Front Elevation

Scale: 1/4"= 1'0"

Rear Elevation

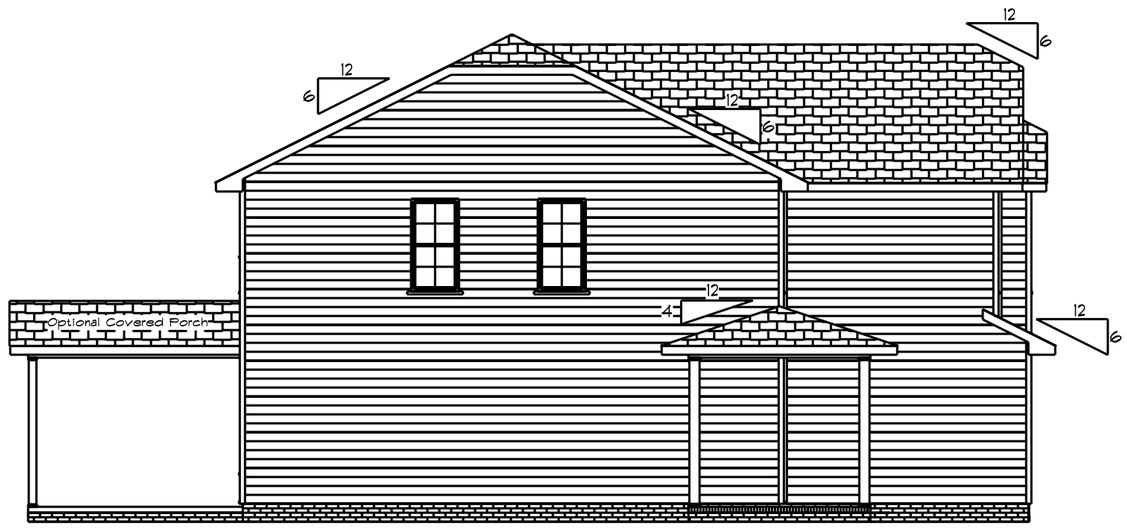
Scale: 1/8"= 1'0"

NOTICE TO CONTRACTOR
All construction must comply with current NC Building Codes and is subject to field inspection and verification.

APPROVED
Limited building only review
Permit holder responsible for full compliance with the code

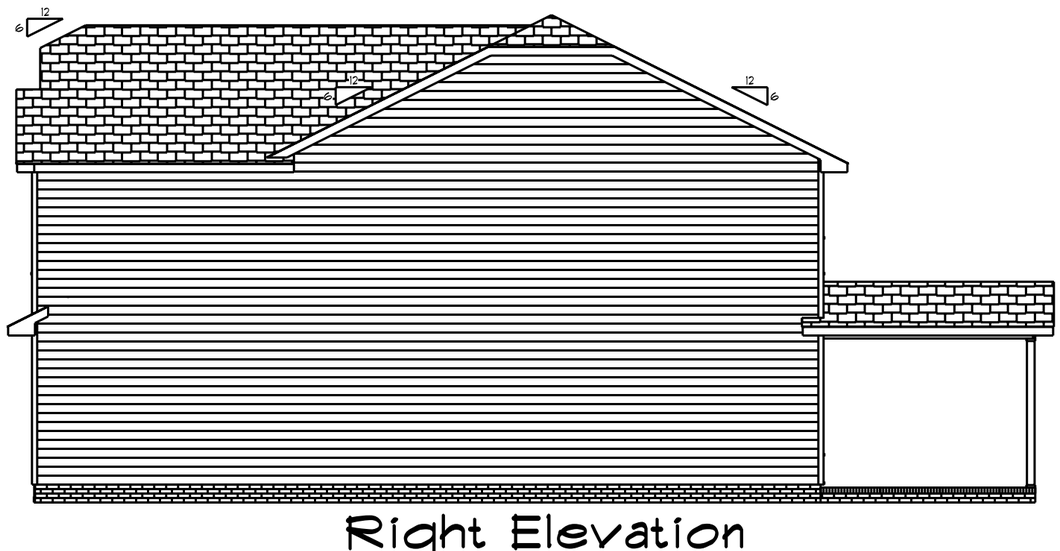
05/13/2024

Harnett COUNTY NORTH CAROLINA



Left Elevation

Scale: 1/8"= 1'0"

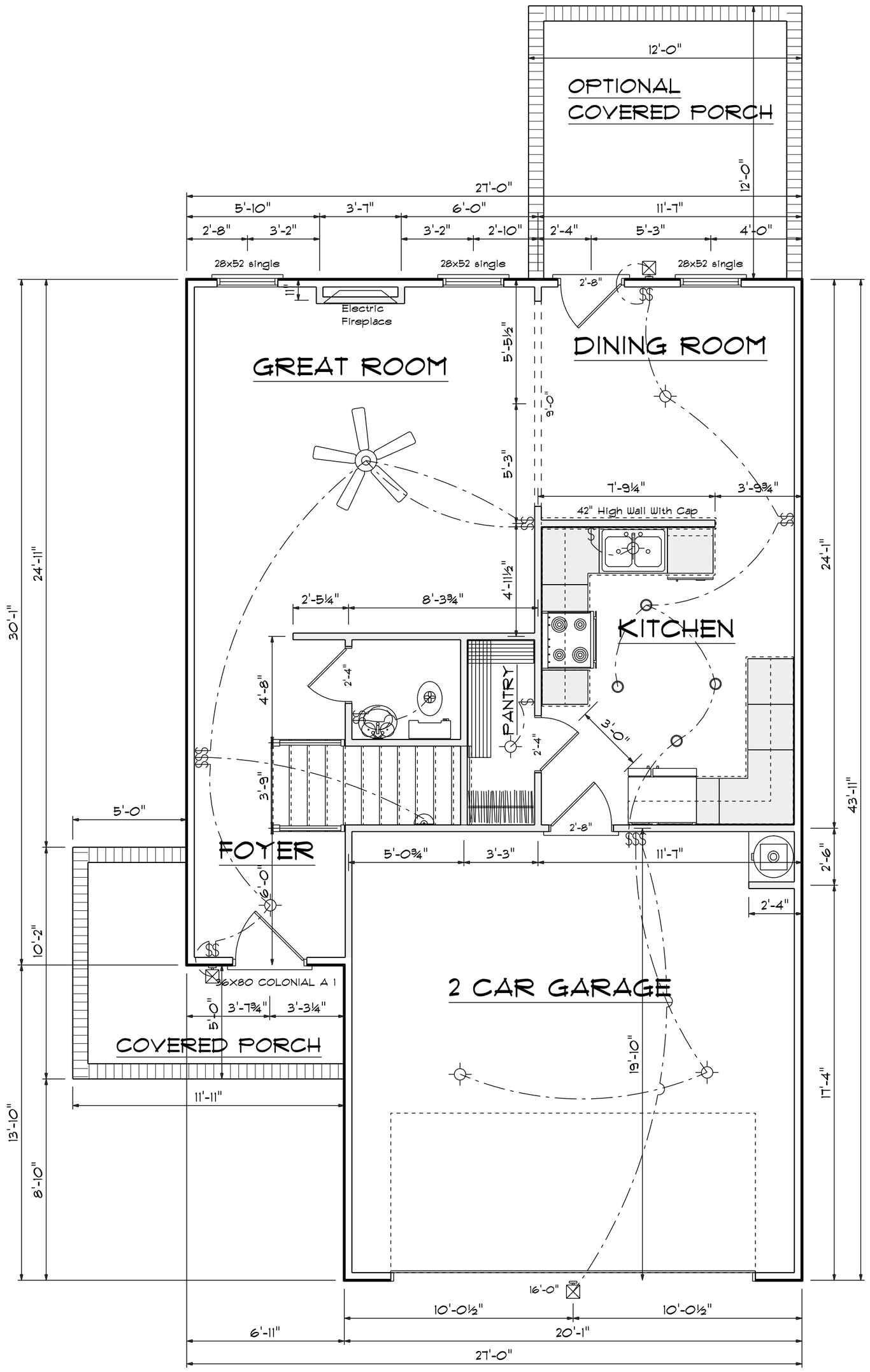


Right Elevation

Scale: 1/8"= 1'0"

Plan #12

SCALE: 1/4"		DATE: 1/4/2024	
DRAWN BY		REVISED	
APPROVED		DRAWING#	



FIRST FLOOR OPENING SCHEDULE				
PRODUCT CODE	SIZE	HINGE	REVERSED	COUNT
2-4 Door Unit	2'-4"	R	NO	2
2-8 Door Unit	2'-8"	L	NO	1
28x52 single	2'-8" x 5'-2"	N	NA	3
32X80 FRENCH A 1	2'-8"	L	NO	1
36X80 COLONIAL A 1	3'-0"	R	NO	1
192X84 - 8 PANEL GARAGE DR	16'-0"	U	NO	1

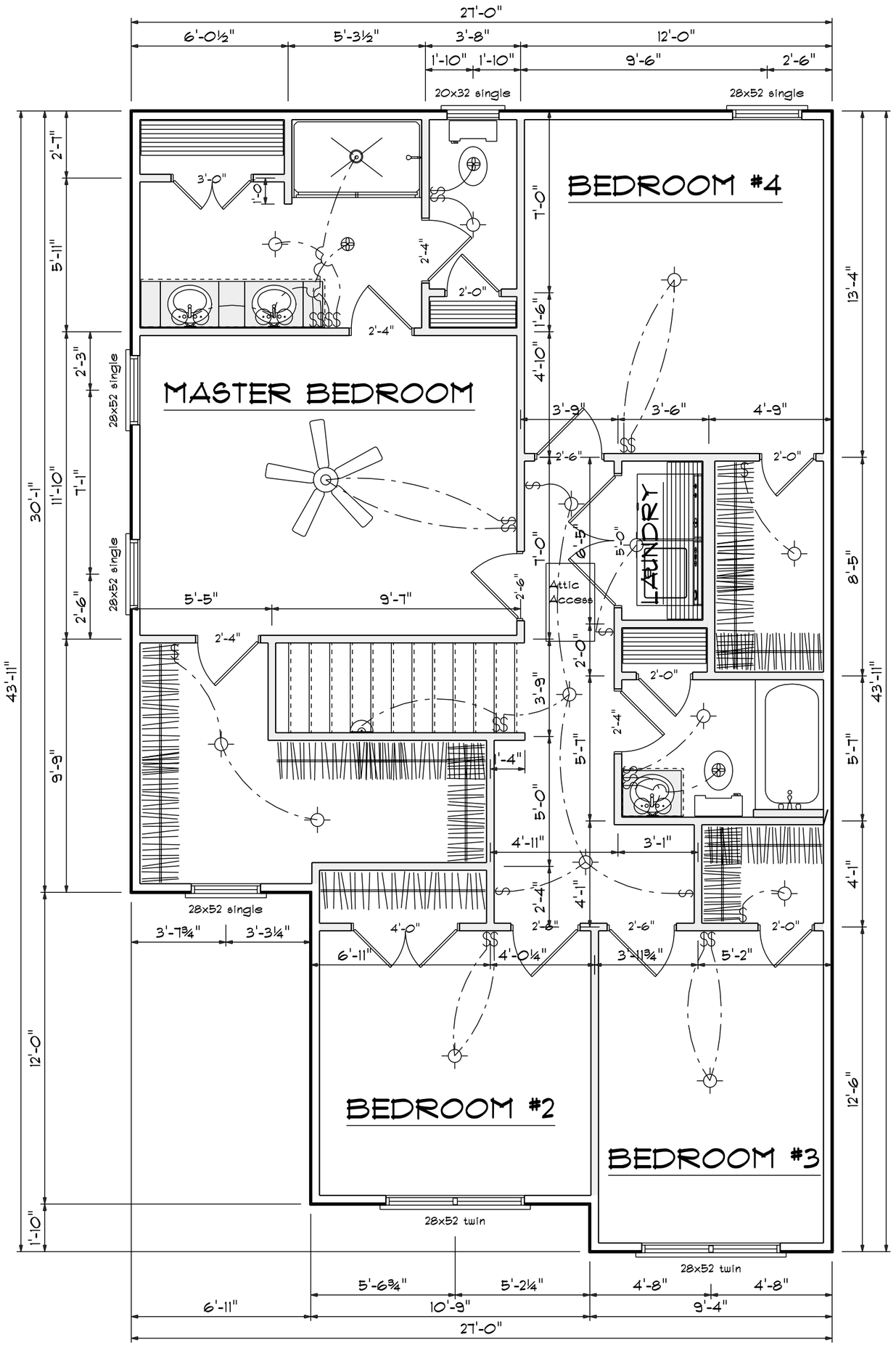
Areas

First Floor	700
Second Floor	1036
	=========
Total Heated	1736
Garage	395
Front Porch	85
Optional Porch	144

First Floor Plan

Scale: 1/4"= 1'-0"

Plan #12	SCALE: 1/4"		DATE: 1/4/2024
	DRAWN BY		REVISED
	APPROVED		DRAWING#



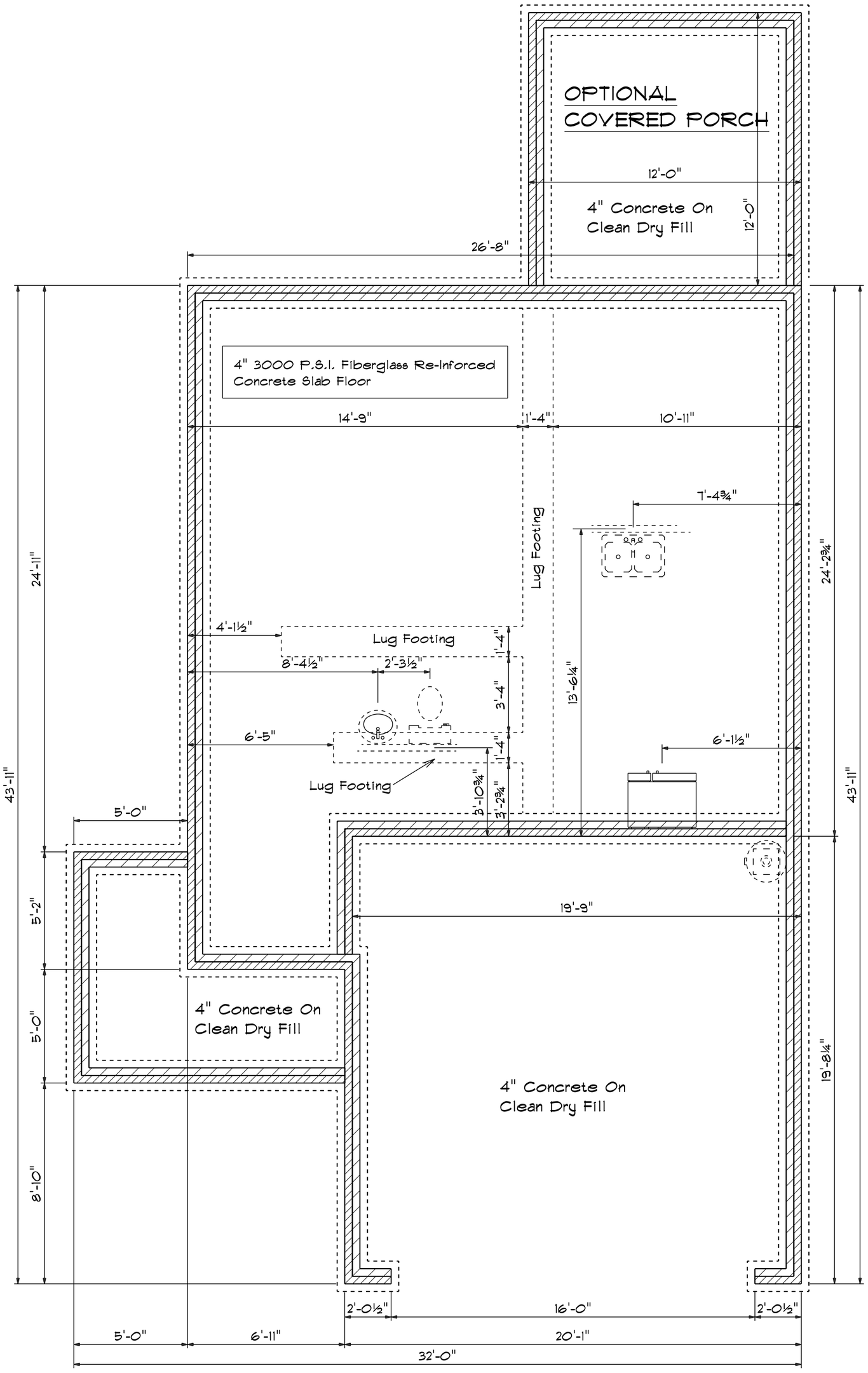
Second Floor Plan

Scale: 1/4"= 1'-0"

SECOND FLOOR OPENING SCHEDULE				
PRODUCT CODE	**SIZE**	**HINGE**	**REVERSED**	**COUNT**
2-0 Door Unit	2'-0"	L	NO	2
2-0 Door Unit	2'-0"	R	NO	2
2-4 Door Unit	2'-4"	L	NO	2
2-4 Door Unit	2'-4"	R	NO	2
2-6 Door Unit	2'-6"	L	NO	3
2-6 Door Unit	2'-6"	R	NO	1
3-0 Doublehung Door Unit	3'-0"	LR	NO	1
4-0 Doublehung Door Unit	4'-0"	LR	NO	1
5-0 Doublehung Door Unit	5'-0"	LR	NO	1
20x32 single	2'-0" x 3'-2"	N	NA	1
28x52 single	2'-8" x 5'-2"	N	NA	4
28x52 twin	5'-4" x 5'-2"	NN	NA	2

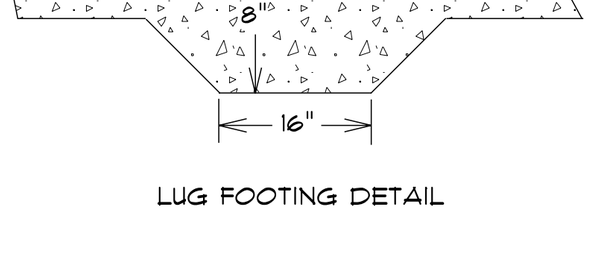
Plan #12

SCALE: 1/4"		DATE: 1/4/2024	
DRAWN BY		REVISED	
APPROVED		DRAWING#	

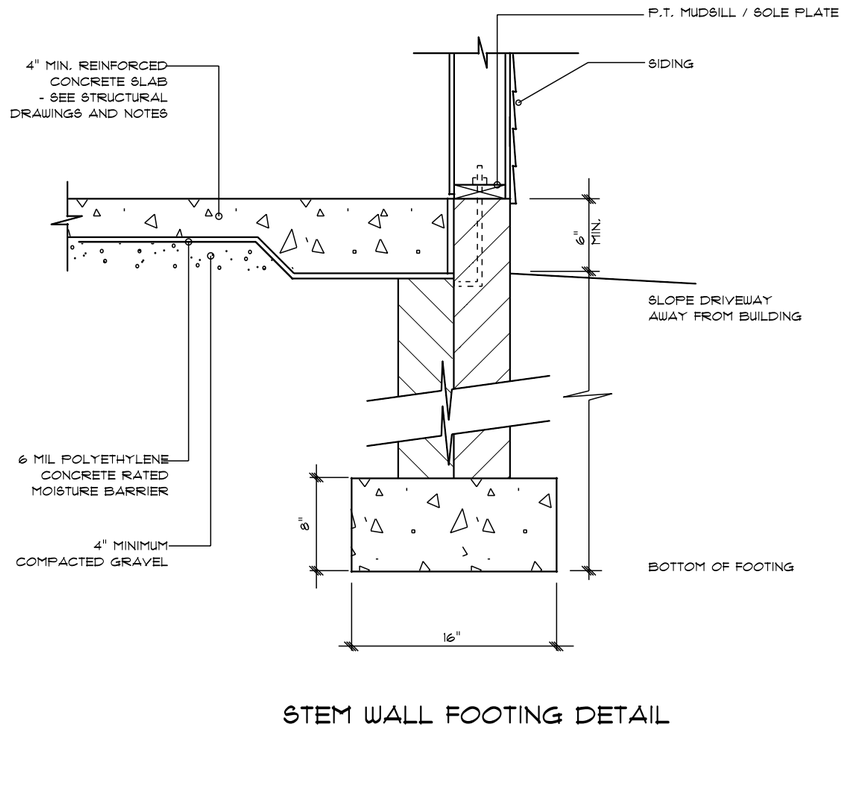


Foundation Plan

Scale: 1/4"= 1'-0"

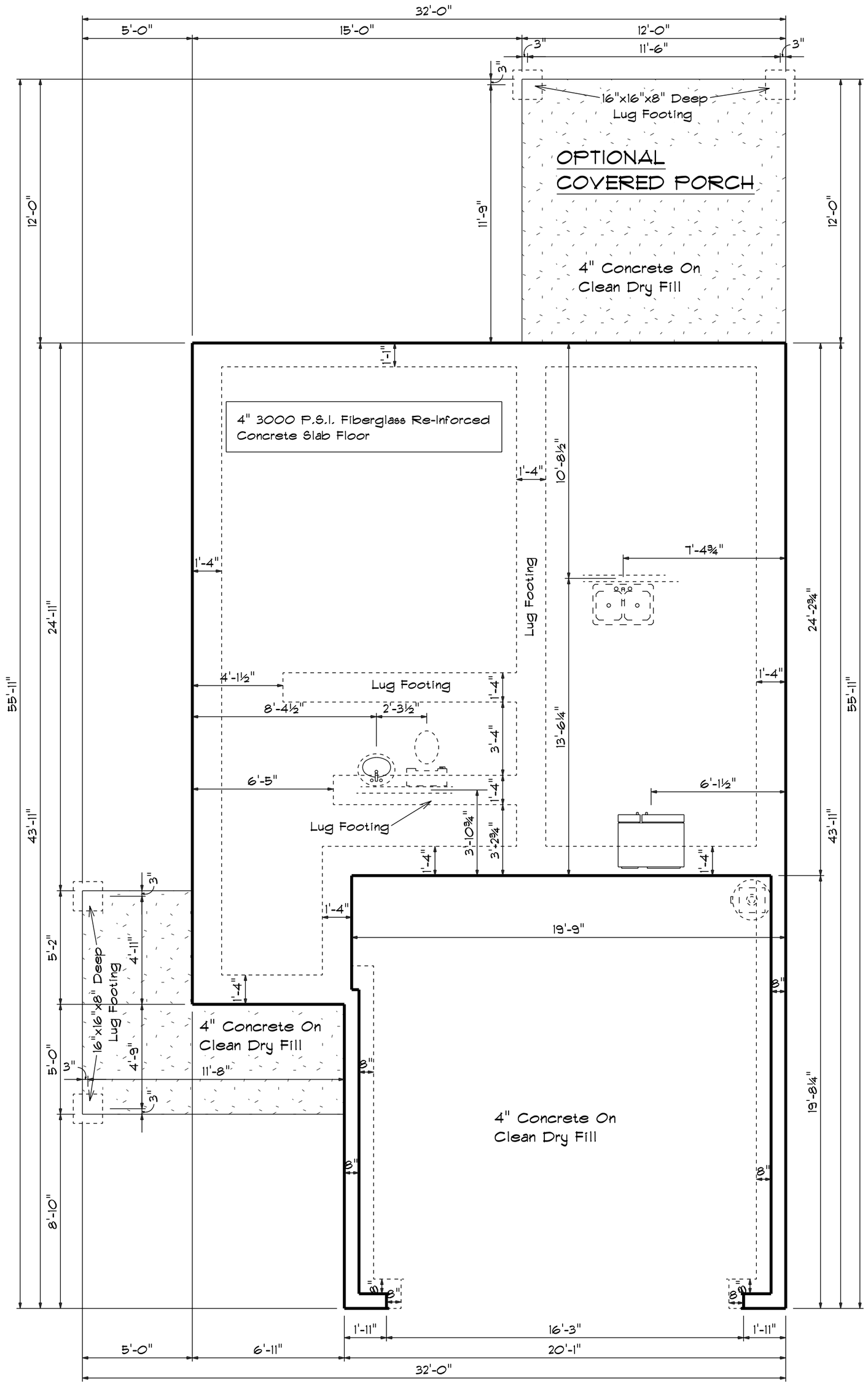


LUG FOOTING DETAIL



STEM WALL FOOTING DETAIL

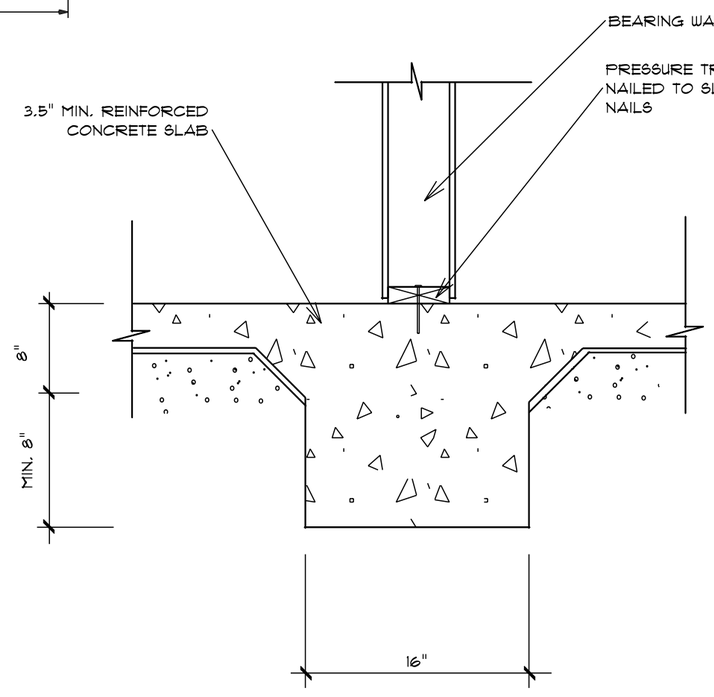
Plan #12	SCALE: 1/4"		DATE: 1/4/2024
	DRAWN BY		REVISED
	APPROVED		DRAWING#



Foundation Plan

Scale: 1/4"= 1'-0"

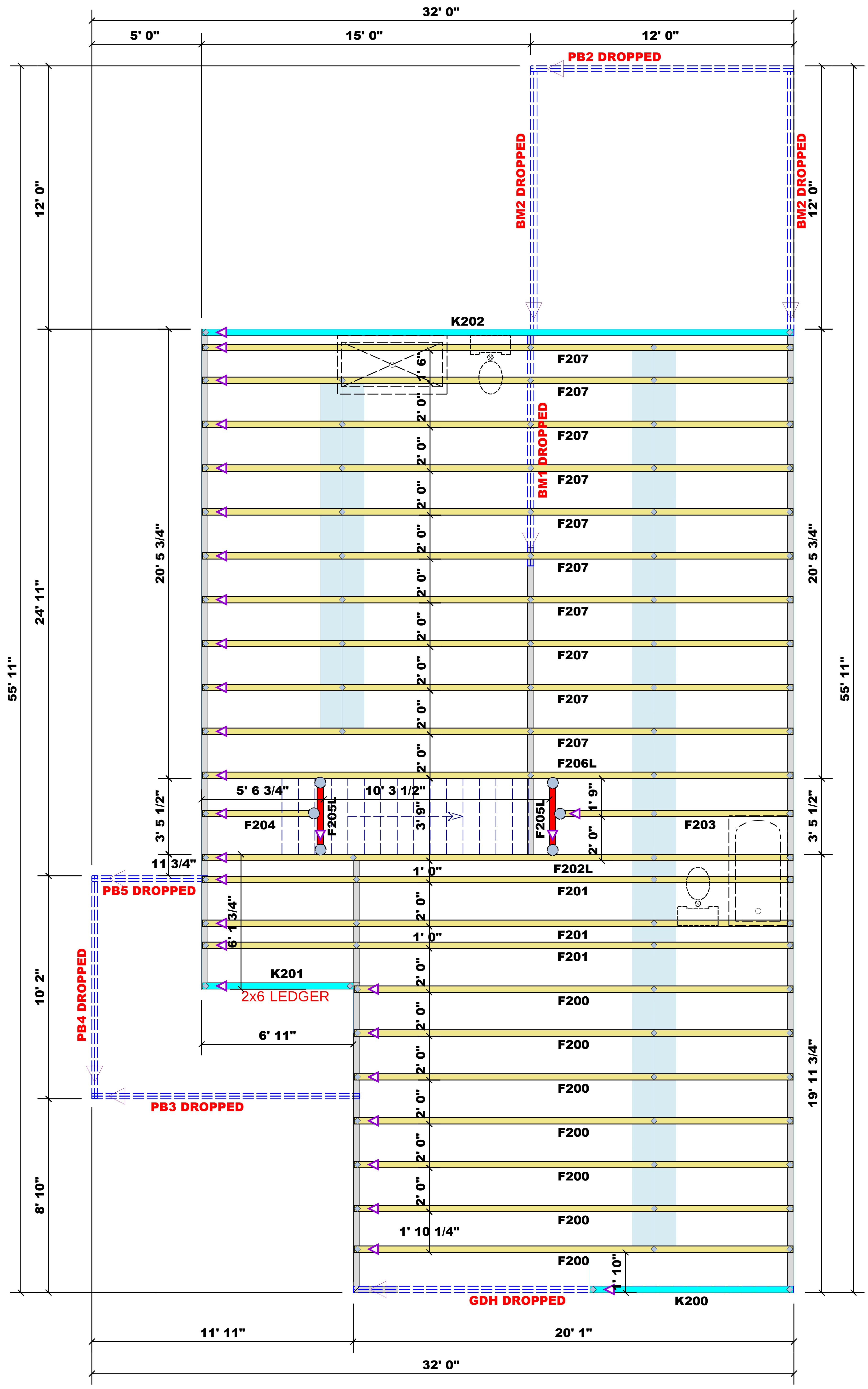
TURN-DOWN
FOOTING DETAIL



INTEGRAL SLAB FOOTING
DETAIL AT BEARING WALL

Plan #12

SCALE: 1/4"		DATE: 1/4/2024	
DRAWN BY		REVISED	
APPROVED		DRAWING#	



Connector Information					Nail Information	
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
◯	MSH422	USP	6	Varies	10d/3"	10d/3"

GENERAL NOTES
1. ◯ AVOID ALL PLUMBING DROP LOCATIONS
2. PB SERIES BEAMS ARE PROVIDED BY OTHERS

Products				
PlotID	Length	Product	Plies	Net Qty
BM2 DROPPED	13' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	4
BM1 DROPPED	11' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
GDH DROPPED	21' 0"	1-3/4"x 14" LVL Kerto-S	2	2
PB3 DROPPED	14' 0"	2x10 SPF No.2	2	2
PB2 DROPPED	12' 0"	2x10 SPF No.2	2	2
PB4 DROPPED	12' 0"	2x10 SPF No.2	2	2
PB5 DROPPED	6' 0"	2x10 SPF No.2	2	2

△ = Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards

Truss Placement Plan
SCALE: NTS

LOAD CHART FOR JACK STUDS
(BASED ON TABLES R502.5(1) & (b))
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (4) PLY HEADER
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				

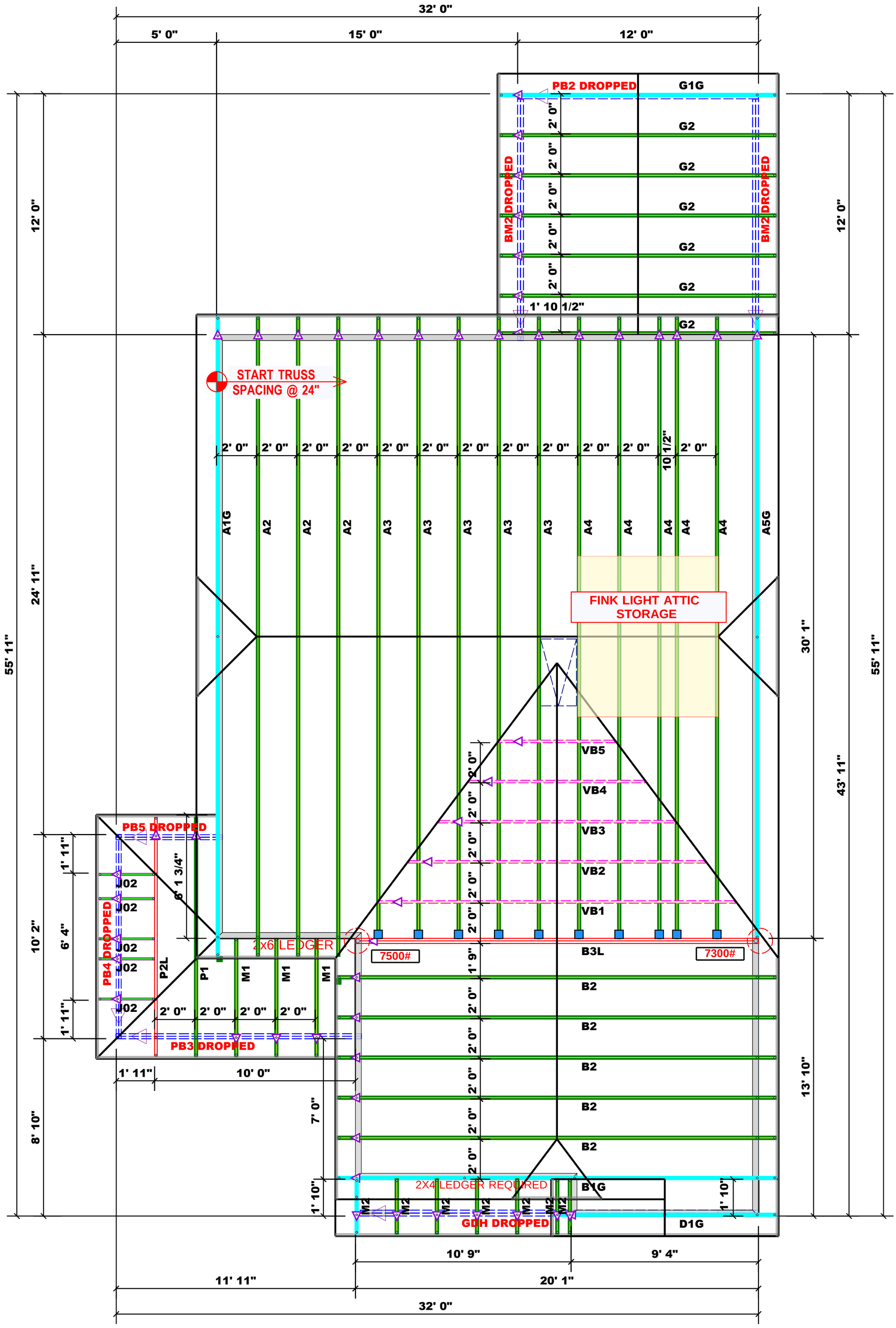
BUILDER	Wellco Construction	CITY / CO.	Harnett County / Harnett
JOB NAME	Lot 4 Overhills Creek 2ND FL	ADDRESS	340 Caldwell Street
PLAN	Plan #12	MODEL	FLOOR
SEAL DATE	Seal Date	DATE REV.	/ /
QUOTE #	B0424-1916	DRAWN BY	Michael Turner
JOB #	J0424-1916	SALES REP.	Lenny Norris

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature Michael Turner
Michael Turner

COMTECH
ROOF & FLOOR TRUSSES & BEAMS
Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444



Connector Information					Nail Information	
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
■	HUS26	USP	10	NA	16d/3-1/2"	16d/3-1/2"

GENERAL NOTES
1. FLAS IN A4 TRUSSES
2. M2 TRUSSES REQUIRE LEDGER

Truss Placement Plan
SCALE: NTS

▲ = Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards

LOAD CHART FOR JACK STUDS
(BASED ON TABLES R502.5(1) & (b))
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (4) PLY HEADER
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				

BUILDER	Wellco Construction	**CITY / CO.**	Harnett County / Harnett
JOB NAME	Lot 4 Overhills Creek	**ADDRESS**	340 Caldwell Street
PLAN	Plan #12	**MODEL**	Roof
SEAL DATE	Seal Date	**DATE REV.**	04/03/24
QUOTE #	B0324-1878	**DRAWN BY**	Michael Turner
JOB #	J0324-1878	**SALES REP.**	Lenny Norris

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
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Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

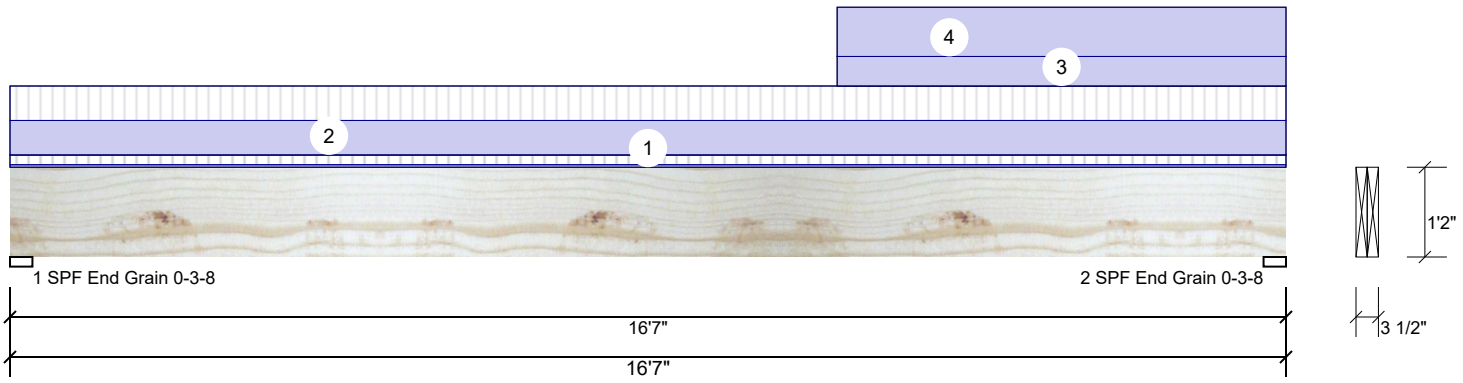
Signature Michael Turner
Michael Turner

COMTECH
ROOF & FLOOR TRUSSES & BEAMS
Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

Client:		Date:	4/4/2024
Project:		Input by:	
Address:		Job Name:	
		Project #:	

GDH DROPPED Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal - II		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	1493	1646	0	0	0
2	Vertical	1493	2889	0	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	30%	1646 / 1493	3138	L	D+L
2 - SPF End Grain	3.500"	Vert	43%	2889 / 1493	4382	L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	13735 ft-lb	9'2 1/2"	26999 ft-lb	0.509 (51%)	D+L	L
Unbraced	13735 ft-lb	9'2 1/2"	13754 ft-lb	0.999 (100%)	D+L	L
Shear	3430 lb	15'1 1/2"	10453 lb	0.328 (33%)	D+L	L
LL Defl inch	0.185 (L/1047)	8'3 9/16"	0.403 (L/480)	0.458 (46%)	L	L
TL Defl inch	0.437 (L/442)	8'6 3/16"	0.538 (L/360)	0.814 (81%)	D+L	L

Design Notes

1. Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
2. Girders are designed to be supported on the bottom edge only.
3. Multiple plies must be fastened together as per manufacturer's details.
4. Top loads must be supported equally by all plies.
5. Top must be laterally braced at a maximum of 7'4 13/16" o.c.
6. Bottom must be laterally braced at end bearings.
7. Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform		1-0-0	Top	10 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
2	Uniform			Top	140 PLF	140 PLF	0 PLF	0 PLF	0 PLF	M2 TRUSSES
3	Part. Uniform	10-9-0 to 16-7-0		Top	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
4	Part. Uniform	10-9-0 to 16-7-0		Top	200 PLF	0 PLF	0 PLF	0 PLF	0 PLF	ROOF TRUSS
	Self Weight				11 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

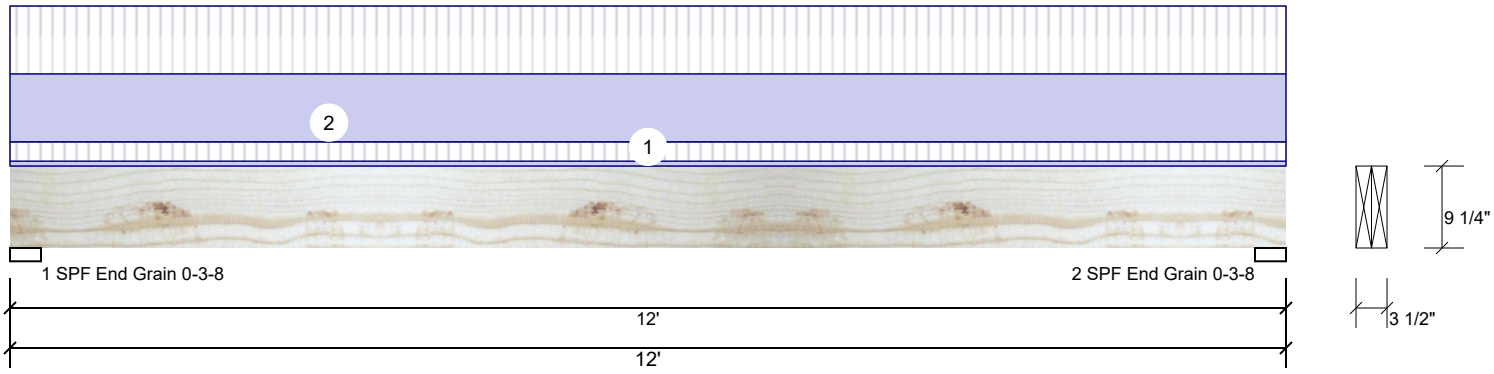
Manufacturer Info

Metsä Wood
301 Merritt 7 Building, 2nd Floor
Norwalk, CT 06851
(800) 622-5850
www.metsawood.com/us

Client:		Date:	4/4/2024
Project:		Input by:	
Address:		Job Name:	
		Project #:	

BM2 DROPPED Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal - II		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	1080	943	0	0	0
2	Vertical	1080	943	0	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	20%	943 / 1080	2023	L	D+L
2 - SPF End Grain	3.500"	Vert	20%	943 / 1080	2023	L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5615 ft-lb	6'	12542 ft-lb	0.448 (45%)	D+L	L
Unbraced	5615 ft-lb	6'	6498 ft-lb	0.864 (86%)	D+L	L
Shear	1670 lb	10'11 1/4"	6907 lb	0.242 (24%)	D+L	L
LL Defl inch	0.166 (L/833)	6'	0.289 (L/480)	0.576 (58%)	L	L
TL Defl inch	0.312 (L/445)	6'	0.385 (L/360)	0.810 (81%)	D+L	L

Design Notes

1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
2 Girders are designed to be supported on the bottom edge only.
3 Multiple plies must be fastened together as per manufacturer's details.
4 Top loads must be supported equally by all plies.
5 Top must be laterally braced at end bearings.
6 Bottom must be laterally braced at end bearings.
7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform		1-0-0	Top	10 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
2	Uniform			Top	140 PLF	140 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				7 PLF					

Notes

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Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info

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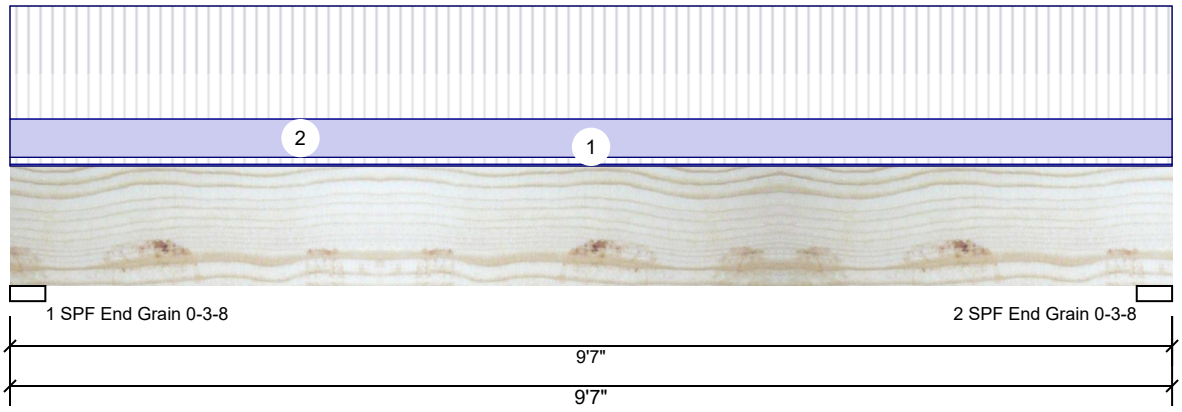
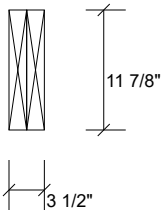
301 Merritt 7 Building, 2nd Floor
Norwalk, CT 06851
(800) 622-5850
www.metsawood.com/us

Client:		Date:	4/4/2024
Project:		Input by:	
Address:		Job Name:	
		Project #:	

BM1 DROPPED Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level





Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal - II		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	3306	1146	0	0	0
2	Vertical	3306	1146	0	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	43%	1146 / 3306	4453	L	D+L
2 - SPF End Grain	3.500"	Vert	43%	1146 / 3306	4453	L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9672 ft-lb	4'9 1/2"	19911 ft-lb	0.486 (49%)	D+L	L
Unbraced	9672 ft-lb	4'9 1/2"	9964 ft-lb	0.971 (97%)	D+L	L
Shear	3271 lb	8'3 5/8"	8867 lb	0.369 (37%)	D+L	L
LL Defl inch	0.130 (L/842)	4'9 1/2"	0.228 (L/480)	0.570 (57%)	L	L
TL Defl inch	0.175 (L/625)	4'9 1/2"	0.304 (L/360)	0.576 (58%)	D+L	L

Design Notes

1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
2 Girders are designed to be supported on the bottom edge only.
3 Multiple plies must be fastened together as per manufacturer's details.
4 Top loads must be supported equally by all plies.
5 Top must be laterally braced at end bearings.
6 Bottom must be laterally braced at end bearings.
7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform		1-0-0	Top	10 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
2	Uniform			Top	220 PLF	650 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				9 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

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2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

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