Mark Morris, P.E.

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The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 47557 JOB: 24-2853-R01 JOB NAME: LOT 9 PROVIDENCE CREEK Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 35 These truss designs comply with IRC 2015 as well as IRC 2018. 22 Truss Design(s)

Trusses:

J01, J03, J04, J05, J06, R01, R02, R02A, R02B, R03, R03A, R04, R05, R06, R07, R08, R09, SP01, SP02, V01, V02, V03



Warning !—Verify design parameters and read notes before use.



- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.

LOAD CASE(S) Standard

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=123.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum
- sheetrock be applied directly to the bottom chord.







- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.
- LOAD CASE(S) Standard





REACTIONS. (lb/size) 4=251/Mechanical, 2=344/0-3-8 (min. 0-1-8) Max Horz 2=83(LC 11) Max Uplift4=-77(LC 8), 2=-114(LC 8) Max Grav 4=290(LC 19), 2=449(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (11)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=114.

9) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 4-5=-20 Concentrated Loads (lb) Vert: 8=-43(F) 9=-123(F)





- non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.

LOAD CASE(S) Standard

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.





of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583



14

4x8 =

13

4x8 =

12

4x4 =

ALITANA MA

NOINEE

ARK K. MORA

4/15/2024

11

4x6 =

23



2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 13-11-1, Exterior(2R) 13-11-1 to 24-0-15, Interior(1) 24-0-15 to 32-9-2, Exterior(2E) 32-9-2 to 37-6-12 zone; cantilever left and right exposed ; end vertical left and right

exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TH CARO 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

16

4x4 =

15

4x8 =

22

- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide with the between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 17=207 11 = 185

10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

4x8 =



Job	Truss	Truss Type	Qty	Ply	LOT 9 PROVIDENCE CREEK 213 PROVIDEN	ICE CREEK DRIVE FUQUAY-VA
24-2853-R01	R02A	Roof Special	1	1	Job Reference (optional)	# 47557
		Run: 8	430 s Feb 1	2 2021 Print	: 8.430 s Feb 12 2021 MiTek Industries. Inc. Mor	Apr 15 15:30:46 2024 Page 2

ID:zSnl_VDJTy_ht?pmOJJgrKyZRJv-aH4KCnu17D.?VJVpr8BmbFKGQKNhrYetEPp_FzQRoN 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 9 PROVIDENCE CREEK 213 PROV	IDENCE CREEK DRIVE FUQUAY-VAR
24-2853-R01	R02B	Roof Special	4	1	Job Reference (optional)	# 47557
		Run: 8.4	30 s Feb 12	2 2021 Print	: 8.430 s Feb 12 2021 MiTek Industries. Inc.	Mon Apr 15 15:30:49 2024 Page 2

ID:zSnl_VDJTy_hu?pmOjJgrKyZRJv-_slSqpwvQ8hZNnDOXGkTDtyktYPu2sgJwN2eyazQRoK

11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

Connected Wood Trustees for additional bracing guidelines, including diagonal bracing. 14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard











	7-1-1	2 14-0-0	19-0-0	24-0-0		30-10-4	38-0-0	
Plate Offse	ets (X,Y) [13:0-	2-8,0-1-12], [22:0-2-8,0-1-12]	3-0-0	5-0-0		0-10-4	7-1-12	
LOADING TCLL (roof Snow (Pf) TCDL BCLL BCDL	(psf)) 20.0 20.0 10.0 0.0 * 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.58 BC 0.90 WB 0.88 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.61 18 -1.01 18 0.09 12	l/defl L/d >733 240 >445 180 n/a n/a	PLATES GRIP MT20 244/190 Weight: 236 lb FT = 20%	
LUMBER- TOP CHOP BOT CHOP WEBS	RD 2x4 SP No.2 RD 2x4 SP No.2 B3: 2x4 SP No.3 2x4 SP No.3 W1: 2x6 SP	*Except* No.1, B2: 2x4 SP SS *Except* No.2 3=1658/0-3-8 (min. 0-1-15), 12=	1658/0-3-8 (min. 0-1-15)	BRACING- TOP CHORD BOT CHORD	Structural v Rigid ceilin 6-0-0 oc bra MiTek rec be installe Installatio	vood sheathing direc g directly applied. E: acing: 17-19 ommends that Stabi d during truss erecti n guide.	tly applied, except end verticals. xcept: lizers and required cross bracing on, in accordance with Stabilizer	
	Max Horz 23 Max Uplift23 Max Grav 23	B=100(LC 14) B=-163(LC 14), 12=-163(LC 15) B=1660(LC 3), 12=1660(LC 3)						
FORCES. TOP CHOP	(Ib) - Max. Com RD 2-24=-2882 5-6=-2703/ 9-27=-2809	o./Max. Ten All forces 250 (lb) 2/276, 3-24=-2809/289, 3-4=-27 367, 6-7=-2703/367, 7-26=-258- 9/289, 10-27=-2882/276, 2-23=-	or less except when shown. 6/260, 4-25=-2649/282, 5-2 /284, 8-26=-2649/282, 8-9= 575/245, 10-12=-1575/245	5=-2584/284, -2716/260,				
BOT CHOP	RD 22-23=-183 16-29=-7/1 12-13=-89/	3/495, 21-22=-253/2593, 20-21= 904, 29-30=-7/1904, 15-30=-7/1 482	253/2593, 20-28=-7/1904, 1 004, 14-15=-162/2593, 13-1	l6-28=-7/1904, 4=-162/2593,				
WEBS	6-17=-156/ 6-19=-156/	1140, 15-17=-199/1025, 7-15=-⁄ 1140, 5-20=-482/190, 3-20=-37	82/190, 9-15=-371/175, 19- /175, 2-22=-117/2133, 10-1	20=-200/1025, 3=-117/2133				
NOTES- 1) Unbalan 2) Wind: A: (envelop 24-0-0 tr member 3) TCLL: A Cat B; P 4) Unbalan 5) This trus non-con 6) This trus 7) * This tru between 8) Provide 12=163. 9) This trus sheetroo	 6-19=-156/1140, 5-20=-482/190, 3-20=-371/175, 2-22=-117/2133, 10-13=-117/2133 NOTES- (10) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 14-0-0, Exterior(2R) 14-0-0 to 24-0-0, Interior(1) 24-0-0 to 34-0-14, Exterior(2E) 34-0-14 to 38-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough (24 B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Unbalanced snow loads have been considered for this design. 5) This truss has been designed for a 10.0 psf bottom chord live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide with fit between the bottom chord and any other members, with BCDL = 10.0psf. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 23=163. 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord. 							
LOAD CAS	E(S) Standard						4/15/2024	



Job	Truss	Truss Type	Qty	Ply	LOT 9 PROVIDENCE CREEK 213 PROVI	DENCE CREEK DRIVE FUQUAY-VA
24-2853-R01	R03A	Roof Special	1	1	Job Reference (optional)	# 47557
		Run: 8.43	30 s Feb 12	2 2021 Print	: 8.430 s Feb 12 2021 MiTek Industries, Inc.	Mon Apr 15 15:30:55 2024 Page 2

10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 11) Bearing symbols are not considered in the structural design of the truss to support the

loads indicated. 12) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

12) Yes blacking shown is to hater support of individual web members only. Telef to boot a back to boot a factor of hatering, instanting, nestanting a blacking of index a factor of hatering of individual web members only. Telef to boot a back to boot a factor of hatering, nestanting a blacking of index a factor of hatering of individual web members only. Telef to boot a back to boot a factor of hatering, nestanting a blacking of index a factor of hatering of individual web members only. Telef to boot a back to boot a factor of hatering, nestanting a blacking of individual web members only. Telef to boot a back to boot a factor of hatering, nestanting, nestanting a blacking of individual web members only. Telef to boot a back to boot a factor of individual web members only. Telef to boot a back to boot a factor of individual web members only. Telef to boot a back to boot a factor of individual web members only. Telef to boot a back to boot a factor of individual web members on a distribution of the boot a back to boot a factor of the boot a back to boot a ba

LOAD CASE(S) Standard









Warning 1—Verity design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	LOT 9 PROVIDENCE CREEK 213 PROVIDENCE CREEK D	RIVE FUQUAY-VAF
24-2853-R01	R07	Common Girder	1	2	Job Reference (optional) # 47	7557
		Run: 8.43 ID:zSr	0 s Feb 12 nl_VDJTy	2021 Print hu?pmOj	: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Apr 15 15:31 JgrKyZRJv-aYbImb5i7SSa2wl4LD_InqXB_BCvKFGN8	:03 2024 Page 2 3YRORmzQRo6

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 11-15=-20

Concentrated Loads (lb)

Vert: 17=-1546(B) 19=-1476(B) 20=-1472(B) 21=-1472(B) 22=-1472(B) 23=-1472(B)





NOTES-(14)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 4-0-0, Corner(3R) 4-0-0 to 8-0-0, Corner(3E) 8-0-0 to 12-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.

- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will be the bottom chord and any other members.
 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift of isotometric.
 LOAD CASE(0) of the bottom chord and any other members.

8) Gable requires composition of the second state of the MORPHS INTERNAL MORPHS NOINEE ARK K. MORR 4/15/2024 Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded

vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusse Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Trusse Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	LOT 9 PROVIDENCE CREEK 213 PRO	OVIDENCE CREEK DRIVE FUQUAY-VAR
24-2853-R01	R09	Common Girder	1	2	Job Reference (optional)	# 47557
		Rup: 8/13	80 s Eeh 12	2021 Print	8 430 s Eeb 12 2021 MiTek Industries In	nc Mon Apr 15 15:31:07 2024 Page 2

n: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 Mi Tek Industries, Inc. Mon Apr 15 15:31:07 2024 Page 2 ID:zSnI_VDJTy_hu?pmOjJgrKyZRJv-SJrGcz8CAgz0XYbra23hxghwAoXmG1rz3APbaXzQRo2

LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 7-11=-20

Concentrated Loads (lb) Vert: 6=-1476(B) 15=-1476(B) 16=-1476(B) 17=-1476(B) 18=-1476(B)







vertically. Applicability of design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be instanted and toaded vertically. Applicability of design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be instanted and toaded of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



- **NOTES-** (9)
- 1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard





H			4-1-0					
Plate Offsets (X,Y) [2:0-3-	0,Edge]		4-1-0					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.03 BC 0.14 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 10 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD	Structural w Rigid ceiling	ood shea g directly	athing direct applied or	ctly applied or 4-1-0 od 10-0-0 oc bracing.	c purlins.
				MiTek rec be installe Installatior	ommends d during n guide.	s that Stab truss erect	ilizers and required cr ion, in accordance wit	oss bracing h Stabilizer

REACTIONS. (lb/size) 1=105/4-1-0 (min. 0-1-8), 3=105/4-1-0 (min. 0-1-8) Max Horz 1=-8(LC 15) Max Uplift1=-13(LC 14), 3=-13(LC 15) Max Grav 1=111(LC 20), 3=111(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(9)

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough
- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

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LOAD CASE(S) Standard
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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard

