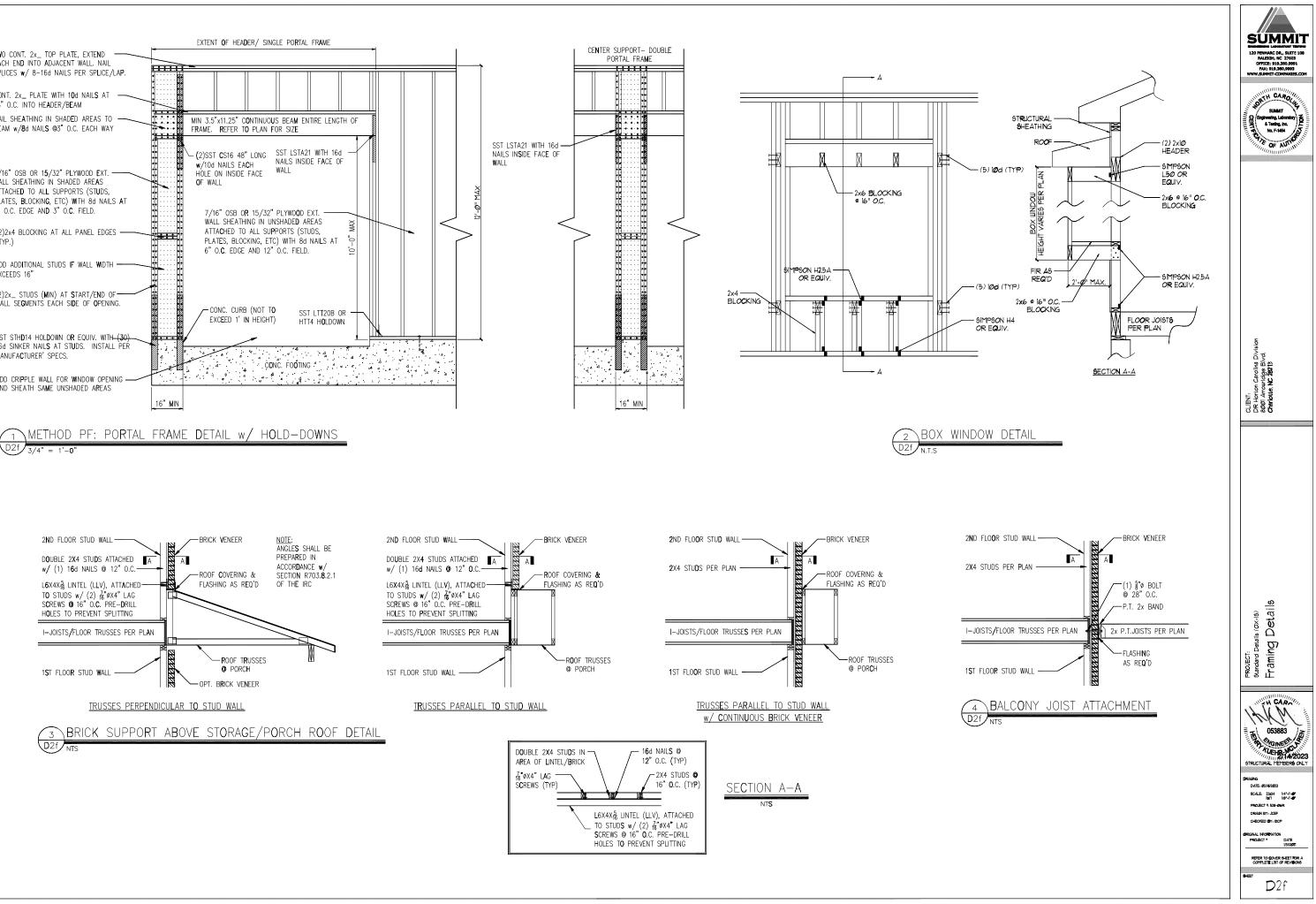


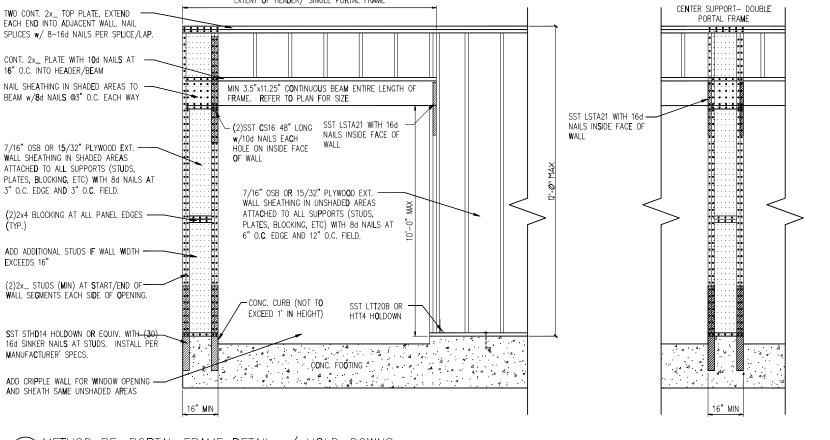
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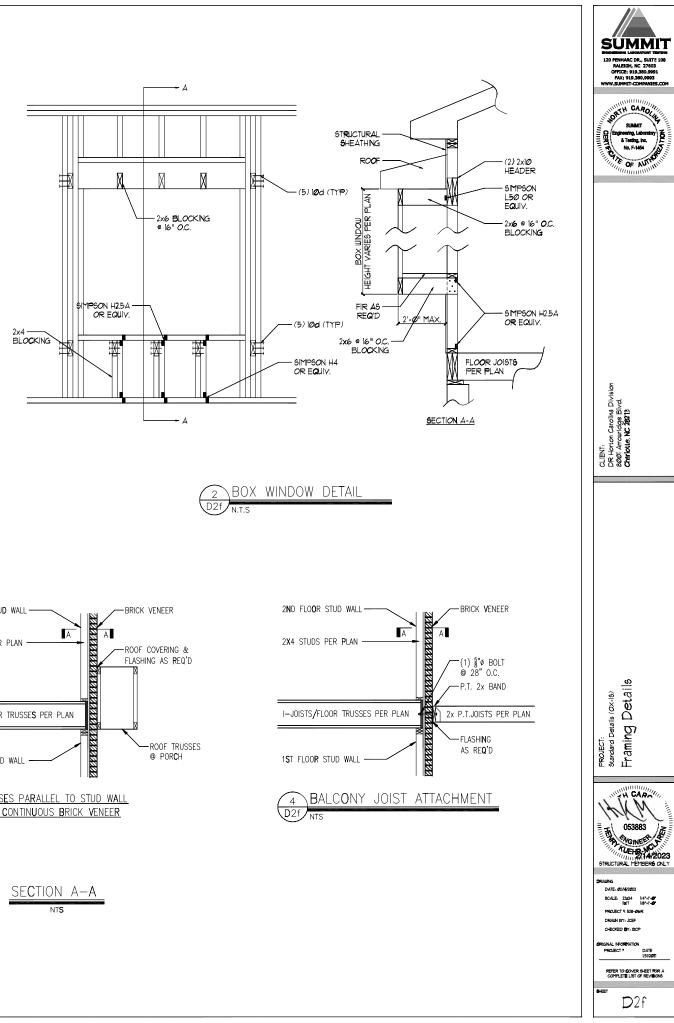


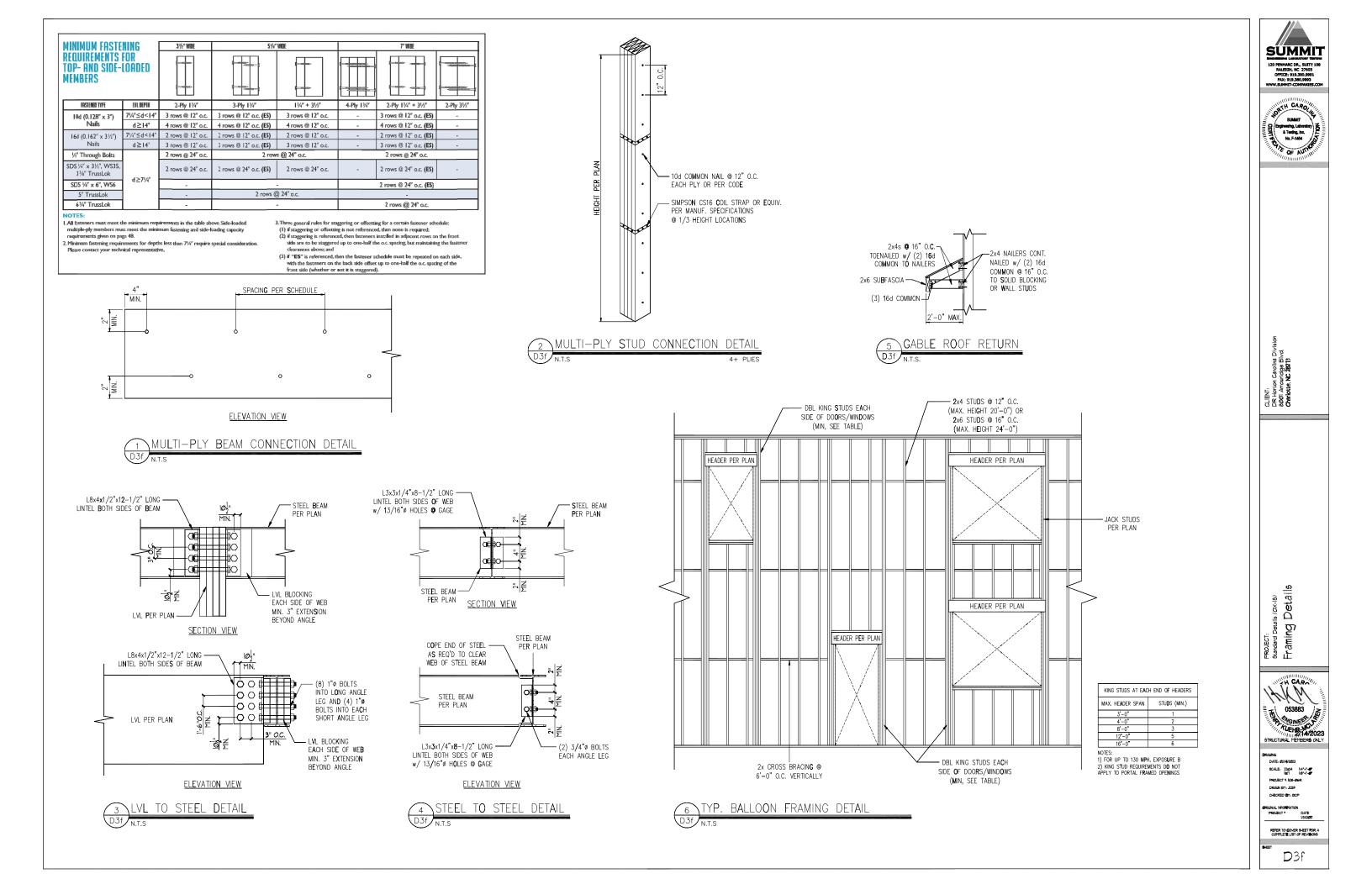


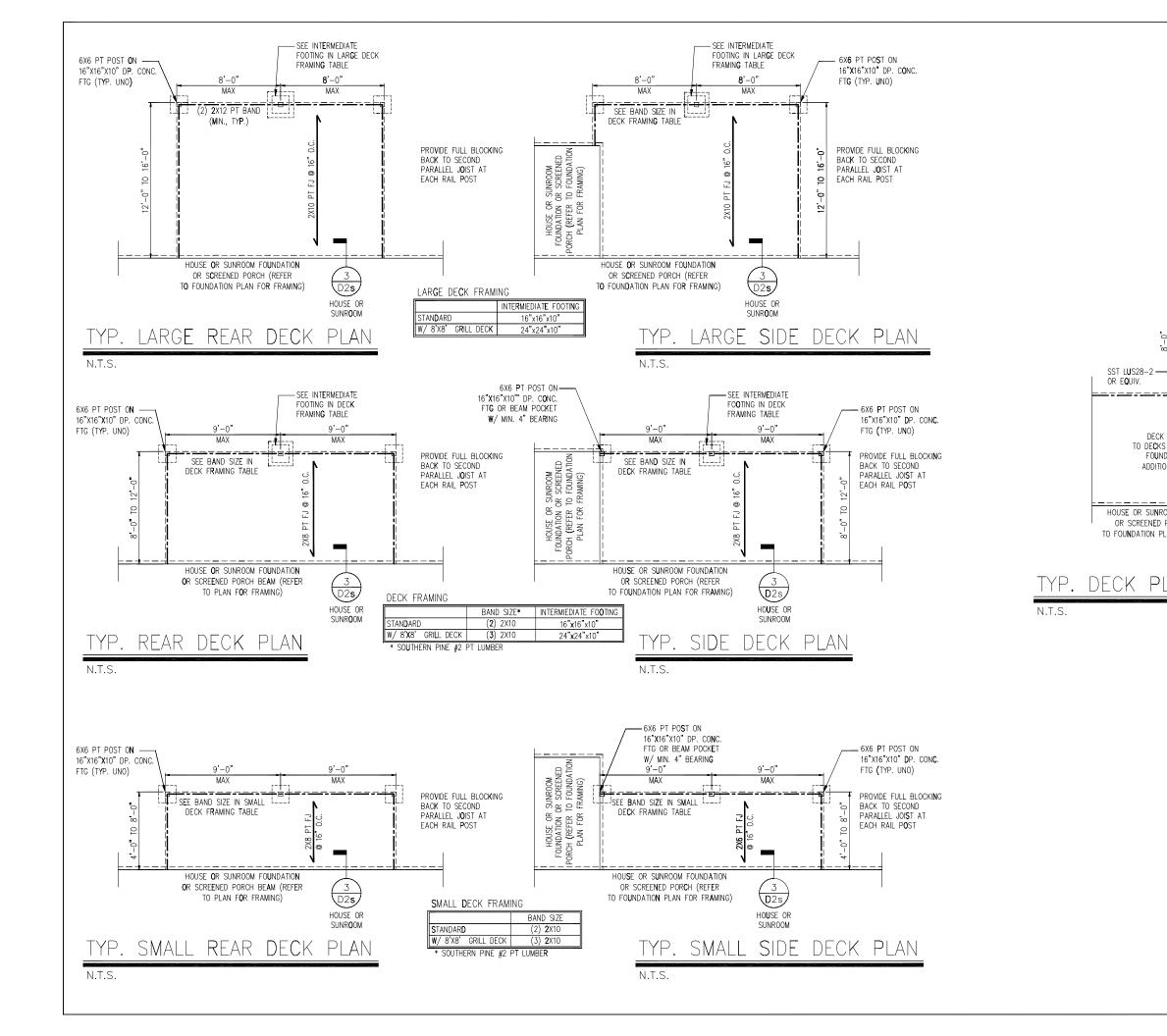


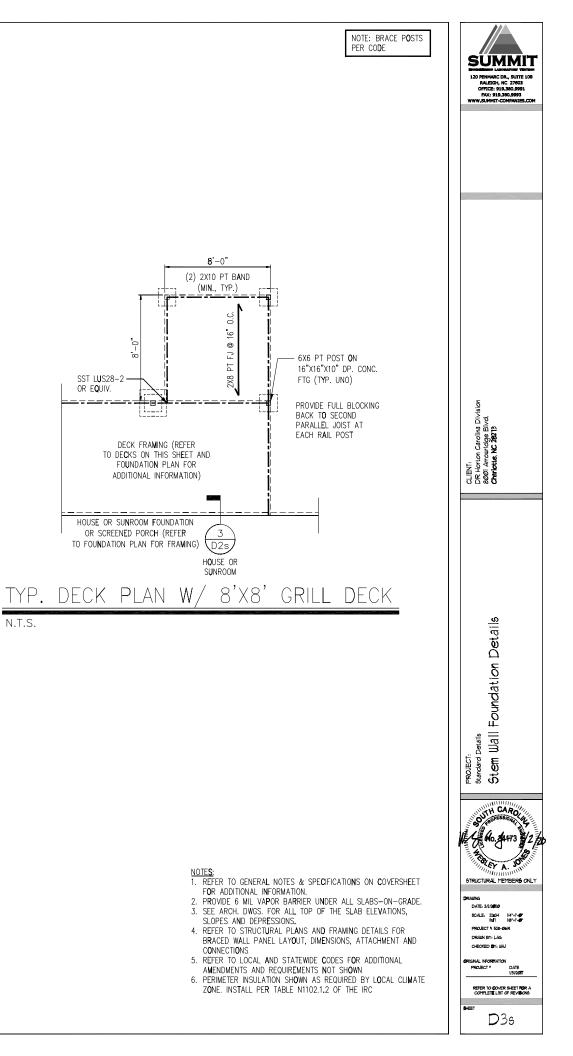








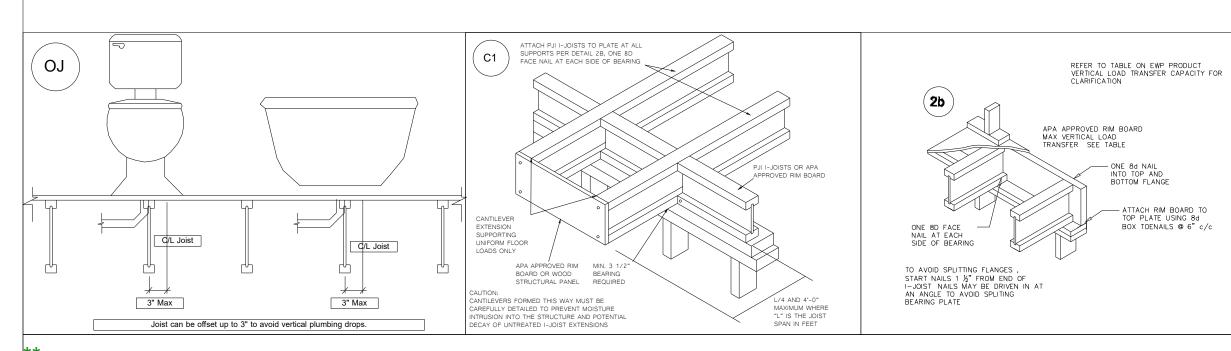




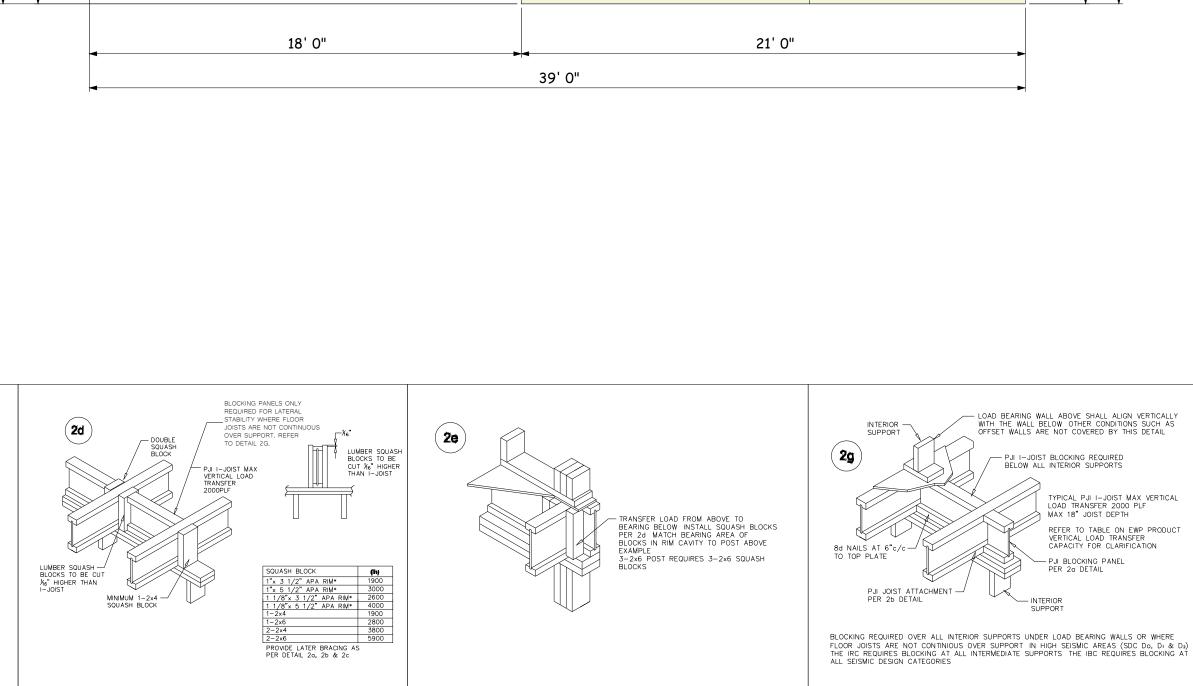
<b>General N</b>	lotes:	** CUTTING OR DRILLING OF COMPONENTS SHOULD NOT BE DONE WITHOUT CONTACTING COMPONENT SUPPLIER
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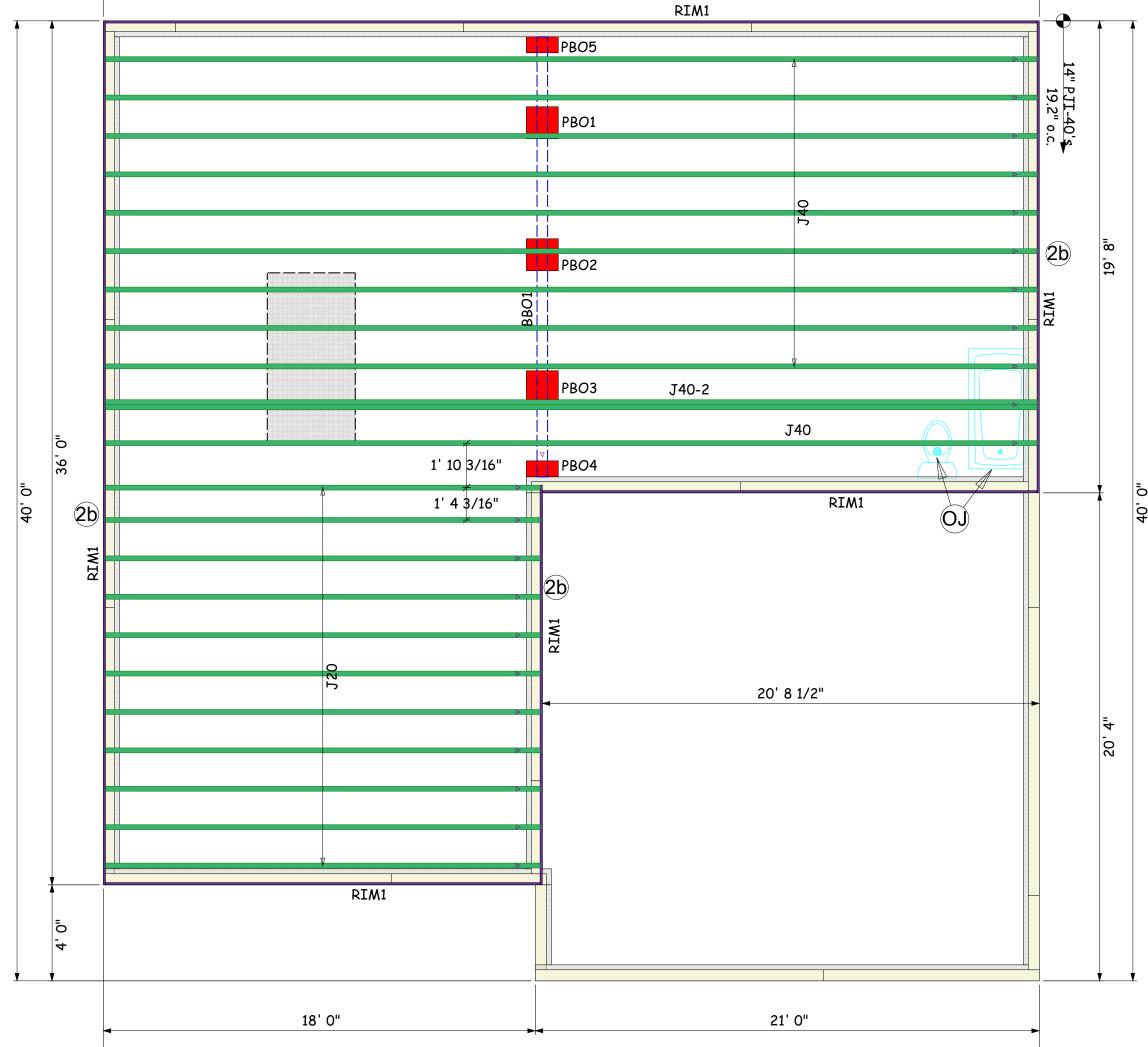
			Pr	oducts		
Net Qty	Plies			Product	Length	PlotID
10	1			14" PJI-40	40' 0"	J40
2	2			14" PJI-40	40' 0"	J40-2
11	1			14" PJI-40	20' 0"	J20
13	1	1 1/	8" × 14"	APA Rim Board	12' 0"	RIM1
				Accessories		
	Net (	Qty	Plies	Product	Length	PlotID
		34	1	3/4" 4x8 OSB		

KEMPSVILLE BUILDING MATERIALS IS NOT RESPONSIBLE FOR THE DESIGN OR CALCULATION OF ANY AND ALL I-JOIST AND LVL/PSL BEAM MATERIAL. ALL ENGINEERING AND INFORMATION FOR THIS MATERIAL IS TO BE PROVIDED BY THE ENGINEER OF RECORD MARKED ON APPROVED SET OF PLANS. ALL BEAM PLACEMENTS ARE PER THE ENGINEERING RECEIVED. ALL CONNECTION DETAILS TO BE PROVIDED BY ENGINEER OF RECORD. REFER TO ENGINEER OR RECORD FOR ALL MULTI-PLY LVL/ I-JOIST CONNECTION PATTERNS. BUILDER TO VERIFY ALL MATERIAL LENGTHS, QUANTITIES, AND SIZES PRIOR TO ORDERING.



\*\* PLUMBING DROPS NOTED ARE IN APPROXIMATE LOCATIONS PER PLAN. BUILDER MUST VERY LOCATIONS BEFORE SETTING JOISTS.

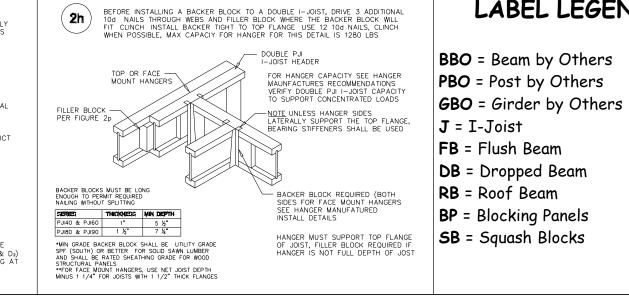




39' 0"

Scale: Date: / Designe Project		This is an I-Joist Placement Plan Only. All designs of It loist follow the IBC/IRC Code Requirements about with	
ne rt			09/20 00/00 00/00 00/00
<b>9</b> 1/4" / 06/0 er: D	dge	placement plan is created from plans ner using Manufactures guidelines. It is e EOR, or builder to review and approve connections, spans, loading, product	)/00 )/00 )/00
= 1'-0 04/24 0W	A Division of the Certer Lumber Company	usage, and quantities. Do not notch or drill holes in beams or flanges on joists without prior approval from the manufacturing Representative unless following hole guidlines in the installation quide of product. Builder takes full responsibility for doing so	ons DW Nam Nam Nam
		and NO Back charge will be accepted.	e e e

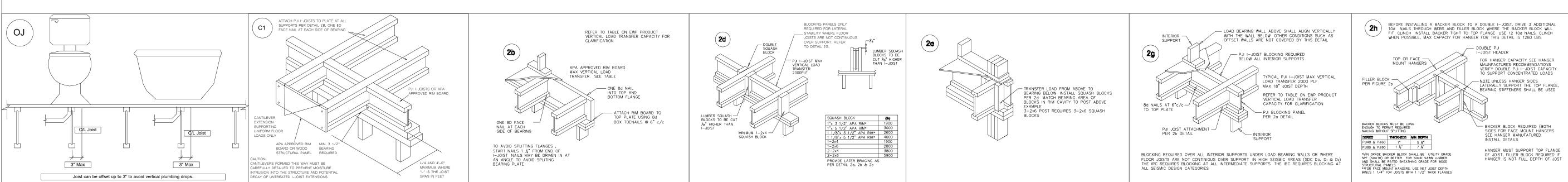
## LABEL LEGEND



		Products
Net Qty	Plies	Product
9	1	14" PJI-40
3	1	14" PJI-40
18	1	14" PJI-40
4	2	14" PJI-40
6	1	14" PJI-40
1	1	14" PJI-40
2	2	2.1 RigidLam SP LVL 1-3/4 x 9-1/4
2	2	2.1 RigidLam SP LVL 1-3/4 x 9-1/4
4	4	2.1 RigidLam SP LVL 1-3/4 x 14
1	1	2.1 RigidLam SP LVL 1-3/4 x 14
2	2	2.1 RigidLam SP LVL 1-3/4 x 16
16	1	1 1/8" x 14" APA Rim Board
2	1	14" PJI-40

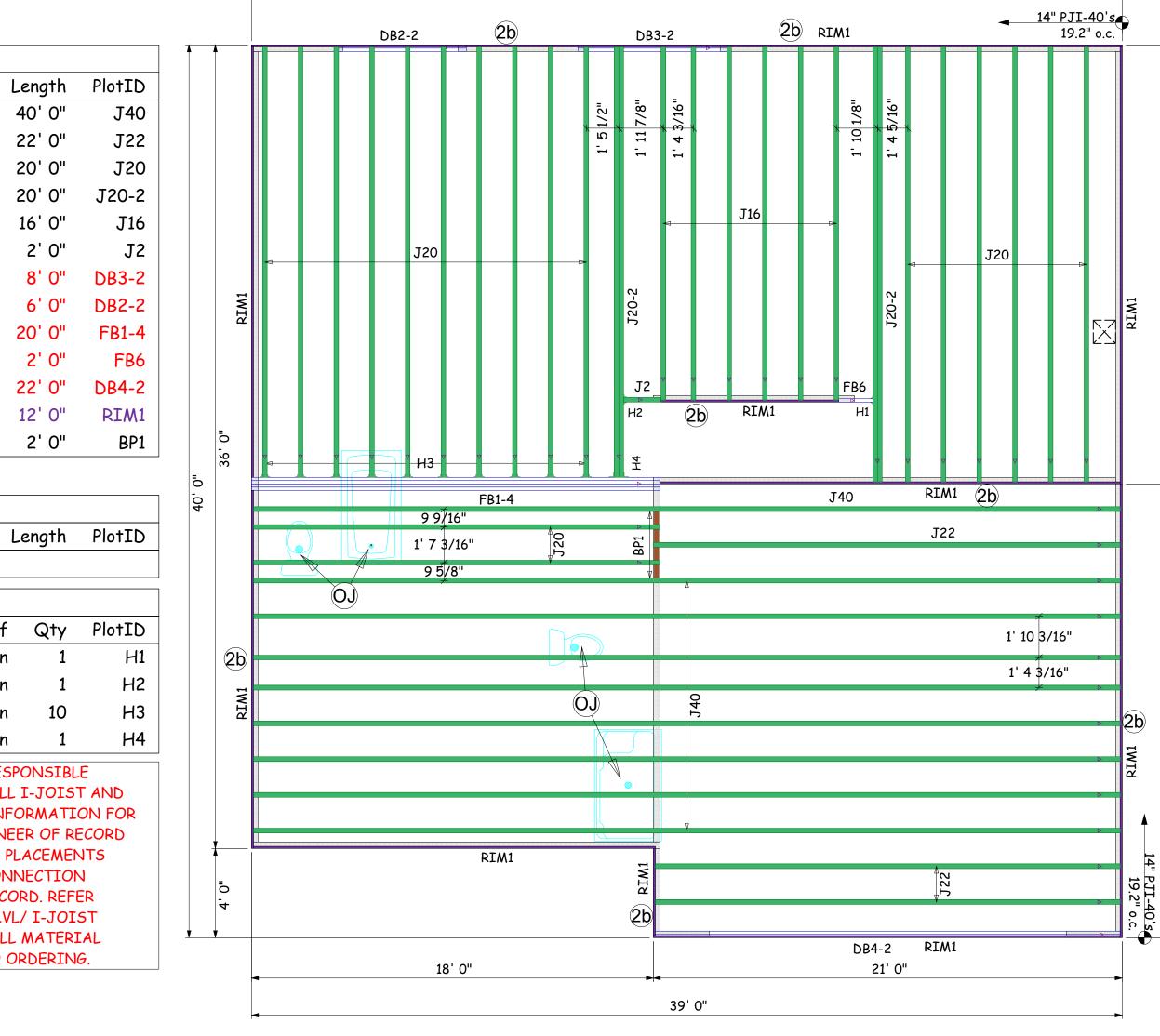
			Acces	sories	
	Net Qty	Plies	F	Product	L
	46	1	3/4" 4>	<8 OSB	
	Со	nnector	' Summar	У	
Web Stiff	Backer Blocks		Product	Manu	ıf
No	No	HUS	51.81/10	Simpso	n
No	2 and Filler	IUS	2.56/14	Simpso	n
No	No	IUS	2.56/14	Simpso	n
No	No	MIU	5.12/14	Simpso	n

KEMPSVILLE BUILDING MATERIALS IS NOT RESPONSIBLE FOR THE DESIGN OR CALCULATION OF ANY AND ALL I-JOIST AND LVL/PSL BEAM MATERIAL. ALL ENGINEERING AND INFORMATION FOR THIS MATERIAL IS TO BE PROVIDED BY THE ENGINEER OF RECORD MARKED ON APPROVED SET OF PLANS. ALL BEAM PLACEMENTS ARE PER THE ENGINEERING RECEIVED. ALL CONNECTION DETAILS TO BE PROVIDED BY ENGINEER OF RECORD. REFER TO ENGINEER OR RECORD FOR ALL MULTI-PLY LVL/ I-JOIST CONNECTION PATTERNS. BUILDER TO VERIFY ALL MATERIAL LENGTHS, QUANTITIES, AND SIZES PRIOR TO ORDERING.



\*\* ALL POINT LOADS FROM ABOVE MUST BE TRANSFERRED TO BEARING FROM UNDER SIDE OF SHEATHING.

\*\* PLUMBING DROPS NOTED ARE IN APPROXIMATE LOCATIONS PER PLAN. BUILDER MUST VERY LOCATIONS BEFORE SETTING JOISTS.



39' 0"

** DAMAGED FLOOR JOISTS SHOULD NOT BE	DAMAGED FLOOR JOISTS SHOULD NOT BE INSTALLED UNLESS APPROVED BY COMPONENT PLANT.	** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH.	** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS.	JENTS.
Date: Desigi Projec	DR Horton		<b>This is an I-Joist Placement Plan Only</b> . All designs of I-Joist follow the IBC/IRC Code Requirements along with Manufacturer's guidelines. This is NOT an engineered	09/20 00/00 00/00 00/00
1/4" = // 06/0 ner: D t #: 240 Sheet Num	45 Mason Ridge Havden K		procernent plan. This procement plan is created from plans provided by the customer using Manufactures guidelines. It is the responsibility of the EOR, or builder to review and approve all bearing conditions, connections, spans, loading, product usage, and quantities. Do not notch or drill holes in beams or	)/00 )/00 )/00
4/24 N 050245	FLOOR JOIST LAYOUT	A Division of the Certer Lumber Company		ns DW Name Name Name Name

# 2ND FLOOR LAYOUT

**J** = I-Joist

**FB** = Flush Beam

**RB** = Roof Beam

**DB** = Dropped Beam

**BP** = Blocking Panels

**SB** = Squash Blocks

LABEL LEGEND

**BBO** = Beam by Others

**PBO =** Post by Others

**GBO** = Girder by Others

\*\* REFER TO INSTALLATION GUIDE FOR PLY TO PLY CONNECTIONS.

0

**A** 

14"

40

-**Ö**-

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