

RE: 24-2118-A RVF-LOT #14 ROOF Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: 24-2118-A Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2018/TPI2014 Wind Code: ASCE 7-16 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.5 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 19 individual, dated Truss Design Drawings and 0 Additional Drawings.

No. 1 2 3 4 5 6 7 8 9 10 11	Seal# I64440671 I64440672 I64440673 I64440674 I64440675 I64440676 I64440677 I64440678 I64440679 I64440680 I64440681	Truss Name HG01 J01 M01GE M02 M03 PB01 SD01 SD02 SM01G T01	Date 3/26/2024 3/26/2024 3/26/2024 3/26/2024 3/26/2024 3/26/2024 3/26/2024 3/26/2024 3/26/2024 3/26/2024 3/26/2024
12	164440682	T01GE	3/26/2024
13 14	l64440683 l64440684	T01S T01SGE	3/26/2024 3/26/2024
14 15 16 17 18 19	I64440685 I64440685 I64440686 I64440687 I64440688 I64440689	T02 T02GE T03 T04 T04GE	3/26/2024 3/26/2024 3/26/2024 3/26/2024 3/26/2024 3/26/2024

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

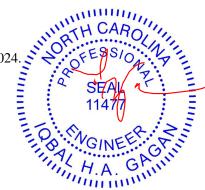
based on the parameters provided by Riverside Roof Truss.

Truss Design Engineer's Name: Gagan, Iqbal

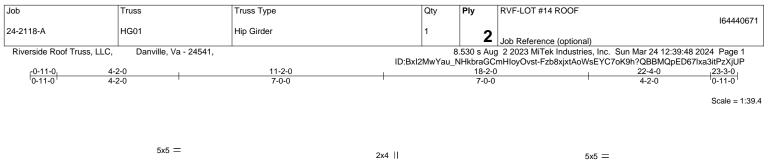
My license renewal date for the state of North Carolina is December 31, 2024

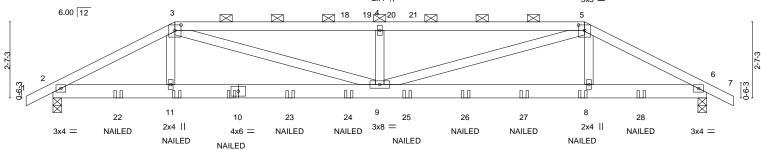
North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gagan, Iqbal





4-2-0	11-2-0			<u>18-2-0</u> 7-0-0		<u>22-4-0</u> 4-2-0	
	0-0-0], [3:0-2-8,0-2-4], [5:0-2-8,0-2-4], [6	6:0-0-0,0-0-0]		100		420	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. TC 0.78 BC 0.45	DEFL. Vert(LL) Vert(CT)	in (loc) -0.13 9 -0.22 9-11	l/defl L/d >999 240 >999 180	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr NO Code IRC2018/TPI2014	WB 0.39 Matrix-MS	Horz(CT)	0.03 6	n/a n/a	Weight: 239 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3		T	BRACING- TOP CHORD BOT CHORD	2-0-0 oc purlins	l sheathing directly (5-0-2 max.): 3-5. ectly applied or 10		s, except
TOP CHORD 2-3=-2904/425 BOT CHORD 2-11=-341/258	lax. Ten All forces 250 (lb) or less exc 5, 3-4=-4361/626, 4-5=-4361/626, 5-6=- 30, 9-11=-340/2552, 8-9=-339/2545, 6-8 3-9=-307/1898, 4-9=-600/158, 5-9=-30	2897/423 8=-340/2573					
 Top chords connected as follows: 3 Bottom chords connected as Webs connected as follows: 3 All loads are considered equaply connections have been pr Unbalanced roof live loads hat Wind: ASCE 7-16; Vult=130n II; Exp B; Enclosed; MWFRS plate grip DOL=1.60 TCLL: ASCE 7-16; Pr=20.0 p DOL=1.15); Is=1.0; Rough C; surfaces with slopes less that Unbalanced snow loads have This truss has been designed non-concurrent with other live Provide adequate drainage to This truss has been designed to the strust has been designed will fit between the bottom c This truss is designed in acc referenced standard ANSI/T Graphical purlin representation 	follows: 2x6 - 2 rows staggered at 0-9-0 2x4 - 1 row at 0-9-0 oc. ally applied to all plies, except if noted as rovided to distribute only loads noted as ave been considered for this design. nph (3-second gust) Vasd=103mph; TCI (directional); cantilever left and right exp sf (roof LL: Lum DOL=1.15 Plate DOL= at B; Partially Exp.; Ce=1.0; Cs=1.00; C n 0.500/12 in accordance with IBC 1608 been considered for this design. If or greater of min roof live load of 12.0 e loads. o prevent water ponding. If or a 10.0 psf bottom chord live load no need for a live load of 20.0psf on the botth hord and any other members. tion (by others) of truss to bearing plate cordance with the 2018 International Re	oc. s front (F) or back (B) fa (F) or (B), unless otherw DL=6.0psf; BCDL=6.0ps posed ; end vertical left a 1.15); Pg=15.0 psf; Pf=1 t=1.10, Lu=50-0-0 Rain .3.4. psf or 1.00 times flat roo on chord in all areas wh capable of withstanding sidential Code sections tation of the purlin along	vise indicated. if; h=25ft; B=45ft; and right exposed i6.5 psf (Lum DOL surcharge applied of load of 11.6 psf her live loads. here a rectangle 3- i241 lb uplift at joi R502.11.1 and R8	L=24ft; eave=4ft ; Lumber DOL=1 =1.15 Plate d to all exposed on overhangs -6-0 tall by 2-0-0 nt 2 and 241 lb to 302.10.2 and	; Cat. 1.60 wide	SEAL 11477 March 26,20	
WARNING - Verify design parame Design valid for use only with MiTe a truss system. Before use, the bui building design. Bracing indicated is always required for stability and fabrication, storage, delivery, erect	eters and READ NOTES ON THIS AND INCLUDED k® connectors. This design is based only upon pa Idling designer must verify the applicability of desig is to prevent buckling of individual truss web and/c to prevent collapse with possible personal injury ar- ion and bracing of trusses and truss systems, see Safety Information available from the Structural f	MITEK REFERENCE PAGE MI rameters shown, and is for an in parameters and properly inco or chord members only. Addition d property damage. For gene ANSI/TP11 Quality Criteria a	individual building com orporate this design int onal temporary and per aral guidance regarding nd DSB-22 available	ponent, not o the overall manent bracing the from Truss Plate Inst	itute (www.tpinst.org)	ENGINEERING BY A MITEK / B18 Soundside Road Edenton, NC 27932	f filiate

ſ	Job	Truss	Truss Type	Qty	Ply	RVF-LOT #14 ROOF
						I64440671
	24-2118-A	HG01	Hip Girder	1	2	
					_	Job Reference (optional)
	Riverside Roof Truss, LLC,	Danville, Va - 24541,		8	.530 s Aug	2 2023 MiTek Industries, Inc. Sun Mar 24 12:39:48 2024 Page 2
			ID:Bxl2	MwYau_N	HkbraGC	mHloyOvst-Fzb8xjxtAoWsEYC7oK9h?QBBMQpED67Ixa3itPzXjUP

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-43, 3-5=-53, 5-7=-43, 12-15=-20

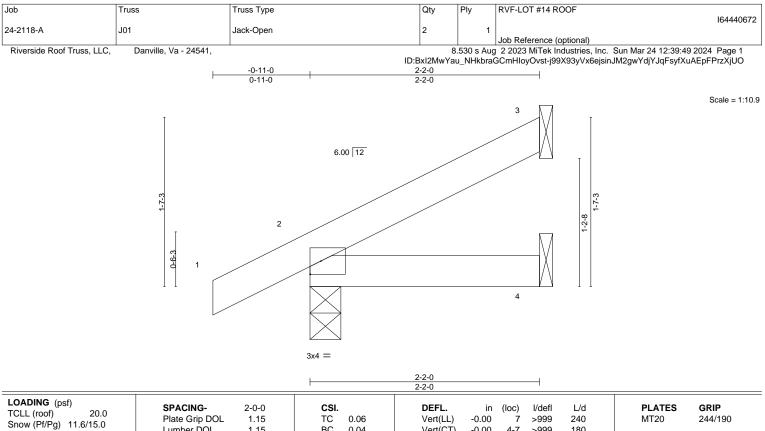
Concentrated Loads (lb)

Vert: 10=-118(B) 11=-118(B) 8=-118(B) 22=-117(B) 23=-118(B) 24=-118(B) 25=-118(B) 26=-118(B) 27=-118(B) 28=-117(B) 28=-117(B) 28=-117(B) 28=-118(B) 28=-11



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





Snow (Pf/Pg) 11.6/15.0 Plate Grip DOL 1.15 IC 0.06 Vert(LL) -0.00 7 >999 240 M120 244/190 TCDL 10.0 Lumber DOL 1.15 BC 0.04 Vert(CT) -0.00 4-7 >999 240 M120 244/190 TCDL 10.0 Rep Stress Incr VES WB 0.00 Hor?(CT) 0.00 3 n/a	TOLL (reaf)	20.0	SFACING-	2-0-0	USI.				(100)	i/ueii	L/u	FLATES	GRIF
Show (Pf/Pg) 11.6/15.0 Lumber DOL 1.15 BC 0.04 Vert(CT) -0.00 4-7 >999 180 TCDL 10.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 3 p/a p/a	TCLL (roof)	20.0	Plate Crip DOI	1 15	TC	0.06	Vort(LL)	0.00	7	~ 000	240	MT20	244/100
TCDL 10.0 Lumber DOL 1.15 BC 0.04 Vert(C1) -0.00 4-7 >999 180 Rep Stress Incr VES WB 0.00 Hor(CT) 0.00 3 p/a p/a	Snow (Pf/Pa)	11 6/15 0	Flate Grip DOL	1.15	10	0.00	Vert(LL)	-0.00		>999	240	101120	244/190
ICDL 10.0 Rep Stress Incr. YES WB 0.00 Horr/CT) 0.00 3 p/a p/a	(0)	11.0/13.0	Lumber DOI	1 15	BC	0.04	Vert(CT)	-0.00	4-7	<u>_</u> aaa	180		
Rep Stress Incr YES $WB = 0.00$ $Borz(C1) = 0.00$ 3 n/a n/a	TCDI	10.0			_				- <i>1</i>	2000	100		
			Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
	BCLL	0.0 *										Mainhe Olle	FT 000/
BCDL 0.0 Code IRC2018/TPI2014 Matrix-MP Weight: 9 lb FT = 20%	PCDI	10.0		212014	Iviatri	X-IVIP						vveignt: 9 ib	FI = 20%
	DUDL	10.0											

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=50(LC 16)

Max Uplift 3=-18(LC 16), 2=-30(LC 16)

Max Grav 3=52(LC 21), 2=156(LC 21), 4=37(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 3 and 30 lb uplift at joint 2.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 2-2-0 oc purlins.

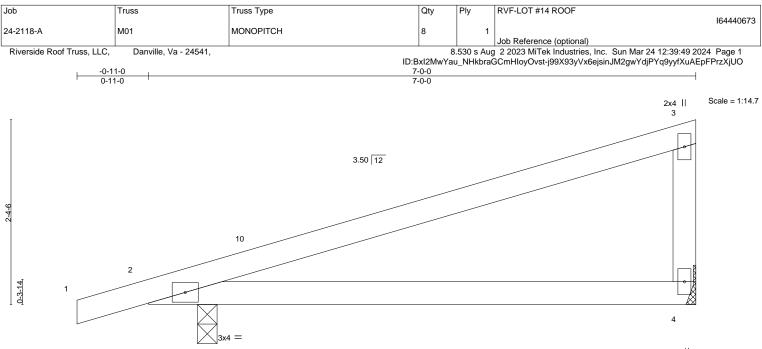
Rigid ceiling directly applied or 10-0-0 oc bracing.

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T RENCO

818 Soundside Road

Edenton, NC 27932



2x4 ||

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals

	0-7-8		7-0-0 6-4-8						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.62 BC 0.41 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.07 -0.14 0.00	(loc) 4-9 4-9 2	l/defl >999 >574 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2018/TPI2014	Matrix-MP	ACING-					Weight: 25 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WFBS

REACTIONS. (size) 4=Mechanical, 2=0-3-0

Max Horz 2=74(LC 15) Max Uplift 4=-12(LC 12), 2=-50(LC 16)

Max Grav 4=246(LC 21), 2=366(LC 2)

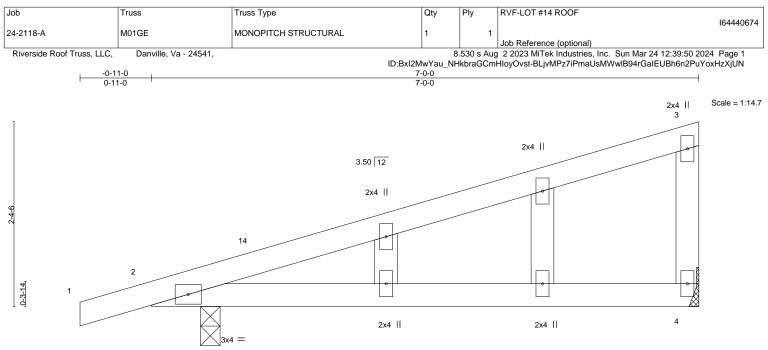
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 6-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 4 and 50 lb uplift at joint 2.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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2x4 ||

Rigid ceiling directly applied or 10-0-0 oc bracing.

	0-7-8		7-0-0 6-4-8	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	TC 0.62 Ver BC 0.41 Ver	FL. in (loc) l/defl L/d rt(LL) 0.07 4-13 >999 240 rt(CT) -0.14 4-13 >574 180 rz(CT) 0.00 2 n/a n/a	PLATES GRIP MT20 244/190 Weight: 28 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2		BRACING- TOP CHORE	D Structural wood sheathing directl except end verticals.	y applied or 6-0-0 oc purlins,

BOT CHORD

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

REACTIONS. (size) 4=Mechanical, 2=0-3-0

Max Horz 2=74(LC 15) Max Uplift 4=-12(LC 12), 2=-50(LC 16)

Max Grav 4=246(LC 21), 2=366(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

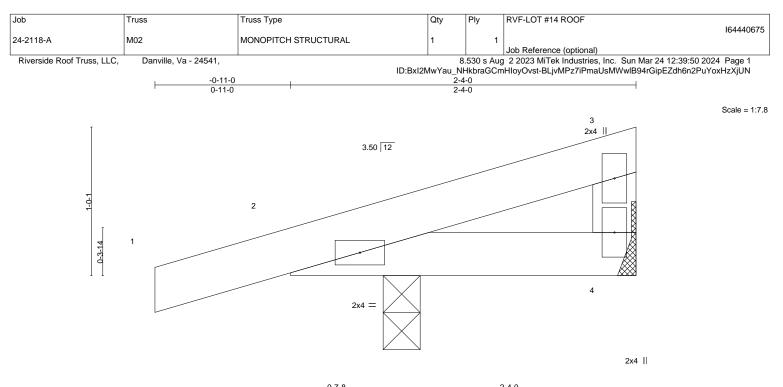
NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 6-10-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 4 and 50 lb uplift at joint 2.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

SEAL 11477 March 26,2024



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	F	0-7-8	2-4-0 1-8-8	-	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.08 BC 0.07 WB 0.00 Matrix-MP	DEFL. in (loc) l/defl L/d Vert(LL) -0.00 5 >999 240 Vert(CT) -0.00 5 >999 180 Horz(CT) 0.00 4 n/a n/a	PLATES MT20 Weight: 9 lb	GRIP 244/190 FT = 20%

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WFBS

REACTIONS. (size) 4=Mechanical, 2=0-3-0

Max Horz 2=27(LC 15) Max Uplift 4=-1(LC 20), 2=-56(LC 16)

Max Grav 4=27(LC 7), 2=216(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate
- DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1 lb uplift at joint 4 and 56 lb uplift at ioint 2.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

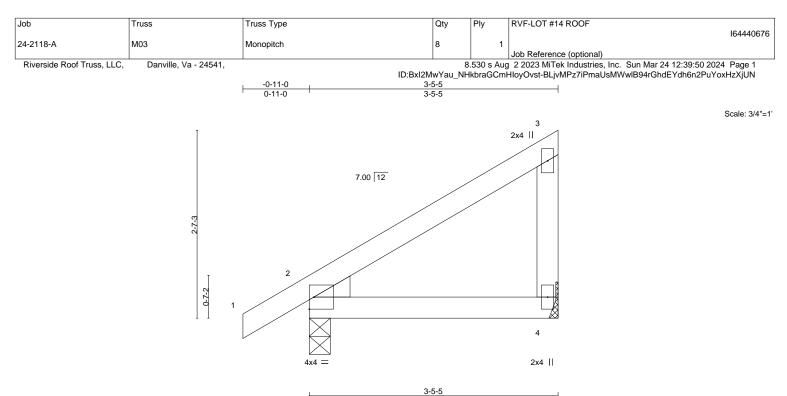


Structural wood sheathing directly applied or 2-4-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals

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			3-5-5						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2018/TPI2014	CSI. TC 0.15 BC 0.13 WB 0.00 Matrix-MP	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.01 0.00	(loc) 4-7 4-7 2	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 17 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2			BRACING- OP CHORD	Structura	al wood	sheathir	ng directly ap	plied or 3-5-5 oc purlir	ıs,

BOT CHORD

except end verticals

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WFBS WEDGE Left: 2x4 SP No.3

REACTIONS. (size) 4=Mechanical, 2=0-3-8 Max Horz 2=82(LC 15) Max Uplift 4=-19(LC 13), 2=-37(LC 16)

Max Grav 4=138(LC 21), 2=194(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

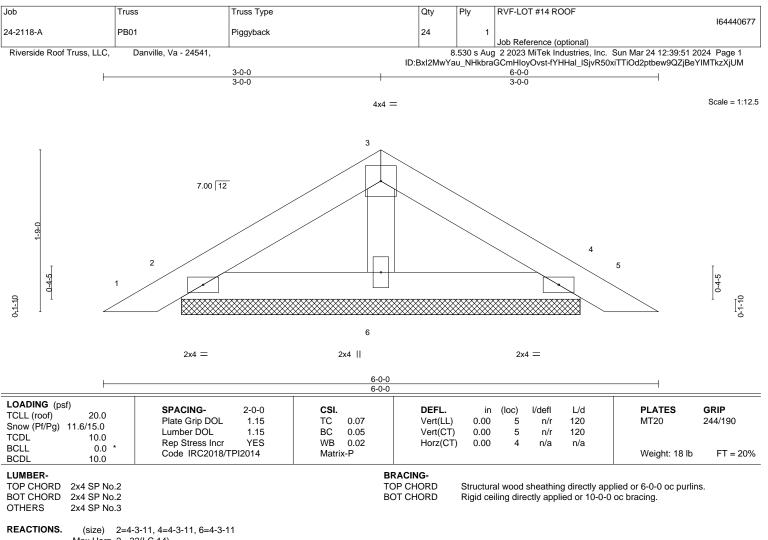
NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 3-3-9 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 4 and 37 lb uplift at ioint 2.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.





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Max Horz 2=-32(LC 14) Max Uplift 2=-37(LC 16), 4=-37(LC 16)

Max Grav 2=129(LC 21), 4=129(LC 22), 6=157(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) Gable requires continuous bottom chord bearing.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 2 and 37 lb uplift at joint 4.

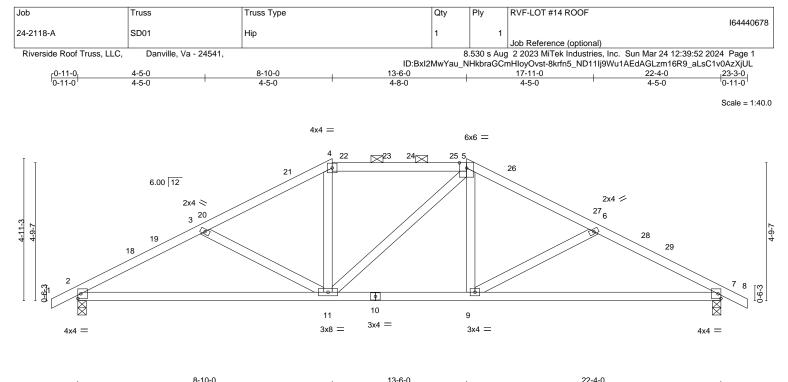
10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



TRENCO AMITEK AFFIIIATO

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	8-10-0	13-0-0	22-4-0	
	8-10-0	4-8-0	8-10-0	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2018/TPI2014	CSI. DEF TC 0.43 Vertu BC 0.68 Vertu WB 0.17 Horz Matrix-MS Horz Horz	(LL) -0.12 9-17 >999 240 MT20 2 (CT) -0.25 9-17 >999 180 (CT) 0.04 7 n/a n/a	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3		BRACING- TOP CHORD BOT CHORD	except 2-0-0 oc purlins (5-2-13 max.): 4-5.	

REACTIONS. (size) 2=0-3-8, 7=0-3-8 Max Horz 2=-79(LC 14) Max Uplift 2=-83(LC 16), 7=-83(LC 16) Max Grav 2=1024(LC 39), 7=1024(LC 39)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1605/222, 3-4=-1259/178, 4-5=-1071/191, 5-6=-1259/178, 6-7=-1604/222

BOT CHORD 2-11=-132/1373 9-11=-39/1071 7-9=-141/1373

WEBS 3-11=-354/117, 4-11=0/317, 5-9=0/317, 6-9=-355/116

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; B=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 8-10-0, Exterior(2R) 8-10-0 to 13-0-15, Interior(1) 13-0-15 to 13-6-0, Exterior(2R) 13-6-0 to 17-8-15, Interior(1) 17-8-15 to 23-3-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint 2 and 83 lb uplift at ioint 7.

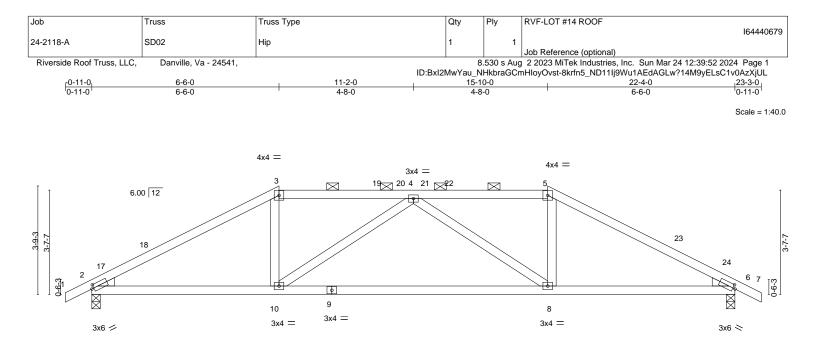
10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



March 26,2024

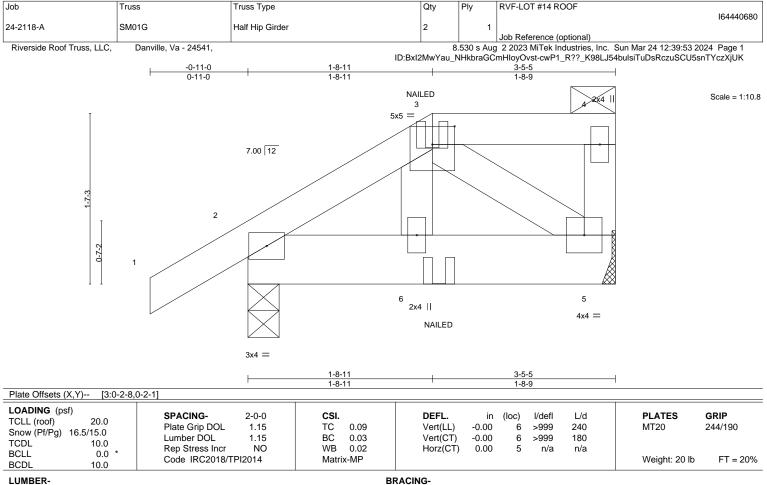
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



		6-6-0			15-1						2-4-0 5-6-0	-1
Plate Offsets (X,Y) [2:0-0-15	<u>6-6-0</u> 5,0-1-8], [6:0-0-15,0-1-8]			9-4	-0				6	5-6-0	
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	20.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TP	2-0-0 1.15 1.15 YES 12014	CSI. TC BC WB Matriz	0.61 0.82 0.26 x-MS	DEFL. Vert(LL) Vert(CT Horz(CT) -0.44	8-10	l/defl >999 >612 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 100 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS WEDGE	2x4 SP No.2	P No.3			Т	BRACING- OP CHORD	2-0-0 oc	purlins	(5-0-7 m	ax.): 3-5.	pplied or 4-0-1 oc purlins 0 oc bracing.	, except
REACTIONS.	Max Horz 2=-6 Max Uplift 2=-8											
FORCES. (III TOP CHORD BOT CHORD WEBS	2-3=-1511/152 2-10=-48/1274	lax. Ten All forces 250 (2, 3-4=-1261/177, 4-5=-12 4, 8-10=-119/1495, 6-8=-5 -10=-383/95, 4-8=-383/95	61/175, 5-6=-1 5/1274		own.							
 Wind: ASCI II; Exp B; Ei 10-8-15, Inth exposed; e grip DOL=1 TCLL: ASC DOL=1.15); surcharge a Unbalancec This truss h non-concur Provide ade This truss h swill fit betwe Provide me joint 6. 	E 7-16; Vult=130n nclosed; MWFRS erior(1) 10-8-15 tr and vertical left and .60 E 7-16; Pr=20.0 p ; Is=1.0; Rough C applied to all expo d snow loads have as been designed rent with other live equate drainage to as been designed has been designed been the bottom ch chanical connection	ave been considered for the hph (3-second gust) Vasd- (directional) and C-C Exter of 15-10-0, Exterior(2R) 15 d right exposed;C-C for mean sf (roof LL: Lum DOL=1.1 at B; Partially Exp.; Ce=1.1 sed surfaces with slopes I been considered for this to greater of min roof live beats. o prevent water ponding. I for a 10.0 psf bottom cho ed for a live load of 20.0ps ord and any other membe on (by others) of truss to b cordance with the 2018 Ini-	=103mph; TCD erior(2E) -0-11- -10-0 to 20-0-1 embers and for 5 Plate DOL=1 0; Cs=1.00; Ct= ess than 0.500 design. e load of 12.0 p ord live load noo f on the bottom rs. wearing plate ca	0 to 2-1-0, I 5, Interior(1 rcces & MWF .15); Pg=15 =1.10, Lu=5 /12 in accor osf or 1.00 ti nconcurrent n chord in al apable of wit	Interior(1) 2:) 20-0-15 to TRS for read 5.0 psf; Pf=1 i0-0-0; Min. dance with imes flat roo t with any ot Il areas whe thstanding 8	1-0 to 6-6-0, Ex 23-3-0 zone; ca tions shown; Lu 6.5 psf (Lum DC flat roof snow loa IBC 1608.3.4. of load of 11.6 ps her live loads. re a rectangle 3- 33 lb uplift at join	terior(2R) antilever le mber DOL DL=1.15 Pl ad govern of on overh 6-0 tall by t 2 and 83	6-6-0 to ft and ri =1.60 p ate s. Rain hangs 2-0-0 v Ib uplift	ght late vide	and the lot	SEAL 11477	New
referenced	d standard ANSI/T										March 26,20	24

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A MiTek Aff 818 Soundside Road Edenton, NC 27932



TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x6 SP No 2 BOT CHORD

WFBS 2x4 SP No 3

REACTIONS. (size) 2=0-3-8, 5=Mechanical

Max Horz 2=49(LC 11) Max Uplift 2=-41(LC 12), 5=-22(LC 9)

Max Grav 2=227(LC 32), 5=137(LC 31)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 41 lb uplift at joint 2 and 22 lb uplift at ioint 5.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-43, 3-4=-53, 5-7=-20

Continued on page 2

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Structural wood sheathing directly applied or 3-5-5 oc purlins,

except end verticals, and 2-0-0 oc purlins: 3-4.

Rigid ceiling directly applied or 10-0-0 oc bracing.





818 Soundside Road Edenton, NC 27932

Job	1	Truss	Truss Type	Qty	ty .	Ply	RVF-LOT #14 ROOF
							I64440680
24-2118-A	5	SM01G	Half Hip Girder	2		1	
							Job Reference (optional)
Riverside Roof	Truss, LLC,	Danville, Va - 24541,			8.	530 s Aug	2 2023 MiTek Industries, Inc. Sun Mar 24 12:39:53 2024 Page 2
				ID:BxI2Mw	wYau_N	IHkbraGC	mHloyOvst-cwP1_R??_K98LJ54bulsiTuDsRczuSCU5snTYczXjUK

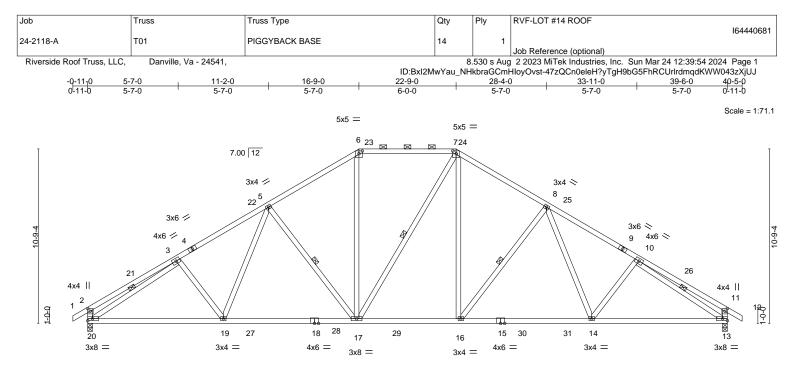
LOAD CASE(S) Standard Concentrated Loads (lb)

Vert: 6=-3(B) 3=-5(B)

SEAL 11477 PB VGINEER CHINING March 26,2024

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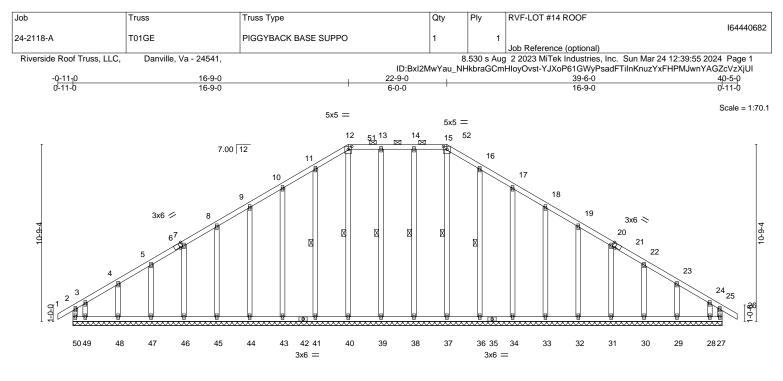




	<u>8-4-8</u> 8-4-8	<u> </u>		22-9-0 6-0-0	31-1-8 8-4-8		<u>39-6-0</u> 8-4-8	
Plate Offsets (X,Y)	[2:0-2-0,0-1-12], [6:0-2-8	0-2-1], [7:0-2-8,0-2-1], [11:0-2-0,0-1-12]					
Snow (Pf/Pg) 16.5/15 TCDL 10 BCLL	0.0 * Code IR	DOL 1.15 DOL 1.15	CSI. TC 0.86 BC 0.88 WB 0.64 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	-0.23 14-16 > -0.39 14-16 >	/defl L/d •999 240 •999 180 n/a n/a	PLATES MT20 Weight: 259 lb	GRIP 244/190 FT = 20%
BCDL 10	0.0	2010,1112011					110.g.m. 200 ib	
				BRACING- TOP CHORD BOT CHORD WEBS		ls, and 2-0-0 oc pu ly applied or 10-0-	oplied or 3-3-13 oc purli Irlins (2-10-4 max.): 6-7 0 oc bracing. 17, 8-16, 3-20, 10-13	
Max I Max I	e) 20=0-3-8, 13=0-3-8 Horz 20=-232(LC 14) Jplift 20=-128(LC 16), 13= Grav 20=1863(LC 28), 13:							
TOP CHORD 2-3= 8-10 BOT CHORD 19-2 WEBS 5-19	Comp./Max. Ten All foi -525/102, 3-5=-2605/210, =-2611/210, 10-11=-525/1 0=-57/2321, 17-19=-21/21 =0/405, 5-17=-650/131, 6 =-2248/88, 10-13=-2254/8	5-6=-2062/241, 6-7=-16 02, 2-20=-452/120, 11- 74, 16-17=0/1732, 14-1 17=-22/766, 7-16=-22/8	90/236, 7-8=-2068/2 13=-452/120 6=0/2038, 13-14=-5	7/2152				
 Wind: ASCE 7-16; II; Exp B; Enclosed 22-4-1, Interior(1) 2 end vertical left and DOL=1.60 TCLL: ASCE 7-16; DOL=1.15); Is=1.0; surcharge applied t Unbalanced snow I This truss has beer non-concurrent with Provide adequate c This truss has beer will fit between the I Provide mechanica at joint 13. This truss is desig referenced standard 	rainage to prevent water p designed for a 10.0 psf b en designed for a live load bottom chord and any othe l connection (by others) of ned in accordance with th	ust) Vasd=103mph; TCE I C-C Exterior(2E) -0-11. R) 22-9-0 to 28-4-0, Inter mbers and forces & MW DOL=1.15 Plate DOL=1 p.; Ce=1.0; Cs=1.00; Ct h slopes less than 0.500 d for this design. in roof live load of 12.0 p oonding. ottom chord live load no of 20.0psf on the bottom er members, with BCDL truss to bearing plate ca e 2018 International Res	0 to 3-0-6, Interior(1 rior(1) 28-4-0 to 40- /FRS for reactions sl .15); Pg=15.0 psf; P =1.10, Lu=50-0-0; M /12 in accordance w bsf or 1.00 times flat inconcurrent with any in chord in all areas v = 10.0psf. apable of withstandir sidential Code sectio) 3-0-6 to 16-9-0, Ex 5-0 zone; cantilever I hown; Lumber DOL= #=16.5 psf (Lum DOI iin. flat roof snow loav ith IBC 1608.3.4. roof load of 11.6 psf y other live loads. where a rectangle 3-6 ng 128 lb uplift at join ns R502.11.1 and R	802.10.2 and	at. ed;	SEAL 11477 SEAL 11477 H.A.G March 26,20	1111
	esign parameters and READ NOT						ENGINEERING BY	

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)





		39-6-						
Plate Offsets (X,Y) [6:0-2-3	,Edge], [12:0-2-8,0-2-1], [15:0-2-8,0-2-1]	39-6- , [21:0-2-3,Edge]	0					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.18 BC 0.09 WB 0.20 Matrix-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.00 26 -0.00 26 0.01 27	n/r	L/d 120 120 n/a	PLATES MT20 Weight: 310 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3		TC			ticals, and rectly appl	2-0-0 oc pu ied or 6-0-0	pplied or 6-0-0 oc purlins Irlins (6-0-0 max.): 12-1 oc bracing. 4-38, 13-39, 12-40, 11-4	5.
REACTIONS. All bearings 3	9-6-0.							

(lb) - Max Horz 50=-232(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 38, 39, 41, 43, 44, 45, 46, 47, 48, 36, 34, 33, 32, 31, 30, 29 except 50=-215(LC 14), 27=-104(LC 15), 49=-154(LC 15), 28=-106(LC 12) Max Grav All reactions 250 lb or less at joint(s) 27, 37, 38, 39, 40, 41, 43, 44, 45, 46, 47, 48, 36, 34, 33,

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 10-11=-149/258, 11-12=-177/303, 12-13=-155/278, 13-14=-155/278, 14-15=-155/278, 15-16=-177/303, 16-17=-149/258

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=40ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -0-11-0 to 2-9-0, Exterior(2N) 2-9-0 to 16-9-0, Corner(3R) 16-9-0 to 20-9-0, Exterior(2N) 20-9-0 to 22-9-0, Corner(3R) 22-9-0 to 26-9-0, Exterior(2N) 26-9-0 to 40-5-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 11) Gable studs spaced at 2-0-0 oc.
- 12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 13) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer muxel worlfy the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)





818 Soundside Road Edenton, NC 27932

 $[\]begin{array}{c} \text{31.3} \text{All reactions 250 ib 0 ress at joint(s) $27, $7, $6, $3, $40, $41, $43, $44, $45, $46, $47, $46, $56, $54, $55, $32, $31, $30, $29, $28 except 50=271(LC 29), $49=259(LC 14) \\ \end{array}$

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #14 ROOF
04 0440 A	TOLOF				I64440682
24-2118-A	T01GE	PIGGYBACK BASE SUPPO	1	1	
					Job Reference (optional)
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8	.530 s Aug	2 2023 MiTek Industries, Inc. Sun Mar 24 12:39:56 2024 Page 2
		ID:Bxl2l	MwYau N	HkbraGCr	nHloyOvst-0V4AdS2uHFXjCngfG0IZK6Wihfde5mAwng?79xzXjUH

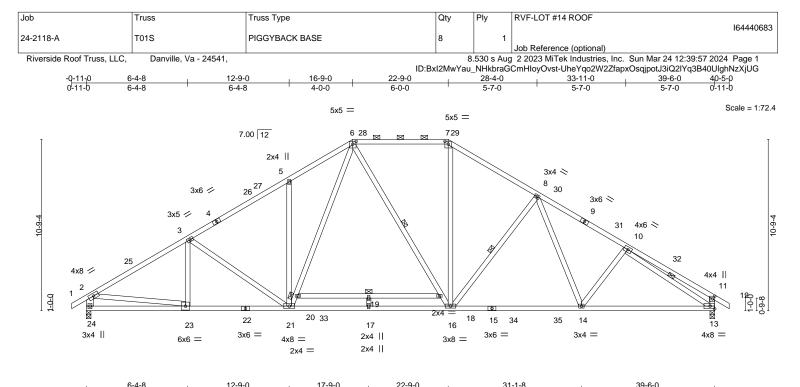
NOTES-

15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





0-4-	8 12-9-0	17-9-0	22-9-0	31-1-8	39-0-0	1
6-4-	8 6-4-8	5-0-0	5-0-0	8-4-8	8-4-8	
Plate Offsets (X,Y) [6:0-2	-8,0-2-1], [7:0-2-8,0-2-1], [11:0-2-0,0-1-12					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.88 BC 0.94 WB 0.85 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT		L/d PLATES 240 MT20 180 n/a Weight: 272 lb	GRIP 244/190 FT = 20%
BCDL 10.0					5	
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 15-22: 2x4 SP WEBS 2x4 SP No.3 2-24: 2x6 SP	P DSS *Except*		BRACING- TOP CHORD BOT CHORD WEBS	except end verticals, and	ng directly applied or 2-8-14 oc purli J 2-0-0 oc purlins (2-5-14 max.): 6-7 lied or 10-0-0 oc bracing, Except: 6-16, 8-16, 10-13	7.
Max Horz 24 Max Uplift 24 Max Grav 24	I=0-3-8, 13=0-3-8 I=-233(LC 14) I=-70(LC 16), 13=-79(LC 16) I=1975(LC 28), 13=1960(LC 29)					
. ,	./Max. Ten All forces 250 (lb) or less ex 77, 3-5=-2616/93, 5-6=-2633/191, 6-7=-18		1,			

8-10=-2754/129, 10-11=-551/89, 2-24=-1859/107, 11-13=-467/112 BOT CHORD 23-24=-99/511, 21-23=0/2508, 17-21=0/1862, 16-17=0/1862, 14-16=0/2183, 13-14=0/2267 WEBS 3-21=-276/116, 5-21=-465/150, 20-21=-92/1052, 6-20=-47/1151, 6-18=-105/250,

7-16=0/860, 8-16=-615/144, 8-14=-6/352, 2-23=0/2074, 10-13=-2369/21

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=40ft; eave=5ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 3-0-6, Interior(1) 3-0-6 to 16-9-0, Exterior(2R) 16-9-0 to 22-4-1, Interior(1) 22-4-1 to 22-9-0, Exterior(2R) 22-9-0 to 28-4-0, Interior(1) 28-4-0 to 40-5-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

4) Unbalanced snow loads have been considered for this design.5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs

non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 13.
 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

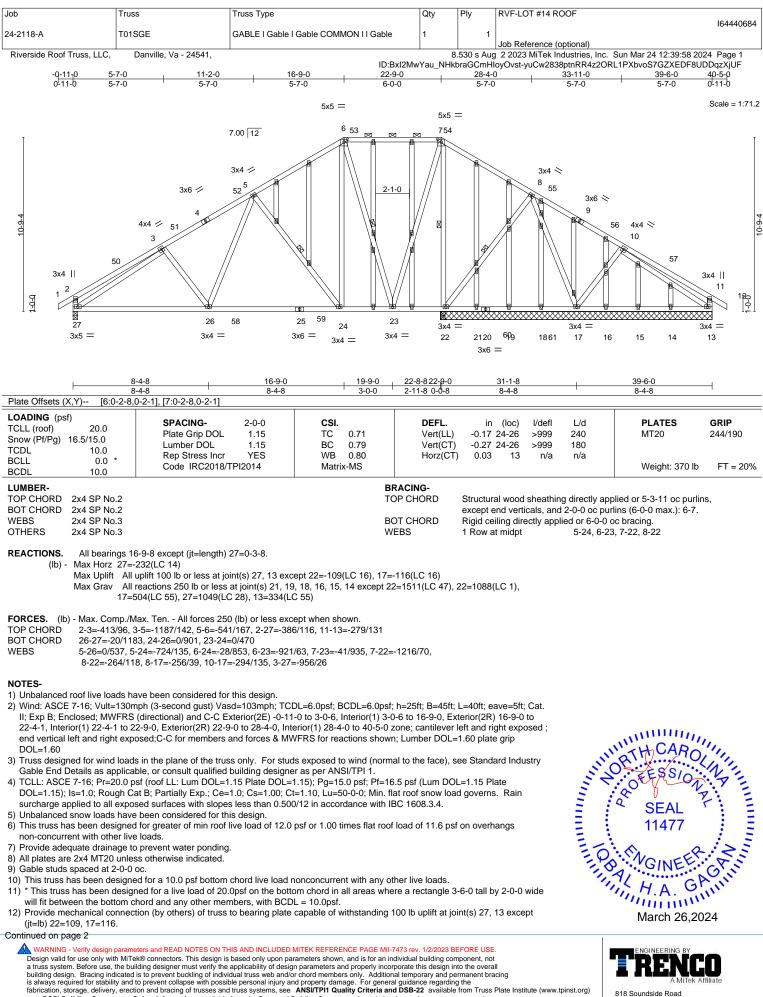
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)





818 Soundside Road

Edenton, NC 27932



and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

[Job	Truss	Truss Type	Qty	Ply	RVF-LOT #14 ROOF
						164440684
	24-2118-A	T01SGE	GABLE I Gable I Gable COMMON I I Gable	1	1	
						Job Reference (optional)
	Riverside Roof Truss, LLC,	Danville, Va - 24541,		8	.530 s Aug	2 2023 MiTek Industries, Inc. Sun Mar 24 12:39:58 2024 Page 2
			ID:Bxl2Mw	Yau NHk	braGCmHI	oyOvst-yuCw2838ptnRR4z2ORL1PXbvoS7GZXEDF8UDDqzXjUF

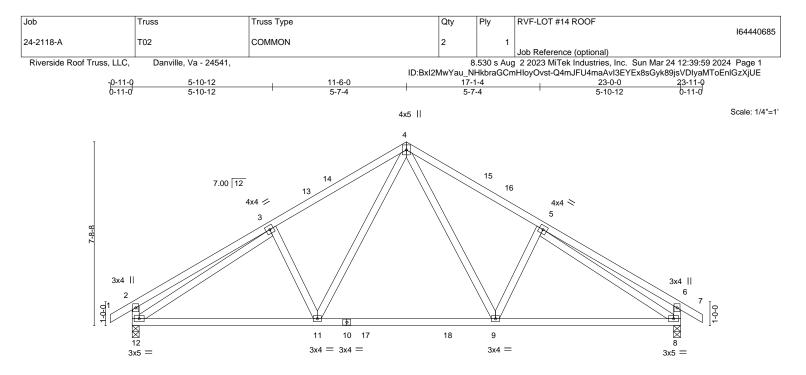
NOTES-

13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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	<u>7-9-3</u> 7-9-3	<u>15-2-13</u> 7-5-11				3-0-0 7-9-3		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	TC 0.45 BC 0.67	Vert(CT) -0	in (loc)).11 9-11).17 8-9).04 8	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 135 lb	GRIP 244/190 FT = 20%
LUMBER.		BRACIN	G.					

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

REACTIONS. (size) 12=0-3-8, 8=0-3-8 Max Horz 12=170(LC 15)

Max Uplift 12=-88(LC 16), 8=-88(LC 16)

Max Grav 12=1075(LC 28), 8=1075(LC 29)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-420/112, 3-4=-1253/179, 4-5=-1253/179, 5-6=-419/112, 2-12=-393/122, 6-8=-393/122

BOT CHORD 11-12=-55/1182, 9-11=0/831, 8-9=-44/1080

WEBS 4-9=-42/562, 4-11=-42/562, 3-12=-1011/46, 5-8=-1010/46

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 11-6-0, Exterior(2R) 11-6-0 to 14-6-0, Interior(1) 14-6-0 to 23-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 8.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. SEAL 11477 BANGINEER CHINA

Structural wood sheathing directly applied or 5-0-8 oc purlins,

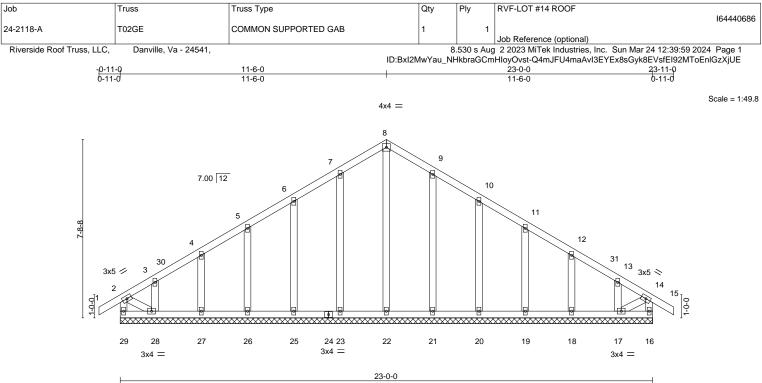
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals

March 26,2024



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		23-0-0)					1	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.08 BC 0.03 WB 0.13	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 0.00	(loc) 15 15 16	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2018/TPI2014	Matrix-S	()					Weight: 148 lb	FT = 20%
LUMBER-		BR	ACING-						

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x4 SP No.2		except end verticals.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
OTHERS	2x4 SP No.3		6-0-0 oc bracing: 28-29,16-17.
			-

REACTIONS. All bearings 23-0-0.

(lb) -Max Horz 29=-170(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 29, 23, 25, 26, 27, 28, 21, 20, 19, 18, 17 Max Grav All reactions 250 lb or less at joint(s) 29, 16, 22, 23, 25, 26, 27, 28, 21, 20, 19, 18, 17

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; B=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -0-11-0 to 2-1-0, Exterior(2N) 2-1-0 to 11-6-0, Corner(3R) 11-6-0 to 14-6-0, Exterior(2N) 14-6-0 to 23-11-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

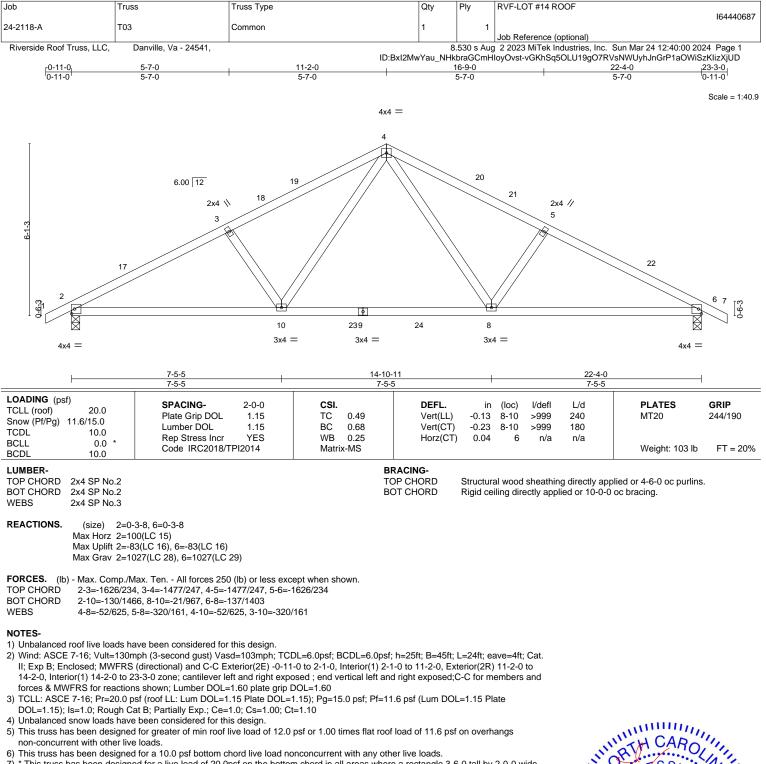
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 29, 23, 25, 26,
- 27, 28, 21, 20, 19, 18, 17.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



March 26,2024



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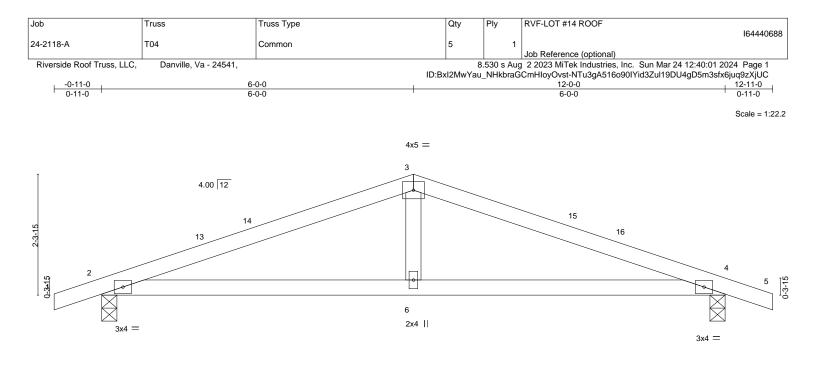


- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

HILL CONTRACTOR SEAL minin March 26,2024

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818 Soundside Road Edenton, NC 27932



	<u> 6-0-0</u> 6-0-0				12-0-0 6-0-0			1
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.46 BC 0.52 WB 0.11	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.05 6-12 -0.10 6-12 0.01 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2018/1PI2014	Matrix-MS					Weight: 42 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3		тс		Structural wood Rigid ceiling dire			blied or 5-5-14 oc purl) oc bracing.	ins.
Max Grav 2=53	4(LC 15) 8(LC 16), 4=-58(LC 16) 35(LC 2), 4=535(LC 2) lax. Ten All forces 250 (lb) or less exc	ept when shown.						
BOT CHORD 2-6=-198/813, WEBS 3-6=0/277	4-6=-198/813							
 2) Wind: ASCE 7-16; Vult=130rr II; Exp B; Enclosed; MWFRS , Interior(1) 9-0-0 to 12-11-0 z & MWFRS for reactions show 3) TCLL: ASCE 7-16; Pr=20.0 p DOL=1.15); Is=1.0; Rough Cz 4) Unbalanced snow loads have 	ave been considered for this design. nph (3-second gust) Vasd=103mph; TCI (directional) and C-C Exterior(2E) -0-11 zone; cantilever left and right exposed ; n; Lumber DOL=1.60 plate grip DOL=1 sf (roof LL: Lum DOL=1.15 Plate DOL= at B; Partially Exp.; Ce=1.0; Cs=1.00; Cf been considered for this design. If or greater of min roof live load of 12.0	-0 to 2-1-0, Interior(1) 2-1 end vertical left and right 6 .60 1.15); Pg=15.0 psf; Pf=11 l=1.10	-0 to 6-0-0, Exteri exposed;C-C for n .6 psf (Lum DOL=	or(2R) 6-0-0 to nembers and fo 1.15 Plate	9-0-0		TH CAR	1.

with any other live loads.

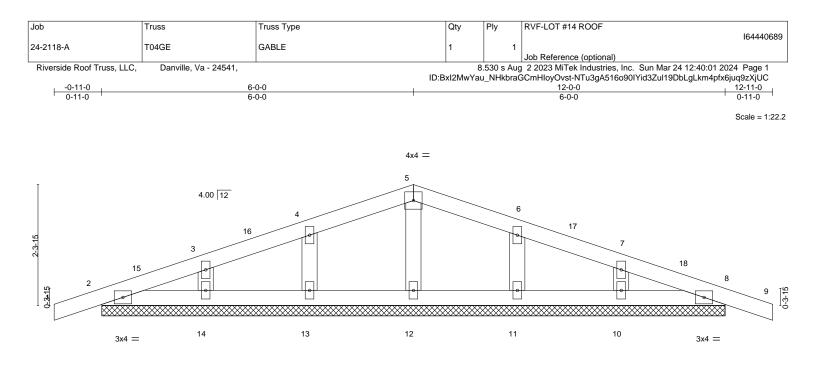
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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		12-0-0 12-0-1						4
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.06 BC 0.03 WB 0.04 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.00 8 -0.00 9 0.00 8	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 47 lb	GRIP 244/190 FT = 20%
BCDL 10.0 LUMBER- TOP CHORD 2x4 SP №.2		BF	ACING-	Structural wood	l sheathir		plied or 6-0-0 oc purlir	

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. All bearings 12-0-0.

Max Horz 2=-24(LC 14) (lb) -

- Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 13, 14, 11, 10
- Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -0-11-0 to 2-0-0, Exterior(2N) 2-0-0 to 6-0-0, Corner(3R) 6-0-0 to 9-0-0, Exterior(2N) 9-0-0 to 12-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 13, 14, 11, 10.
- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Rigid ceiling directly applied or 10-0-0 oc bracing.

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