Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

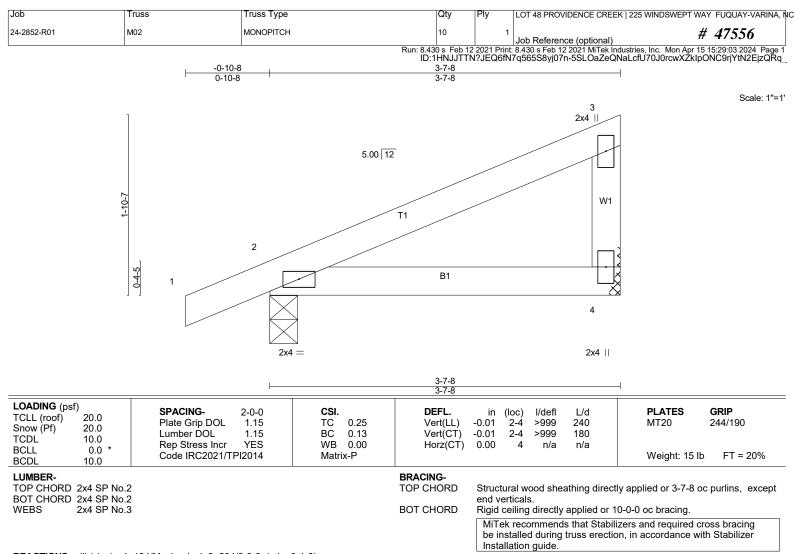
AST #: 47556 JOB: 24-2852-R01 JOB NAME: LOT 48 PROVIDENCE CREEK Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 35 These truss designs comply with IRC 2015 as well as IRC 2018. 22 Truss Design(s)

Trusses:

M02, R01, R02, R03, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, SP01, SP02, VT01, VT02, VT03, VT04, VT05, VT06



Warning !—Verify design parameters and read notes before use.



REACTIONS. (lb/size) 4=124/Mechanical, 2=204/0-3-8 (min. 0-1-8) Max Horz 2=61(LC 11) Max Uplift4=-27(LC 14), 2=-41(LC 14) Max Grav 4=167(LC 21), 2=285(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads. 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

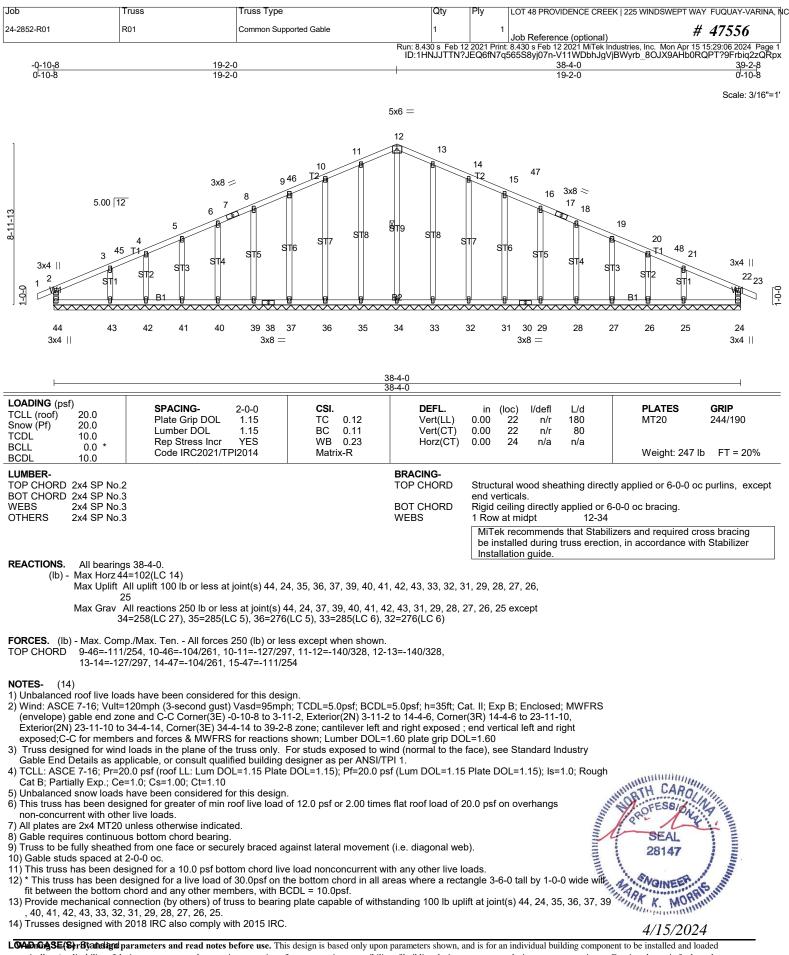
between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

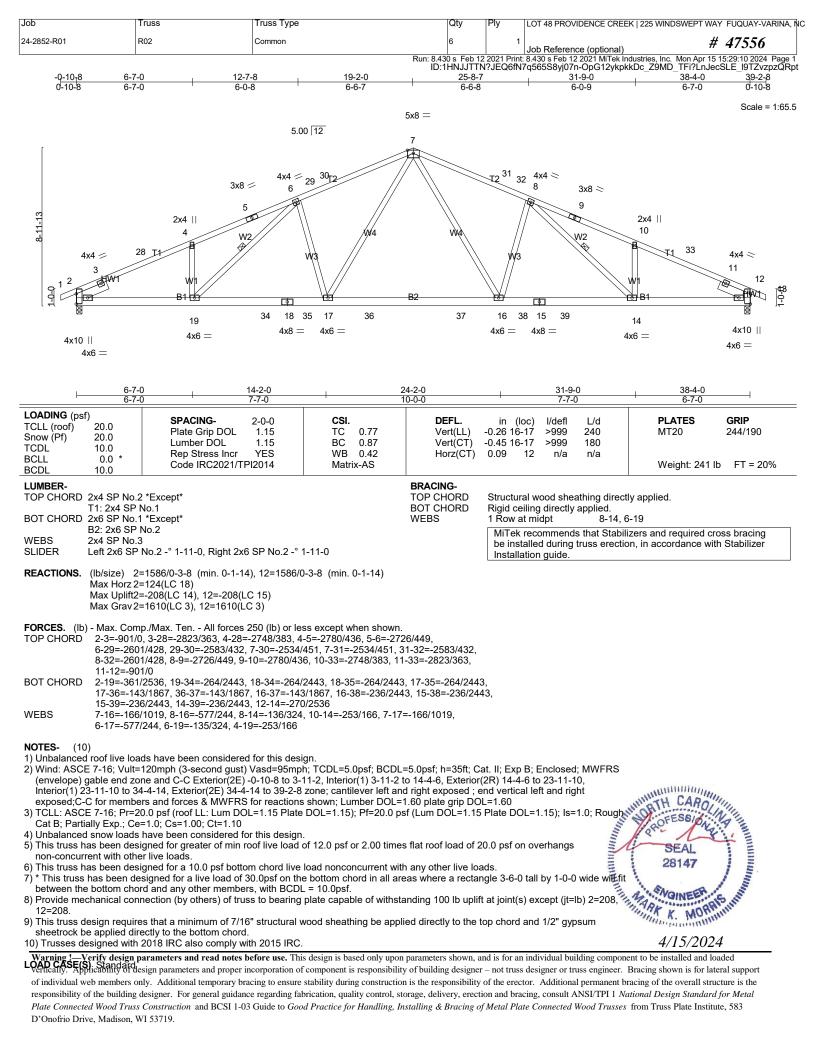
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

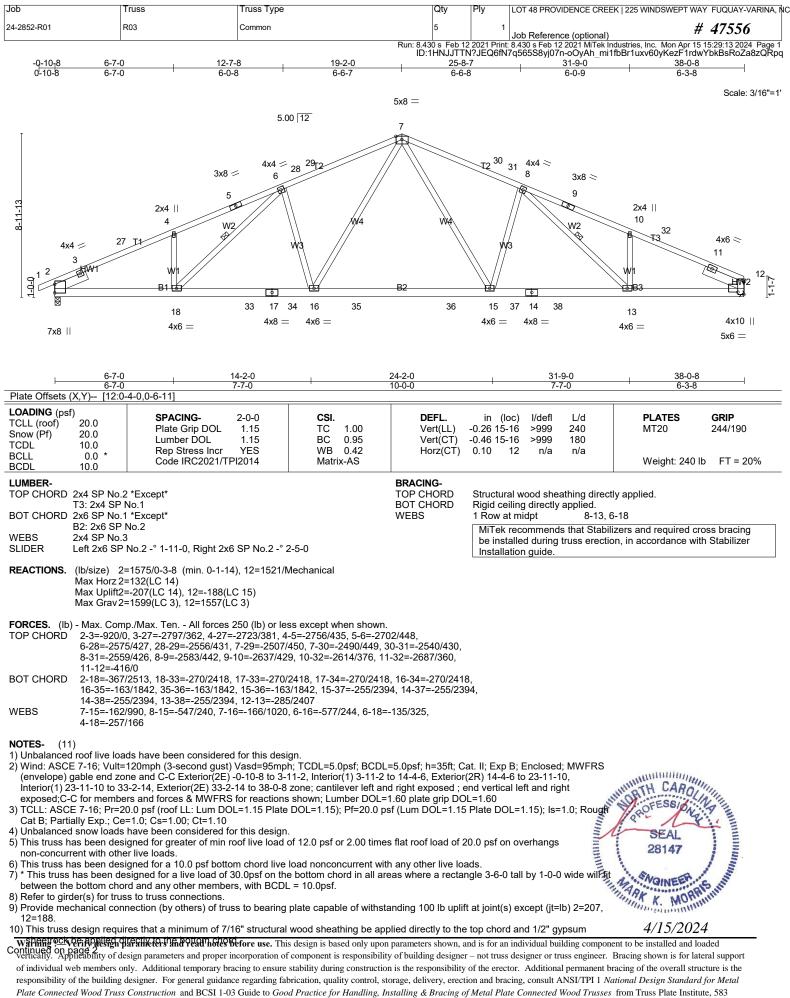
LOAD CASE(S) Standard





vertically. Applicability of design parameters and roter three orthorized in order on point is responsibility of building designer – not truss designer of truss designer. Bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusse Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





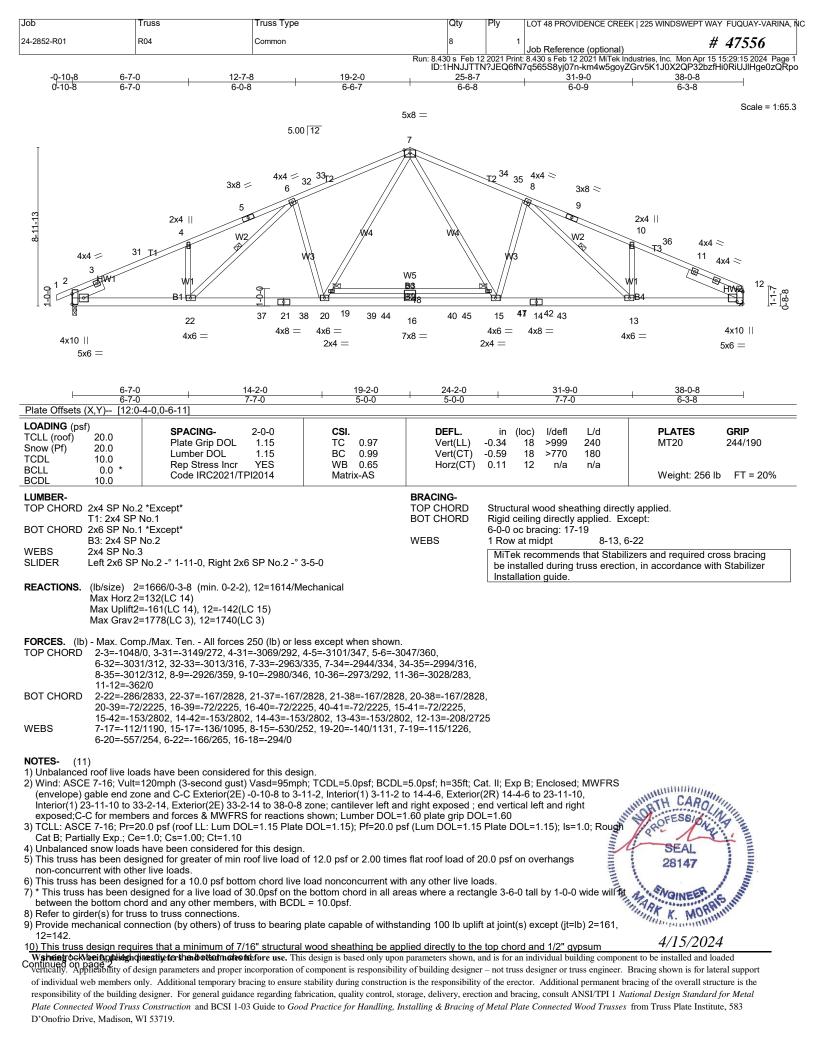
D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 48 PROVIDENCE CREEK 225 WINDSWEPT WAY FUQUAY-VARINA, NC	
24-2852-R01	R03	Common	5	1	Job Reference (optional) # 47556	
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Apr 15 15:29:13 2024 Page 2 ID:1HNJJTTN?JEQ6fN7q565S8yj07n-oOyAh_mi1fbBr1uxv60yKezF1rdwYbkBsRoZa8zQRpq						

11) Trusses designed with 2018 IRC also comply with 2015 IRC.

LOAD CASE(S) Standard



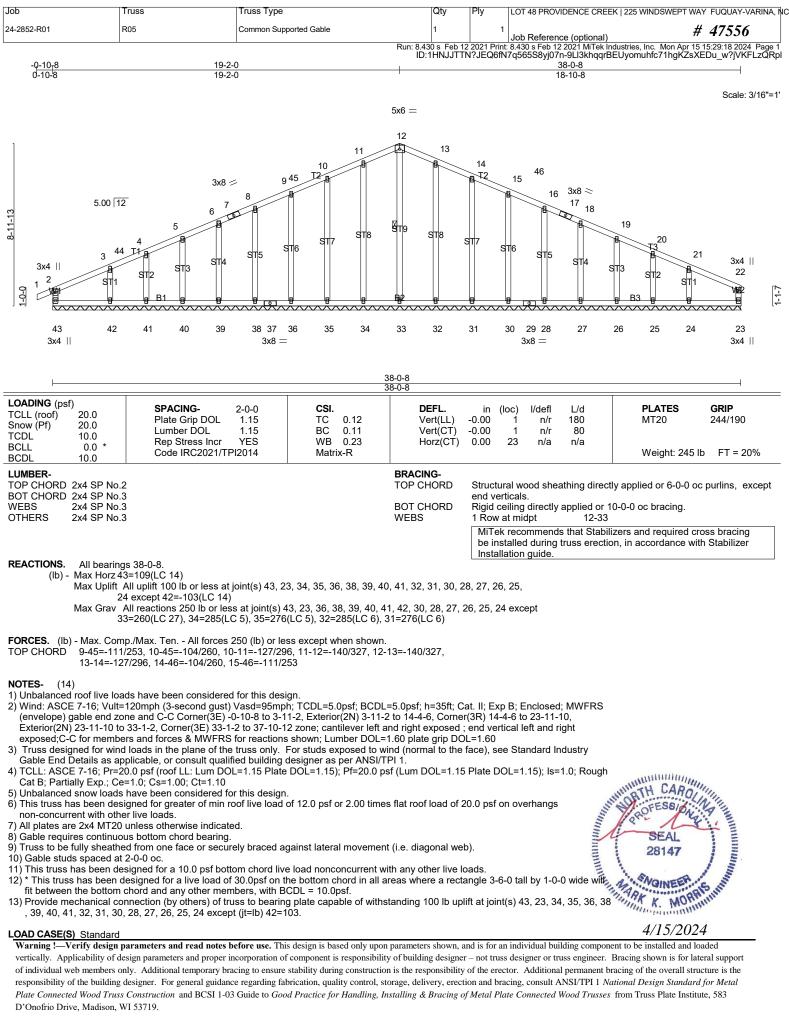


Job	Truss	Truss Type	Qty	Ply	LOT 48 PROVIDENCE CREEK 225 WINDSWEPT WAY FUQUAY-VARINA, NC	
24-2852-R01	R04	Common	8	1	Job Reference (optional) # 47556	
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Apr 15 15:29:16 2024 Page 2 ID:1HNJJTTN?JEQ6fN7q565S8yj07n-CzdIJ?oaJa_miUcWaEafxGbmj2dxluydYP0DATzQRpn						

11) Trusses designed with 2018 IRC also comply with 2015 IRC.

LOAD CASE(S) Standard





of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583

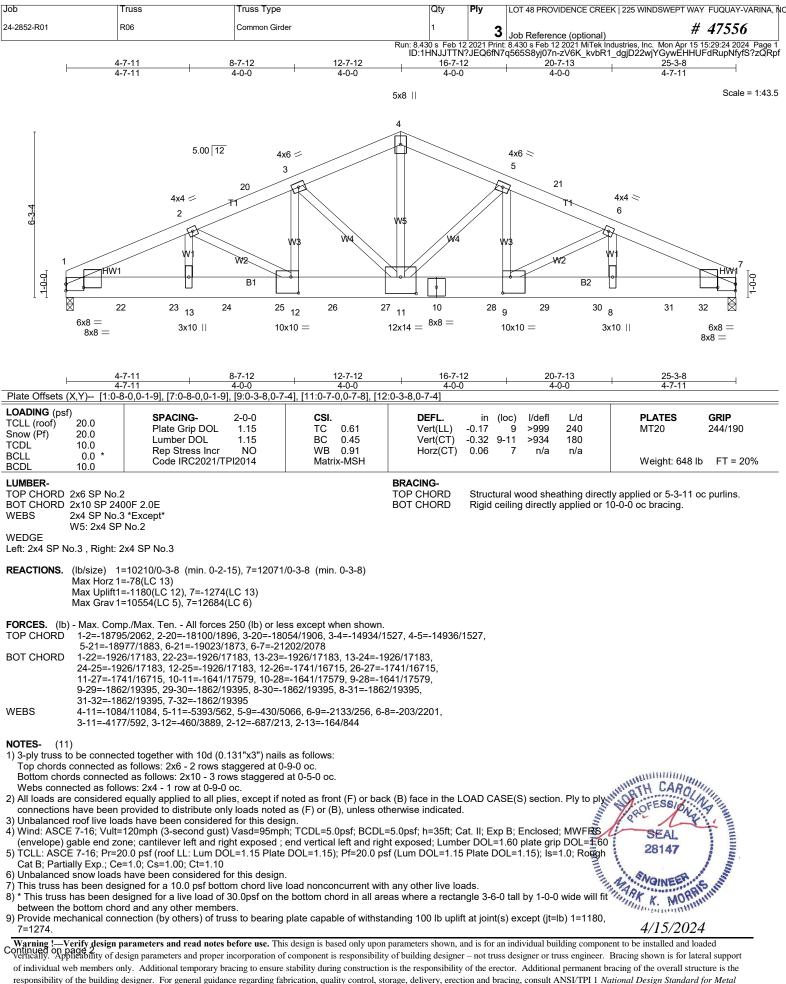


Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 48 PROVIDENCE CREEK 225 WINDSWEPT WAY FUQUAY-VARINA, NC	
24-2852-R01	R06	Common Girder	1	3	Job Reference (optional) # 47556	
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Apr 15 15:29:24 2024 Page 2 ID:1HNJJTTN?JEQ6fN7q565S8yj07n-zV6K_kvbR1_dgjD22wjYGywEHHUFdRupNfyfS?zQRpf						

NOTES- (11)

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1537 lb down and 208 lb up at 2-0-12, 1537 lb down and 208 lb up at 4-0-12, 1537 lb down and 208 lb up at 6-0-12, 1537 lb down and 208 lb up at 6-0-12, 1537 lb down and 208 lb up at 8-0-12, 1720 lb down and 162 lb up at 10-0-12, 1720 lb down and 162 lb up at 12-0-12, 1720 lb down and 162 lb up at 14-0-12, 1720 lb down and 162 lb up at 12-0-12, 1720 lb down and 162 lb up at 20-0-12, 1720 lb down and 208 lb up at 24-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

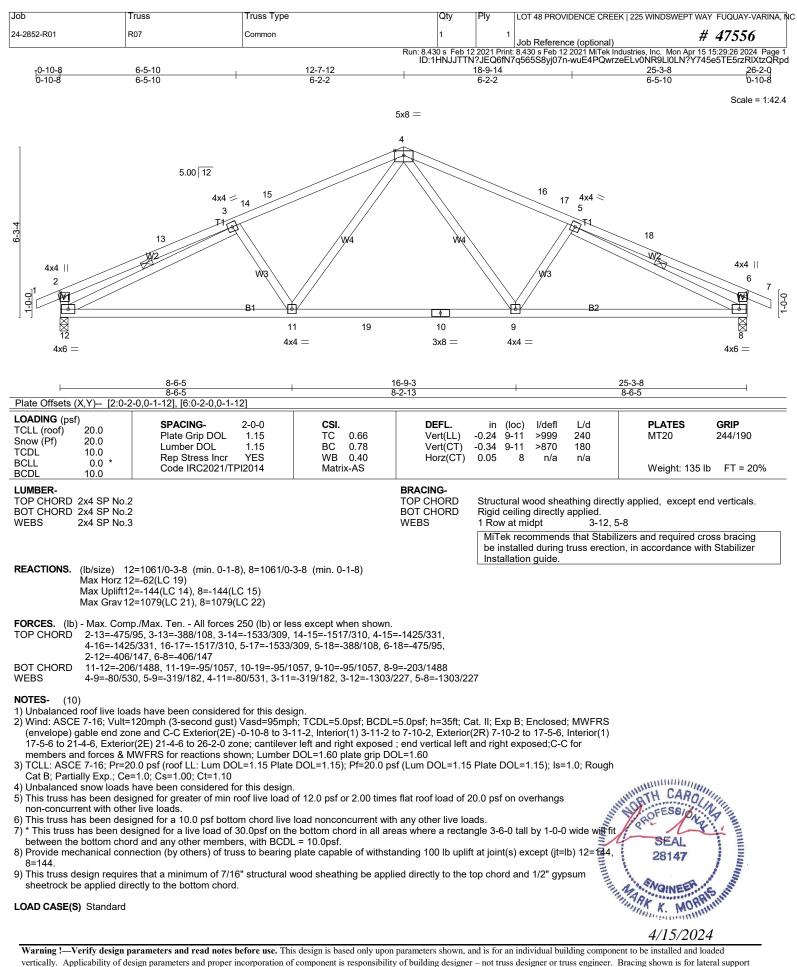
Uniform Loads (plf)

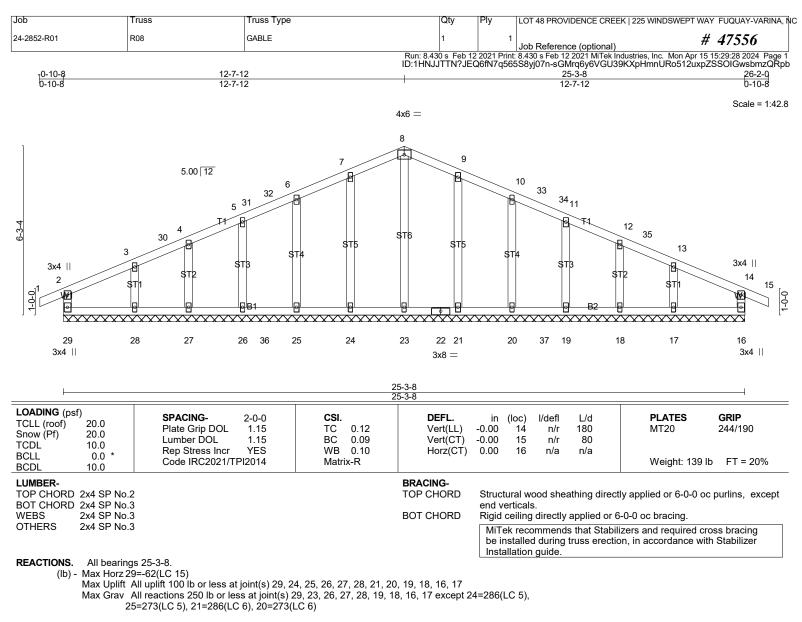
Vert: 1-4=-60, 4-7=-60, 14-17=-20

Concentrated Loads (lb) Vert: 10=-1594(B) 8=-1594(B) 22=-1501(B) 23=-1501(B) 24=-1501(B) 25=-1501(B) 26=-1594(B) 27=-1594(B) 28=-1594(B) 29=-1594(B) 30=-1594(B) 31=-1594(B) 20=-1594(B) 2

32=-1501(B)







FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(14)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 7-10-2, Exterior(2R) 7-10-2 to 17-5-6, Interior(1) 17-5-6 to 21-4-6, Exterior(2E) 21-4-6 to 26-2-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

- non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- TH CARO 12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 29, 24, 25, 26, 2댣28 ANNU ANARX 21, 20, 19, 18, 16, 17.

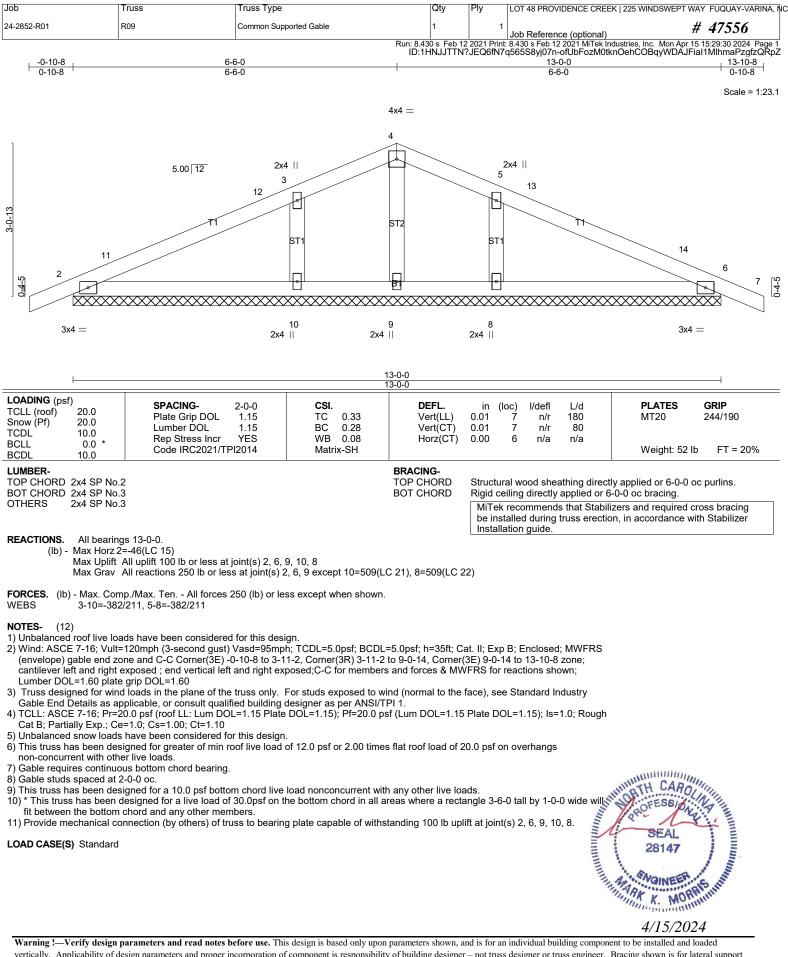
LOAD CASE(S) Standard

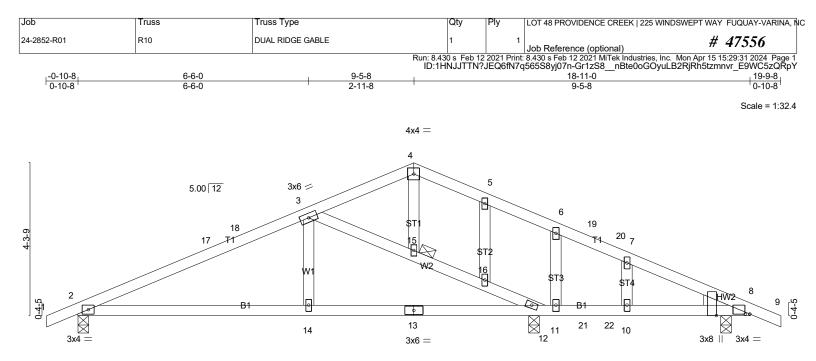
MORPHS INTERNAL MORPHS INTERNA Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

MARK

NOINEE

4/15/2024





.....

4/15/2024

.....

H	6-6-0 6-6-0	13-0-0 6-6-0		<u>18-4-12</u> 5-4-12	<u>18-11-</u> 0 0-6-4			
Plate Offsets (X,Y) [8:0-1								
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. DEFL TC 0.54 Vert(BC 0.51 Vert(WB 0.25 Horz(Matrix-SH	LL) 0.07 8-10 >970 CT) -0.12 2-14 >999	0 240 MT20 9 180	244/190			
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 WEDGE	2	BRACING- TOP CHOF BOT CHOF JOINTS	RD Structural wood s RD Rigid ceiling direc 1 Brace at Jt(s): 1	heathing directly applied or 4- tly applied or 10-0-0 oc bracir 5 nds that Stabilizers and requi	ng.			
Right: 2x4 SP No.3				ng truss erection, in accordan				
Max Horz 2 Max Uplift2	REACTIONS. (Ib/size) 2=590/0-3-8 (min. 0-1-8), 8=308/0-3-8 (min. 0-1-8), 12=715/0-3-8 (min. 0-1-8) Max Horz 2=64(LC 14) Max Uplift2=-97(LC 14), 8=-75(LC 11), 12=-100(LC 11) Max Grav 2=660(LC 21), 8=340(LC 22), 12=736(LC 22)							
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-17=-1009/136, 17-18=-921/138, 3-18=-891/148, 5-6=-280/71, 7-8=-267/0 BOT CHORD 2-14=-95/857, 13-14=-95/857, 12-13=-95/857 WEBS 3-15=-817/147, 15-16=-817/142, 12-16=-821/149, 3-14=0/278, 6-11=-404/252								
 NOTES- (10) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2; Interior(1) 3-11-2 to 4-7-14, Exterior(2R) 4-7-14 to 14-3-2; Interior(1) 14-3-2 to 14-11-14, Exterior(2E) 14-11-14 to 19-9-8 zone; cantilever left and right exposed; end vertical left and right exposed; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15) Plate DOL=1.15) plate DOL=1.15); ls=1.0; Rough Cat B; Partially Exp; Ce=1.0; Cs=1.00; Ct=1.10 4) Unbalanced snow loads have been considered for this design. 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. 6) All plates are 2x4 MT20 unless otherwise indicated. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will between the bottom chord and any other members. 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8 except (jt=lb) LOAD CASE(S) Standard 								
LOAD CASE(S) Standard				ANGINE ANGINE ARK K. N	ORAS INTERNET			

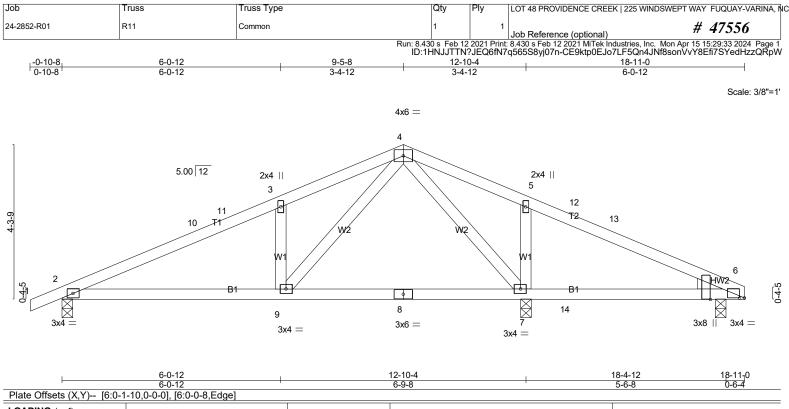


Plate Offsets (X,Y) [6:0-	1-10,0-0-0], [6:0-0-8,Edge]								
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.52 BC 0.53 WB 0.36 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl 0.10 6-7 >744 0.08 6-7 >905 0.01 7 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 83 lb	GRIP 244/190 FT = 20%		
LUMBER- TOP CHORD 2x4 SP No.: BOT CHORD 2x4 SP No.: WEBS 2x4 SP No.: WEDGE Right: 2x4 SP No.3	2		BRACING- TOP CHORD BOT CHORD	Structural wood sheat Rigid ceiling directly a MiTek recommends be installed during tr Installation guide.	applied or 6-0-0 that Stabilizer) oc bracing. s and required cro	oss bracing		
Max Hórz 2 Max Uplift2	2=516/0-3-8 (min. 0-1-8), 7=918/0-3-8 2=69(LC 14) 2=-91(LC 14), 7=-100(LC 11), 6=-52(LC 2=586(LC 21), 7=946(LC 22), 6=151(LC	C 11)	-8 (min. 0-1-8)						
TOP CHORD 2-10=-861 BOT CHORD 2-9=-94/72	BOT CHORD 2-9=-94/726								
 2) Wind: ASCE 7-16; Vult= (envelope) gable end zc Exterior(2E) 13-11-10 tc members and forces & I 3) TCLL: ASCE 7-16; Pr=2 Cat B; Partially Exp.; Ce 4) Unbalanced snow loads 5) This truss has been des non-concurrent with othe 6) This truss has been des 7) * This truss has been des 7) * This truss has been des 8) Provide mechanical con 7=100. 9) Graphical bracing represe the member must be brace 10) Bearing symbols are o structural design of the 11) Web bracing shown is Installing, Restraining of 	nly graphical representations of a pose e truss to support the loads indicated. for lateral support of individual web me & Bracing of Metal Plate Connected W	 TCDL=5.0psf; BCDL= 11-2, Interior(1) 3-11-2 t exposed; end vertical le DOL=1.60 plate grip DC DOL=1.15); Pf=20.0 ps of 12.0 psf or 2.00 times load nonconcurrent with e bottom chord in all are plate capable of withsta e or the orientation of the sible bearing condition. I embers only. Refer to Br (ood Trusses for additio) 	o 4-7-14, Exterior oft and right expose DL=1.60 of (Lum DOL=1.15 of any other live load of 2 oft any other live load oft any other load oft any other live load oft any other load oft any othe	(2R) 4-7-14 to 13-11-10 sed; porch right exposed Plate DOL=1.15); Is=1 20.0 psf on overhangs ads. ngle 3-6-0 tall by 1-0-0 v t at joint(s) 2, 6 except (ember. Symbol only indi are not considered in th nod Practice for Handlin lines, including diagona), d;C-C for .0; Rough	SEAL 28147	A aded		
MINIMUM BRACING F	ARY SHEET- PERMANENT RESTRAIL REQUIREMENTS OF TOP CHORD, B S, ALWAYS CONSULT THE PROJEC	OTTOM CHORD, AND CT ARCHITECT OR EN	WEB PLANES. I GINEER FOR AD	N ADDITION TO THES DITIONAL BRACING	ENDED E	4/15/2024	4		
	parameters and read notes before use. This esign parameters and proper incorporation of nly. Additional temporary bracing to ensure				ss engineer. Brac	cing shown is for late	eral support		

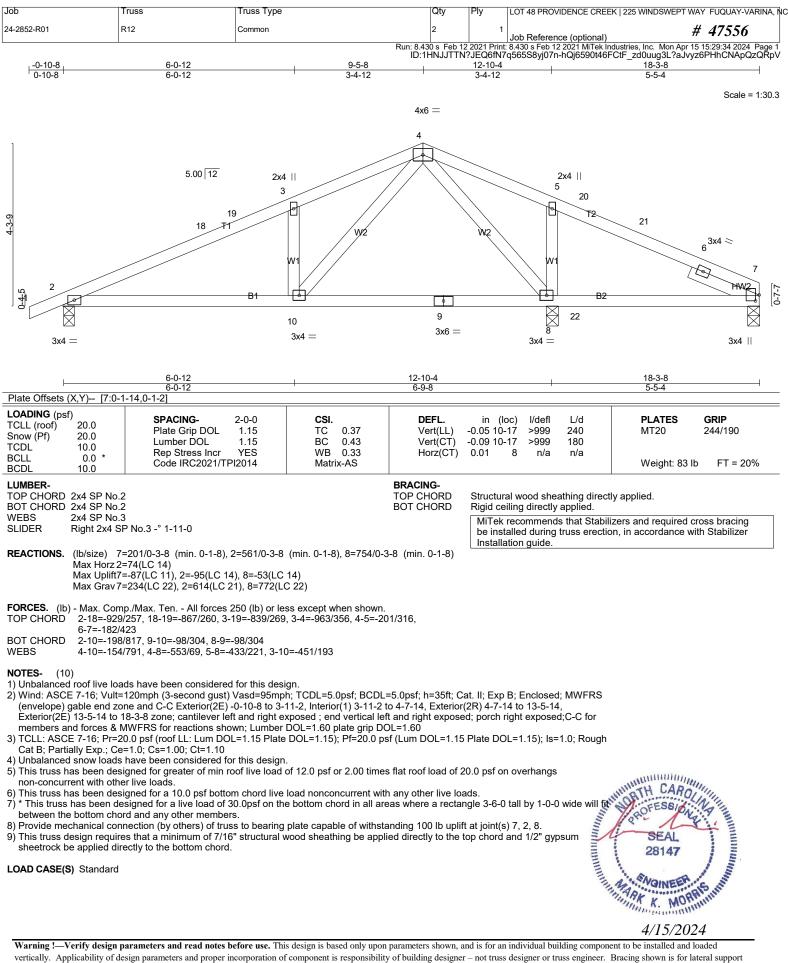
ONLINES OF DEGENERATION OF DEGENERATION OF DEGENERATION OF COMPONENT IN CONTROL OF DEGENERATION OF DEGENERA

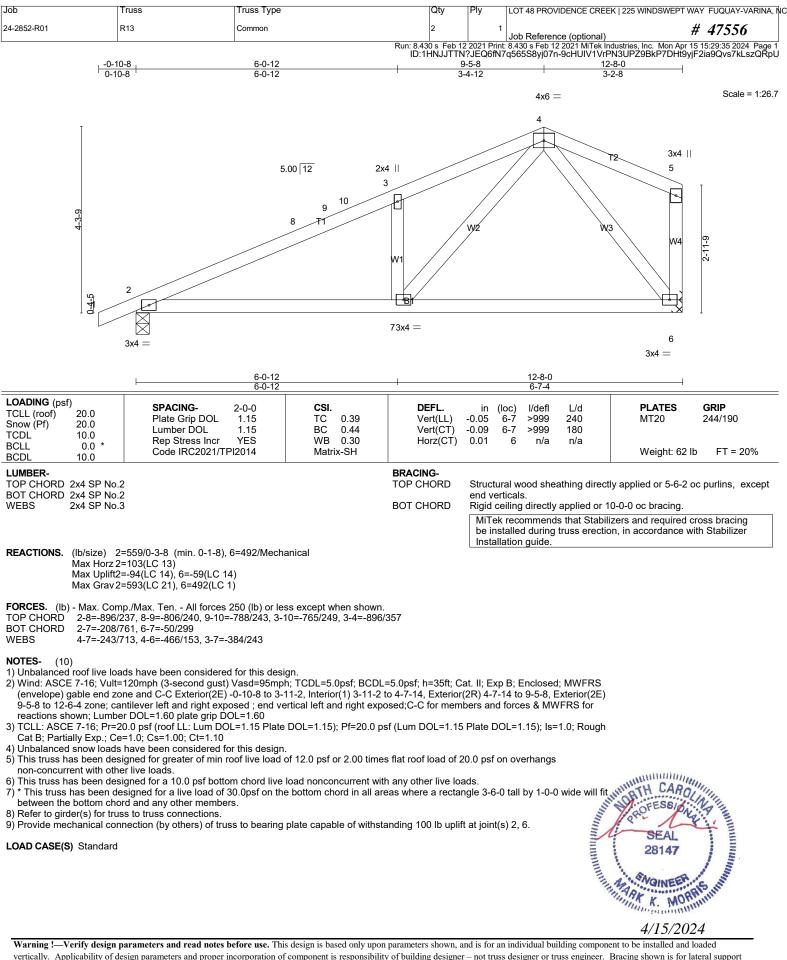
Job	Truss	Truss Type	Qty	Ply	LOT 48 PROVIDENCE CREEK 225 WINDSWEPT WAY FUQUAY-VARINA, NC
24-2852-R01	R11	Common	1	1	Job Reference (optional) # 47556

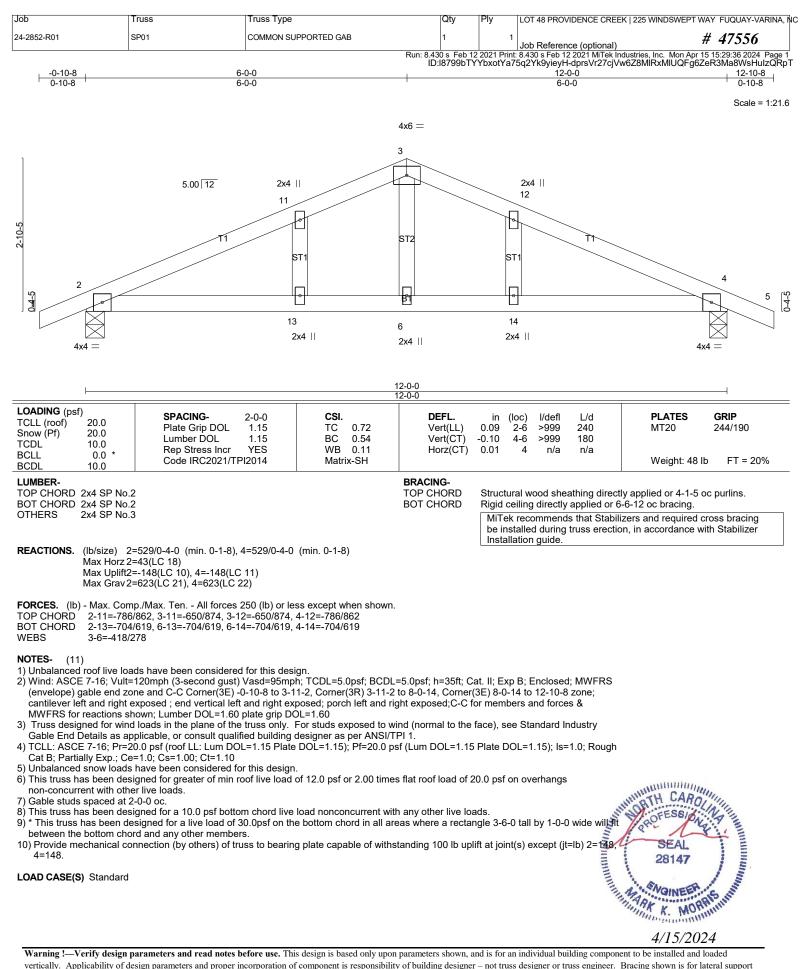
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Apr 15 15:29:33 2024 Page 2 ID:1HNJJTTN?JEQ6fN7q565S8yj07n-CE9ktp0EJo7LF5Qn4JNf8sonVvY8Efi7SYedHzzQRpW

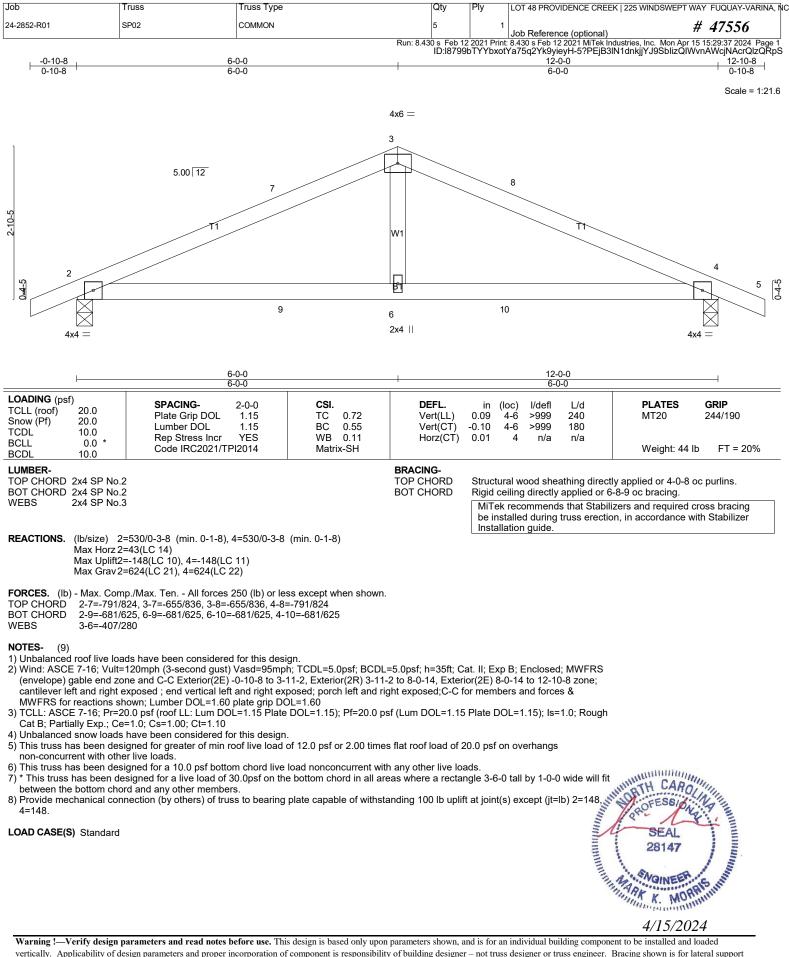
LOAD CASE(S) Standard

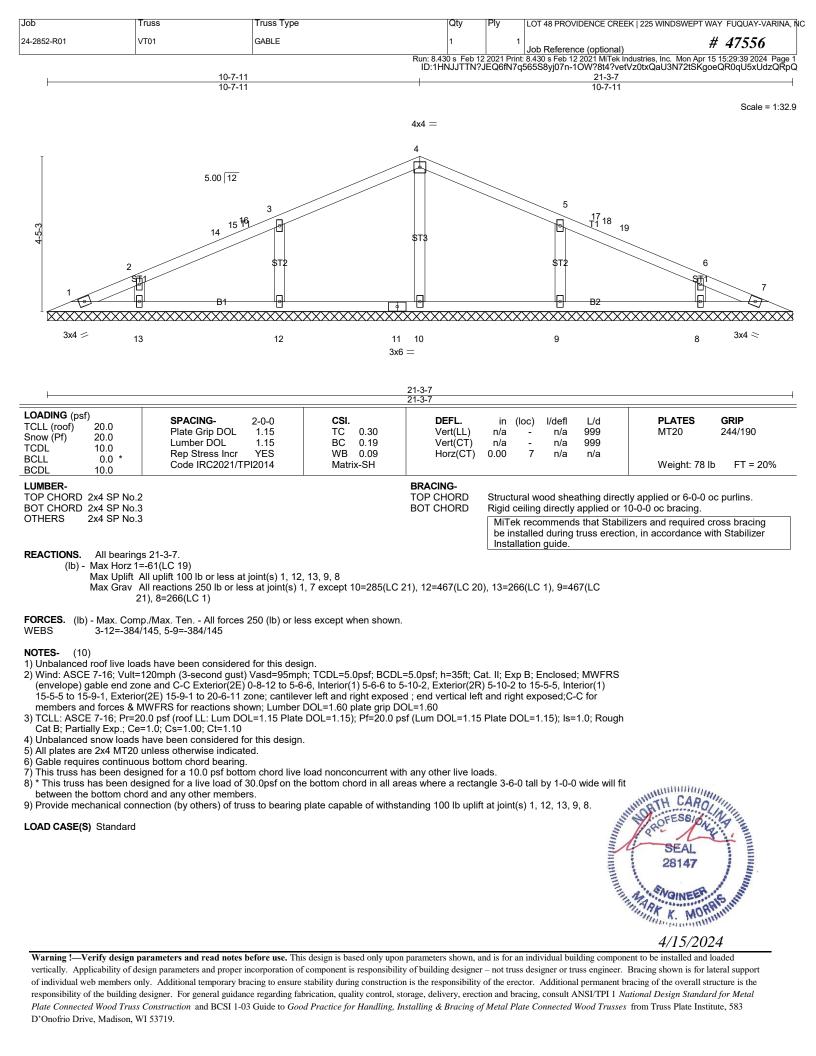


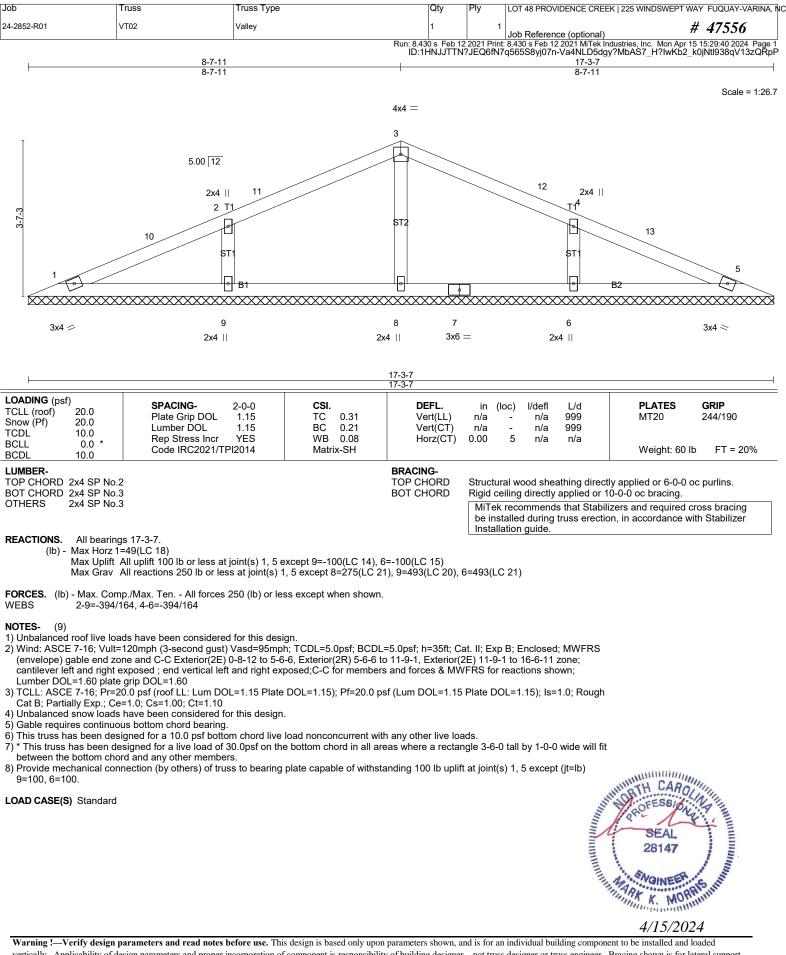


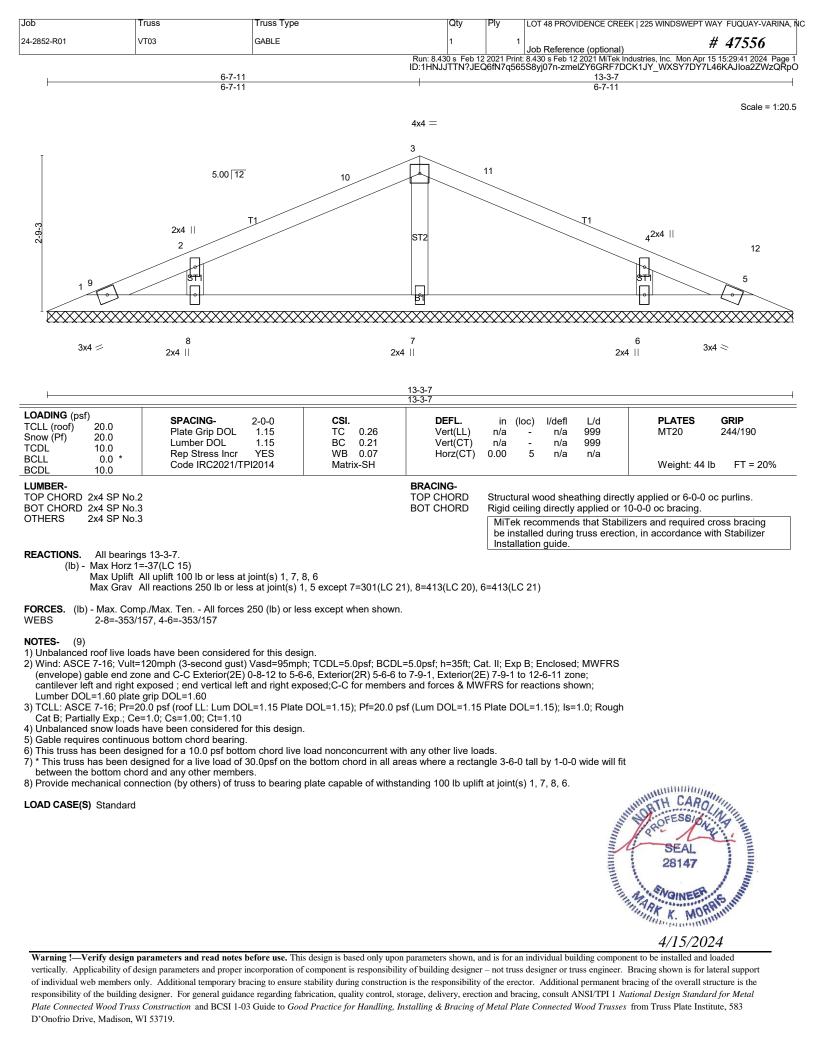


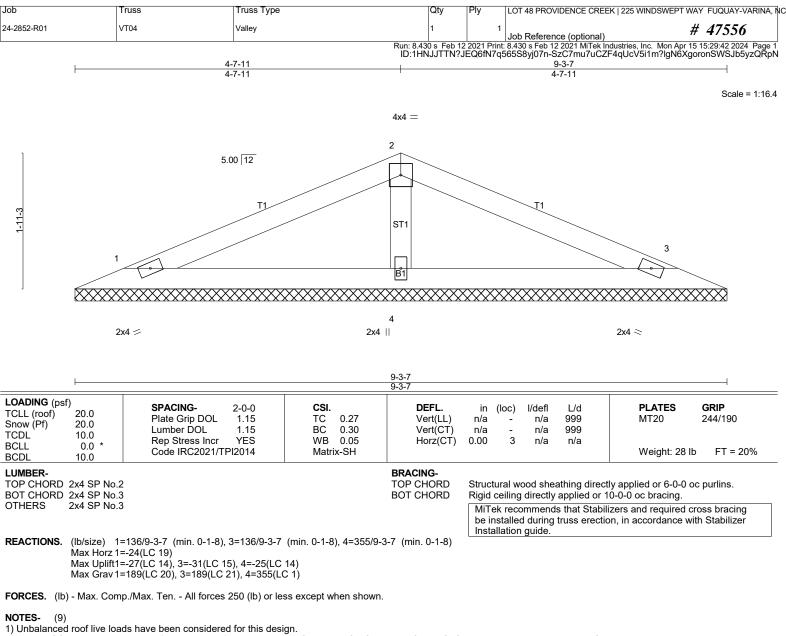












2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

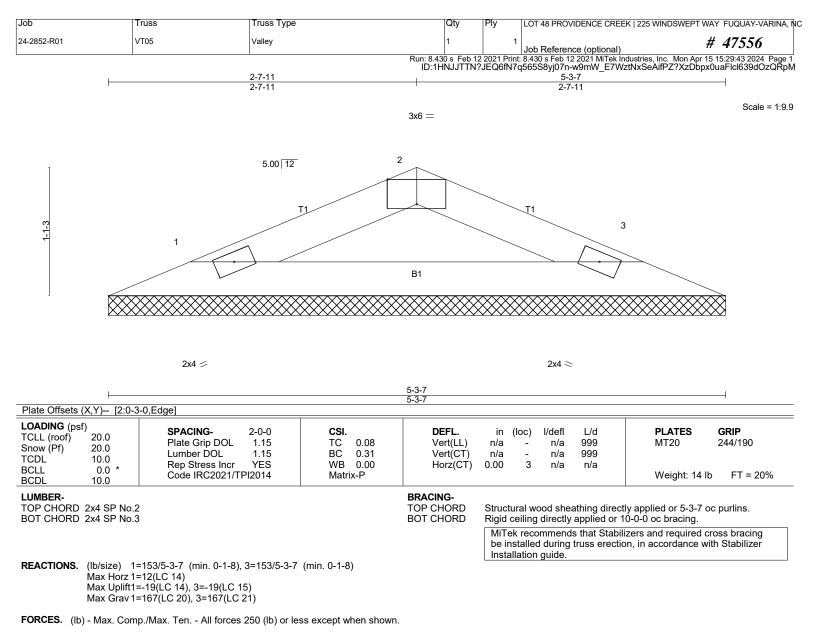
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard





NOTES- (9

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough
- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

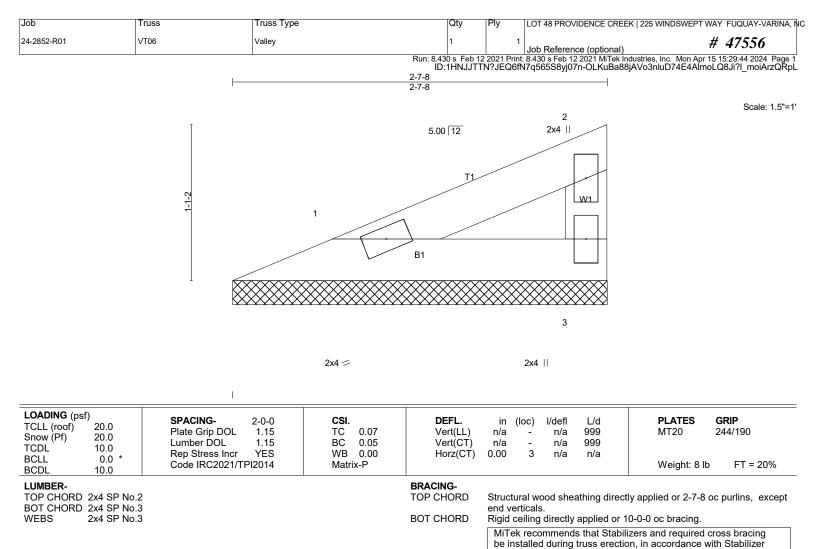
7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

```
LOAD CASE(S) Standard
```





Installation guide

REACTIONS. (lb/size) 1=70/2-7-8 (min. 0-1-8), 3=70/2-7-8 (min. 0-1-8) Max Horz 1=27(LC 11) Max Uplift1=-10(LC 14), 3=-15(LC 14) Max Grav 1=86(LC 20), 3=86(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard

