



ELEVATION NOTES:
 GRADE ELEVATIONS SHOWN DO NOT NECESSARILY REFER TO THIS OR ANY OTHER LOT. THEY ARE FOR DIAGRAMMATIC PURPOSES ONLY AND MAY VARY. BUILDER IS RESPONSIBLE FOR ADAPTING THIS PLAN TO SUIT THE EXISTING TOPOGRAPHY OF THE SITE.

ROOF VENTILATION TO BE DETERMINED BY BUILDER AS PER CODE.

ALL EGRESS OR RESCUE WINDOWS FROM SLEEPING ROOMS MUST HAVE A MIN. NET CLEAR OPENING OF 4.0 SQ FT. THE MIN NET CLEAR OPENING HEIGHT DIMENSION SHALL BE 22". THE MIN NET CLEAR OPENING WIDTH SHALL BE 20".

EACH EGRESS WINDOW FROM SLEEPING ROOMS MUST HAVE A SILL HEIGHT OF NO MORE THAN 44" FROM THE FLOOR. ALL WINDOW SIZES ARE NOMINAL AND ARE TO BE VERIFIED WITH MANUFACTURER FOR AVAILABILITY AND CONFORMITY TO STATE AND LOCAL CODE REQUIREMENTS.

PORCHES, BALCONIES, OR RAISED FLOOR SURFACES LOCATED MORE THAN 30" ABOVE THE FLOOR OR GRADE BELOW SHALL HAVE GUARDRAILS NOT LESS THAN 32" IN HEIGHT.

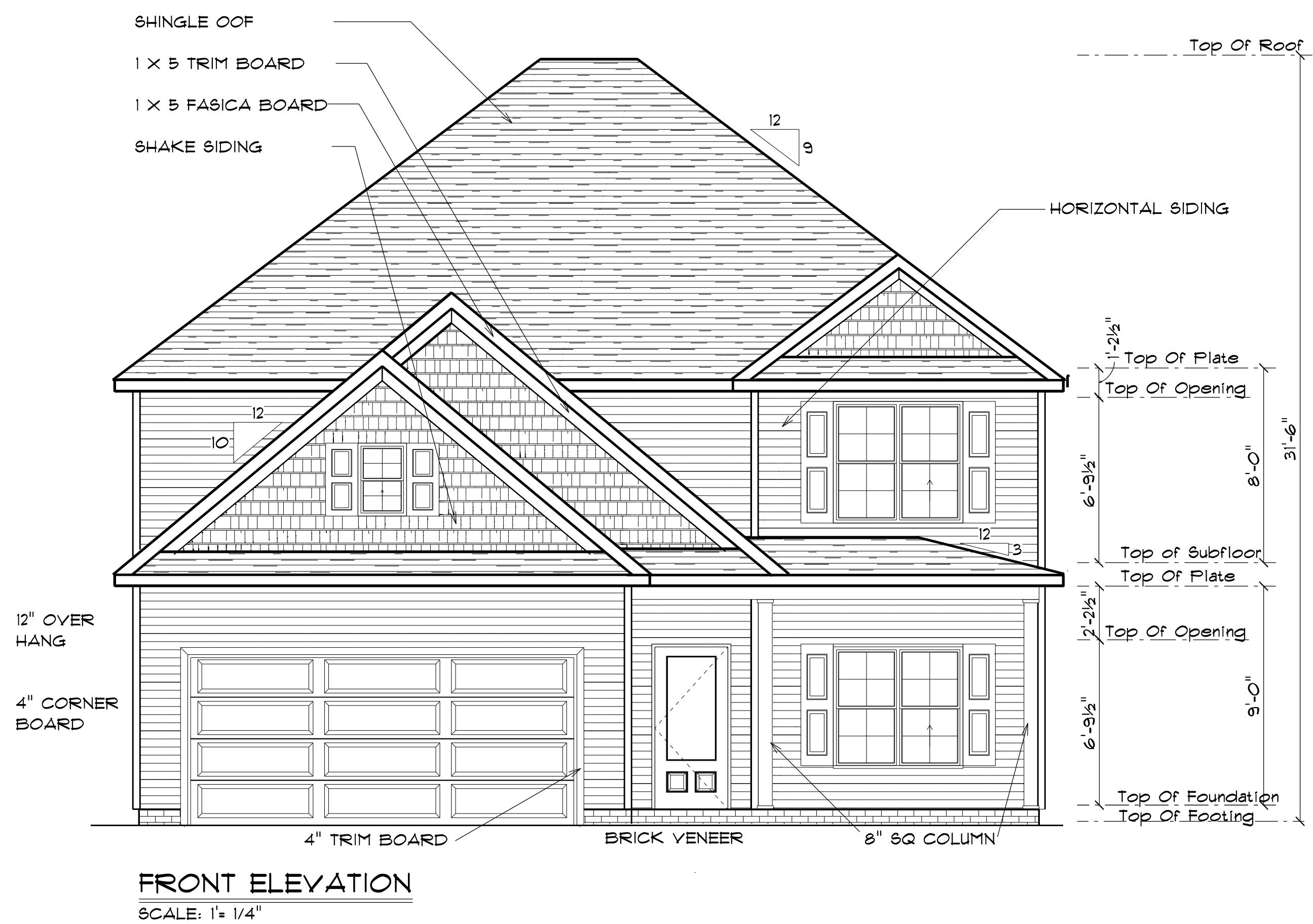
I ASSUME NO RESPONSIBILITY FOR ANY DISTANCES AFTER START OF CONSTRUCTION. CONTRACTOR/BUILDER SHALL CONSULT WITH HOME OWNER ON ALL INTERIOR AND EXTERIOR MOLDINGS, TRIMS, COLORS, FINISHES, CABINET LAYOUTS, AND MANUFACTURERS BEFORE CONSTRUCTION BEGINS. ALL BEAMS AND FRAMING MEMBERS ARE SIZED BY OTHERS.

1.1 This plan has been drawn to comply with the 2018 NC Building Code

- 1.2 Minimum Design Loads for Building and Other Structures A&CE 1-9B
- 2 Roof Dead Load 15 P&F
- 3 Roof Live Load 20 P&F
- 4 Typical Floor Dead Load 10 P&F
- 5 Floor Live Loads
 - 5.1 Rooms other than sleeping rooms 40 P&F
 - 5.2 Sleeping Rooms 30 P&F
 - 5.3 Stairs 40 P&F
 - 5.4 Decks 40 P&F
 - 5.5 Exterior Balconies 60 P&F
- 6 Wind Loads
 - 6.1 Ultimate Design Wind Speeds 15 MPH
 - 6.2 Wind Importance Factor, Iw 1.00
 - 6.3 Exposure B
 - 6.4 Walls (Component and Cladding) 25 P&F
 - 6.5 Roofs (Component and Cladding)
 - 6.5.1 Roof Slopes 2.25/12 to 1/12 34.8 P&F
 - 6.5.2 Roof Slopes 1/12 to 12/12 21 P&F

It is the sole responsibility of the Contractor and/or Builder to conform to all standards, provisions, requirements, methods of construction and uses of materials provided in buildings and/or structures as required by NC Uniform Building Code, Local Agencies and in accordance with good engineering practices. Verify all dimensions prior to construction.

CRH HOMES RETAINS TITLE AND OWNERSHIP OF ALL PLANS. THESE PLANS CAN NOT BE COPIED OR REPRODUCED. THESE PLANS CAN NOT BE BUILT BY ANYONE BUILDER OTHER THAN CRH HOMES



Diane Rives Design
 6205 Mockingbird Lane
 Sanford, N.C. 27332
 919-710-0953
 grh@harnettcounty.net

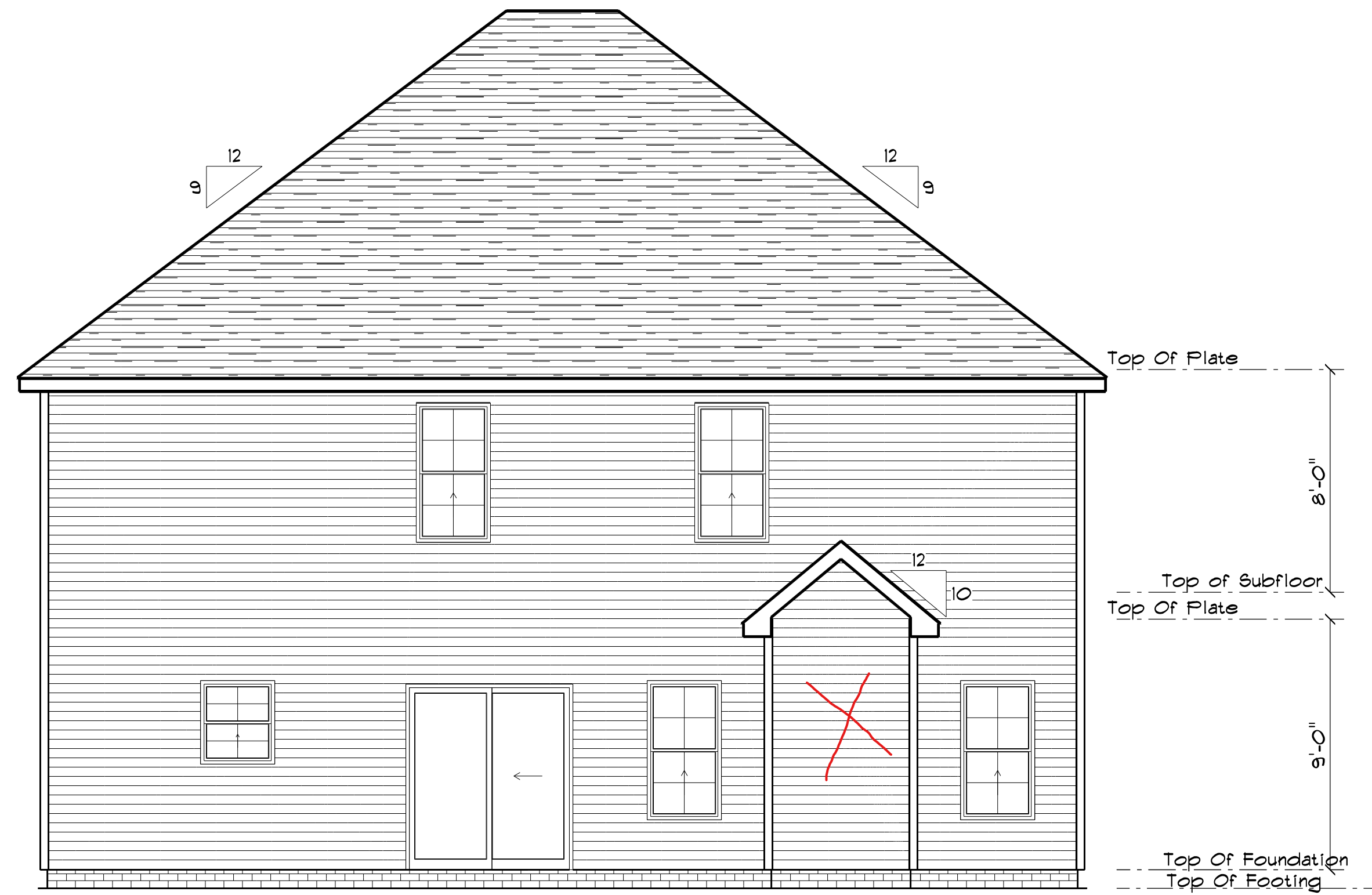
SCALE: 1" = 1/4"
 DRAWN BY:
 DATE: 9/11/2023

CRH HOMES

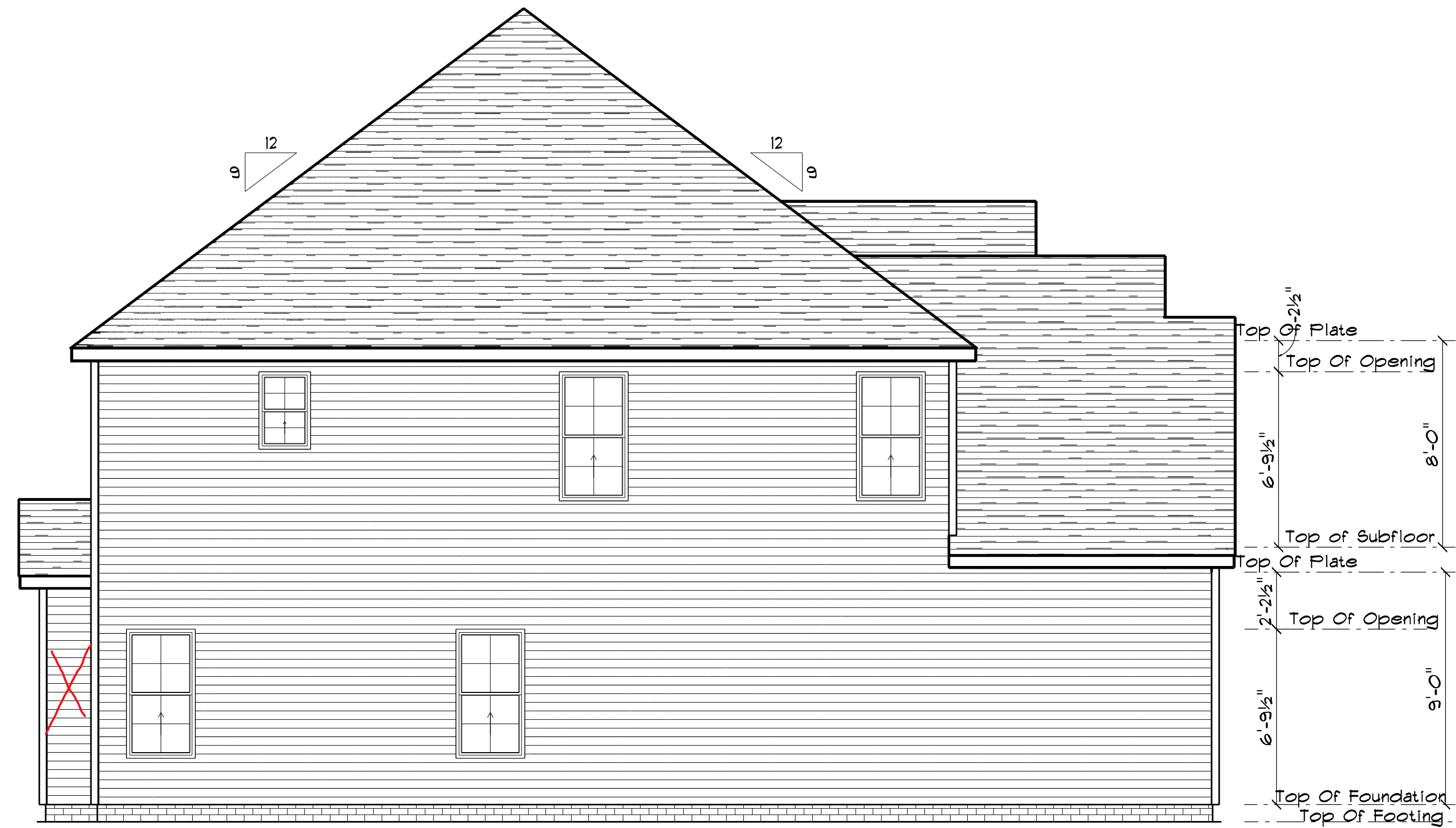
THE OAKTON II
 LEFT GARAGE

FRONT ELEVATION C

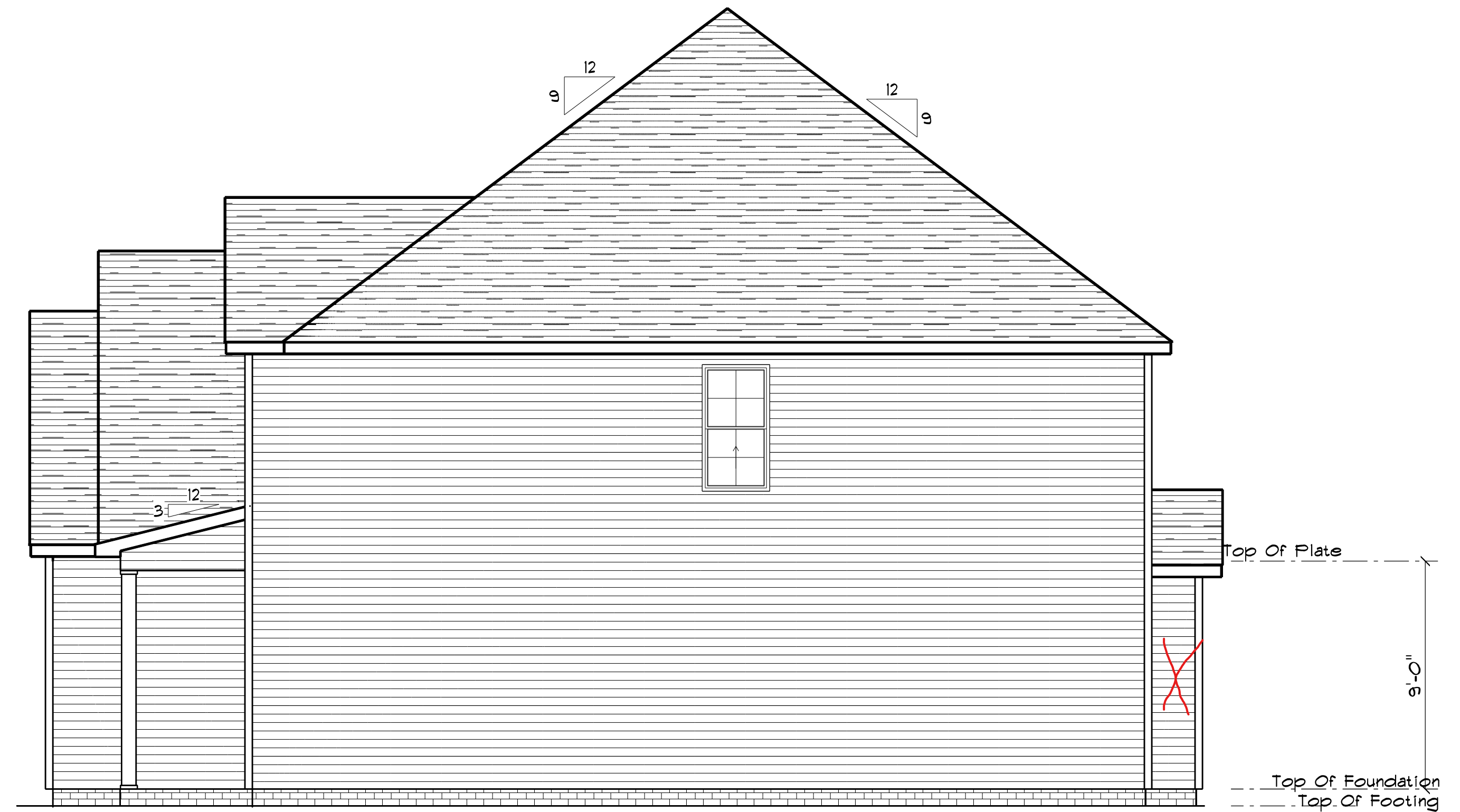
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REAR ELEVATION
SCALE: 1" = 1/4"



LEFT ELEVATION
SCALE: 1" = 1/4"



RIGHT ELEVATION
SCALE: 1" = 1/4"

2A

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6205 Mockingbird Lane
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gotuonarchitect.net

DRD

SCALE: 1" = 1/4"
DRAWN BY:
DATE: 9/11/2023

CRH HOMES

THE OAKTON II
LEFT GARAGE

ELEVATIONS

FOUNDATION NOTES:
 ALL FOOTINGS SHALL BEAR ON ORIGINAL UNDISTURBED SOIL.
 THE 28 DAY COMPRESSIVE STRENGTH OF ALL FOOTINGS IS 3000 PSI

PROVIDE WATER PROOFING AND PERIMETER DRAINS AS REQUIRED.

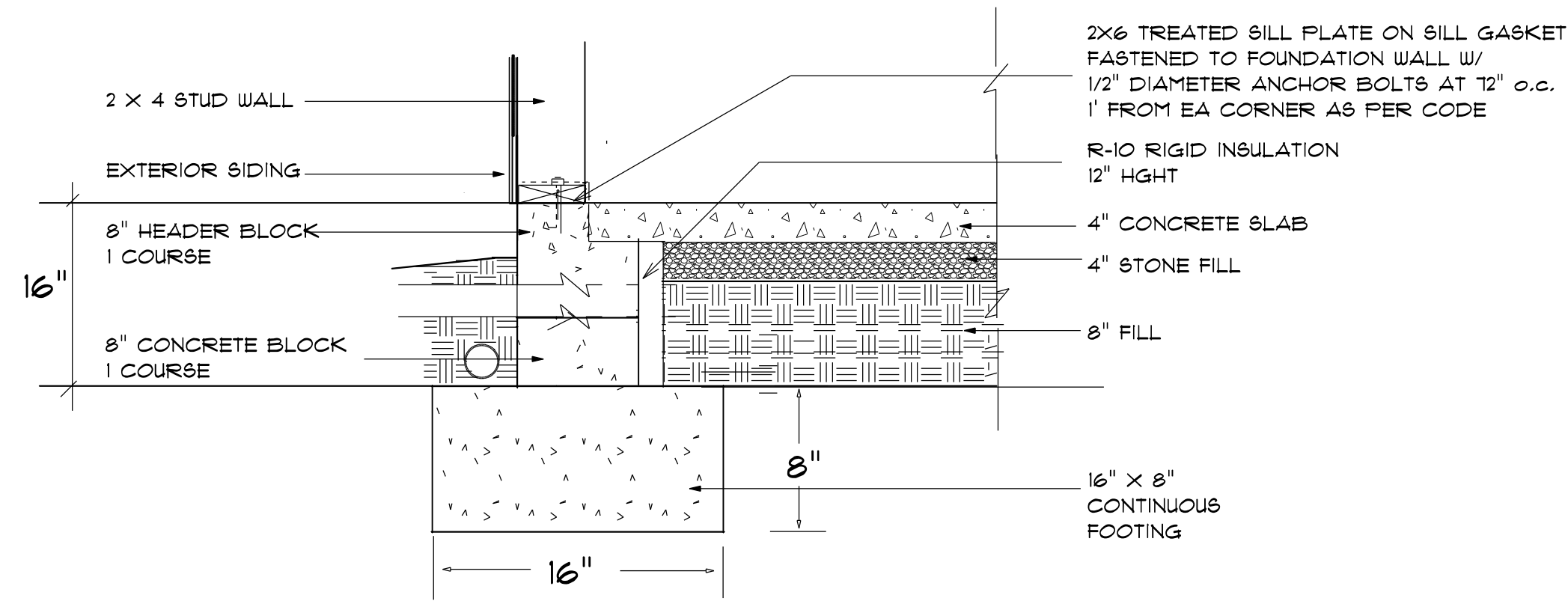
FOUNDATION CONCRETE MIX TO HAVE 1-1/2" MAX AGGREGATE SIZE. CONCRETE FILL MIX TO HAVE 1/2" MAX AGGREGATE SIZE.

FOOTING WIDTHS ARE BASED ON A LOAD-BEARING SOIL CAPACITY OF 2000 PSI.

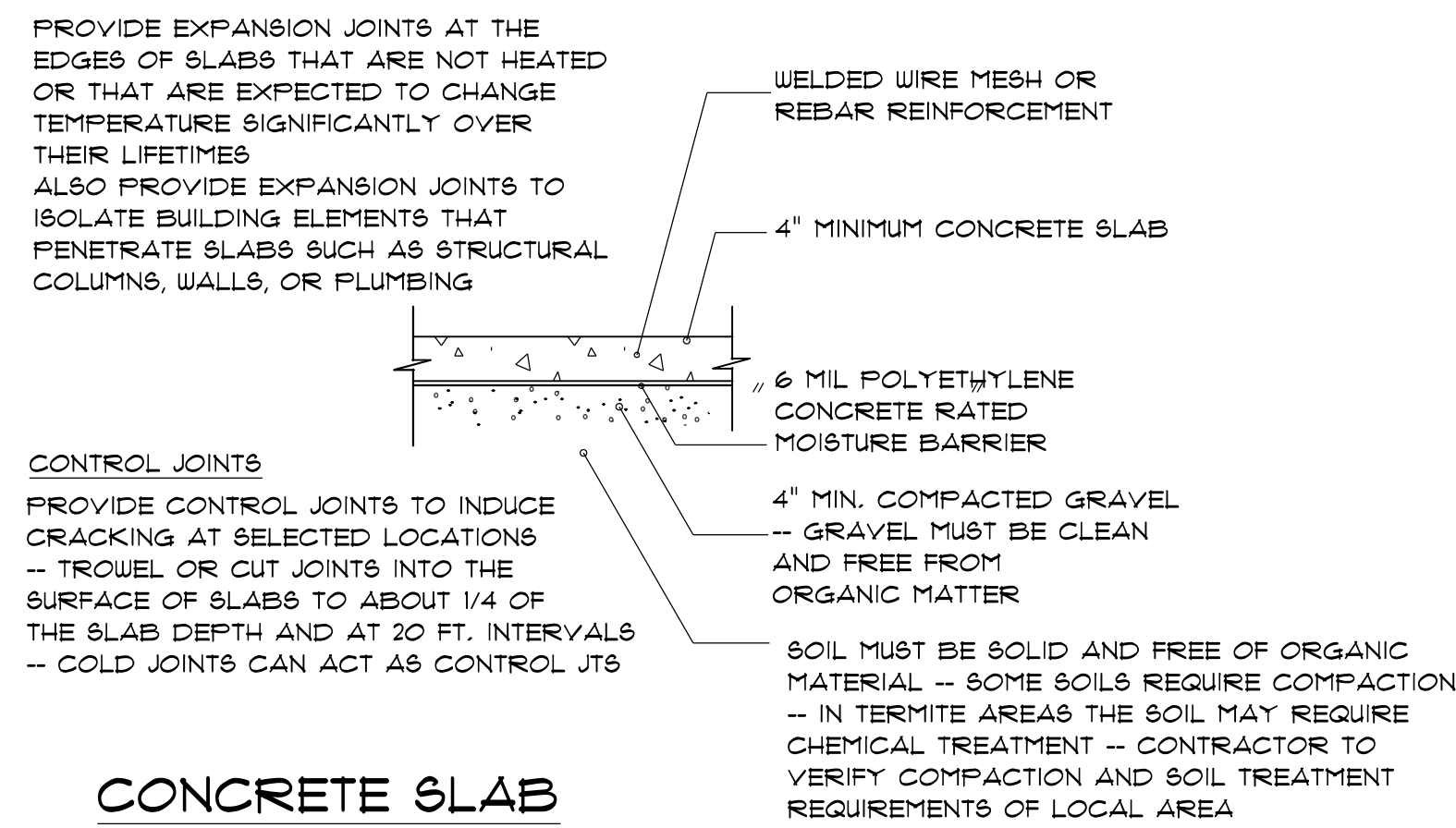
PROVIDE 6 MIL POLY VAPOR BARRIER TO COVER GROUND SURFACE IN CRAWL SPACE

ALL ANCHOR BOLTS TO BE 12" LONG, 1/2" DIA. A36 UNO ANCHOR BOLTS SHALL BE SPACED AT A MAX OF 6' OC AND NO MORE THAN 1' FROM EA CORNER.

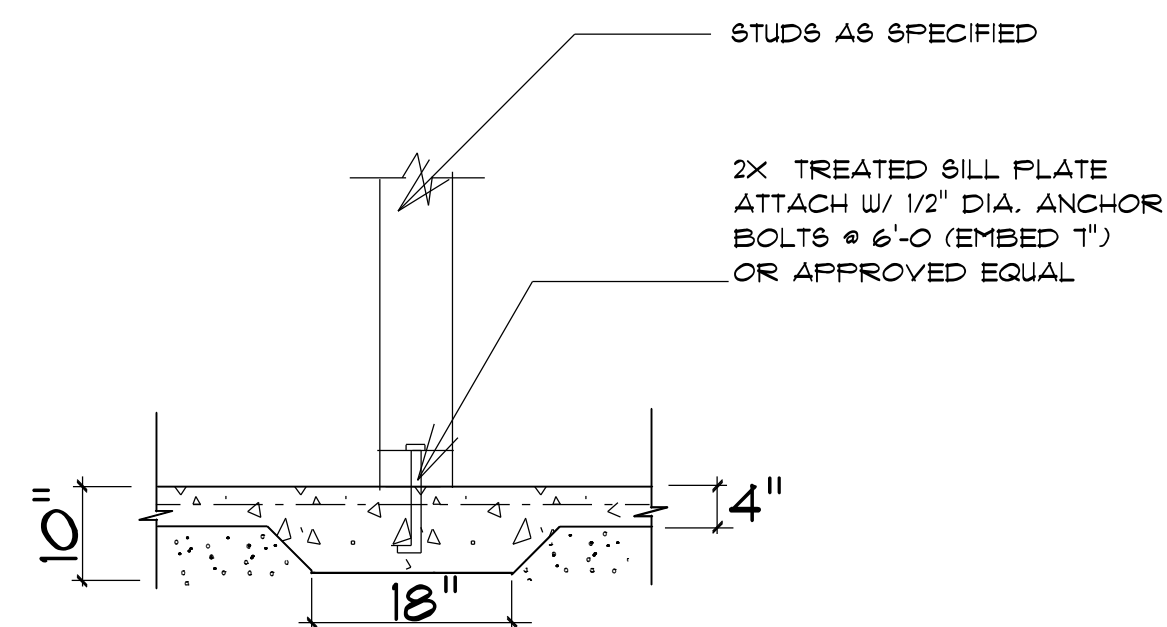
Termite Soil Treatment: Treat entire slab area soil or crawl space surface before vapor barrier is installed and slab is poured with a state approved termiticide. Termiticide should be applied by a licensed and certified pest control professional by the state of North Carolina.



STEM WALL FOUNDATION Detail
 NOT TO SCALE

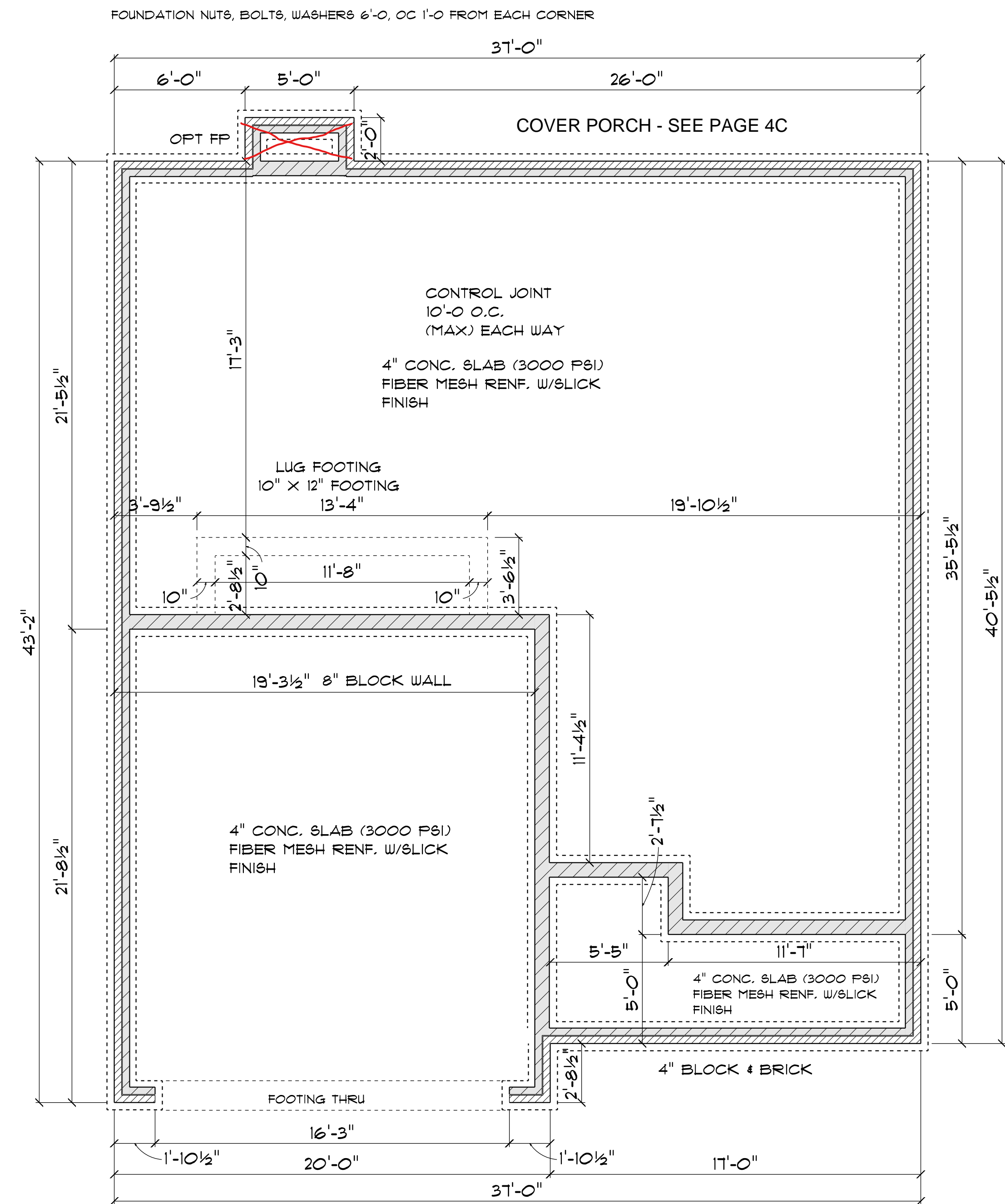


CONCRETE SLAB DETAILS / NOTES
 NOT TO SCALE

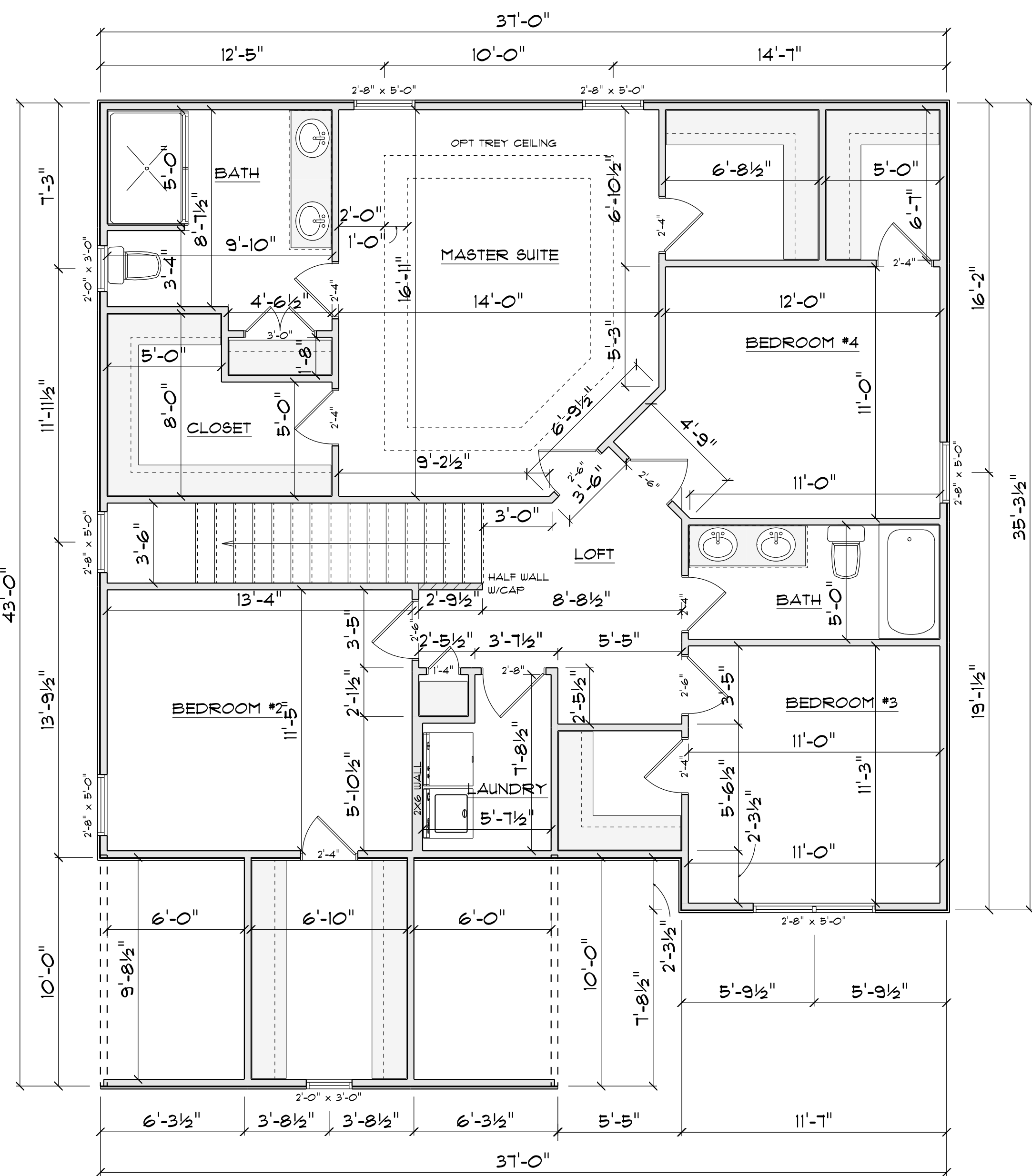


TYPICAL THICKENED SLAB
 NOT TO SCALE

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FOUNDATION PLAN
 SCALE: 1" = 1/4"



2ND FLOOR PLAN

SCALE: 1" = 1/4"

AREA SCHEDULE	
NAME	AREA
Heated	1283 sq ft.

GENERAL FRAMING NOTES:

ALL LUMBER IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED.

FRAMING LUMBER SHALL BE SYP #2 GRADE AND/OR SPRUCE PINE FIR #1 AND/OR #2, KILN DRIED.

WHERE PRE-ENGINEERED JOISTS ARE USED, JOIST MANUFACTURER SHALL PROVIDE SHOP DRAWINGS, WHICH BEAR SEAL OF A N.C. ENGINEER.

STUDS AND JOISTS SHALL NOT BE CUT TO INSTALL PLUMBING OR WIRING WITHOUT ADDING METAL OR WOOD SIDE PANELS TO STRENGTHEN THE MEMBER TO ITS ORIGINAL CAPACITY.

NAIL MULTIPLE MEMBERS WITH 2 ROWS OF 16d NAILS STAGGERED 32" ON AN USE 3-16d NAILS 2" IN AT EACH END. DOUBLE ALL STUDS UNDER ROOF POST DOWNS UNO.

NAIL FLOOR JOISTS TO SILL PLATE WITH 8d TOE NAILS.

ALL EXPOSED FRAMING ON PORCHES AND DECKS SHALL BE PRESSURE TREATED.

PROVIDE WATERPROOFING AND DRAINS AS REQUIRED.

ALL FRAMING TO BE 16" OC UNO. WALL FRAMING DIMENSIONS ARE BASED ON 2 X 4 STUDS UNO, DOUBLE STUDS UNDER ALL HEADERS.

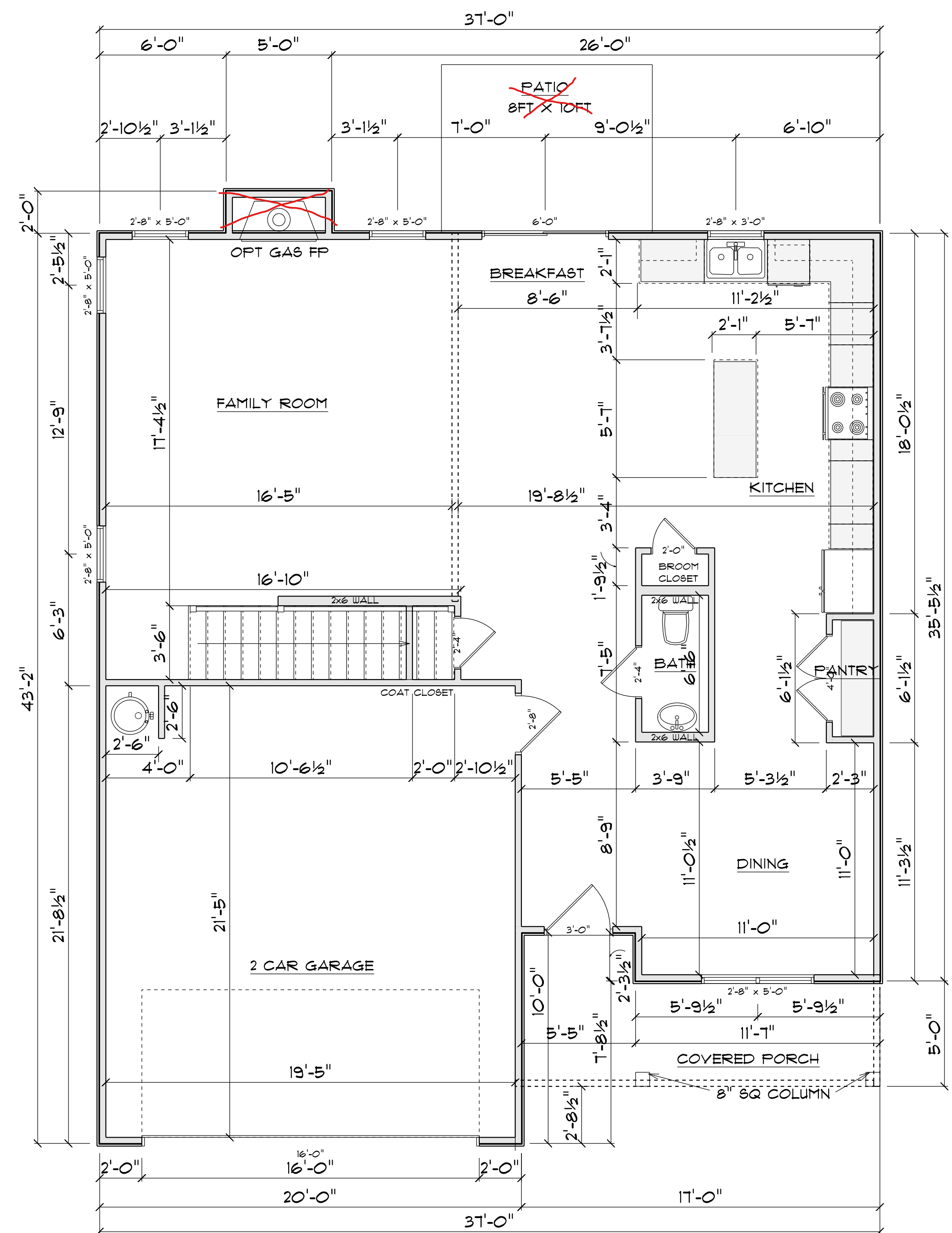
LVL'S AND TJI'S TO BE SIZED BY OTHERS

EXTERIOR WALLS IN LIVING AREAS ARE 2 X 4

WINDOW SCHEDULE				
SIZE	COUNT	LIBRARY NAME	R.O. WIDTH	R.O. HEIGHT
2'-0" x 3'-0"	2	Window\Single Hung	24"	36"
2'-8" x 3'-0"	1	Window\Single Hung	32"	36"
2'-8" x 5'-0"	9	Window\Single Hung	32"	60-1/2"
2'-8" x 5'-0"	2	Window\Single Hung	64"	60-1/2"

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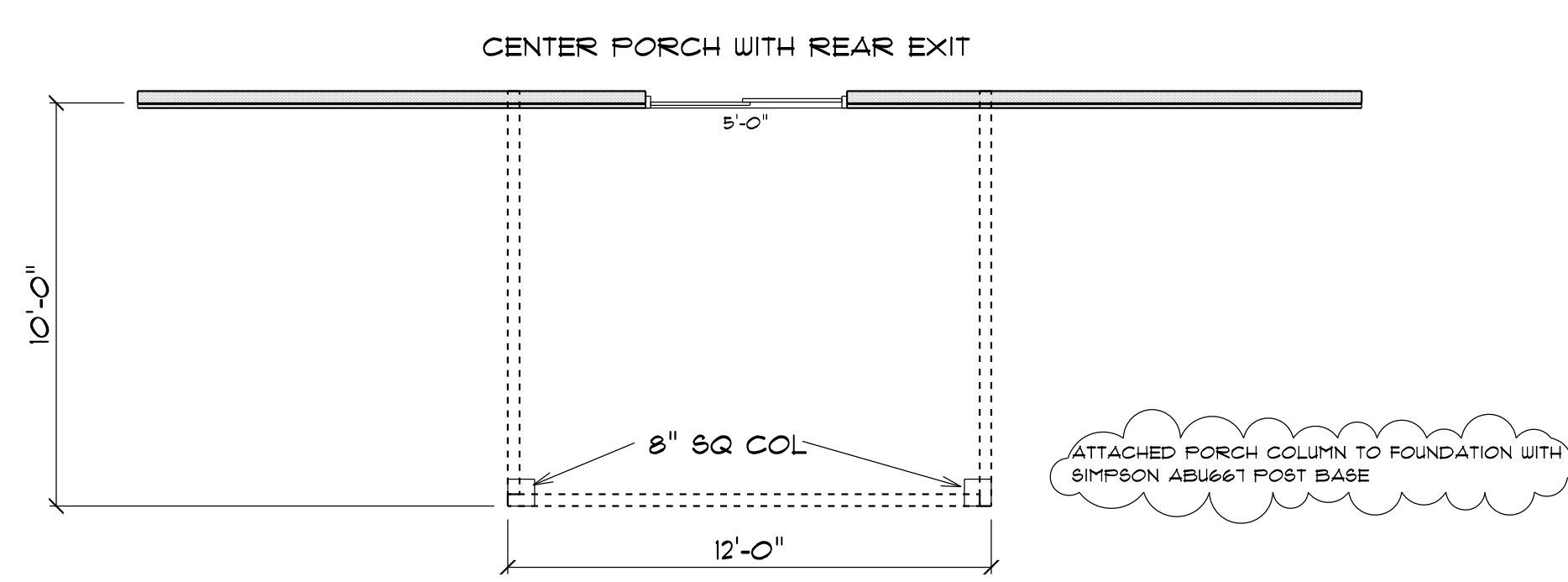
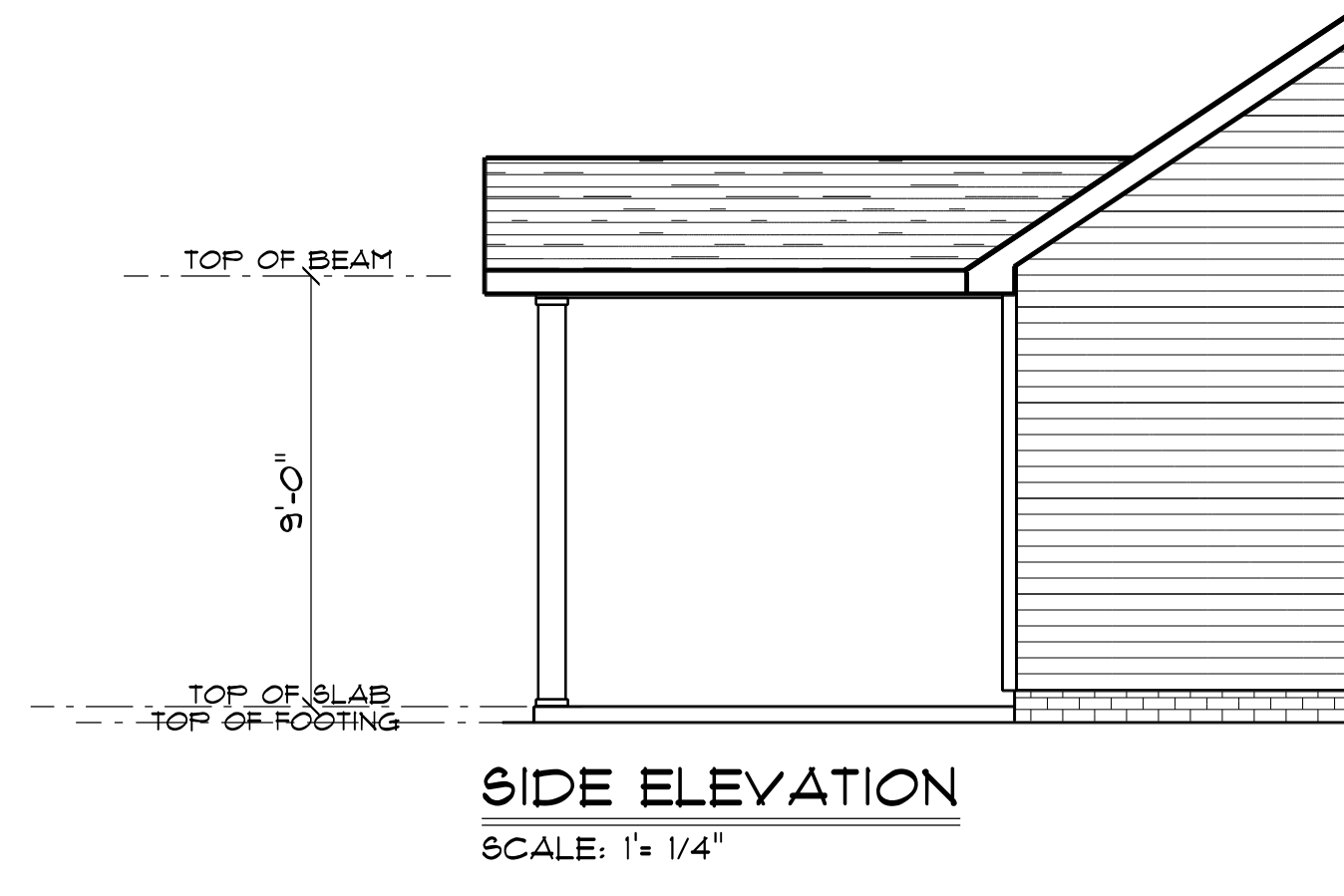
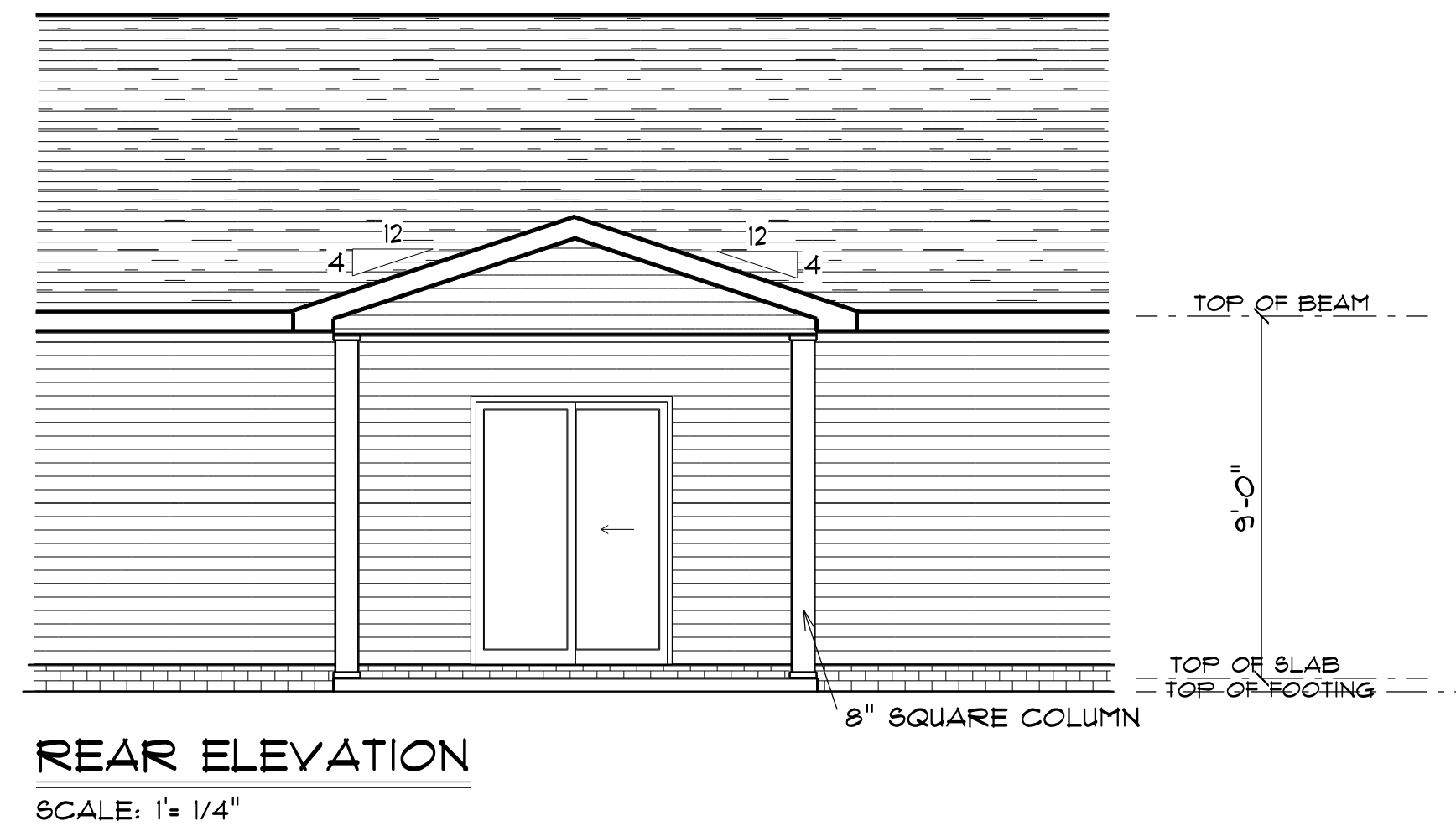
COVER PORCH SEE PAGE 4C



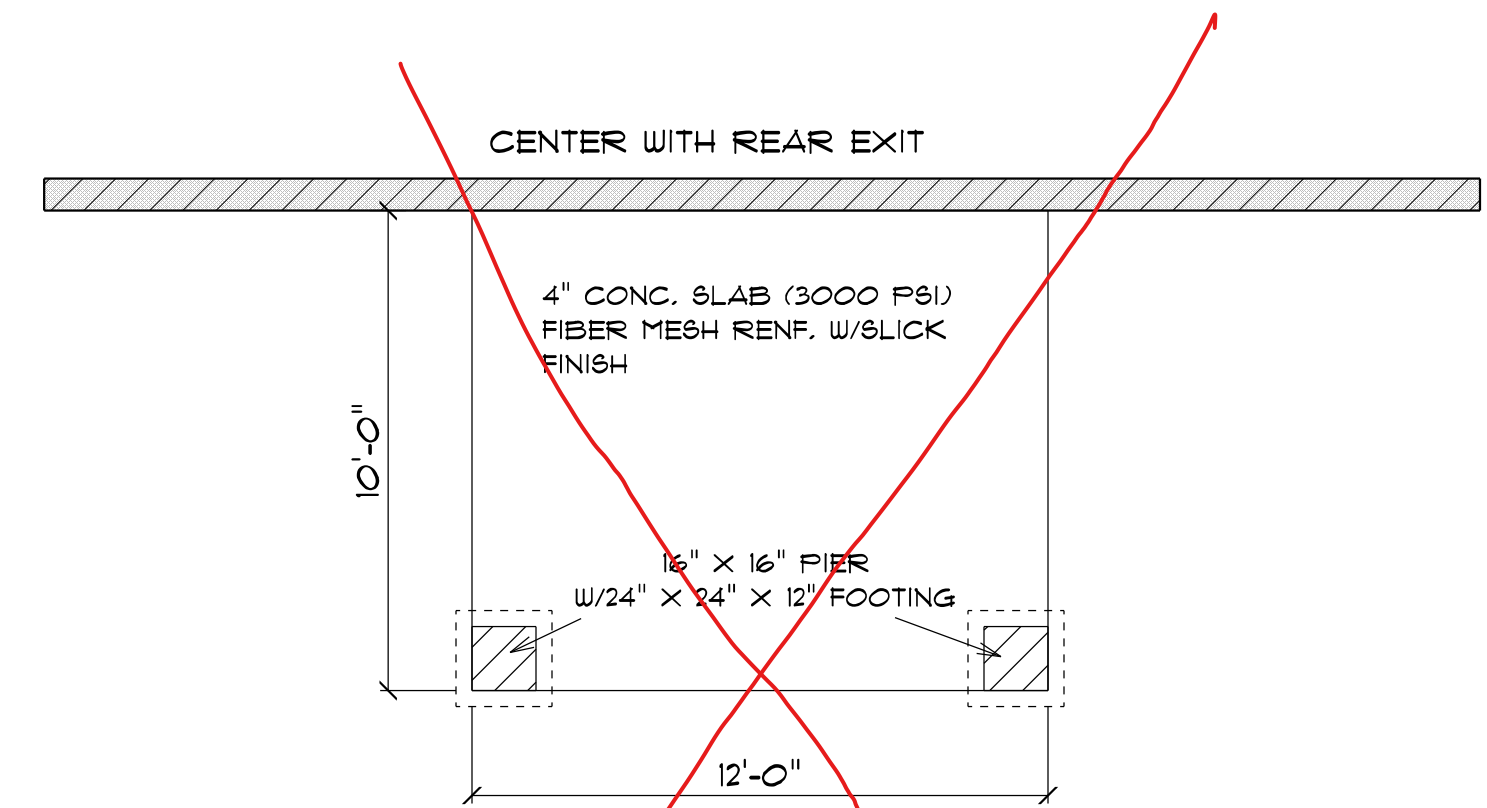
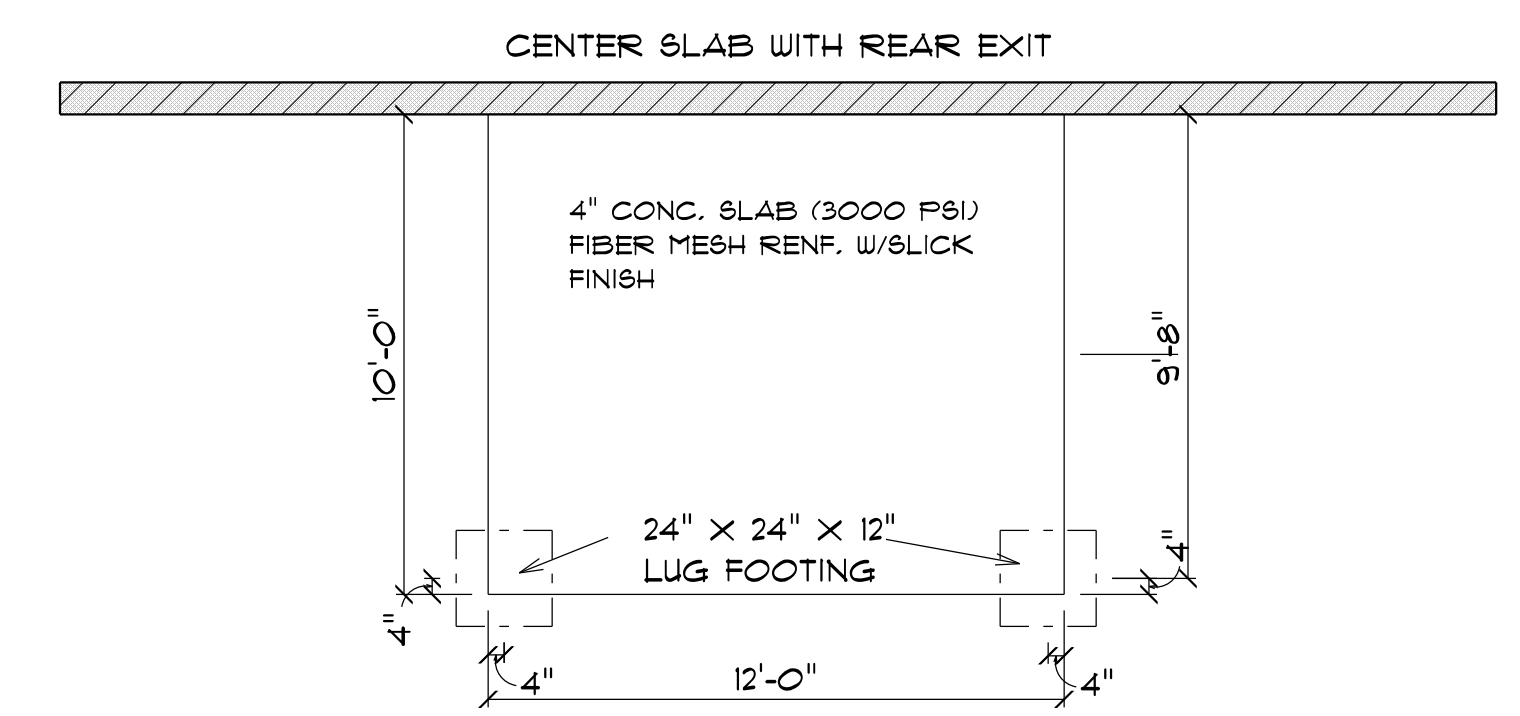
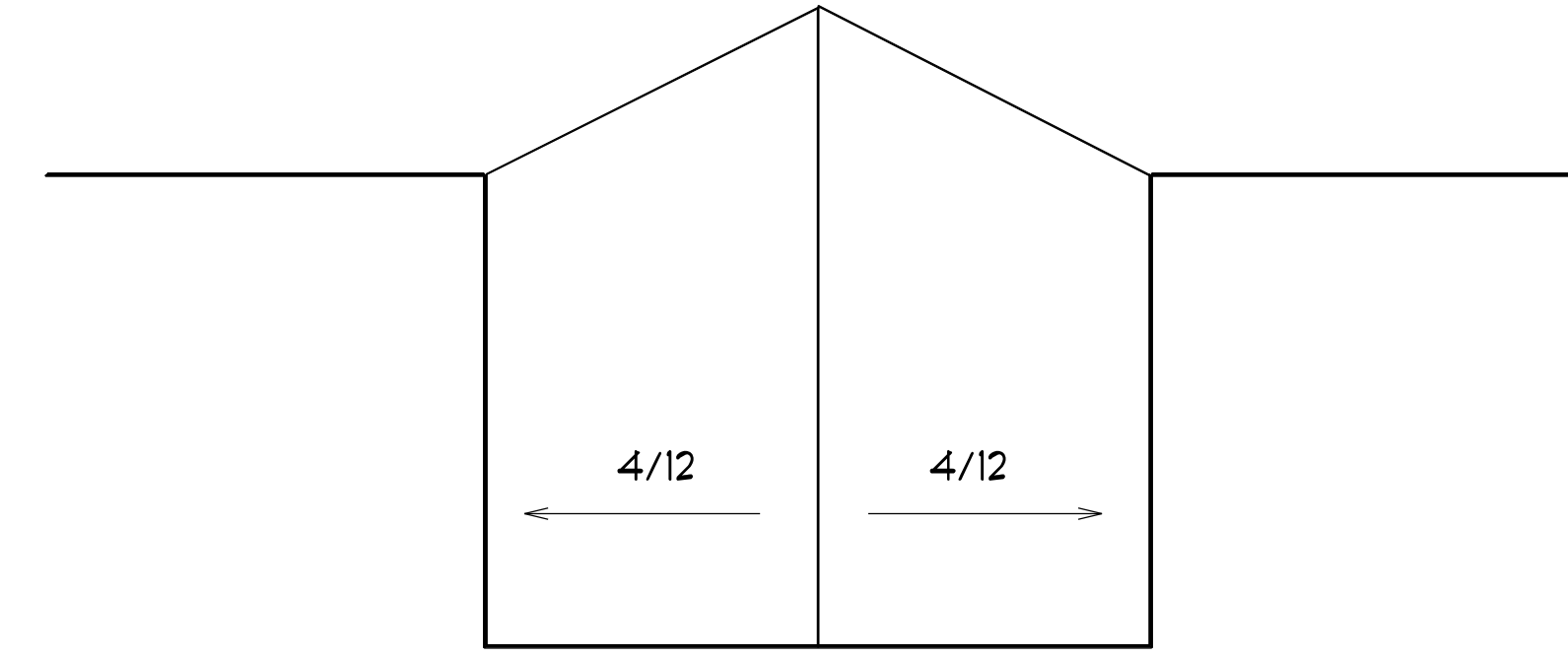
1ST FLOOR PLAN

SCALE: 1" = 1/4"

AREA SCHEDULE	
NAME	AREA
Heated	1023 sq ft.
Garage	440 sq ft.
Covered Front Porch	91 sq ft.



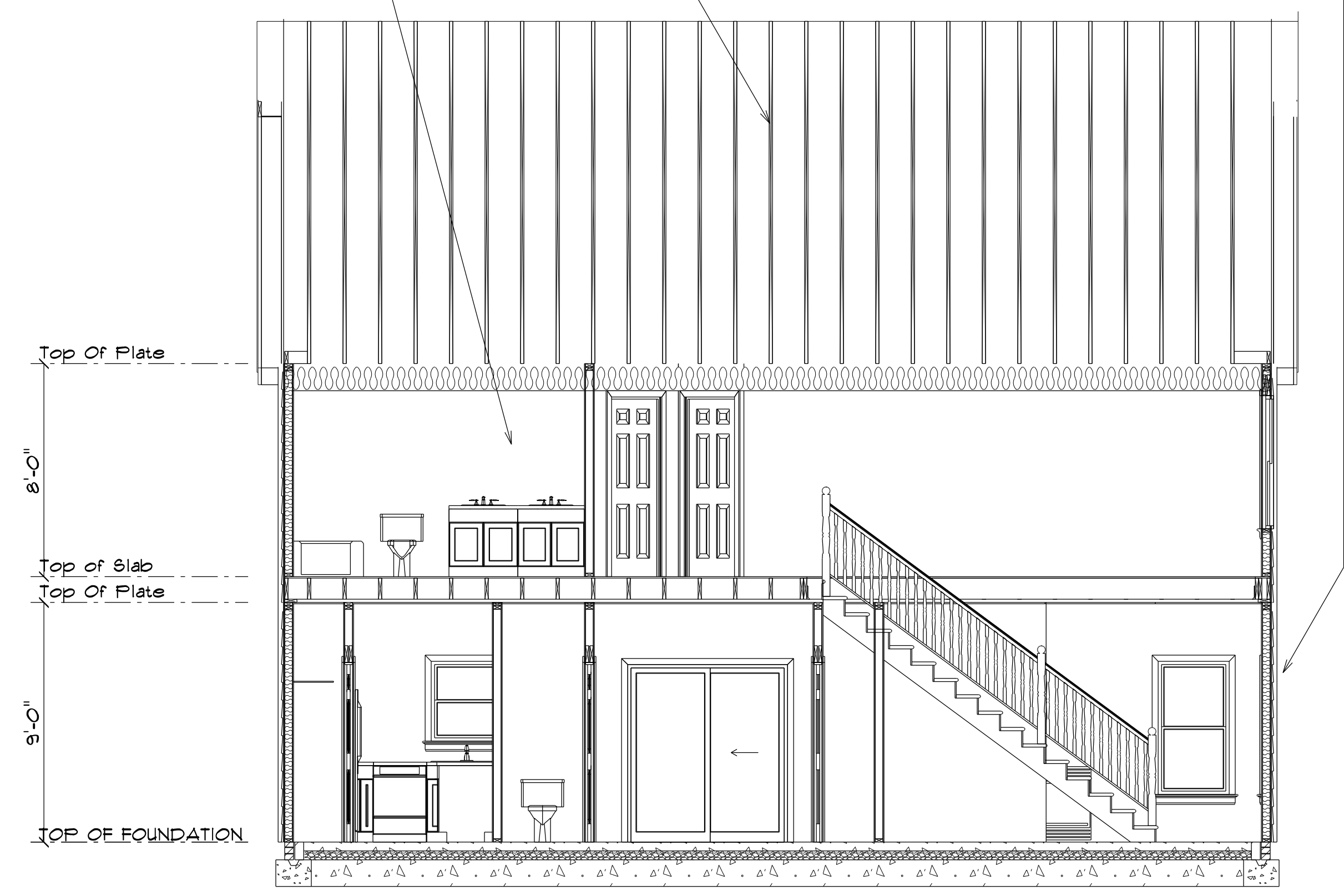
AREA SCHEDULE	
NAME	AREA
Covered Porch	120 sq. ft.



TYPICAL 2x4 WALL:
 1/2" DRYWALL
 TAPED & SANDED
 2x4 STUDS @ 16" o.c.
 1/2" DRYWALL
 TAPED & SANDED

TYPICAL TRUSS ROOF:
 SHINGLES
 7/16" ROOFING PLYWOOD c/w
 'H' CLIPS
 BLOCK & BRACE PER TRUSS MGR.
 PRE-ENGINEERED TRUSSES @ 24" o.c.
 2x4 TRUSS BRACING
 R38 BLOWN INSULATION
 1/2" CEILING BOARD
 TAPED & SANDED

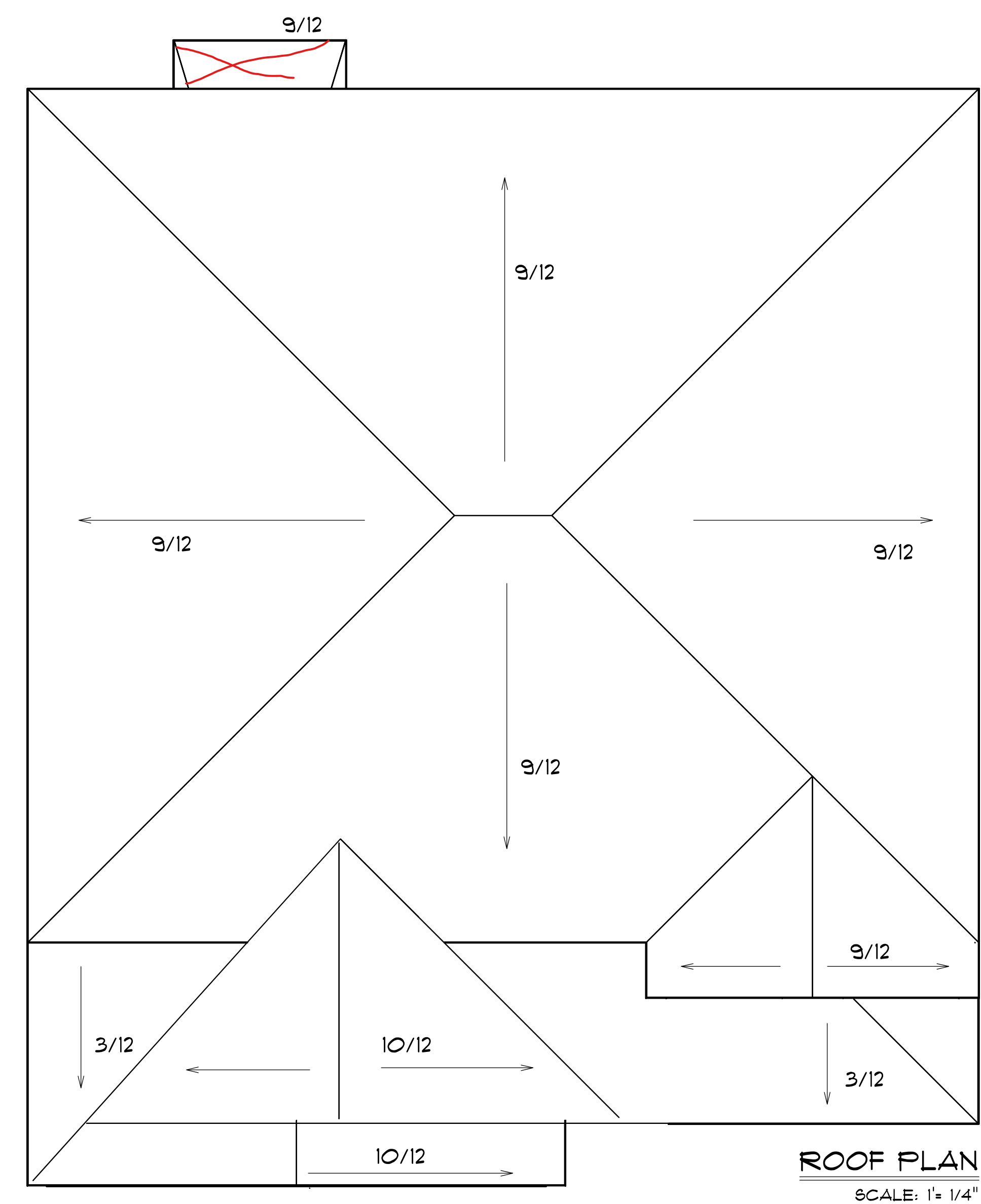
TYPICAL 2x4 SIDING EXTERIOR WALL:
 HORIZONTAL SIDING
 7/16" PLYWOOD SHEATHING
 2x4 STUDS @ 16" o.c.
 R15 BATT INSULATION
 1/2" DRYWALL
 TAPED & SANDED



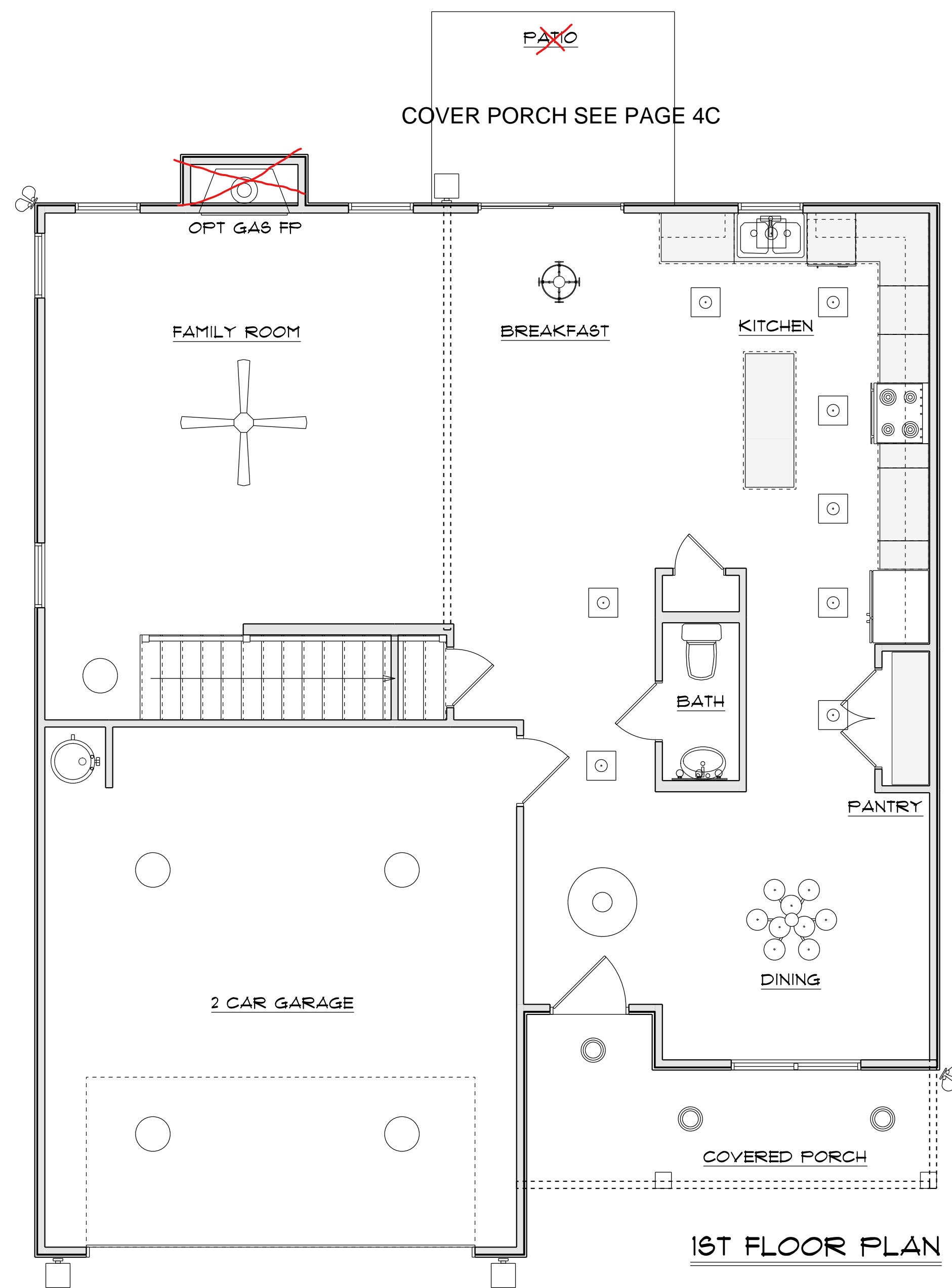
SECTION THROUGH
 SCALE: 1" = 1/4"

ROOF NOTES:
 TRUSSES, BRACINGS, BRIDGING AND CONNECTORS ARE TO BE DESIGNED BY THE TRUSS MANUFACTURER.
 IDENTIFY LUMBER BY OFFICIAL GRADE MARKINGS.
 DO NOT CUT OR REMOVE CHORDS OR OTHER TRUSS MEMBERS.
 DO NOT NOTCH OR DRILL TRUSS MEMBERS.
 WHERE PRE-ENGINEERED ROOF TRUSSES ARE USED, TRUSS MANUFACTURER SHALL PROVIDE SHOP DRAWINGS, WHICH BEAR SEAL OF A N. C. REGISTERED ENGINEER.

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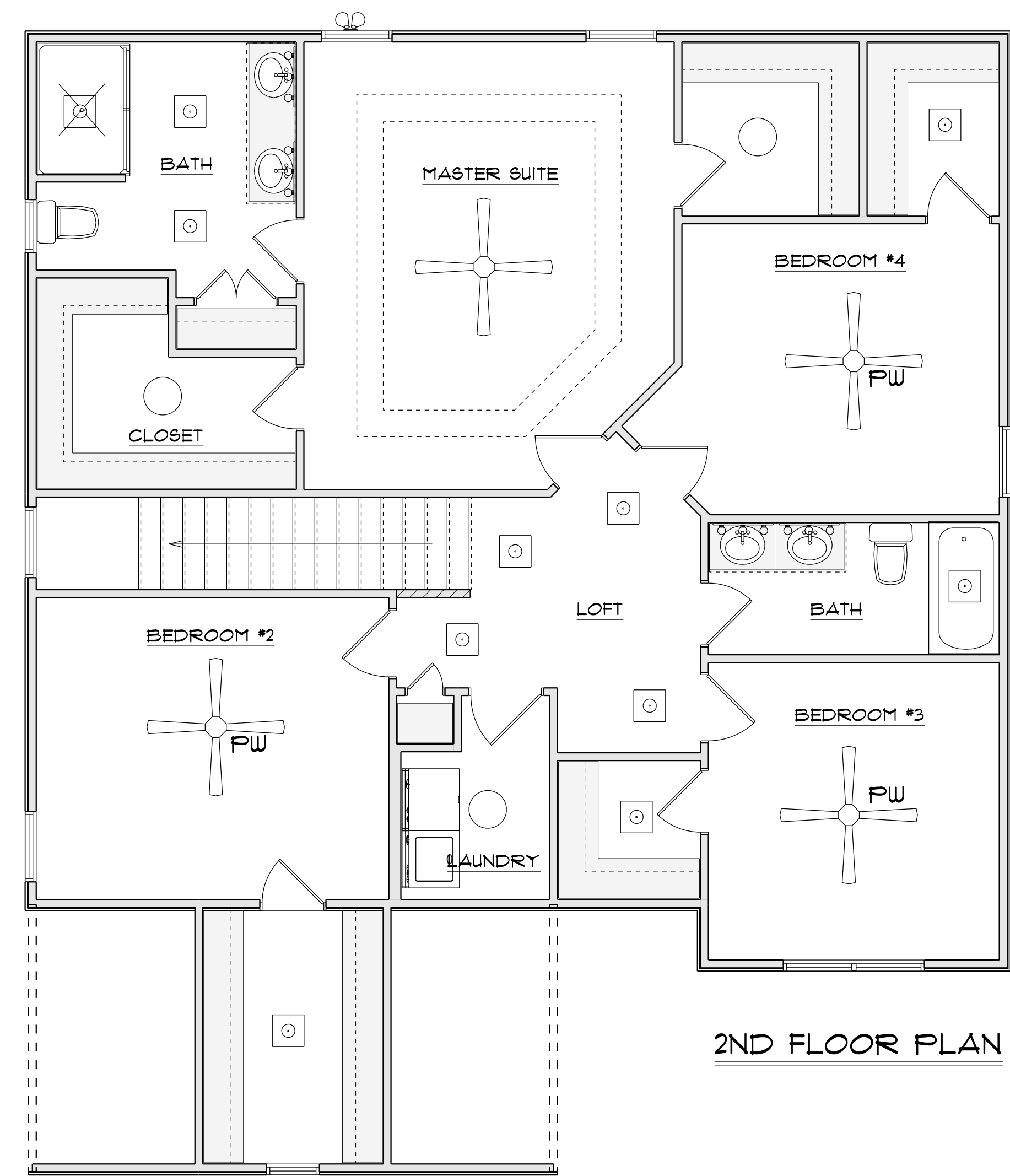


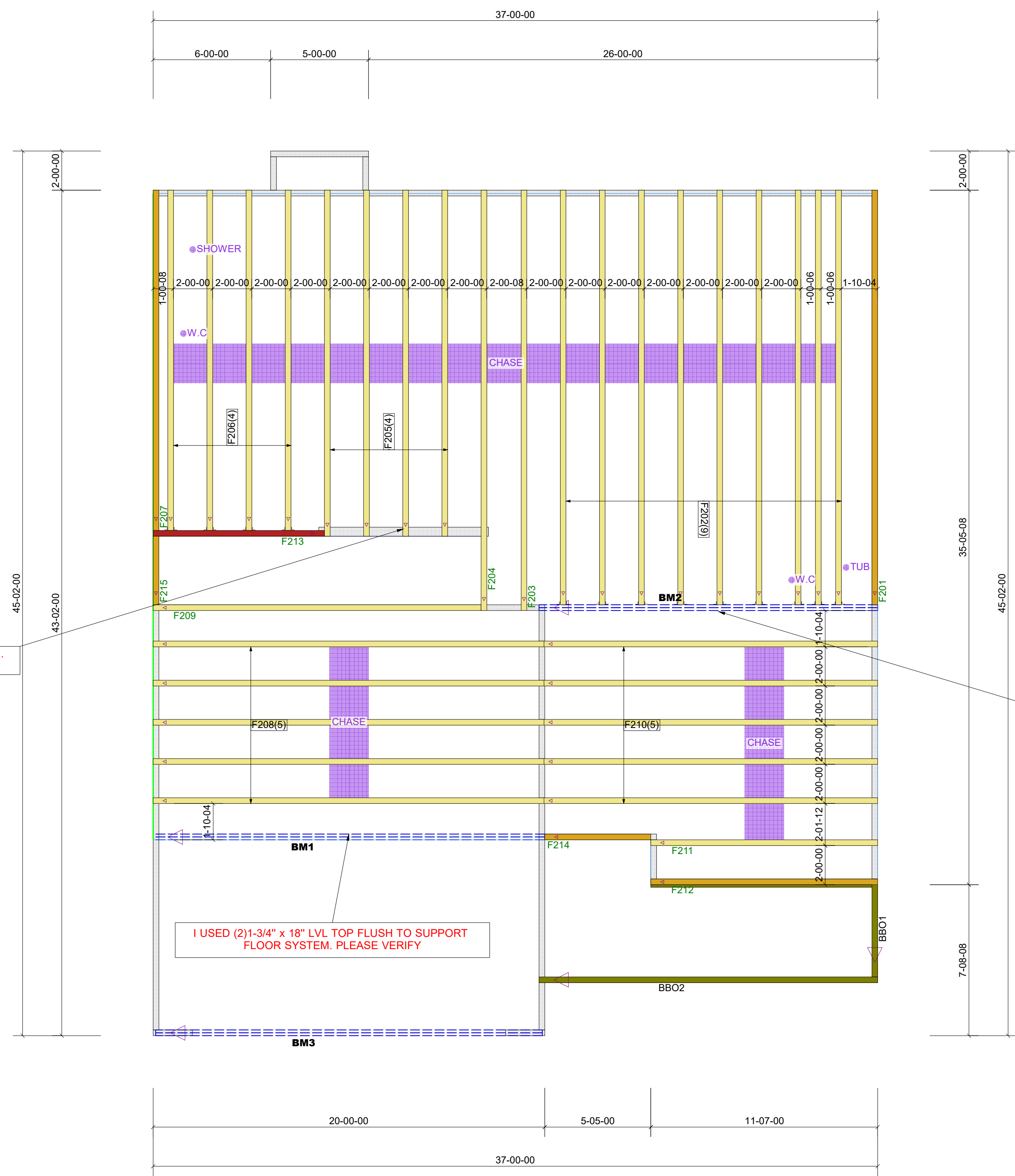
ROOF PLAN
 SCALE: 1" = 1/4"



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ELECTRICAL LEGEND		
ELECTRICAL	COUNT	SYMBOL
ceiling fan	2	
10" led	8	
7" led	21	
foyer light	1	
dinning room light	1	
coach light	3	
exterior over head	3	
flood light	3	
vanity bar light	5	
wall sconce		
nook light	1	





ProdID	Length	Product	Plates	Net Qty	File Type
BM3	20'-00"-00"	2.0 RigidLam DF LVL 1-3/4 x 11-7/8	2	2	FF
BM2	18'-00"-00"	2.0 RigidLam DF LVL 1-3/4 x 14	2	2	FF
BM1	20'-00"-00"	2.0 RigidLam DF LVL 1-3/4 x 18	2	2	FF

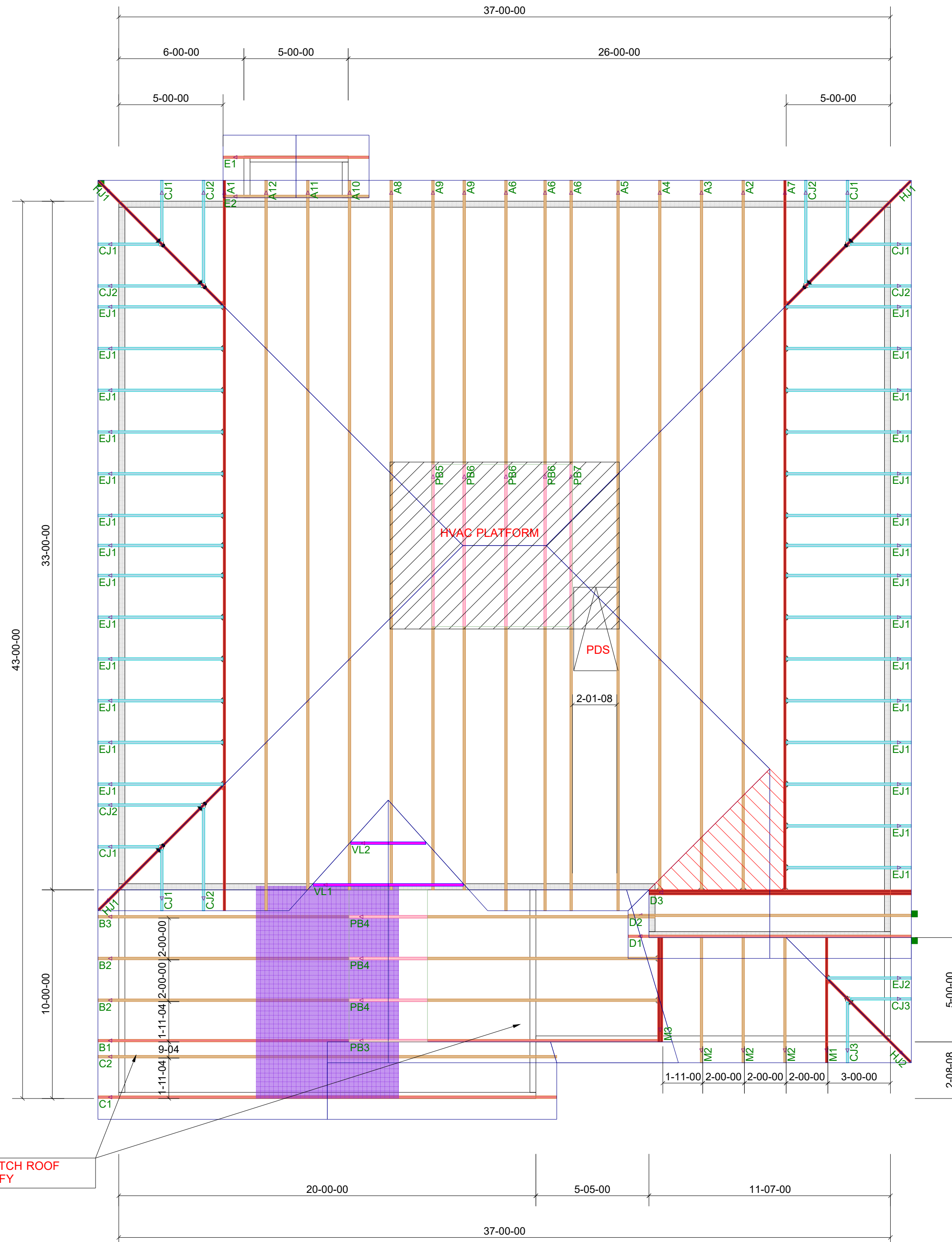
Truss Connector Total List		
Manuf	Product	Qty
Simpson	TH4K22	13

CRH
Oakton II - Elev B
2nd FLOOR TRUSS PLACEMENT PLAN

REVISIONS	
DATE	BY
9-14-23	TH

FLOOR TRUSS FRAMING
DRAWING SCALE : NTS

PROJECT NUMBER 23090033
SHEET NUMBER 1 / 2



I USED 2X6 HEEL HEIGHT & 12" OVERHANG FOR THIS JOB. PLEASE VERIFY

Truss Connector Total List		
Manuf	Product	Qty
Simpson	HTU26	6
Simpson	One H2.5A	19

I USED 2-03-10 HEEL HEIGHT FOR 10/12 PITCH ROOF FOR LOWER ROOF. PLEASE VERIFY

ROOF TRUSS FRAMING

DRAWING SCALE : NTS



CRH
Oakton II - Elev B
ROOF TRUSS PLACEMENT PLAN

REVISIONS	
DATE	BY
9-14-23	TH

PROJECT NUMBER
23090033
SHEET NUMBER
2 / 2



Customer:
Job Name:
City:
Customer Ph...

Job Name: **02**
Level: **2nd floor**
Label: **BM1 - i186**
Type: **Beam**

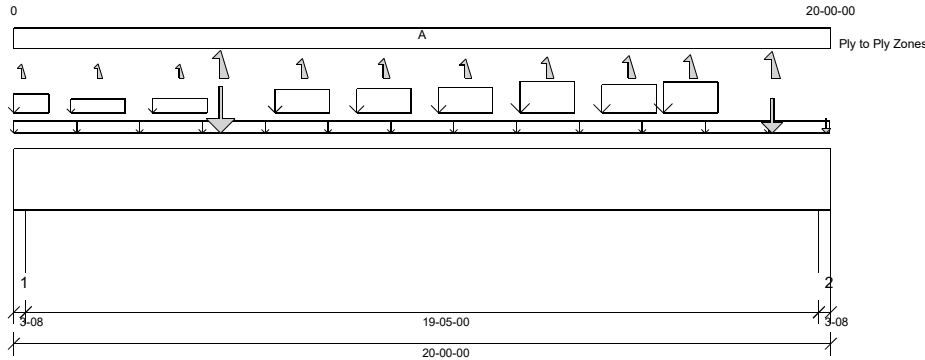
2 Ply Member
2.0 RigidLam DF LVL 1-3/4
x 18

Status:
Design Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.7.3.303.Update9.26

Report Version: 2021.03.26 06/12/2024 12:23



DESIGN INFORMATION

Building Code: IRC 2018
Design Methodology: ASD
Risk Category: II (General Construction) Residential
Service Condition: Dry
LL Deflection Limit: L/360, 0.75" (absolute)
TL Deflection Limit: L/240, 1.00" (absolute)

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 19'- 5"

Bearing Stress of Support Material:

- 875 psi Wall @ 0'- 2 1/2"
- 875 psi Wall @ 19'- 9 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Max Pos. Moment:	10'- 7 15/16"	D + Lr	1.15	29546 lb ft	53375 lb ft	Passed - 55%
Max Neg. Moment:	11'- 3/4"	0.6D + 0.6W	1.60	3107 lb ft	30374 lb ft	Passed - 10%
Max Shear:	18'- 2 1/2"	D + Lr	1.15	6212 lb	14007 lb	Passed - 44%
Live Load (LL) Neg. Defl.:	10'- 2 1/16"	0.6W		0.266"	L/360	Passed - L/876
Total Load (TL) Pos. Defl.:	10'- 15/16"	D + 0.75(L + Lr + 0.6W)		0.610"	L/240	Passed - L/381

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Downward Reaction	Uplift Reaction	Resistance of Member	Resistance of Support	Result
1	3-08	D + Lr	1.15	5232 lb		9187 lb	10719 lb	Passed - 57%
1	3-08	0.6D + 0.6W	1.60		-386 lb	-	-	
2	3-08	D + Lr	1.15	6853 lb		9187 lb	10719 lb	Passed - 75%
2	3-08	0.6D + 0.6W	1.60		-651 lb	-	-	

LOADING

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
Self Weight	0'	20'	Self Weight	Top	17 lb/ft	-	-	-	-
Uniform	0'	19'- 11 1/2"	FC1 Floor Decking (Plan View Fill)	Top	10 lb/ft	40 lb/ft	-	-	-
Uniform	-0'	0'- 10 1/4"	W17(i148)	Top	204 lb/ft	-	65 lb/ft	188 lb/ft	57 lb/ft
Uniform	1'- 4 3/4"	2'- 8 3/4"	W17(i148)	Top	-	-	25 lb/ft	65 lb/ft	30 lb/ft
Uniform	3'- 4 3/4"	4'- 8 3/4"	W17(i148)	Top	62 lb/ft	-	26 lb/ft	68 lb/ft	32 lb/ft
Uniform	6'- 4 3/4"	7'- 8 3/4"	W17(i148)	Top	372 lb/ft	-	97 lb/ft	253 lb/ft	109 lb/ft
Uniform	8'- 4 3/4"	9'- 8 3/4"	W17(i148)	Top	386 lb/ft	-	107 lb/ft	280 lb/ft	115 lb/ft
Uniform	10'- 4 3/4"	11'- 8 3/4"	W17(i148)	Top	404 lb/ft	-	119 lb/ft	317 lb/ft	107 lb/ft
Uniform	12'- 4 3/4"	13'- 8 3/4"	W17(i148)	Top	572 lb/ft	-	184 lb/ft	502 lb/ft	165 lb/ft
Uniform	14'- 4 3/4"	15'- 8 3/4"	W17(i148)	Top	509 lb/ft	-	143 lb/ft	404 lb/ft	126 lb/ft
Uniform	15'- 10 3/4"	17'- 2 3/4"	W17(i148)	Top	536 lb/ft	-	166 lb/ft	521 lb/ft	147 lb/ft
Point	0'- 2 1/4"	0'- 2 1/4"	W17(i148)	Top	-	-	-	-16 lb	-202 lb
Point	2'- 3/4"	2'- 3/4"	W17(i148)	Top	-	-	-	-	-230 lb
Point	4'- 3/4"	4'- 3/4"	W17(i148)	Top	-	-	-	-	-210 lb
Point	5'- 3/4"	5'- 3/4"	W17(i148)	Top	1299 lb	-	396 lb	1080/-42 lb	362/-1393 lb
Point	7'- 3/4"	7'- 3/4"	W17(i148)	Top	-	-	-	-	-707 lb
Point	9'- 3/4"	9'- 3/4"	W17(i148)	Top	-	-	-	-	-721 lb
Point	11'- 3/4"	11'- 3/4"	W17(i148)	Top	-	-	-	-4 lb	-669 lb
Point	13'- 3/4"	13'- 3/4"	W17(i148)	Top	-	-	-	-	-848 lb
Point	15'- 3/4"	15'- 3/4"	W17(i148)	Top	-	-	-	-36 lb	-930 lb
Point	16'- 6 3/4"	16'- 6 3/4"	W17(i148)	Top	-	-	-	-76 lb	-977 lb
Point	18'- 6 3/4"	18'- 6 3/4"	W17(i148)	Top	771 lb	-	264 lb	778/-79 lb	245/-1255 lb
Point	19'- 10 5/8"	19'- 10 5/8"	W17(i148)	Top	120 lb	-	36 lb	107 lb	34 lb

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
1	0'	0'- 3 1/2"	W1(i130)	2973 lb	400 lb	827 lb	2257/-76 lb	0 lb/-3620 lb
2	19'- 8 1/2"	20'	W11(i134)	3774 lb	400 lb	1081 lb	3082/-178 lb	0 lb/-3620 lb

DESIGN NOTES

- CAUTION: The maximum net analysis reaction exceeds the user-defined maximum uplift value at one or more supports.
- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Customer:
Job Name:
City:
Customer Ph...

Job Name: **02**
Level: **2nd floor**
Label: **BM1 - i186**
Type: **Beam**

2 Ply Member
2.0 RigidLam DF LVL 1-3/4
x 18

Status:
Design
Passed

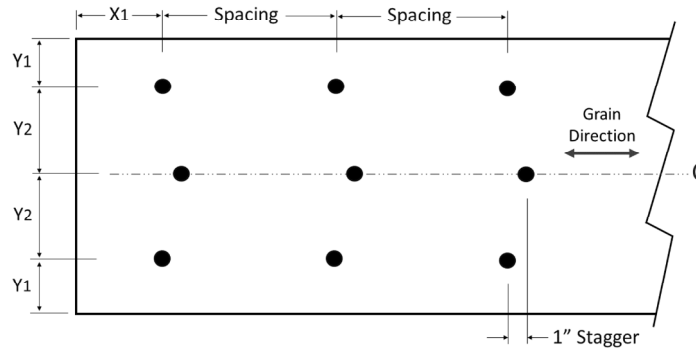
DESIGN NOTES

- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- Beam Stability Factor used in the calculation for Allowable Max Pos Moment (CL) = 1.00

PLY TO PLY CONNECTION

- Zone A: Factored load = 0 plf. Use 12d (0.148"x3.25") nails. LDF = 1.00. Qty = 63. Row = 3, Spacing = 12"
12d (0.148"x3.25") nails properties: D = 0.148" , L = 3.25". Fastener capacity = 117 lbs. X1 = 2.25" , Y1 = 0.75", Y2 = 1.5"
Install fasteners from one face.
X1 = Minimum end distance, X2 = Minimum edge distance, Y2 = Minimum row spacing.

FASTENER INSTALLATION – 3 ROWS (FROM ONE FACE)





Customer:
Job Name:
City:
Customer Ph...

Job Name: **02**
Level: **2nd floor**
Label: **BM2 - i185**
Type: **Beam**

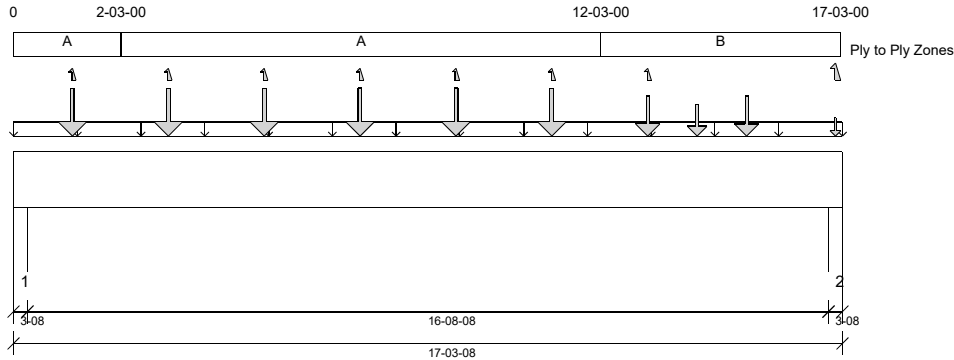
2 Ply Member
2.0 RigidLam DF LVL 1-3/4
x 14

Status:
Design Passed

Illustration Not to Scale. Pitch: 0/12

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Report Version: 2021.03.26 06/12/2024 12:23



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Design Methodology: ASD
Risk Category: II (General Construction)
Residential
Service Condition: Dry
LL Deflection Limit: L/360, 0.75" (absolute)
TL Deflection Limit: L/240, 1.00" (absolute)

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 8 1/2"

Bearing Stress of Support Material:

- 875 psi Wall @ 0'- 2 1/2"
- 875 psi Wall @ 17'- 1"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Max Pos. Moment:	9'- 2 3/4"	D + L	1.00	23329 lb ft	28972 lb ft	Passed - 81%
Max Shear:	1'- 5 1/2"	D + L	1.00	5394 lb	9473 lb	Passed - 57%
Live Load (LL) Pos. Defl.:	8'- 7 3/4"	L		0.532"	L/360	Passed - L/377
Total Load (TL) Pos. Defl.:	8'- 7 13/16"	D + L		0.745"	L/240	Passed - L/269

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Downward Reaction	Uplift Reaction	Resistance of Member	Resistance of Support	Result
1	3-08	D + L	1.00	5481 lb		9188 lb	10719 lb	Passed - 60%
2	3-08	D + L	1.00	5178 lb		9188 lb	10719 lb	Passed - 56%

LOADING

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
Self Weight	0'	17'- 3 1/2"	Self Weight	Top	13 lb/ft	-	-	-	-
Uniform	0'	17'- 3 1/2"	FC1 Floor Decking (Plan View Fill)	Top	9 lb/ft	37 lb/ft	-	-	-
Point	1'- 2 3/4"	1'- 2 3/4"	F202(Cond08)	Back	322 lb	858 lb	1 lb	3/0 lb	1/4 lb
Point	3'- 2 3/4"	3'- 2 3/4"	F202(Cond07)	Back	321 lb	858 lb	1 lb	2 lb	1/3 lb
Point	5'- 2 3/4"	5'- 2 3/4"	F202(Cond06)	Back	321 lb	858 lb	1 lb	2 lb	1/3 lb
Point	7'- 2 3/4"	7'- 2 3/4"	F202(Cond05)	Back	321 lb	858 lb	1 lb	1 lb	1/3 lb
Point	9'- 2 3/4"	9'- 2 3/4"	F202(Cond04)	Back	321 lb	858 lb	0 lb	1 lb	0/2 lb
Point	11'- 2 3/4"	11'- 2 3/4"	F202(Cond03)	Back	322 lb	858 lb	1 lb	2/0 lb	1/3 lb
Point	13'- 2 3/4"	13'- 2 3/4"	F202(Cond02)	Back	269 lb	650 lb	1 lb	2/0 lb	1/2 lb
Point	14'- 3 1/8"	14'- 3 1/8"	F202(Cond09)	Back	215 lb	442 lb	-	-	-
Point	15'- 3 1/2"	15'- 3 1/2"	F202(Cond01)	Back	267 lb	650 lb	-	-	-
Point	17'- 1 3/4"	17'- 1 3/4"	W14(i144)	Top	65 lb	-	30 lb	78 lb	32/-175 lb

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
1	0'	0'- 3 1/2"	W19(i150)	1558 lb	3923 lb	3 lb	8 lb	3 lb / -14 lb
2	17'	17'- 3 1/2"	W7(i139)	1570 lb	3608 lb	32 lb	83 lb	3 lb / -14 lb

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- Beam Stability Factor used in the calculation for Allowable Max Pos Moment (CL) = 1.00

PLY TO PLY CONNECTION

- Zone A: Factored load = 606 plf. Use 12d (0.148"x3.25") nails. LDF = 1.00. Qty = 39. Row = 3, Spacing = 12"
 - Zone B: Factored load = 637 plf. Use 12d (0.148"x3.25") nails. LDF = 1.00. Qty = 18. Row = 3, Spacing = 12"
- 12d (0.148"x3.25") nails properties: D = 0.148", L = 3.25". Fastener capacity = 117 lbs. X1 = 2.25", Y1 = 0.75", Y2 = 1.5"
- Install fasteners from one face.
X1 = Minimum end distance, X2 = Minimum edge distance, Y2 = Minimum row spacing.



Customer:
Job Name:
City:
Customer Ph...

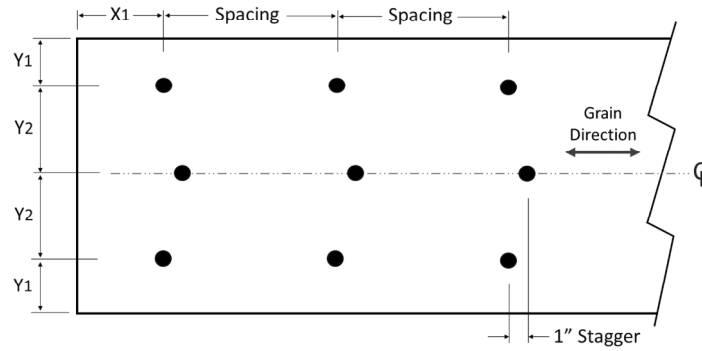
Job Name: **02**
Level: **2nd floor**
Label: **BM2 - i185**
Type: **Beam**

2 Ply Member
2.0 RigidLam DF LVL 1-3/4
x 14

Status:
Design
Passed

PLY TO PLY CONNECTION

FASTENER INSTALLATION – 3 ROWS (FROM ONE FACE)





Customer:
Job Name:
City:
Customer Ph...

Job Name: **02**
Level: **2nd floor**
Label: **BM3 - i176**
Type: **Beam**

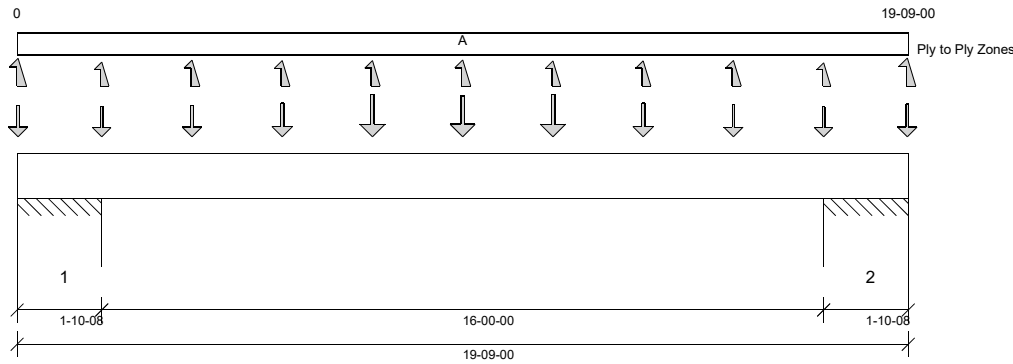
2 Ply Member
2.0 RigidLam DF LVL 1-3/4
x 11-7/8

Status:
Design Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.7.3.303.Update9.26

Report Version: 2021.03.26 06/12/2024 12:23



DESIGN INFORMATION

Building Code: IRC 2018
Design Methodology: ASD
Risk Category: II (General Construction) Residential
Service Condition: Dry
LL Deflection Limit: L/360, 0.75" (absolute)
TL Deflection Limit: L/240, 1.00" (absolute)

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 19'- 9" Bottom: 19'- 9"

Bearing Stress of Support Material:

- 875 psi Wall @ 0'- 1 1/2"
- 875 psi Wall @ 1'- 9"
- 875 psi Wall @ 18'
- 875 psi Wall @ 19'- 7 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Max Pos. Moment:	9'- 10 1/2"	D + 0.75(L + Lr + 0.6W)	1.60	1792 lb ft	19326 lb ft	Passed - 9%
Max Neg. Moment:	18'	D + 0.75(L + Lr + 0.6W)	1.60	2708 lb ft	19326 lb ft	Passed - 14%
Max Shear:	16'- 10 5/8"	D + 0.75(L + Lr)	1.15	797 lb	9241 lb	Passed - 9%
Live Load (LL) Pos. Defl.:	9'- 11 5/16"	0.75(L + Lr + 0.6W)		0.028"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	9'- 10 15/16"	D + 0.75(L + Lr + 0.6W)		0.055"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Downward Reaction	Uplift Reaction	Resistance of Member	Resistance of Support	Result
1	8-04	0.6D + 0.6W	1.60	131 lb		30130 lb	25266 lb	Passed - 1%
1	8-04	D + 0.75(L + Lr)	1.15		-1430 lb	-	-	
1	1-02-04	D + 0.75(L + Lr)	1.15	2463 lb		37406 lb	43641 lb	Passed - 7%
1	1-02-04	0.6D + 0.6W	1.60		-401 lb	-	-	
2	1-02-08	D + 0.75(L + Lr)	1.15	2459 lb		38063 lb	44406 lb	Passed - 6%
2	1-02-08	0.6D + 0.6W	1.60		-385 lb	-	-	
2	8-00	0.6D + 0.6W	1.60	124 lb		29217 lb	24500 lb	Passed - 1%
2	8-00	D + 0.75(L + Lr)	1.15		-1344 lb	-	-	

LOADING

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
Self Weight	0'	19'- 9"	Self Weight	Top	11 lb/ft	-	-	-	-
Point	0'- 1/4"	0'- 1/4"	C1(Cond01)	Top	63 lb	5 lb	30 lb	99 lb	49/-195 lb
Point	1'- 10 1/2"	1'- 10 1/2"	C1(Cond01)	Top	74 lb	-4 lb	25 lb	93 lb	37/-142 lb
Point	3'- 10 1/2"	3'- 10 1/2"	C1(Cond01)	Top	78 lb	-4 lb	29 lb	93 lb	46/-167 lb
Point	5'- 10 1/2"	5'- 10 1/2"	C1(Cond01)	Top	91 lb	24/-5 lb	28 lb	91 lb	44/-161 lb
Point	7'- 10 1/2"	7'- 10 1/2"	C1(Cond01)	Top	128 lb	85 lb	29 lb	91 lb	47/-165 lb
Point	9'- 10 1/2"	9'- 10 1/2"	C1(Cond01)	Top	127 lb	84 lb	30 lb	101 lb	27/-174 lb
Point	11'- 10 1/2"	11'- 10 1/2"	C1(Cond01)	Top	128 lb	85 lb	29 lb	91 lb	47/-161 lb
Point	13'- 10 1/2"	13'- 10 1/2"	C1(Cond01)	Top	91 lb	24/-5 lb	28 lb	91 lb	44/-157 lb
Point	15'- 10 1/2"	15'- 10 1/2"	C1(Cond01)	Top	79 lb	-4 lb	29 lb	94 lb	46/-164 lb
Point	17'- 10 1/2"	17'- 10 1/2"	C1(Cond01)	Top	73 lb	-4 lb	24 lb	92 lb	37/-133 lb
Point	19'- 8 3/4"	19'- 8 3/4"	C1(Cond01)	Top	64 lb	5 lb	31 lb	102 lb	51/-199 lb

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
1	0'	1'- 10 1/2"	W22(i172)	1342/-737 lb	494/-355 lb	303/-147 lb	1105/-588 lb	349 lb/-1056 lb
==>	0'- 1 1/2"	0'- 1 1/2"	W22(i172)	-737 lb	13/-329 lb	-147 lb	107/-580 lb	-
==>	1'- 9"	1'- 9"	W22(i172)	1342 lb	481/-26 lb	303 lb	998/-8 lb	-
2	17'- 10 1/2"	19'- 9"	W12(i140)	1348/-741 lb	494/-355 lb	305/-149 lb	1101/-580 lb	349 lb/-1056 lb
==>	18'	18'	W12(i140)	1348 lb	481/-26 lb	305 lb	999 lb	-
==>	19'- 7 1/2"	19'- 7 1/2"	W12(i140)	-741 lb	13/-329 lb	-149 lb	102/-580 lb	-

DESIGN NOTES

- CAUTION: The maximum net analysis reaction exceeds the user-defined maximum uplift value at one or more supports.
- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.



Customer:
Job Name:
City:
Customer Ph...

Job Name: **02**
Level: **2nd floor**
Label: **BM3 - i176**
Type: **Beam**

2 Ply Member
2.0 RigidLam DF LVL 1-3/4
x 11-7/8

Status:
Design
Passed

- Beam Stability Factor used in the calculation for Allowable Max Pos Moment (CL) = 0.57

PLY TO PLY CONNECTION

- Zone A: Factored load = 0 plf. Use 12d (0.148"x3.25") nails. LDF = 1.00. Qty = 40. Row = 2, Spacing = 12"
12d (0.148"x3.25") nails properties: D = 0.148", L = 3.25". Fastener capacity = 117 lbs. X1 = 2.25", Y1 = 0.75", Y2 = 1.5"
Install fasteners from one face.

X1 = Minimum end distance, X2 = Minimum edge distance, Y2 = Minimum row spacing.

FASTENER INSTALLATION – 2 ROWS (FROM ONE FACE)

