

RE: 24-2117-A RVF-LOT #4 ROOF Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: 24-2117-A Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2018/TPI2014 Wind Code: ASCE 7-16 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.5 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 32 individual, dated Truss Design Drawings and 0 Additional Drawings.

No	Seal#	Truss Name	Date	No	Seal#	Truss Name	Date
1	164650598	C 101	4/3/2024	21	164650618	T06	4/3/2024
2	16/650500	HG01	4/3/2024	27	16/650610	T07	4/3/2024
2	164650600	101	4/3/2024	22	164650620	V01	4/3/2024
1	164650601	M01	4/3/2024	20	164650621	V01 V02	4/3/2024
- 1 5	104050001	MO2	4/3/2024	24	104030021	V02 V02	4/3/2024
5	104030002	MO2	4/3/2024	20	104050022	V03	4/3/2024
0	104030003		4/3/2024	20	104030023	V04 V05	4/3/2024
1	164650604	PB01	4/3/2024	27	164650624	V05	4/3/2024
8	164650605	PB01GE	4/3/2024	28	164650625	V06	4/3/2024
9	164650606	PB02	4/3/2024	29	164650626	V07	4/3/2024
10	164650607	T01GE	4/3/2024	30	164650627	V08	4/3/2024
11	164650608	T02G	4/3/2024	31	164650628	V09	4/3/2024
12	164650609	T02SGE	4/3/2024	32	164650629	V10	4/3/2024
13	164650610	T03	4/3/2024				
14	164650611	T03A	4/3/2024				
15	164650612	T03GE	4/3/2024				
16	164650613	T04	4/3/2024				
17	164650614	T04A	4/3/2024				
18	164650615	T05	4/3/2024				
19	164650616	T05GE	4/3/2024				

4/3/2024

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

T05S

based on the parameters provided by Riverside Roof Truss.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2024

North Carolina COA: C-0844

164650617

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IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric







and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #4 ROOF	
					10	64650599
24-2117-A	HG01	HIP GIRDER	1	2		
				-	Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.530 s Au	g 2 2023 MiTek Industries, Inc. Tue Apr 2 17:09:25 2024 F	Page 2

ID:tdHS5IWyLng?jaR9E1eBtqyIy9_-hA3sONjFcuxravOFqmvcAhXBDlq9?f3nh7VOZOzUhhe

NOTES-

14) Use Simpson Strong-Tie THJA26 (THJA26 on 2 ply, Left Hand Hip) or equivalent at 4-0-6 from the left end to connect truss(es) to front face of bottom chord.

15) Use Simpson Strong-Tie THJA26 (THJA26 on 2 ply, Right Hand Hip) or equivalent at 7-11-10 from the left end to connect truss(es) to front face of bottom chord. 16) Fill all nail holes where hanger is in contact with lumber.
17) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-43, 2-3=-43, 3-4=-53, 4-5=-43, 5-6=-43, 7-10=-20

Concentrated Loads (lb)

Vert: 9=-338(F) 8=-338(F) 13=-134(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCEL Building Component Schut Information, purplication for the trust structure Bucking Component Advancement and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





BCI 0.0 * Rep Stress Incr YES WB 0.03 Hol2(CT) -0.00 3 h/a h/a	LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL	sf) 20.0 11.6/15.0 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.08 BC 0.03 WB 0.03	DEFL. Vert(LL) -0. Vert(CT) -0. Horz(CT) -0.	in (loc) .00 6 .00 5-6 .00 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL 0.0 Code IRC2018/TPI2014 Matrix-MP Weight: 11 lb FT = 20%	BCDL	10.0	Code IRC2018/TPI2014	Matrix-MP					Weight: 11 lb	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 1-11-11 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 6=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 6=67(LC 16) Max Uplift 6=-20(LC 16), 3=-10(LC 13), 4=-14(LC 16)

Max Grav 6=159(LC 21), 3=37(LC 21), 4=36(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 3, 4.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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April 3,2024

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			4-0-0			
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.26 BC 0.16 WB 0.06 Matrix-MP	DEFL. in (loc) Vert(LL) -0.01 4-5 Vert(CT) -0.02 4-5 Horz(CT) -0.00 4	l/defl L/d >999 240 >999 180 n/a n/a	PLATES GRIP MT20 244/190 Weight: 24 lb FT = 20	۵%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-

 TOP CHORD
 Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 4=Mechanical Max Horz 5=97(LC 13) Max Uplift 5=-39(LC 16), 4=-26(LC 13)

Max Grav 5=221(LC 2), 4=155(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 3-10-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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818 Soundside Road



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- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 Coblecture spaced at 2.0.0 ac
- 10) Gable studs spaced at 2-0-0 oc.11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 20.0ps on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 12, 11.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

SEAL 036322 April 3,2024

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and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #4 ROOF	
					16465	0608
24-2117-A	T02G	COMMON GIRDER	1	2		
				5	Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.530 s Au	g 2 2023 MiTek Industries, Inc. Tue Apr 2 17:09:31 2024 Page	2
		ID:tdF	-IS5IWyLn	q?jaR9E1e	Btgyly9 -WKQ8fQo0Ckh lgrOA100Pym84AnRPJOf32yin1zUhh	Y

NOTES-

13) Use Simpson Strong-Tie LUS26 (4-10d Girder, 3-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 13-5-4 from the left end to 17-5-4 to connect truss(es) to front face of bottom chord.

14) Fill all nail holes where hanger is in contact with lumber.
15) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-43, 5-9=-43, 10-16=-20

Concentrated Loads (lb)

Vert: 13=-246(F) 11=-132(F) 19=-501(F) 20=-501(F) 21=-501(F) 22=-499(F) 24=-686(F) 26=-686(F) 27=-246(F) 29=-246(F) 31=-133(F) 33=-132(F) 34=-132(F) 34=-132(F) 32=-132(F) 32=-1 35=-132(F)

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Max Grav 15=466(LC 34), 13=1559(LC 28), 10=719(LC 29)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-362/68, 3-5=0/372, 7-8=-746/78, 2-15=-394/108, 8-10=-619/111
- BOT CHORD 14-15=-104/461, 13-14=-111/271, 11-13=0/544, 10-11=-66/284
- WEBS 5-13=-569/37, 7-13=-823/118, 7-11=0/396, 3-13=-629/143, 8-11=0/303

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=30ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 14-9-0, Exterior(2R) 14-9-0 to 17-9-0, Interior(1) 17-9-0 to 30-5-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

All plates are 2x4 MT20 unless otherwise indicated. 7)

8) Gable studs spaced at 2-0-0 oc.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 13, 10. 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



April 3,2024

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- 9) Refer to girder(s) for truss to truss connections.
- 10) Bearing at joint(s) 17 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14 except (it=lb) Continue 10501 7age72

April 3,2024

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818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #4 ROOF	
						164650610
24-2117-A	103	PIGGYBACK BASE	3	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.530 s Au	ig 2 2023 MiTek Industries, Inc. Tue Apr 2 17:09:33 2024	Page 2
		ID:tdF	IS5IWyLn	g?jaR9E1e	eBtqyly9SiYu46pGkLxiY7?nIR2UUNsPS_Qqt71yXMRprw	zUhhW

NOTES-

12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Job	Truss	Truss Type	Qty	Ply	RVF-LOT #4 ROOF	
04 0447 4	TODA		1	1	16	64650611
24-2117-A	103A	PIGGYBACK BASE	1	1	lob Reference (ontional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.530 s Au	g 2 2023 MiTek Industries, Inc. Tue Apr 2 17:09:34 2024 Pa	age 2
		ID:tdHS	5lWyLng?	aR9E1eBt	qyly9wv6GHSqvVf3ZAHazs9Zj1aOZCOm3caH6l0AMNMzL	JhhV

NOTES-

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





		58-7-8						
		58-7-8	4 0 0 4 01					
Plate Offsets (X, Y) [14:0-3-	0,0-4-0], [19:0-4-0,0-4-8], [23:0-3-0,0-4-0]], [43:0-4-0,0-4-8], [51:0-4	4-0,0-4-8]					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr. YES	CSI. TC 0.14 BC 0.05 WB 0.17	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (lc -0.00 0.00 0.01	oc) l/defl 1 n/r 1 n/r 35 n/a	L/d 120 120 p/a	PLATES MT20	GRIP 244/190
BCLL 0.0 *	Code IRC2018/TPI2014	Matrix-S		0.01	00 II/a	n/a	Weight: 588 lb	FT = 20%
BCDL 10.0							_	
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3 SLIDER Left 2x4 SP No	.3 1-6-4	BF TC BC W	R ACING- DP CHORD DT CHORD EBS	Structural w except end v Rigid ceiling 1 Row at mi	ood sheathin verticals, and directly appl dpt	g directly app 2-0-0 oc pur lied or 10-0-0 23-45, 22 17-51, 16 24-44, 25	blied or 6-0-0 oc purlins lins (6-0-0 max.): 14-2: oc bracing. -46, 21-47, 20-48, 19-4 -52, 15-53, 14-54, 13-5 -43	s, 3. 19, 18-50, 15, 12-56,
 Reactions. All bearings 58-7-8. (lb) - Max Horz 2=233(LC 15) Max Uplift All uplift 100 lb or less at joint(s) 2, 46, 47, 48, 49, 50, 51, 52, 55, 56, 58, 59, 60, 61, 62, 63, 64, 44, 43, 42, 41, 40, 39, 38, 37, 36 Max Grav All reactions 250 lb or less at joint(s) 35, 2, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 58, 59, 60, 61, 62, 63, 64, 44, 43, 42, 41, 40, 39, 38, 37, 36 								
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 11-12=-120/251, 12-13=-123/298, 13-14=-137/336, 14-15=-123/322, 15-16=-123/322, 16-17=-123/322, 17-18=-122/321, 18-19=-122/321, 19-20=-122/321, 20-21=-122/321, 20-21=-122/321, 21-22=-122/321, 22-23=-122/321, 23-24=-136/336, 24-25=-122/297								
 NOTES- 1) Unbalanced roof live loads h 2) Wind: ASCE 7-16; Vult=1300 II; Exp B; Enclosed; MWFRS 26-6-0, Exterior(2N) 26-6-0 t exposed; end vertical left an grip DOL=1.60 3) Truss designed for wind load Gable End Details as applicat 4) TCLL: ASCE 7-16; Pr=20.0 p DOL=1.15); Is=1.0; Rough C surcharge applied to all expo 5) Unbalanced snow loads have 6) This truss has been designe non-concurrent with other liv 7) Provide adequate drainage t 8) All plates are 2x4 MT20 unle 9) Gable requires continuous b 10) Gable studs spaced at 2-0- 11) This truss has been designe 12) * This truss has been designe 	ave been considered for this design. mph (3-second gust) Vasd=103mph; TCE & (directional) and C-C Corner(3E) -0-11-(o 38-6-0, Corner(3R) 38-6-0 to 44-6-0, E: d right exposed;C-C for members and fo als in the plane of the truss only. For stud able, or consult qualified building designe bsf (roof LL: Lum DOL=1.15 Plate DOL=1 iat B; Partially Exp.; Ce=1.0; CS=1.00; CC bsed surfaces with slopes less than 0.500 e been considered for this design. d for greater of min roof live load of 12.0 [e loads. o prevent water ponding. ss otherwise indicated. otom chord bearing. 0 oc. ed for a 10.0 psf bottom chord live load n ned for a live load of 20.0psf on the botto chord and any other members.	DL=6.0psf; BCDL=6.0psf; 0 to 4-11-6, Exterior(2N) 4 xterior(2N) 44-6-0 to 58-5 rces & MWFRS for reacti s exposed to wind (norm: r as per ANSI/TPI 1. 1.5); Pg=15.0 psf; Pf=16 =1.10, Lu=50-0-0; Min. fit /12 in accordance with IE psf or 1.00 times flat roof onconcurrent with any ot m chord in all areas whe	h=25ft; B=45ft; L 4-11-6 to 20-6-0, 5-12 zone; cantile ons shown; Luml al to the face), se 5.5 psf (Lum DOL at roof snow load 3C 1608.3.4. load of 11.6 psf of her live loads. re a rectangle 3-0	=59ft; eave= Corner(3R) ver left and i ber DOL=1.6 e Standard I =1.15 Plate governs. R on overhang	=2ft; Cat. 20-6-0 to right 30 plate Industry ain s	The community of the second se	SEAL 036322	

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	RVF-LOT #4 ROOF	
					16-	4650612
24-2117-A	T03GE	PIGGYBACK BASE SUPPO	1	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.530 s Au	Ig 2 2023 MiTek Industries, Inc. Tue Apr 2 17:09:36 2024 Pa	age 2
		ID:tdH	IS5IWyLnd	?jaR9E1e	Btgyly9 -sHD1i7s90GJHPbkMzabB6?U3xBcp4d8ODKfTSFzL	JhhT

NOTES-

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 46, 47, 48, 49, 50, 51, 52, 55, 56, 58, 59, 60, 61, 62, 63, 64, 44, 43, 42, 41, 40, 39, 38, 37, 36.

4) Beveld plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





L	10-4-12	20-6-0	29-6-0	36-6-4	48-10-4	58-7-8	
Г	10-4-12	10-1-4	9-0-0	7-0-4	12-4-0	9-9-4	
Plate Offsets	s (X,Y) [6:0-5-4,0-3-0],	[13:Edge,0-2-4]					
LOADING (p TCLL (roof) Snow (Pf/Pg) TCDL BCLL	psf) 20.0) 16.5/15.0 10.0 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.79 BC 0.68 WB 0.94 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d -0.21 14-16 >693 240 -0.31 14-16 >477 180 0.06 16 n/a n/a	PLATES MT20 Weight: 445 lb	GRIP 244/190 FT = 20%
BCDL	10.0						
LUMBER-				BRACING-			
TOP CHORE	2x6 SP No.2			TOP CHORD	Structural wood sheathing directly ap	plied or 3-11-15 oc purli	ns,
BOT CHORE	2x6 SP No.2				except end verticals, and 2-0-0 oc pu	rlins (6-0-0 max.): 6-9.	
WEBS	2x4 SP No.3 *Except*			BOT CHORD	Rigid ceiling directly applied or 10-0-0) oc bracing, Except:	
	7-16: 2x4 SP No.1				6-0-0 oc bracing: 14-16.		
SLIDER	Left 2x4 SP No.3 2-6-0)		WEBS	1 Row at midpt 4-19, 9-1	6, 12-14	

REACTIONS. All bearings 0-3-8 except (jt=length) 13=Mechanical.

(lb) - Max Horz 2=236(LC 15)

Max Uplift All uplift 100 lb or less at joint(s) 16 except 2=-124(LC 16), 14=-135(LC 16)

Max Grav All reactions 250 lb or less at joint(s) 13 except 2=1623(LC 28), 16=2882(LC 28), 14=1062(LC 49)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-4=-2368/257, 4-6=-1404/268, 6-7=-517/265, 7-9=0/641, 9-11=-42/489, 11-12=-83/412

 BOT CHORD
 2-21=-150/2245, 19-21=-150/2245, 17-19=0/1253, 16-17=0/546, 14-16=-465/137

 WEBS
 4-21=0/508, 4-19=-1168/190, 6-19=0/998, 6-17=-1214/61, 7-17=0/1209, 7-16=-1980/179, 9-16=-769/135, 9-14=-24/274, 11-14=-714/307, 12-14=-496/170

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=59ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 4-11-6, Interior(1) 4-11-6 to 20-6-0, Exterior(2R) 20-6-0 to 28-9-8, Interior(1) 28-9-8 to 38-6-0, Exterior(2R) 38-6-0 to 46-9-8, Interior(1) 46-9-8 to 58-5-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

9) Refer to girder(s) for truss to truss connections.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16 except (jt=lb) 2=124, 14=135.

- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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6-17, 7-16

2 Rows at 1/3 pts

A MiTek A 818 Soundside Road



H	10-4-12	20-6-0	29-6-0	36-6-4	48-10-4		58-7-8 9-9-4	
Plate Offsets	(X,Y) [5:0-5-4,0-	3-0], [12:Edge,0-2-4]	000	104	12 4 0		004	
LOADING (p TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI. TC 0.79 BC 0.67 WB 0.94 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.21 13-15 >693 -0.31 13-15 >477 0.06 15 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 443 lb	GRIP 244/190 FT = 20%
LUMBER-			В	RACING-				
TOP CHORD	2x6 SP No.2		Т	OP CHORD	Structural wood sheathing	directly applied or	3-11-15 oc purli	ns,
BOT CHORD	2x6 SP No.2				except end verticals, and 2	-0-0 oc purlins (6-	0-0 max.): 5-8.	
WEBS	2x4 SP No.3 *Exc	ept*	В	OT CHORD	Rigid ceiling directly applie	d or 10-0-0 oc bra	cing, Except:	
	6-15: 2x4 SP No.	1			6-0-0 oc bracing: 13-15.			
SLIDER	Left 2x4 SP No.3	2-6-0	v	/EBS	1 Row at midpt	3-18, 8-15, 11-13	3	
					2 Rows at 1/3 pts	5-16, 6-15		

REACTIONS. All bearings 0-3-8 except (jt=length) 12=Mechanical.

(lb) - Max Horz 1=229(LC 15)

Max Uplift All uplift 100 lb or less at joint(s) 1, 15 except 13=-135(LC 16)

Max Grav All reactions 250 lb or less at joint(s) 12 except 1=1573(LC 27), 15=2879(LC 27), 13=1063(LC 48)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-3=-2362/267, 3-5=-1407/273, 5-6=-518/269, 6-8=0/638, 8-10=-41/487, 10-11=-83/410

 BOT CHORD
 1-20=-150/2251, 18-20=-150/2251, 16-18=0/1255, 15-16=0/547, 13-15=-462/133

 WEBS
 3-20=0/508, 3-18=-1171/190, 5-18=0/999, 5-16=-1214/61, 6-16=0/1209, 6-15=-1980/179, 8-15=-766/134, 8-13=-24/272, 10-13=-714/307, 11-13=-495/170

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=59ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0 to 5-10-6, Interior(1) 5-10-6 to 20-6-0, Exterior(2R) 20-6-0 to 28-9-8, Interior(1) 28-9-8 to 38-6-0, Exterior(2R) 38-6-0 to 46-9-8, Interior(1) 46-9-8 to 58-5-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

- 4) Unbalanced snow loads have been considered for this design.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 15 except (jt=lb) 13=135.

10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Max Horz 2=222(LC 15) Max Uplift 2=-130(LC 16), 14=-104(LC 16), 18=-109(LC 16) Max Grav 2=1598(LC 28), 14=828(LC 29), 18=3321(LC 28)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-4=-2320/271, 4-6=-1345/276, 6-7=-532/276, 7-9=-532/276, 9-10=0/797,

10-12=-989/349, 12-14=-810/170

BOT CHORD	2-23=-128/2207, 21-23=-128/2207, 19-21=0/1203, 18-19=-771/137, 16-18=-527/115,
	14-16=-38/702
WEBS	4-23=0/521, 4-21=-1184/185, 6-21=0/964, 6-19=-1174/46, 7-19=-667/170,

9-19=-124/1978, 9-18=-1897/208, 10-18=-1353/166, 10-16=-213/1706, 12-16=-698/297

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=59ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 4-11-13, Interior(1) 4-11-13 to 20-6-0, Exterior(2R) 20-6-0 to 28-10-2, Interior(1) 28-10-2 to 38-6-0, Exterior(2R) 38-6-0 to 46-10-2, Interior(1) 46-10-2 to 59-11-0 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Bearing at joint(s) 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=130, 14=104, 18=109.

11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and Continuities and a continuity of the continuity of

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Job	Truss	Truss Type	Qty	Ply	RVF-LOT #4 ROOF	
						164650615
24-2117-A	T05	PIGGYBACK BASE	2	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.530 s Au	ug 2 2023 MiTek Industries, Inc. Tue Apr 2 17:09:40 2024	Page 2
		ID:tdHS5IWyLng?jaR9E1eBtqyly9I3TXYVvf4VqiuC17CQg7Hreb4op20GO_8ydgb0zUhhP				

NOTES-

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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59-0-0								
59-0-0								
Plate Offsets (X,Y) [2:1-10-12,0-2-0], [14:0-3-0,0-4-0], [19:0-4-0,0-4-8], [23:0-3-0,0-4-0], [33:0-2-12,0-2-1], [35: ⁻	1-8-10,0-2-0]						
LOADING (psf) SPACING- 2-0-0 TCLL (roof) 20.0 Plate Grip DOL 1.15 Snow (Pf/Pg) 16.5/15.0 Lumber DOL 1.15 TCDL 10.0 Rep Stress Incr YES BCLL 0.0 * Code IRC2018/TPI2014 Code IRC2018/TPI2014	CSI. DEFL. TC 0.05 Vert(LL) BC 0.02 Vert(CT) WB 0.17 Horz(CT) Matrix-S Vertix-S Vertix-S	in (loc) l/defl L/d -0.00 35 n/r 120 -0.00 35 n/r 120 0.01 35 n/a n/a	PLATES C MT20 2 Weight: 597 lb	FT = 20%				
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 OTHERS 2x4 SP No.3 SLIDER Left 2x4 SP No.3 2-6-0, Right 2x4 SP No.3 2-6-0	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing directly app 2-0-0 oc purlins (6-0-0 max.): 14-23. Rigid ceiling directly applied or 10-0-0 1 Row at midpt 23-47, 22 17-54, 16 24-46, 25	plied or 6-0-0 oc purlins, e) oc bracing. -48, 21-49, 20-50, 19-51, -55, 15-56, 14-57, 13-58, i-45	xcept 18-52, 12-59,				
REACTIONS. All bearings 59-0-0. (lb) - Max Horz 2=-216(LC 14) Max Uplift All uplift 100 lb or less at joint(s) 2, 48, 49, 5 67, 46, 45, 43, 42, 41, 40, 39, 38, 37	0, 51, 52, 54, 55, 58, 59, 61, 62, 63, 64, 6	5, 66,						

Max Grav All reactions 250 lb or less at joint(s) 2, 47, 48, 49, 50, 51, 52, 54, 55, 56, 57, 58, 59, 61, 62, 63, 64, 65, 66, 67, 46, 45, 43, 42, 41, 40, 39, 38, 37, 35

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 12-13=-104/261, 13-14=-120/300, 14-15=-112/289, 15-16=-112/289, 16-17=-112/289, 17-18=-112/289, 18-19=-112/289, 19-20=-112/289, 20-21=-112/289, 21-22=-112/289, 22-23=-112/289, 23-24=-120/300, 24-25=-104/261

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=59ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -0-11-0 to 4-11-13, Exterior(2N) 4-11-13 to 20-6-0, Corner(3R) 20-6-0 to 26-6-0, Exterior(2N) 26-6-0 to 38-6-0, Corner(3R) 38-6-0 to 44-6-0, Exterior(2N) 44-6-0 to 59-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)





Job	Truss	Truss Type	Qty	Ply	RVF-LOT #4 ROOF	
						64650616
24-2117-A	T05GE	PIGGYBACK BASE SUPPO	1	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.530 s Au	g 2 2023 MiTek Industries, Inc. Tue Apr 2 17:09:42 2024 F	Page 2
		ID:tdHS5IWyLng?jaR9E1eBtqyIy9hRbIzBwwc64Q7WBWKqibMGk7rcfdULdHbG6ngvzUhhN				

NOTES-

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 48, 49, 50, 51, 52, 54, 55, 58, 59, 61, 62, 63, 64, 65, 66, 67, 46, 45, 43, 42, 41, 40, 39, 38, 37.

14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)







	10-4-12	20.6.0	29.6.2	36-6-4	19-7-1		50-0-0			
	10-4-12	10-1-4	8-0-2	8-0-2	12-1-0		10-4-12			
Plate Offset	ts (X,Y) [6:0-5-4,	0-3-0], [17:0-4-0,0-5-4], [21:0-2-4,0-4-12	1							
	(nsf)									
TCLL (roof)	20.0	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES	GRIP		
Snow (Pf/P	a) 16.5/15.0	Plate Grip DOL 1.15	TC 0.76	Vert(LL)	-0.37 19-20 >	>720 240	MT20	244/190		
TCDL	10.0	Lumber DOL 1.15	BC 0.87	Vert(CT)	-0.58 19-20	>462 180				
BCLL	0.0 *	Rep Stress Incr YES	WB 1.00	Horz(CT)) 0.03 21	n/a n/a	Woight: 471 lb	ET - 20%		
BCDL	10.0	Code IRC2016/TFI2014	Matrix-MS				vveignt. 47 i ib	FT = 20%		
LUMBER-			F	BRACING-						
TOP CHOR	2x6 SP No.2		1	TOP CHORD	Structural wood st	neathing directly as	plied or 4-6-8 oc purling	s. except		
BOT CHOR	2x6 SP 2400F 2	.0E *Except*			2-0-0 oc purlins (6	6-0-0 max.): 6-10.		.,		
	21-23,23-25: 2x	6 SP No.2, 18-20: 2x4 SP No.1	E	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:						
WEBS	2x4 SP No.3 *E	xcept*			6-0-0 oc bracing:	18-20	5 I			
	10-21,10-16: 2x	4 SP No.2	V	NEBS	1 Row at midpt	4-24, 7-2	22			
SLIDER	Left 2x4 SP No.	3 2-6-0, Right 2x4 SP No.3 2-6-0			2 Rows at 1/3 pts	6-22, 9-2	21, 10-20			
REACTION	IS. (size) 2=0- Max Horz 2=22 Max Uplift 2=-1 Max Grav 2=15	3-8, 14=0-3-8, 21=0-3-8 22(LC 15) 41(LC 16), 14=-87(LC 16) 584(LC 28), 14=923(LC 29), 21=3717(L0	C 28)							
FORCES. TOP CHOR BOT CHOR WEBS	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-2291/291, 4-6=-1315/299, 6-7=-512/305, 7-9=-512/305, 9-10=0/801, 10-12=-1220/310, 12-14=-1048/121 BOT CHORD 2-26=-146/2182, 24-26=-146/2182, 22-24=0/1176, 21-22=-775/99, 17-21=-257/107, 16-17=-257/107, 14-16=-2/910, 19-20=-353/0, 18-19=-353/0 WEBS 4-26=0/523, 4-24=-1184/183, 6-24=0/970, 6-22=-1193/31, 7-22=-661/169, 9-22=-114/1951, 9-21=-1922/204, 20-21=-1645/66, 10-20=-1418/106, 10-18=-117/2036, 16-18=-142/1738, 12-16=-689/300, 17-19=-292/0									
NOTES-										
1) Unbalan	ced roof live loads ha	ave been considered for this design.					MITTIN			
2) Wind: AS II; Exp B to 28-10- exposed arin DOI	SCE 7-16; Vult=130n ; Enclosed; MWFRS -2, Interior(1) 28-10-2 ; end vertical left and =1 60	nph (3-second gust) Vasd=103mph; TCI (directional) and C-C Exterior(2E) -0-11 2 to 38-6-0, Exterior(2R) 38-6-0 to 46-10 d right exposed;C-C for members and fo	DL=6.0psf; BCDL=6.0ps -0 to 4-11-13, Interior(1) -2, Interior(1) 46-10-2 to rces & MWFRS for read	sf; h=25ft; B=45ft;) 4-11-13 to 20-6- o 59-11-0 zone; ca ctions shown; Lun	L=59ft; eave=7ft; C 0, Exterior(2R) 20-6 antilever left and rig nber DOL=1.60 plat	cat. 6-0 ht ce	OP FESSE	IN IN		
3) TCLL: AS	SCE 7-16: Pr=20.0 p	sf (roof LL: Lum DOL=1.15 Plate DOL=	1.15): Pa=15.0 psf: Pf=1	6.5 psf (Lum DOI	L=1.15 Plate	2 .	. K	1		
DOL=1.1	15); Is=1.0; Rough Ca	at B; Partially Exp.; Ce=1.0; Cs=1.00; Cf	=1.10, Lu=50-0-0; Min.	flat roof snow loa	d governs. Rain		SEAL	: =		
surcharg	surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.									
4) Unbalan	ced snow loads have	been considered for this design.				- E 1	036322	- E		
5) This trus	s has been designed	for greater of min roof live load of 12.0	psf or 1.00 times flat roo	of load of 11.6 psf	on overhangs	3 3	:	1 E		
non-concurrent with other live loads.										
6) Provide adequate drainage to prevent water ponding.										
7) This trus	() I his truss has been designed for a 10.0 pst bottom chord live load nonconcurrent with any other live loads.									
8) * I his tru	o) This truss has been designed of a live load of 20.0ps on the portion chord in all areas where a rectangle 3-6-0 tail by 2-0-0 wide									
	with between the bottom order and any other interflores, with bottom = 10.0psi. 9) Bearing at joint(s) 21 considers parallel to arise value using ANS/IZP1 andle to arise formula. Building designer should verify									
J Deaning	a joint(3) 21 conside	To paraller to grain value using ANOI/TF	i angio to grain tormul	a. Dununy uesiy			A COLUMN A			

9) Bearing at joint(s) 21 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **PCB Building Component Scitut Information**. Building from the Structure Building Component Advancement description (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



April 3,2024

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #4 ROOF	
					164	4650617
24-2117-A	T05S	PIGGYBACK BASE	8	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.530 s Au	ug 2 2023 MiTek Industries, Inc. Tue Apr 2 17:09:43 2024 Pa	age 2
		ID:tdHS5IWyLng?jaR9E1eBtqyIy99d9gAXxYNQCHIgmitYDquUG7W0odDbxQqwsLCLzUhhM				

NOTES-

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14 except (jt=lb) 2=141.
11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.
 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.

 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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April 3,2024



April 3,2024







TRENCO

818 Soundside Road











818 Soundside Road



- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 4-7-3, Exterior(2R) 4-7-3 to 7-7-3, Interior(1) 7-7-3 to 8-7-14 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.





818 Soundside Road



1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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- exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate
- DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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- Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



