

Roof Area = 2705.11 sq.ft.
Ridge Line = 43.83 ft.
Hip Line = 16.34 ft.
Horiz. OH = 226.13 ft.
Raked OH = 101.27 ft.
Decking = 93 sheets

All Walls Shown Are Considered Load Bearing

Truss Placement Plan
Scale: 1/4"=1'

▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)

Dimension Notes

1. All exterior wall to wall dimensions are to face of stud unless noted otherwise
2. All interior wall dimensions are to face of stud unless noted otherwise
3. All exterior wall to truss dimensions are to face of stud unless noted otherwise

Hatch Legend
Padded HVAC
Drop Beam

	Conne	Nail Info	ormation			
Sym	Product	Header	Truss			
	JUS24	USP	NA	10d/3"	10d/3"	
L	SKH26L	USP	16d/3-1/2"	10d/1-1/2"		
R	SKH26R	USP	1	Varies	16d/3-1/2"	10d/1-1/2"

		Products		
PlotID	Length	Product	Plies	Net Qty
DB1	44' 0"	2x12 SP No.2	2	2
DB2	40' 0"	2x12 SP No.2	2	2

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Reaction / # of Studs



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables ( derived from the prescriptive Code requirements ) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Neil Baggett

Neil Baggett

LOAD CHART FOR JACK STUDS
(BASED ON TABLES R502.5(1) & (b))

NUM	MBER C	STUDS R HEADER/		A END OF	-
END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				

CITY / CO.	Spring Lake / Harnett
ADDRESS	184 Donnas Lane
MODEL	Roof
DATE REV.	5/9/2024
DRAWN BY	DRAWN BY Neil Baggett
SALES REP.	SALES REP. Neil Baggett

JOB NAME Shirey Resion Custom Custom SEAL DATE Seal Date Quote # Quote # JO524-2717

Cash/Brian Shirey

BUILDER

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

					DATE	05/09/24 PAGE 1
Reaction Summary of Orde	er	REQ. QUOTE DATE	11		ORDER #	J0524-2717
^		ORDER DATE	05/07/24		QUOTE #	
		DELIVERY DATE	11		CUSTOMER ACCT#	000127
ROOF & FLOOR		DATE OF INVOICE	11		CUSTOMER PO#	
ComTech   TRUSSES & BEAMS		ORDERED BY	NONE		INVOICE #	
eilly Road Industrial Park P.O. Box 40408		COUNTY	Harnett		TERMS	TO BE PRE-PAID
ayetteville, N.C. 28309 (910) 864-TRUS		SUPERINTENDANT	NONE		SALES REP	Neil Baggett
		JOBSITE PHONE #	( ) -		SALES AREA	Neil Baggett
	JOB NAME: S	Shirey Residence	•	LOT #	SUBDIV:	•
L	MODEL: Roof	TAG: Cu	ustom	JOB 0	CATEGORY: WCall - Will	Call
	DELIVERY INS					

**REQ. LAYOUTS** 

**JOBSITE** 

Joint 2

Joint 2

Joint 2

484.2 lbs.

-192.6 lbs.

484.2 lbs.

-79.0 lbs.

219.4 lbs.

-83.7 lbs.

Joint 6

Joint 6

Joint 7

394.5 lbs.

-160.3 lbs.

394.5 lbs.

-48.4 lbs.

3.7 lbs

-9.8 lbs

Joint 8

70.7 lbs.

-25.7 lbs.

Joint 9

168.9 lbs.

-47.2 lbs.

Joint 10

99.0 lbs.

-39.6 lbs.

**REQ. ENGINEERING** 

JOBSITE

Cash/Brian Shirey

184 Donnas Lane Spring Lake, NC

Roof Order

BUILDING DEPARTMENT OVERHANG INFO

END CUT RETURN

MONOPITCH

Х3

MONOPITCH

X4

MONOPITCH

X5

16

1

3.00

3.00

3 00

0.00

0.00

0.00

10-00-08

10-00-08

10-00-08

10-00-08 2 X 4 2 X 6 01-02-08 00-03-08

10-00-08 | 2 X 4 | 2 X 6 | 01-02-08 | 00-03-08

10-00-08 2 X 4 2 X 6 01-02-08

PLUMB

SPECIAL INSTRUCTIONS:

HEEL HEIGHT

**GABLE STUDS** 

00-06-08

24 IN. OC

PLAN SEAL DATE: BY

QUOTE

LAYOUT

CUTTING NB

DATE

11

05/07/24

**LOADING** TCLL-TCDL-BCLL-BCDL STRESS INCR. ROOF TRUSSES ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.) INFORMATION 20.0,10.0,0.0,10.0 PITCH TYPE BASE QTY **PROFILE** LUMBER **OVERHANG REACTIONS** O/A PLY ID TOP вот TOP BOT LEFT RIGHT **PIGGYBACK** 29-05-00 Joint 20 Joint 21 Joint 22 Joint 23 Joint 19 8.00 0.00 A1 29-04-00 | 2 X 6 | 2 X 6 | 01-06-08 | 01-03-00 554.4 lbs. 711.0 lbs. 923.4 lbs. 551.1 lbs. 553.4 lbs. -647.8 lbs. -789.9 lbs. -266.7 lbs. -270.1 lbs. -249.9 lbs. **PIGGYBACK** 29-08-08 Joint 7 Joint 11 Α2 29-07-08 | 2 X 6 | 2 X 6 | 01-03-00 | 01-03-00 15 8.00 0.00 1418.2 lbs. 1418.2 lbs. -53.4 lbs. -53.4 lbs. **PIGGYBACK** 29-08-08 Joint 7 Joint 11 29-07-08 2 X 6 2 X 6 01-03-00 01-03-00 0.00 8.00 A3 5 1418.2 lbs. 1418.2 lbs. -53.4 lbs. -53.4 lbs GABLE Joint 19 29-08-08 Joint 20 Joint 21 Joint 22 Joint 23 29-07-08 | 2 X 6 | 2 X 6 | 01-03-00 | 01-03-00 8.00 0.00 A4 544.3 lbs. 626.8 lbs. 178.3 lbs. 182.6 lbs. 176.5 lbs. -484.1 lbs. -514.0 lbs -96.1 lbs. -95.6 lbs -68.5 lbs. **GABLE** 08-05-12 Joint 2 Joint 6 Joint 8 Joint 9 Joint 10 PB1 08-05-12 2 X 4 2 X 4 2 8.00 0.00 225.2 lbs. 225.8 lbs. 113.3 lbs. 113.3 lbs. 111.8 lbs. -19.6 lbs. -17.6 lbs. -114.8 lbs. 24.0 lbs. -115.3 lbs **PIGGYBACK** 08-05-12 Joint 2 Joint 4 Joint 6 08-05-12 2 X 4 2 X 4 PB2 20 8.00 0.00 216.0 lbs. 216.0 lbs. 305.9 lbs. -37.3 lbs. -44.7 lbs. 24.7 lbs. MONOPITCH 10-00-08 Joint 2 Joint 7 Joint 8 Joint 9 Joint 10 0.00 1 3.00 X1 10-00-08 | 2 X 4 | 2 X 6 | 01-02-08 70.7 lbs. 168.9 lbs. 99.0 lbs. 219.4 lbs. 3.7 lbs -83.7 lbs. -25.7 lbs. -47.2 lbs. -39.6 lbs. -9.8 lbs JACK-CLOSED 10-00-08 Joint 2 Joint 7 10-00-08 2 X 4 2 X 6 01-02-08 15 3.00 0.00 464 0 lbs 411.8 lbs -184.5 lbs. -159.0 lbs.

																		DA	TE 05/09	/24	PAGE 2
eaction	Sur	nma	ry of	Order		R	EQ. (	TOUS	E DATE	•	/ /				OR	DER#	<del>‡</del>		J05	24-2	2717
						O	RDE	R DA	TE	С	05/0	7/24			QU	OTE #	ŧ				
						D	ELIV	ERY I	DATE		/ /				CU	STOM	IER A	CCT :	# 00	0127	
		ROOF	& FLO	OR		D	ATE	OF IN	VOICE		//				CU	STOM	IER F	,O #			
omTe	ch∥ˈ	TRUSSE	S & BE	AMS		o	RDE	RED E	вү	N	NONE					/OICE	#				
y Road Ind	ustrial F	Park P.C	). Box 4	0408			OUN			ŀ	Harnett					RMS			ТО	BE F	PRE-PA
etteville, N.						s	UPE	RINTE	NDANT	Г	NON	ΙΕ			SA	LES R	EP		Ne	il Bag	ggett
						J	OBSI	TE PI	HONE #	: (	)	-			SA	LES A	REA				ggett
				Jo	OB N	IAME: Shi	rev R	esideı	nce					LO	Г#	SI	UBD	V:			
CASH	OR CH	IFCKS	ONLY			L:Roof	,			: Custo	m								Vill Call		
NO CRE		_	_	· —		ERY INSTR	UCTIO	ONS:													
	יוום:	ARDS	,			s round trip	00110														
( ) -																					
Cash/Br	ian S	hirey																			
184 Dor	nas L	ane		SF	PECIA	AL INSTRU	CTION	NS:													
Spring I																					
Opinig i	_anc,	.,,																PLAN	SEAL [		
																			BY		DATE
		г						1											1		
	EPART	MENT				IEEL HEIGH	-T	00-0	06-08	REQ	. LA	YOUTS	REC	. ENGI	NEERIN	NG	_	OTE			11
	<u>EPART</u>	MENT	END CU		N					REQ	$\perp$		REQ	. ENGI			LA'	YOUT	NB		11
f Order			PLUMB	T RETUR	N G	ABLE STU	DS	24 1	N. OC		$\perp$	YOUTS	REQ	. ENGI	JOBSITE		LA'		NB	(	
of Order			PLUMB		N G	TCLL-TCDL-BO	DS CLL-BCD	24 I	IN. OC				1		JOBSITE	E 1	CU'	YOUT	NB	(	11
of Order	RUS	SSES	PLUMB LC IN	PADING FORMATION TYPE	N G	TCLL-TCDL-BC 20.0,10.0,0	<b>DS</b> CLL-BCD 0.0,10.	24 I	IN. OC RESS INCR.		RC	OOF TRUS	1 S SPAC		JOBSITE	E 1	CU'	YOUT	NB	(	11
of Order	RUS	SSES	PLUMB LC IN	DADING FORMATION TYPE	G ON	TCLL-TCDL-BC 20.0,10.0,C BASE O/A	DS CLL-BCD 0.0,10.	24 I	IN. OC RESS INCR.	RHAN	RC	OOF TRUS	S SPAC	ING: 2	Jobsite 24.0 IN.	E 1	CU'	YOUT	NB	(	11
of Order  OOF T  ROFILE	QTY PLY	SSES PIT TOP	PLUMB LC IN CH BOT	DADING FORMATION TYPE ID	G ON	TCLL-TCDL-BC 20.0,10.0,0 BASE O/A 02-04-15	DS  CLL-BCD  0.0,10.  LUN  TOP	24 I STR 0 IBER BOT	IN. OC RESS INCR. 1.15 OVE LEFT	ERHAN RIG	RC	OF TRUS REAC	S SPAC	int 3	JOBSITE 24.0 IN.	E 1	CU'	YOUT	NB	(	11
f Order  OOF T  ROFILE	RUS	SSES	PLUMB LC IN	DADING FORMATION TYPE	G ON	TCLL-TCDL-BC 20.0,10.0,C BASE O/A	DS  CLL-BCD  0.0,10.  LUN  TOP	24 I STR 0 IBER BOT	IN. OC RESS INCR. 1.15 OVE LEFT	ERHAN RIG	RC	JOBSITE  POF TRUS  REAC  Joint 2  185.1 lb	S SPAC	int 3	JOBSITE 24.0 IN. J	E 1 . O.C.	CU (TYP)	YOUT	NB	(	11
OOF TOPROFILE	QTY PLY	SSES PIT TOP	PLUMB LC IN CH BOT	DADING FORMATION TYPE ID	G ON	TCLL-TCDL-BC 20.0,10.0,0 BASE O/A 02-04-15	DS  CLL-BCD  0.0,10.  LUN  TOP	24 I STR 0 IBER BOT	IN. OC RESS INCR. 1.15 OVE LEFT	ERHAN RIG	RC	OF TRUS REAC	S SPAC	int 3	JOBSITE 24.0 IN. J	E 1	CU (TYP)	YOUT	NB	(	11
OOF TOPROFILE	QTY PLY	SSES PIT TOP	PLUMB LC IN CH BOT	DADING FORMATION TYPE ID JACK-OPI	ON EN	TCLL-TCDL-BC 20.0,10.0,C BASE O/A 02-04-15 02-04-15	DS  CLL-BCD  0.0,10.  LUN  TOP	24 I STR 0 IBER BOT	IN. OC RESS INCR. 1.15 OVE LEFT	ERHAN RIG	RC	JOBSITE  OF TRUS  REAC  Joint 2  185.1 lb  -86.7 lbs	1 S SPAC	int 3 52.9 lb 19.4 lb	JOBSITE J. S.	loint 4 40.0 lt	CU (TYP)	YOUT	NB	(	11
of Order  OOF T  ROFILE	QTY PLY	SSES PIT TOP	PLUMB LC IN CH BOT	DADING FORMATION TYPE ID	ON EN	TCLL-TCDL-BC 20.0,10.0,0 BASE O/A 02-04-15	DS  CLL-BCD  0.0,10.  LUM  TOP  2 X 4	24 I  L STR 0  IBER BOT 2 X 4	N. OC RESS INCR. 1.15 OVE LEFT 01-02-08	RHAN RIC	RC	JOBSITE  OF TRUS  REAC  Joint 2  185.1 lb  -86.7 lbs	1 S SPAC	int 3 52.9 lb 19.4 lb	JOBSITE 24.0 IN.  J Ss. ss.	loint 4 40.0 lt -12.6 ll	(TYP	YOUT	NB		11
f Order  OOF T  ROFILE	QTY PLY	SSES PIT TOP 3.00	PLUMB LC IN BOT 0.00	DADING FORMATIO TYPE ID JACK-OPI Y1	ON EN	TCLL-TCDL-BC 20.0,10.0,0 BASE O/A 02-04-15 02-04-15	DS  CLL-BCD  0.0,10.  LUM  TOP  2 X 4	24 I  L STR 0  IBER BOT 2 X 4	N. OC RESS INCR. 1.15 OVE LEFT 01-02-08	RHAN RIC	RC	JOBSITE  OF TRUS  REAC  Joint 2  185.1 lb  -86.7 lbs	TIONS Jos.	int 3 52.9 lb 19.4 lb	JOBSITE 24.0 IN.  J ss. ss. J ss.	loint 4 40.0 lt	CU' (TYP)	YOUT	NB	(	11
OF TOOP TOOP TOOP TOOP TOOP TOOP TOOP TO	QTY PLY	SSES PIT TOP 3.00	PLUMB LC IN BOT 0.00	DADING FORMATIO TYPE ID JACK-OPI Y1	ON EN	TCLL-TCDL-BC 20.0,10.0,0 BASE O/A 02-04-15 02-04-15	DS  CLL-BCD  0.0,10.  LUM  TOP  2 X 4	24 I  L STR 0  IBER BOT 2 X 4	N. OC RESS INCR. 1.15 OVE LEFT 01-02-08	RHAN RIC	RC	JOBSITE  OF TRUS  REAC  Joint 2  185.1 lb  -86.7 lbs  Joint 2  256.5 lb	TIONS Jos.	int 3 52.9 lb 19.4 lb int 3 113.1 lk	JOBSITE 24.0 IN.  J ss. ss. J ss.		CU' (TYP)	YOUT	NB	(	11
OF TOOP TOOP TOOP TOOP TOOP TOOP TOOP TO	QTY PLY	SSES PIT TOP 3.00	PLUMB LC IN BOT 0.00	DADING FORMATIO TYPE ID JACK-OPI Y2  JACK-OPI	ON EN	TCLL-TCDL-BC 20.0,10.0,0  BASE O/A  02-04-15 02-04-15  04-04-15 04-04-15	DS CLL-BCD 0.0,10.  LUM TOP 2 X 4	24 I  L STR 0  BER BOT 2 X 4	N. OC  RESS INCR. 1.15  OVE  LEFT  01-02-08	RHAN RIG	RC	JOBSITE  OF TRUS  REAC  Joint 2  185.1 lb  -86.7 lbs  Joint 2  256.5 lb	S SPAC	int 3 52.9 lb 19.4 lb int 3 113.1 lk	JOBSITE 24.0 IN.  J. S.		CU' (TYP)	YOUT	NB	į (	11
of Order  OOF T  ROFILE	QTY PLY	SSES PIT TOP 3.00	PLUMB LC IN BOT 0.00	DADING FORMATIO TYPE ID JACK-OPI Y1  JACK-OPI Y2	ON EN	TCLL-TCDL-BC 20.0,10.0,C BASE O/A 02-04-15 02-04-15 04-04-15	DS CLL-BCD D.0,10.  LUM TOP 2 X 4	24 I  L STR 0  BER BOT 2 X 4	N. OC  RESS INCR. 1.15  OVE  LEFT  01-02-08	RHAN RIG	RC	Joint 2 256.5 lb -110.6 lb	S SPAC	int 3 52.9 lb 19.4 lb int 3 113.1 lb	JOBSITE 24.0 IN.  J. S. S. S. S. S. J. S. S. S. S. J. S. S. S. S. S. J. J. S. S. S. S. J. J. S.	loint 4 40.0 lt -12.6 ll 78.1 lt -24.0 ll	CU' (TYP)	YOUT	NB	į (	11
OOF TOPROFILE	QTY PLY 2	3.00	PLUMB LC IN BOT 0.00	DADING FORMATIO TYPE ID JACK-OPI Y2  JACK-OPI	ON EN	TCLL-TCDL-BC 20.0,10.0,0  BASE O/A  02-04-15 02-04-15  04-04-15 04-04-15	DS CLL-BCD D.0,10.  LUM TOP 2 X 4	24 I  L STR 0  BER BOT 2 X 4	N. OC  RESS INCR. 1.15  OVE  LEFT  01-02-08	RHAN RIG	RC	JOBSITE  OF TRUS  REAC  Joint 2  185.1 lb  -86.7 lbs  Joint 2  256.5 lb  -110.6 lb  Joint 2	S SPAC	int 3 52.9 lb 19.4 lb int 3 113.1 lb 43.4 lb	JOBSITE 24.0 IN.  J. S.		(TYP)	YOUT	NB	Į.	11
OOF TOPROFILE	QTY PLY 2	3.00	PLUMB LC IN BOT 0.00	DADING FORMATIO TYPE ID JACK-OPI Y2  JACK-OPI Y3	ON GON EEN	TCLL-TCDL-BC 20.0,10.0,C BASE O/A 02-04-15 02-04-15 04-04-15 04-04-15 06-04-15	DS CLL-BCD D.0,10.  LUM TOP 2 X 4	24 I  L STR 0  BER BOT 2 X 4	N. OC  RESS INCR. 1.15  OVE  LEFT  01-02-08	RHAN RIG	RC	Joint 2 256.5 lb -110.6 lb Joint 2 333.4 lb -138.4 lb	TIONS  Jo s. s. Jo ss. s	int 3 52.9 lb 19.4 lb 19.4 lb int 3 113.1 lt 43.4 lb int 3 142.0 lt 54.2 lb	JOBSITE 24.0 IN.  J. S.		(TYP)	YOUT	NB	(	11
OOF TOPROFILE	QTY PLY 2 2	3.00 3.00	PLUMB LCIN BOT 0.00  0.00	DADING FORMATIO TYPE ID JACK-OPI Y2  JACK-OPI Y3	ON GON EEN	TCLL-TCDL-BG 20.0,10.0,0  BASE O/A  02-04-15  04-04-15  04-04-15  06-04-15  06-04-15	DS CLL-BCC LO.0,10. LUN TOP 2 X 4	24 I STR	N. OC  RESS INCR.  1.15  OVE  LEFT  01-02-08	ERHAN RIC 8	RC	Joint 2 185.1 lb -86.7 lbs  Joint 2 256.5 lb -110.6 lb -138.4 lb Joint 2	1 Jos	int 3 552.9 lb 19.4 lb 119.4 l	JOBSITE 24.0 IN.  J. S. S. S. S. J. S.		(TYP)	YOUT	NB	(	11
of Order  OOF T  ROFILE	QTY PLY 2	3.00	PLUMB LC IN BOT 0.00	DADING FORMATIO TYPE ID JACK-OPI Y2  JACK-OPI Y3	ON GON EEN	TCLL-TCDL-BC 20.0,10.0,C BASE O/A 02-04-15 02-04-15 04-04-15 04-04-15 06-04-15	DS CLL-BCC LO.0,10. LUN TOP 2 X 4	24 I STR	N. OC  RESS INCR.  1.15  OVE  LEFT  01-02-08	ERHAN RIC 8	RC	Joint 2 185.1 lb -86.7 lbs  Joint 2 256.5 lb -110.6 lb  Joint 2 333.4 lb -138.4 lb  Joint 2 379.3 lb	1 Jos	int 3 52.9 lb 19.4 lb 19.4 lb 143.4 lb 143.4 lb 142.0 lt 54.2 lb int 6 848.2 lt	JOBSITE 24.0 IN.  J s. s. s.  J s. s. s.		(TYP)	YOUT	NB	(	11
PROFILE	QTY PLY 2 2	3.00 3.00	PLUMB LCIN BOT 0.00  0.00	DADING FORMATIO TYPE ID JACK-OPI Y2  JACK-OPI Y3	ON GON EEN	TCLL-TCDL-BG 20.0,10.0,0  BASE O/A  02-04-15  04-04-15  04-04-15  06-04-15  06-04-15	DS CLL-BCC LO.0,10. LUN TOP 2 X 4	24 I STR	N. OC  RESS INCR.  1.15  OVE  LEFT  01-02-08	ERHAN RIC 8	RC	Joint 2 185.1 lb -86.7 lbs  Joint 2 256.5 lb -110.6 lb -138.4 lb Joint 2	1 Jos	int 3 552.9 lb 19.4 lb 119.4 l	JOBSITE 24.0 IN.  J s. s. s.  J s. s. s.		(TYP)	YOUT	NB	į (	11
OF TOOP TOOP TOOP TOOP TOOP TOOP TOOP TO	QTY PLY 2 2	3.00 3.00	PLUMB LCIN BOT 0.00  0.00	JACK-OPI Y3  JACK-CLOS Y4	ON GON EEN	02-04-15 04-04-15 06-04-15 08-04-15	DS CLL-BCC LO.0,10. LUN TOP 2 X 4	24 I STR	N. OC  RESS INCR.  1.15  OVE  LEFT  01-02-08	ERHAN RIC 8	RC	Joint 2 185.1 lb -86.7 lbs  Joint 2 256.5 lb -110.6 lb  Joint 2 333.4 lb -138.4 lb  Joint 2 379.3 lb -150.1 lb	1 Jos	int 3 52.9 lb 19.4 lb 19.4 lb int 3 142.0 lt 54.2 lb int 6 848.2 lt 132.2 lt	JOBSITE 24.0 IN.  J s. s. s.  J s. s. s.		(TYP)	YOUT	NB	į (	11
OF Order	QTY PLY 2 2	3.00 3.00	PLUMB LCIN BOT 0.00  0.00	DADING FORMATIO TYPE ID JACK-OPI Y2  JACK-OPI Y3	ON GON EEN	TCLL-TCDL-BG 20.0,10.0,0  BASE O/A  02-04-15  04-04-15  04-04-15  06-04-15  06-04-15	DS CLL-BCD 0.0,10. LUN TOP 2 X 4 2 X 4	24 I STR	N. OC  RESS INCR.  1.15  OVE  LEFT  01-02-08  01-02-08	ERHAN RIC 8	RC	Joint 2 185.1 lb -86.7 lbs  Joint 2 256.5 lb -110.6 lb  Joint 2 333.4 lb -138.4 lb  Joint 2 379.3 lb	1 Jos	int 3 52.9 lb 19.4 lb 19.4 lb 143.4 lb 143.4 lb 142.0 lt 54.2 lb int 6 848.2 lt	JOBSITE 24.0 IN.  J. S. S. S. J. S.		(TYP)	YOUT	NB	, (	11

### *ITEMS*

QTY	ITEM TYPE	SIZE	<b>LENGTH</b> FT-IN-16	PART NUMBER	NOTES
15	Hangers, USP	JUS24			SIMPSON (LUS24)
1	Hangers, USP	SKH26L			SIMPSON (SUL26)
. 1	Hangers, USP	SKH26R			SIMPSON (SUR26)



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0524-2717

Cash/Shirey Residence/Harnett

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I65454086 thru I65454101

My license renewal date for the state of North Carolina is December 31, 2024.

North Carolina COA: C-0844



May 9,2024

Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job Truss Truss Type Qty Cash/Shirey Residence/Harnett 165454086 J0524-2717 A1 PIGGYBACK BASE GIRDE Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Wed May 8 14:57:42 2024 Page 1

ID:nsIXU10oULrH1SgmyBZWMUzvBdO-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

19-6-4 9-6-4 10-0-0 9-9-12

Scale = 1:77.1

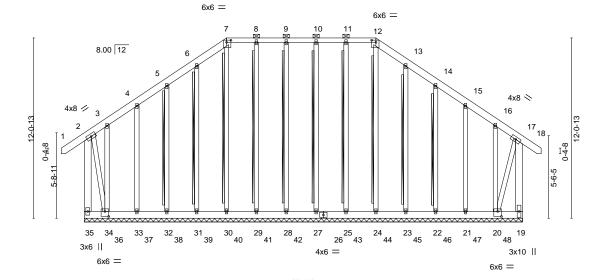


Plate Offsets (X,Y)-- [7:0-3-8,0-1-12], [12:0-3-8,0-1-12], [20:0-3-0,0-4-0], [34:0-3-0,0-4-0]

	• • •	7 7 7			
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.15	TC 0.09	Vert(LL) -0.00 17 n/r 120 MT20 244/190	
TCDL	10.0	Lumber DOL 1.15	BC 0.08	Vert(CT) 0.00 17 n/r 120	
BCLL	0.0 *	Rep Stress Incr NO	WB 0.32	Horz(CT) -0.01 19 n/a n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Weight: 378 lb FT = 25	%

LUMBER-BRACING-2x6 SP No.1 \*Except\* TOP CHORD TOP CHORD

7-12: 2x4 SP No.1 **BOT CHORD** 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1 \*Except\* **WEBS WEBS** 

2-34,17-20: 2x4 SP No.2

2x4 SP No.2 **OTHERS** 

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (10-0-0 max.): 7-12. Rigid ceiling directly applied or 6-0-0 oc bracing, Except:

10-0-0 oc bracing: 19-20.

2x4 SPF No.2 - 12-24, 11-25, 10-27, 9-28, T-Brace: 8-29, 7-30, 6-31, 5-32, 13-23, 14-22

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails. 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 29-4-0.

(lb) -Max Horz 35=-452(LC 6)

Max Uplift All uplift 100 lb or less at joint(s) except 35=-732(LC 4), 19=-648(LC 5), 24=-184(LC 6), 25=-195(LC 4), 27=-236(LC 5), 28=-222(LC 4), 29=-215(LC 4), 30=-158(LC 7), 31=-251(LC 8), 32=-268(LC 8), 33=-278(LC 8), 34=-750(LC 5), 23=-250(LC 9), 22=-270(LC 9), 21=-267(LC 9), 20=-790(LC 4)

Max Grav All reactions 250 lb or less at joint(s) except 35=834(LC 34), 19=711(LC

25), 24=558(LC 1), 25=510(LC 1), 27=587(LC 19), 28=557(LC 20), 29=557(LC 1), 30=548(LC 1), 31=558(LC 1), 32=547(LC 19), 33=575(LC 1), 34=853(LC 25),

23=553(LC 1), 22=554(LC 20), 21=551(LC 1), 20=923(LC 34)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-35=-588/536, 6-7=-56/267, 12-13=-77/278, 17-19=-620/570

**BOT CHORD** 34-35=-408/366, 33-34=-277/247, 32-33=-277/247, 31-32=-277/247, 30-31=-277/247,

29-30=-277/247, 28-29=-277/247, 27-28=-277/247, 25-27=-277/247, 24-25=-277/247,

23-24=-277/247, 22-23=-277/247, 21-22=-277/247, 20-21=-277/247

**WEBS** 2-34=-622/591, 17-20=-625/592

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web)
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide Continuille to be transperate bottom chord and any other members



May 9,2024

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	Cash/Shirey Residence/Harnett	
						165454086
J0524-2717	A1	PIGGYBACK BASE GIRDE	1	1		
					Joh Reference (ontional)	

Fayetteville, NC - 28314, Comtech, Inc,

8.430 s Jan 6 2022 MiTek Industries, Inc. Wed May 8 14:57:42 2024 Page 2 ID:nsIXU10oULrH1SgmyBZWMUzvBdO-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

### NOTES-

- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 732 lb uplift at joint 35, 648 lb uplift at joint 19, 184 lb uplift at joint 24, 195 lb uplift at joint 25, 236 lb uplift at joint 27, 222 lb uplift at joint 28, 215 lb uplift at joint 29, 158 lb uplift at joint 30, 251 lb uplift at joint 31, 268 lb uplift at joint 32, 278 lb uplift at joint 33, 750 lb uplift at joint 34, 250 lb uplift at joint 23, 270 lb uplift at joint 22, 267 lb uplift at joint 21 and 790 lb uplift at joint 20.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 402 lb down and 170 lb up at 0-3-4, 392 lb down and 181 lb up at 2-2-12, 392 lb down and 181 lb up at 4-2-12, 392 lb down and 181 lb up at 4-2-12, 392 lb down and 181 lb up at 6-2-12, 392 lb down and 181 lb up at 6-2-12, 392 lb down and 181 lb up at 12-2-12, 392 lb down and 181 lb up at 14-2-12, 392 lb down and 181 lb up at 16-2-12, 392 lb down and 181 lb up at 18-2-12, 392 lb down and 18-2-12, 392 lb down and 18-2-12, 392 lb down and 18-2-12, 39 181 lb up at 20-2-12, 392 lb down and 181 lb up at 22-2-12, 392 lb down and 181 lb up at 24-2-12, and 393 lb down and 180 lb up at 26-2-12, and 393 lb down and 393 lb lb up at 28-2-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 14) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 15) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-7=-60, 7-12=-60, 12-17=-60, 17-18=-60, 19-35=-20

Concentrated Loads (lb)

Vert: 35=-402(F) 26=-392(F) 36=-392(F) 37=-392(F) 38=-392(F) 39=-392(F) 40=-392(F) 41=-392(F) 42=-392(F) 43=-392(F) 44=-392(F) 45=-392(F) 46=-392(F) 47=-392(F) 48=-393(F)



Job Truss Truss Type Qty Cash/Shirey Residence/Harnett 165454087 J0524-2717 A2 PIGGYBACK BASE 15 Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Wed May 8 14:57:43 2024 Page 1 ID:nsIXU10oULrH1SgmyBZWMUzvBdO-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 30-10-8 14-9-12 9-9-12 1-3-0 Scale = 1:70.2 5x8 = 6x6 = 3 8.00 12 18 4x4 = 19 2x4 =13 12 4x6 🥢 4x6 <> 6 0-4-8 0-4-8 5-6-5 ф ₩ 7 9 21 20 11 10 8 3x6 || 4x8 = 3x6 II 3x4 = 3x4 = 19-9-12 29-7-8 9-9-12 Plate Offsets (X,Y)--[2:0-1-0,0-2-0], [3:0-5-4,0-3-4], [4:0-3-8,0-3-4], [5:0-1-0,0-2-0] LOADING (psf) SPACING-CSI. DEFL. in (loc) I/def L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.40 Vert(LL) -0.18 8-10 >999 360 244/190 MT20 TCDL 10.0 Lumber DOL 1.15 BC 0.46 Vert(CT) -0.22 8-10 >999 240 **BCLL** 0.0 Rep Stress Incr YES WB 0.28 Horz(CT) 0.01 n/a n/a Code IRC2015/TPI2014 FT = 25% **BCDL** 10.0 Wind(LL) 0.20 10-11 >999 240 Weight: 272 lb Matrix-AS **BRACING-**

TOP CHORD

**BOT CHORD** 

**WEBS** 

**JOINTS** 

LUMBER-

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1 WEBS 2x4 SP No.2 \*Except\*

2-11,5-7: 2x6 SP No.1

REACTIONS. (size) 11=0-3-8, 7=0-3-8 Max Horz 11=-356(LC 10)

Max Uplift 11=-53(LC 12), 7=-53(LC 13) Max Grav 11=1418(LC 2), 7=1418(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 $2\text{-}3\text{=-}1118/332,\ 3\text{-}4\text{=-}821/384,\ 4\text{-}5\text{=-}1118/332,\ 2\text{-}11\text{=-}1253/424,\ 5\text{-}7\text{=-}1252/424}$ TOP CHORD

**BOT CHORD** 10-11=-333/377 8-10=-175/864 **WEBS** 2-10=-135/905, 5-8=-136/905

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 9-10-4, Exterior(2) 9-10-4 to 16-0-15, Interior(1) 16-0-15 to 19-10-4, Exterior(2) 19-10-4 to 26-0-15, Interior(1) 26-0-15 to 30-9-7 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 11 and 53 lb uplift at
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied, except end verticals, and

2-0-0 oc purlins (6-0-0 max.): 3-4.

Rigid ceiling directly applied.

1 Brace at Jt(s): 12, 13

1 Row at midpt

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall

building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job Truss Truss Type Qty Cash/Shirey Residence/Harnett 165454088 PIGGYBACK BASE J0524-2717 **A3** 5 Job Reference (optional) Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Wed May 8 14:57:43 2024 Page 1 Comtech, Inc. ID:nsIXU10oULrH1SgmyBZWMUzvBdO-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 30-10-8 9-9-12 14-9-12 19-9-12 29-7<sub>-</sub>8 0-5-8 29-2-0 1-3-0 Scale = 1:74.2 5x8 = 6x6 =3 8.00 12 18 4x4 = 19 13 4x6 / 4x6 💸 0-4-8 0-4-8 5-6-5 7-2-I 20 9 21 10 8 2x4 || 2x4 || 3x6 || 5x12 = 5x12 = 3x6 || 8x8 = 19-9-12 9-9-12 29-7-8 9-9-12 Plate Offsets (X,Y)--[2:0-1-0,0-2-0], [3:0-5-4,0-3-4], [4:0-3-8,0-3-4], [5:0-1-0,0-2-0] LOADING (psf) SPACING-CSI. DEFL. in (loc) I/def L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.40 Vert(LL) -0.18 8-10 >999 360 244/190 MT20 TCDL 10.0 Lumber DOL 1.15 BC 0.46 Vert(CT) -0.22 8-10 >999 240 BCLL 0.0 Rep Stress Incr YES WB 0.28 Horz(CT) 0.01 n/a n/a Code IRC2015/TPI2014 FT = 25% **BCDL** 10.0 Wind(LL) 0.20 10-11 >999 240 Weight: 294 lb Matrix-AS **BRACING-**

TOP CHORD

**BOT CHORD** 

**WEBS** 

**JOINTS** 

LUMBER-

REACTIONS.

2x6 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2 \*Except\*

2-11,5-7: 2x6 SP No.1

Max Horz 11=-356(LC 10) Max Uplift 11=-53(LC 12), 7=-53(LC 13) Max Grav 11=1418(LC 2), 7=1418(LC 2)

(size) 11=0-3-8, 7=0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  $2\text{-}3\text{=-}1118/332,\ 3\text{-}4\text{=-}821/384,\ 4\text{-}5\text{=-}1118/332,\ 2\text{-}11\text{=-}1253/424,\ 5\text{-}7\text{=-}1252/424}$ TOP CHORD

**BOT CHORD** 10-11=-333/377 8-10=-175/864 **WEBS** 2-10=-135/905, 5-8=-136/905

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 9-10-4, Exterior(2) 9-10-4 to 16-0-15, Interior(1) 16-0-15 to 19-10-4, Exterior(2) 19-10-4 to 26-0-15, Interior(1) 26-0-15 to 30-9-7 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 11 and 53 lb uplift at joint 7.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied, except end verticals, and

2-0-0 oc purlins (6-0-0 max.): 3-4.

Rigid ceiling directly applied.

1 Brace at Jt(s): 12, 13

1 Row at midpt

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall

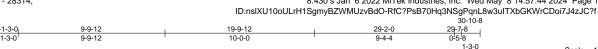
building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job Truss Truss Type Qty Cash/Shirey Residence/Harnett 165454089 J0524-2717 A4 **GABLE** Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Wed May 8 14:57:44 2024 Page 1



Scale = 1:77.0

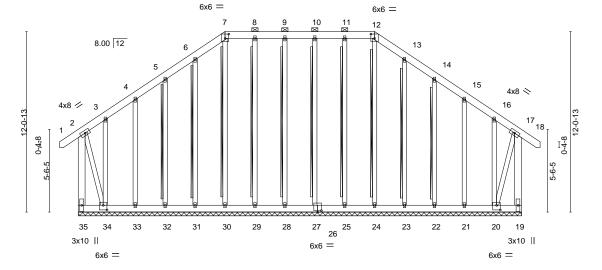


Plate Offsets (X,Y)-- [7:0-3-0,0-3-8], [12:0-3-0,0-3-8], [20:0-3-0,0-3-12], [26:0-3-0,0-1-4], [34:0-3-0,0-3-12]

LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	-0.00	18	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	ВС	0.06	Vert(CT)	-0.00	18	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.29	Horz(CT)	-0.01	19	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-S	, ,					Weight: 386 lb	FT = 25%

LUMBER-BRACING-

TOP CHORD 2x6 SP No.1 TOP CHORD **BOT CHORD** 2x6 SP No.1 2x6 SP No.1 \*Except\* **BOT CHORD WEBS** 

2-34,17-20: 2x4 SP No.2 **WEBS** 

**OTHERS** 2x4 SP No.2 Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (10-0-0 max.): 7-12. Rigid ceiling directly applied or 6-0-0 oc bracing.

2x4 SPF No.2 - 12-24, 11-25, 10-27, 9-28,

8-29, 7-30, 6-31, 5-32, 13-23, 14-22 Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 29-7-8.

Max Horz 35=-446(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 25, 27, 28, 29, 31, 32, 33, 23, 22,

21 except 35=-581(LC 8), 19=-484(LC 9), 34=-593(LC 9), 20=-514(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 24, 25, 27, 28, 29, 30, 31, 32, 33, 23, 22, 21 except 35=645(LC 11), 19=544(LC 10), 34=710(LC 10),

20=627(LC 11)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 5-6=-208/283, 6-7=-258/342, 7-8=-235/317, 8-9=-234/317, 9-10=-234/317,

10-11=-234/317, 11-12=-235/317, 12-13=-258/342, 13-14=-208/283, 2-35=-599/548,

17-19=-502/456

BOT CHORD 34-35=-406/368

WFBS 2-34=-603/575, 17-20=-521/496

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 27, 28, 29, 31, 32, 33, 23, 22, 21 except (jt=lb) 35=581, 19=484, 34=593, 20=514.

OathGraghting representation does not depict the size or the orientation of the purlin along the top and/or bottom chord



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	Cash/Shirey Residence/Harnett
10504.0747		CARLE			I65454089
J0524-2717	A4	GABLE	1	1	Job Reference (ontional)

Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Wed May 8 14:57:44 2024 Page 2

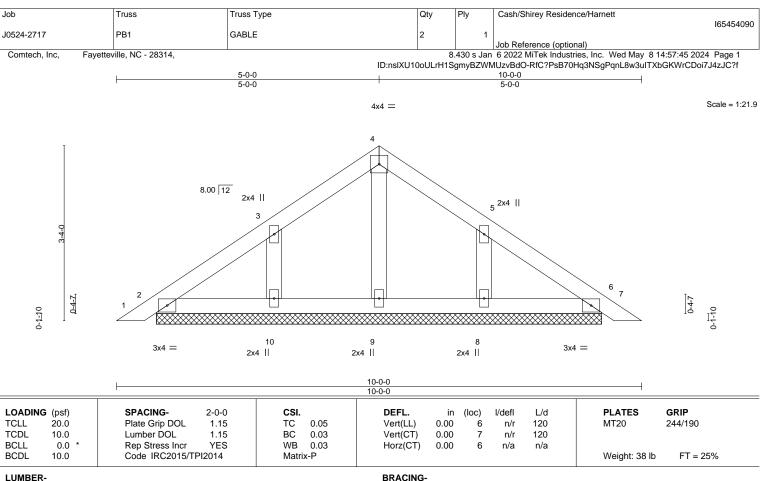
ID:nsIXU10oULrH1SgmyBZWMUzvBdO-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

### NOTES-

13) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



818 Soundside Road Edenton, NC 27932



BOT CHORD

LUMBER-TOP CHORD

2x4 SP No.1 2x4 SP No.1

**BOT CHORD OTHERS** 2x4 SP No.2

REACTIONS. All bearings 8-5-12.

Max Horz 2=-95(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-115(LC 12), 8=-115(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=115. 8=115
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



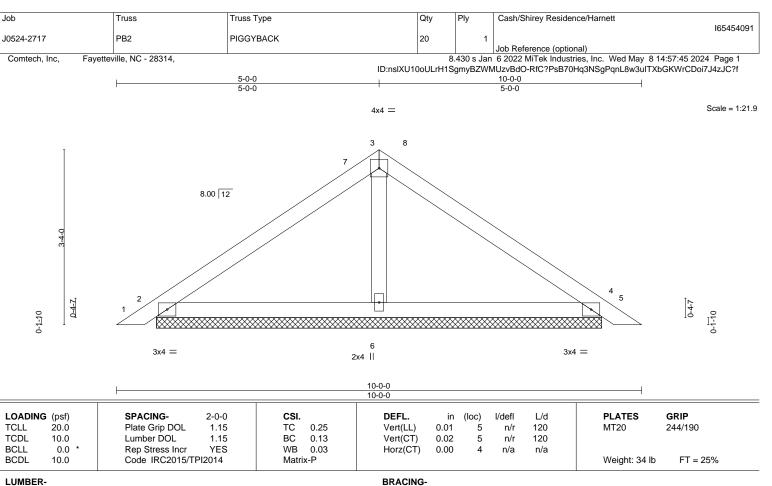
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1 2x4 SP No.1

BOT CHORD **OTHERS** 2x4 SP No.2

> 2=8-5-12, 4=8-5-12, 6=8-5-12 (size) Max Horz 2=-76(LC 10) Max Uplift 2=-37(LC 12), 4=-45(LC 13)

Max Grav 2=216(LC 1), 4=216(LC 1), 6=306(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-2 to 4-7-15, Interior(1) 4-7-15 to 5-0-0, Exterior(2) 5-0-0 to 9-2-14, Interior(1) 9-2-14 to 9-8-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall

building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



b	Truss	Truss Type		Qt	y Ply	Cash	n/Shirey Residen	ce/Harnett		
524-2717	X1	MONOPITCH SUPP	OPTED			1			16	5454092
024-2717	^1	MONOPITCH SUPP	OKTED	'			Reference (option	al)		
Comtech, Inc, Faye	etteville, NC - 28314,					Jan 6 202	22 MiTek Industri	es, Inc. Wed May 8		
				ID:nsIXU10oU	LrH1SgmyB2	WMUzvB	dO-RfC?PsB70F	lq3NSgPqnL8w3uIT>	kbGKWrCDoi7J4z	:JC?f
-1-2-8 1-2-8				10-0-8 10-0-8	3					
1-2-0				10-0-0	)					
									Sca	le = 1:19.4
									3x4	
т									6 7	
								2x4		
								5		
			3.00	12						
					2x4 4	_				
					4					
			2x4							
3-0-3			3							
3-6				ı						
				'						
Т	2									
190								Ů		
Jac										
٠٠١						*****				
	04		11		10			9	8	
	3x4 =		2x4		2x4		2x	4	3x4	
	<u> </u>									
0.4500.0	001000			5==1	. ,	1/1.5		DI 4756		
OADING (psf) CLL 20.0	SPACING- Plate Grip DOL	2-0-0 <b>CSI.</b> 1.15 TC	0.11	<b>DEFL.</b> Vert(LL)	in (loc 0.00			PLATES MT20	<b>GRIP</b> 244/190	
CDL 20.0	Lumber DOL		0.11	Vert(CT)	0.00	l 11/1 l n/r		IVI I ZU	244/190	

Horz(CT) -0.00

n/a

except end verticals.

n/a

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 52 lb

FT = 25%

LUMBER-BRACING-TOP CHORD

YES

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 2x6 SP No.1 **WEBS** 

0.0

10.0

**BOT CHORD** 2x4 SP No.2

REACTIONS. All bearings 10-0-8. Max Horz 2=134(LC 8) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 7, 8, 2, 9, 10, 11

Max Grav All reactions 250 lb or less at joint(s) 7, 8, 2, 9, 10 except 11=307(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Rep Stress Incr

Code IRC2015/TPI2014

### NOTES-

**BCLL** 

**BCDL** 

**OTHERS** 

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WB 0.03

Matrix-S

- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 8, 2, 9, 10, 11.



May 9,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPII Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)







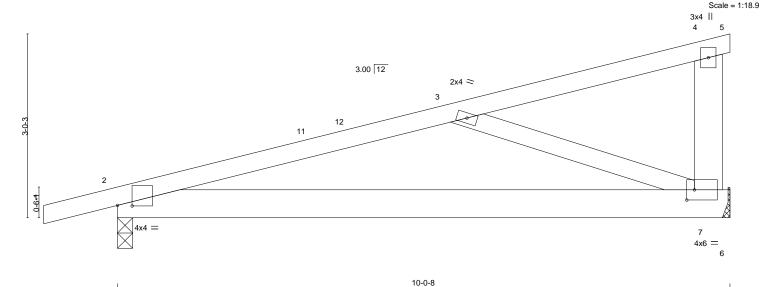


Plate Off	Plate Offsets (X,Y) [2:0-2-14,0-0-2], [7:0-1-8,0-2-0]									
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP						
TCLL	20.0	Plate Grip DOL 1.15	TC 0.23	Vert(LL) -0.05 7-10 >999 360 MT20 244/190						
TCDL	10.0	Lumber DOL 1.15	BC 0.24	Vert(CT) -0.10 7-10 >999 240						
BCLL	0.0 *	Rep Stress Incr YES	WB 0.17	Horz(CT) 0.00 7 n/a n/a						
BCDL	10.0	Code IRC2015/TPI2014	Matrix-AS	Wind(LL) 0.12 7-10 >986 240 Weight: 52 lb FT = 25%						

**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x4 SP No.1 **BOT CHORD** 2x6 SP No.1 WEBS 2x6 SP No.1 \*Except\*

3-7: 2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 7=Mechanical

Max Horz 2=96(LC 8)

Max Uplift 2=-185(LC 8), 7=-159(LC 8) Max Grav 2=464(LC 1), 7=412(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-631/544 **BOT CHORD** 2-7=-633/594 **WEBS** 3-7=-528/491

### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 10-0-8 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=185, 7=159.
- 6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

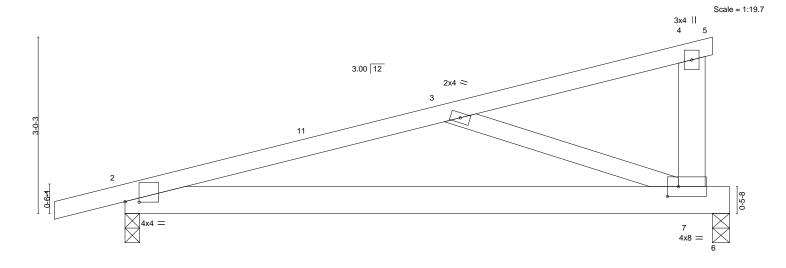
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





Fayetteville, NC - 28314, Comtech, Inc. ID: nslXU10 oULrH1SgmyBZWMUzvBdO-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ff10-0-8 1-2-8 5-8-12 4-3-12



10-4-0 Plate Offsets (X V)-- [2:0-2-14 0-0-2] [7:0-2-4 0-2-0]

TOP CHORD

**BOT CHORD** 

Tidle Offsets (X, T)	Hate Offices (A, 1) [2.0 2 14,0 0 2], [1.0 2 4,0 2 0]								
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP					
TCLL 20.0	Plate Grip DOL 1.15	TC 0.31	Vert(LL) -0.05 7-10 >999 360	MT20 244/190					
TCDL 10.0	Lumber DOL 1.15	BC 0.25	Vert(CT) -0.12 7-10 >999 240						
BCLL 0.0 *	Rep Stress Incr YES	WB 0.17	Horz(CT) 0.01 6 n/a n/a						
BCDL 10.0	Code IRC2015/TPI2014	Matrix-AS	Wind(LL) 0.14 7-10 >902 240	Weight: 53 lb FT = 25%					

LUMBER-**BRACING-**

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x6 SP No.1 \*Except\*

3-7: 2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 6=0-3-8 Max Horz 2=96(LC 8)

Max Uplift 2=-193(LC 8), 6=-160(LC 8) Max Grav 2=484(LC 1), 6=395(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-690/563 **BOT CHORD** 2-7=-645/652 **WEBS** 3-7=-528/431

### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 10-0-8 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=193, 6=160.
- 5) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

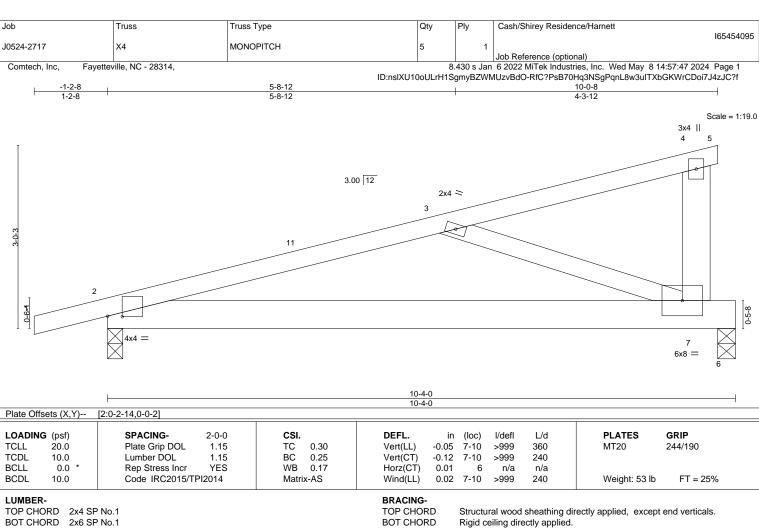
Rigid ceiling directly applied.

May 9,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





LUMBER-

BOT CHORD 2x6 SP No.1 WEBS 2x6 SP No.1 \*Except\* 3-7: 2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 6=0-3-8

Max Horz 2=96(LC 8)

Max Uplift 2=-79(LC 8), 6=-48(LC 12) Max Grav 2=484(LC 1), 6=395(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-690/226 **BOT CHORD** 2-7=-302/652 **WEBS** 3-7=-528/291

### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 10-0-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- 5) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	)	Qt	y Ply	'	Cash/Shirey Res	idence/Harnett	165454096
J0524-2717	X5	MONOPIT	CH SUPPORTED	1		1			105454096
						-	Job Reference (o		
Comtech, Inc, Faye	etteville, NC - 28314,								3 14:57:47 2024 Page 1
-1-2-8				ID:nsIXU10oU 10-0-8		/BZWIN	/IUzvBdO-RfC?Psi	370Hq3NSgPqnL8w3uII	XbGKWrCDoi7J4zJC?f
-1-2-8 1-2-8				10-0-8					
3-0-3	2 3x4 =		2x- 3	3.00 12		10		2x4    5 5 2x4	Scale = 1:19.4 3x4    6 7 8 3x4
	+								
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15	CSI. TC 0.11 BC 0.04	DEFL. Vert(LL) Vert(CT)	in (I 0.00 0.00	oc) 1	l/defl L/d n/r 120 n/r 120	PLATES MT20	<b>GRIP</b> 244/190

**BRACING-**

Horz(CT)

TOP CHORD

-0.00

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Weight: 52 lb

FT = 25%

except end verticals.

n/a

**BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

n/a

REACTIONS. All bearings 10-0-8.

2x4 SP No.1

2x6 SP No.1 2x6 SP No.1

2x4 SP No.2

0.0

10.0

Max Horz 2=134(LC 8) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 7, 8, 2, 9, 10, 11

Max Grav All reactions 250 lb or less at joint(s) 7, 8, 2, 9, 10 except 11=307(LC 1)

YES

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Rep Stress Incr

Code IRC2015/TPI2014

### NOTES-

**BCLL** 

**BCDL** 

**WEBS** 

**OTHERS** 

LUMBER-

TOP CHORD

BOT CHORD

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip

WB

Matrix-S

0.03

- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 8, 2, 9, 10, 11.



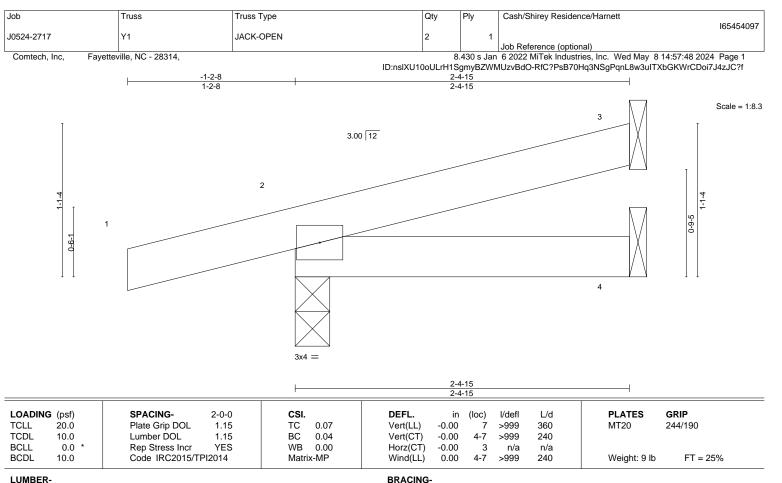




WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPII Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD

2x4 SP No.1

REACTIONS. 3=Mechanical, 2=0-3-0, 4=Mechanical Max Horz 2=34(LC 8)

Max Uplift 3=-19(LC 8), 2=-87(LC 8), 4=-13(LC 9) Max Grav 3=53(LC 1), 2=185(LC 1), 4=40(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.



Structural wood sheathing directly applied or 2-4-15 oc purlins.

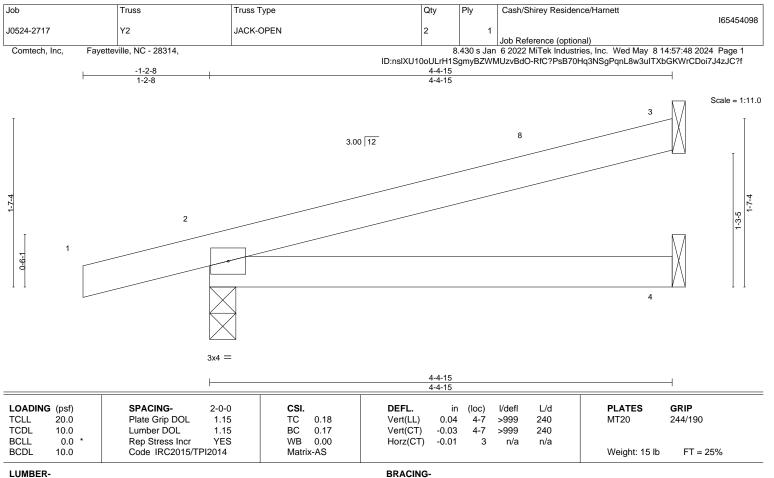
Rigid ceiling directly applied or 10-0-0 oc bracing.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1

**BOT CHORD** 2x4 SP No.1

> 3=Mechanical, 2=0-3-0, 4=Mechanical Max Horz 2=50(LC 8)

Max Uplift 3=-43(LC 8), 2=-111(LC 8), 4=-24(LC 8) Max Grav 3=113(LC 1), 2=257(LC 1), 4=78(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 4-4-3 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=111.
- 6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



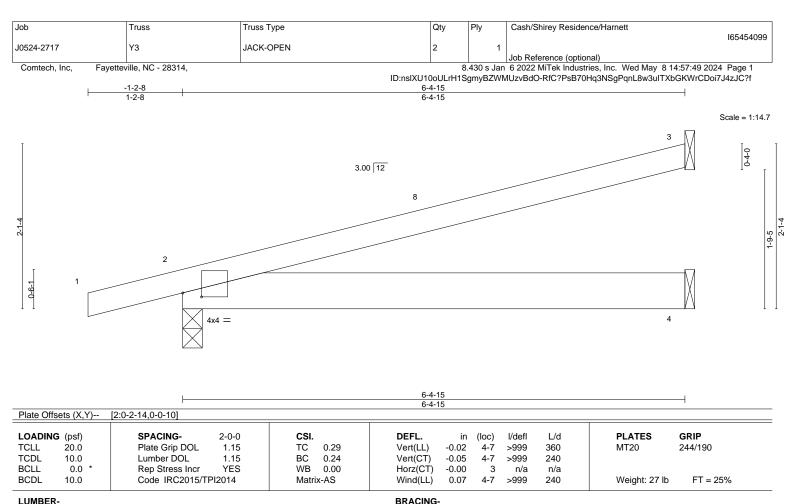


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall

building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





**BOT CHORD** 

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1

(size) 3=Mechanical, 2=0-3-0, 4=Mechanical

Max Horz 2=66(LC 8) Max Uplift 3=-54(LC 8), 2=-138(LC 8), 4=-47(LC 8)

Max Grav 3=142(LC 1), 2=333(LC 1), 4=132(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

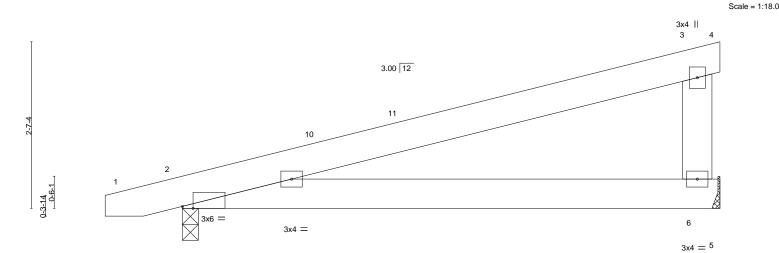
- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 6-4-3 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=138.
- 6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.





Job Truss Truss Type Qty Cash/Shirey Residence/Harnett 165454100 J0524-2717 Υ4 JACK-CLOSED 2 Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Wed May 8 14:57:49 2024 Page 1 ID: nslXU10 oULrH1SgmyBZWMUzvBdO-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ff

8-4-15



8-4-15

TOP CHORD

**BOT CHORD** 

Plate Offsets	(X,Y)	[2:0-2-0,Eage]										
LOADING (p	sf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20	0.0	Plate Grip DOL	1.15	TC	0.28	Vert(LL)	-0.04	6-9	>999	360	MT20	244/190
TCDL 10	.0	Lumber DOL	1.15	BC	0.25	Vert(CT)	-0.10	6-9	>999	240		
BCLL (	.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	2	n/a	n/a		
BCDL 10	.0	Code IRC2015/TP	12014	Matri	x-AS	Wind(LL)	0.12	6-9	>837	240	Weight: 47 lb	FT = 25%

LUMBER-BRACING-

2x6 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 WEBS 2x6 SP No.1

REACTIONS. (size) 6=Mechanical, 2=0-3-0 Max Horz 2=79(LC 8)

1-2-8

Max Uplift 6=-132(LC 8), 2=-150(LC 8) Max Grav 6=348(LC 1), 2=379(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-15 to 3-5-14, Interior(1) 3-5-14 to 8-4-15 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=132, 2=150,
- 6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

May 9,2024

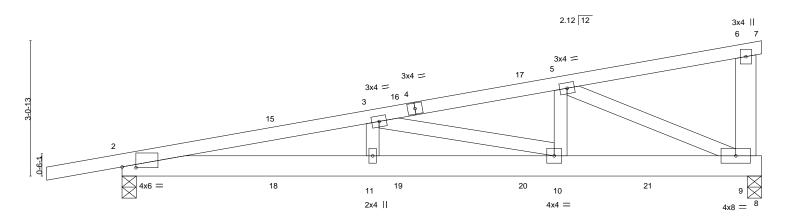
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job Truss Truss Type Qty Cash/Shirey Residence/Harnett 165454101 J0524-2717 **Z**1 ROOF SPECIAL GIRDER Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Wed May 8 14:57:50 2024 Page 1 ID:nsIXU10oULrH1SgmyBZWMUzvBdO-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 9-11-4 1-8-8 5-8-2 4-3-2 4-6-10

Scale = 1:26.1



		-	5-8-2 5-8-2				9-11-4 4-3-2		-	14-5-14 4-6-10	
Plate Offs	ets (X,Y)	[2:0-3-12,0-0-4]	3-0-2				T-3-Z			+0-10	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.31	Vert(LL)	0.12 10-11	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.67	Vert(CT)	-0.17 10-11	>988	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.72	Horz(CT)	0.03	n/a	n/a		
BCDL	10.0	Code IRC2015/Ti	PI2014	Matrix	c-MS	1				Weight: 79 lb	FT = 25%

**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x6 SP No.1
WEBS 2x4 SP No.2 \*Except\*

6-9: 2x6 SP No.1

**REACTIONS.** (size) 2=0-3-14, 9=0-3-13

Max Horz 2=98(LC 19)

Max Uplift 2=-387(LC 4), 9=-528(LC 4) Max Grav 2=924(LC 1), 9=1350(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2839/1114, 3-5=-2109/813

BOT CHORD 2-11=-1155/2774, 10-11=-1155/2774, 9-10=-837/2044 WEBS 3-10=-755/330, 5-10=-346/824, 5-9=-2091/860

### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=387, 9=528.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 20 lb down and 25 lb up at 3-6-0, 20 lb down and 25 lb up at 3-6-0, 48 lb down and 61 lb up at 6-3-15, 48 lb down and 61 lb up at 6-3-15, and 60 lb down and 73 lb up at 9-1-14, and 60 lb down and 73 lb up at 9-1-14 on top chord, and 5 lb down and 26 lb up at 3-6-0, 5 lb down and 26 lb up at 3-6-0, 27 lb down and 51 lb up at 6-3-15, 27 lb down and 51 lb up at 9-1-14, and 320 lb down and 162 lb up at 11-11-13, and 320 lb down and 162 lb up at 11-11-13, and 320 lb down and 162 lb up at 11-11-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

 Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-60, 6-7=-60, 8-12=-20



Structural wood sheathing directly applied or 3-6-11 oc purlins,

Rigid ceiling directly applied or 7-3-4 oc bracing.

except end verticals.

Continued on page 2

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chort Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Cash/Shirey Residence/Harnett
J0524-2717	Z1	ROOF SPECIAL GIRDER	1	1	lob Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc,

8.430 s Jan 6 2022 MiTek Industries, Inc. Wed May 8 14:57:50 2024 Page 2 ID:nsIXU10oULrH1SgmyBZWMUzvBdO-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: 16=-56(F=-28, B=-28) 17=-114(F=-57, B=-57) 18=-4(F=-2, B=-2) 19=-45(F=-23, B=-23) 20=-154(F=-77, B=-77) 21=-640(F=-320, B=-320)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



### Symbols

### PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- <sup>1</sup>/16" from outside edge of truss.

₹

This symbol indicates the required direction of slots in connector plates.

\*Plate location details available in MiTek software or upon request.

### PLATE SIZE

4 × 4

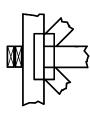
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

### LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

### **BEARING**



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur Min size shown is for crushing only.

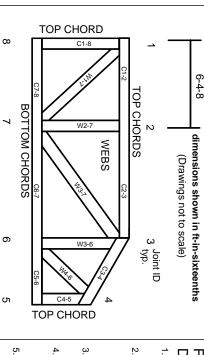
### Industry Standards: ANSI/TPI1: National I

National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing.

Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

DSB-22:

## Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

## Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282 ESR-4722, ESL-1388

## Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

© 2023 MiTek® All Rights Reserved

### MiTek®



MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

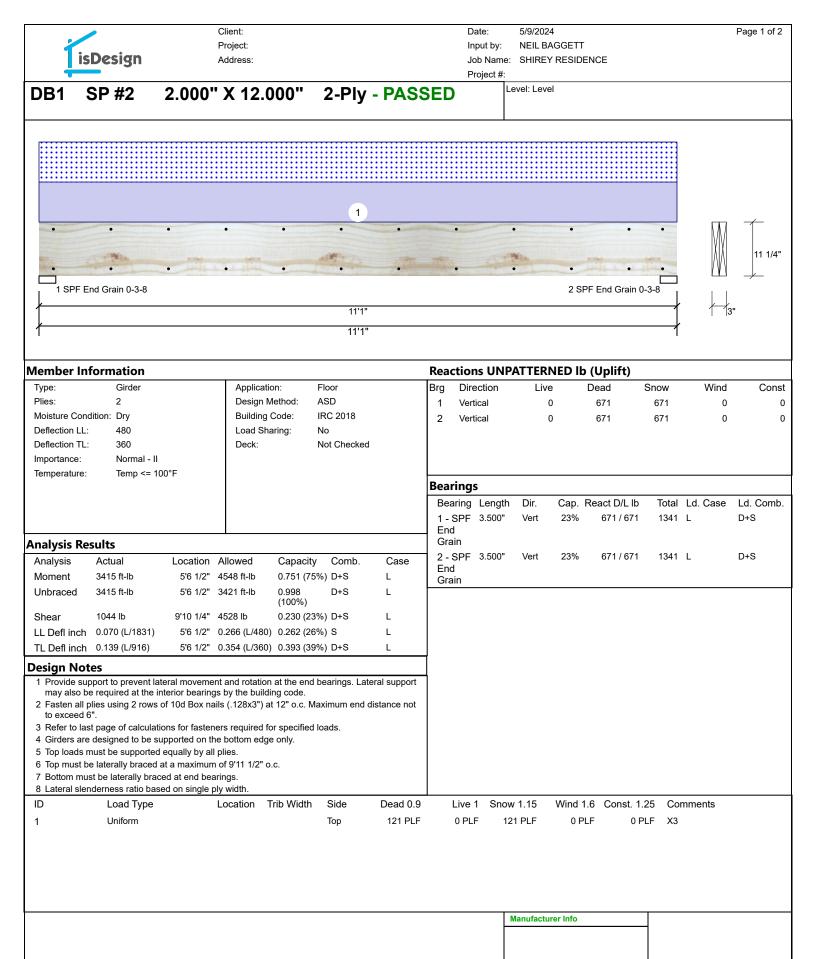
# ▲ General Safety Notes

## Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

œ

- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- The design does not take into account any dynamic or other loads other than those expressly stated.



This design is valid until 6/28/2026

	Client:		Date: 5/9/2024	Page 2 of 2
	Project:		Input by: NEIL BAGGETT	
isDesign	Address:		Job Name: SHIREY RESIDENCE	
<b>—</b>			Project #:	
DB1 SP #2	2.000" X 12.000"	2-Ply - PASSED	Level: Level	
		•		
•	• •	• • •	• • •	<b>₹</b> M 1
				11 1/4"
				<u>V</u> ∭  ′′′′′ <sup>4</sup>
	•			$\overline{\mathcal{X}}$ $\longrightarrow$
1 SPF End Grain 0-3-8			2 SPF End Grain 0-3-8	
<b> </b>		11'1"		3"
<b> </b>				1 1-
		11'1"	1	
Multi-Ply Analysis				
	ours of 10d Pour naile (120-21)	a+ 12" o.c. Massimassus and di	stance not to exceed 6"	
rasten all plies using 2 rd	ows of 10d Box nails (.128x3")	at 12 o.c Maximum end dis	stance not to exceed 6".	
Capacity Load	0.0 % 0.0 PLF			
Yield Limit per Foot	202.6 PLF			
Yield Limit per Fastener	101.3 lb.			
CM	1			
Yield Mode Edge Distance	IV 1 1/2"			
Min. End Distance	3"			
Load Combination				
Duration Factor	1.00			
			Manufacturer Info	
		This design is valid until 6/28/	2026	
Version 23.40.705 Powered by iStruct				