Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 47606 JOB: 24-3026-R01 JOB NAME: LOT 0.0042 HONEYCUTT HILLS Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. *41 Truss Design(s)*

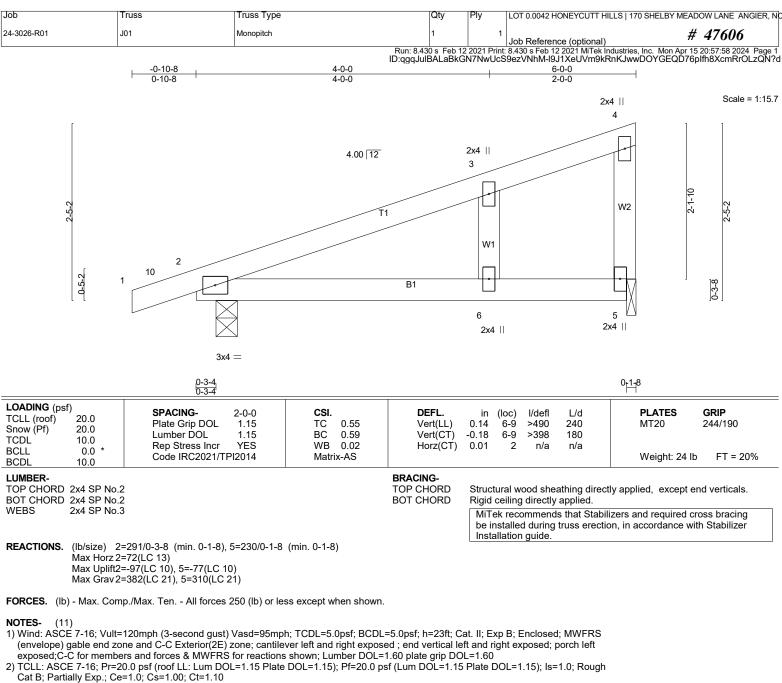
Trusses:

J01, J01A, J02, J04, J05, J06, J08, J09, J10, J11, J12, P02, R01, R02, R05, R06, R06A, R07, R08, R09, R10, R11, R12, R13, R14, R14A, R15, R16, R17, V01, V02, V03, V04, V05, V06, V07, V08, V09, V10, V11, V12



Warning !--- Verify design parameters and read notes before use.

This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for*



3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

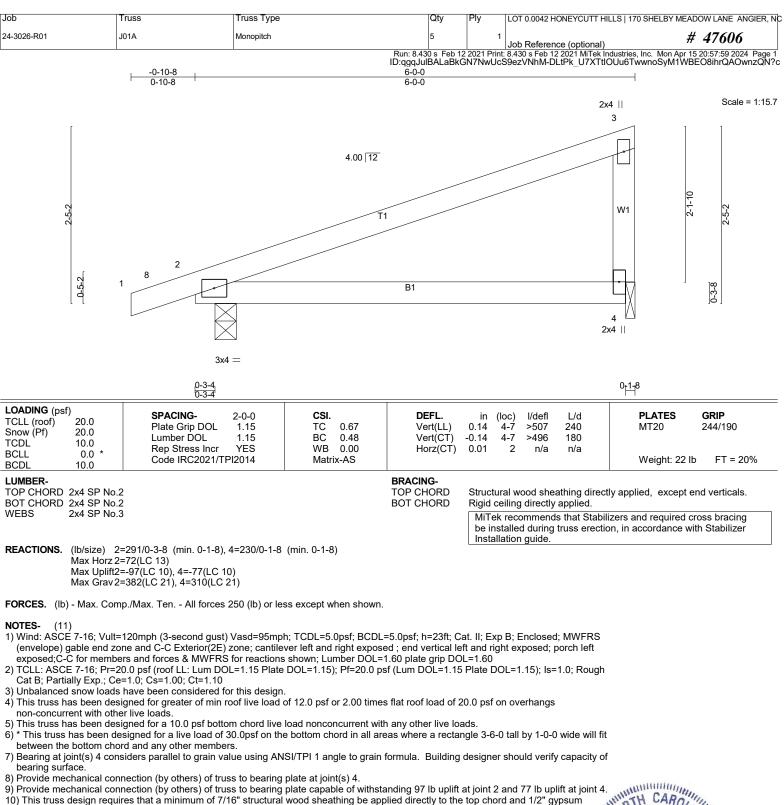
non-concurrent with other live loads.

LOAD CASE(S) Standard

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 97 lb uplift at joint 2 and 77 lb uplift at joint 5.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

joint 5. SEAL 28147 SEAL 28147

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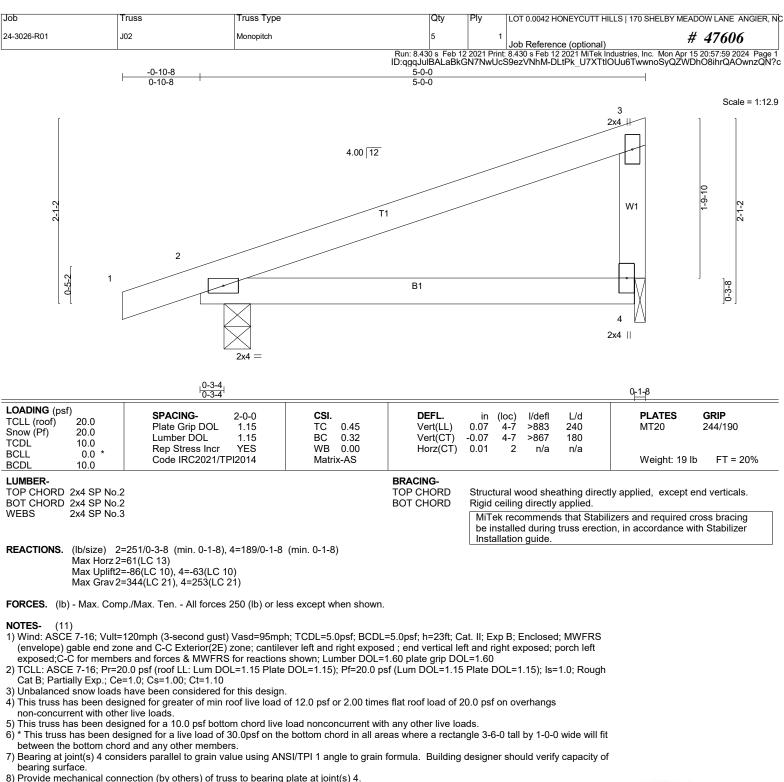


sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

lift at joint 4. /psum // CROFESSION / P // SEAL 28147 // RANKERRE

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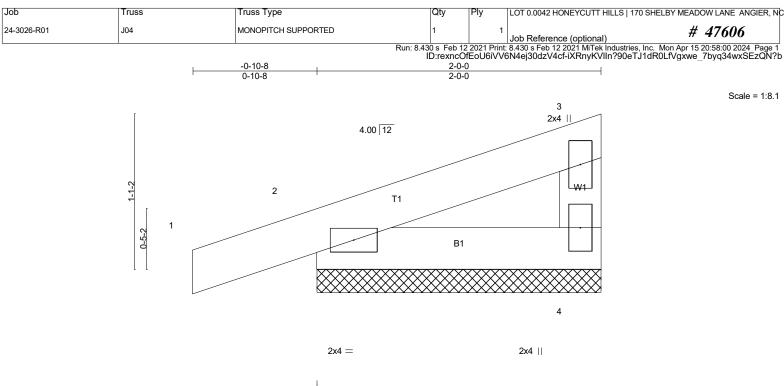


- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 2 and 63 lb uplift at joint 4.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum
- sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



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LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * DOD 40.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.09 BC 0.06 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d 0.00 1 n/r 180 0.00 1 n/r 80 0.00 4 n/a n/a	PLATES GRIP MT20 244/190 Weight: 8 lb FT = 20%
BCDL10.0LUMBER-TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.3WEBS2x4 SP No.3			BRACING- TOP CHORD BOT CHORD	end verticals. Rigid ceiling directly applied or MiTek recommends that Stabi	tly applied or 2-0-0 oc purlins, except 10-0-0 oc bracing. lizers and required cross bracing on, in accordance with Stabilizer

REACTIONS. (lb/size) 4=62/2-0-0 (min. 0-1-8), 2=139/2-0-0 (min. 0-1-8) Max Horz 2=27(LC 11) Max Uplift4=-9(LC 14), 2=-41(LC 10) Max Grav 4=76(LC 21), 2=180(LC 21)

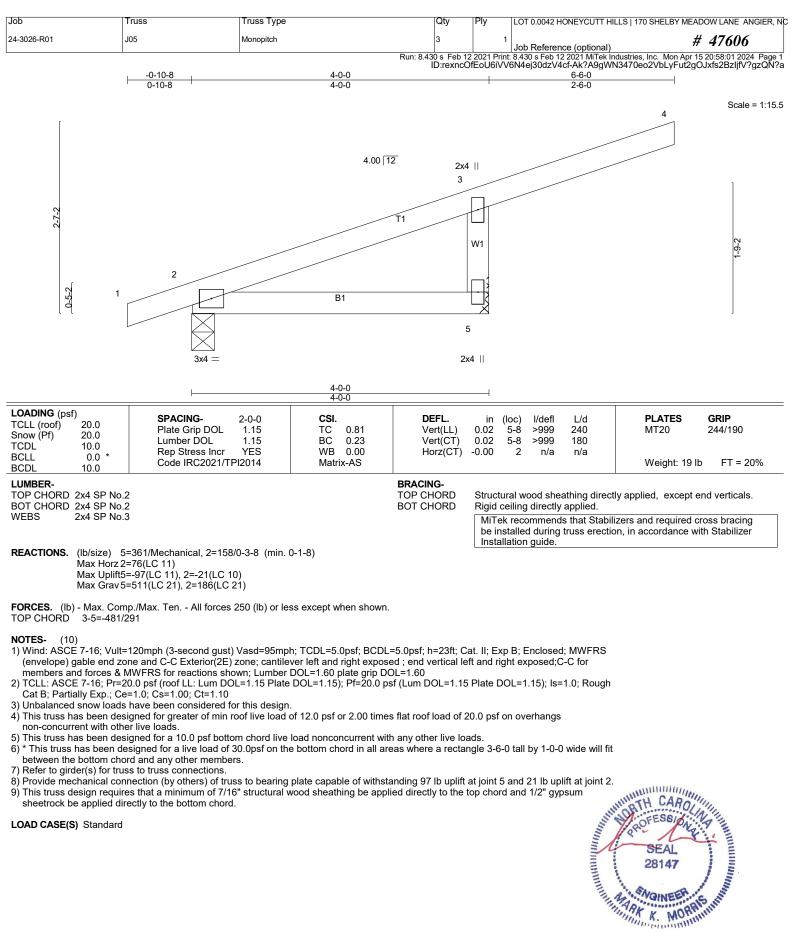
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

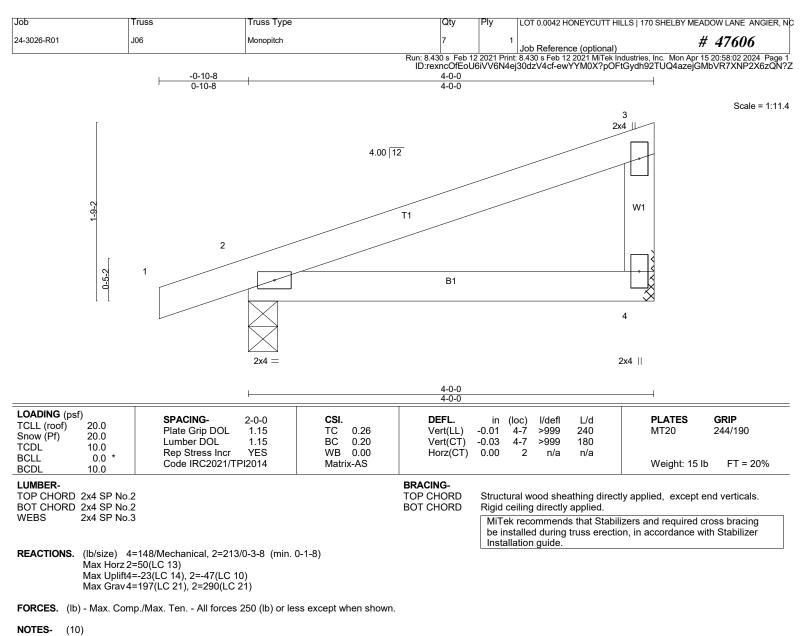
- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads. 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- between the bottom chord and any other members. 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 4 and 41 lb uplift at joint **2 LOAD CASE(S)** Standard

A Summer and A State SEAL 28147 VOINE K. MORP

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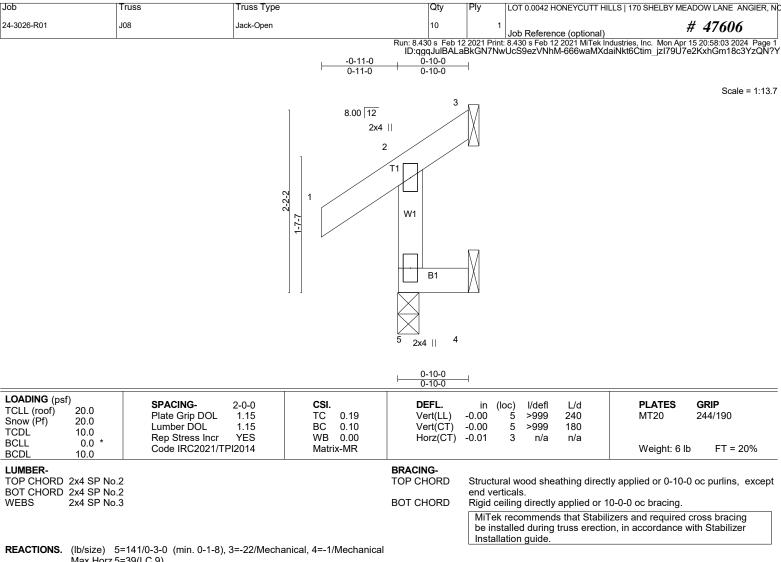
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.

LOAD CASE(S) Standard

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 4 and 47 lb uplift at joint 2.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum
- sheetrock be applied directly to the bottom chord.



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Max Horz 5=39(LC 9)

Max Uplift3=-63(LC 18), 4=-29(LC 9) Max Grav 5=203(LC 18), 3=12(LC 10), 4=33(LC 10)

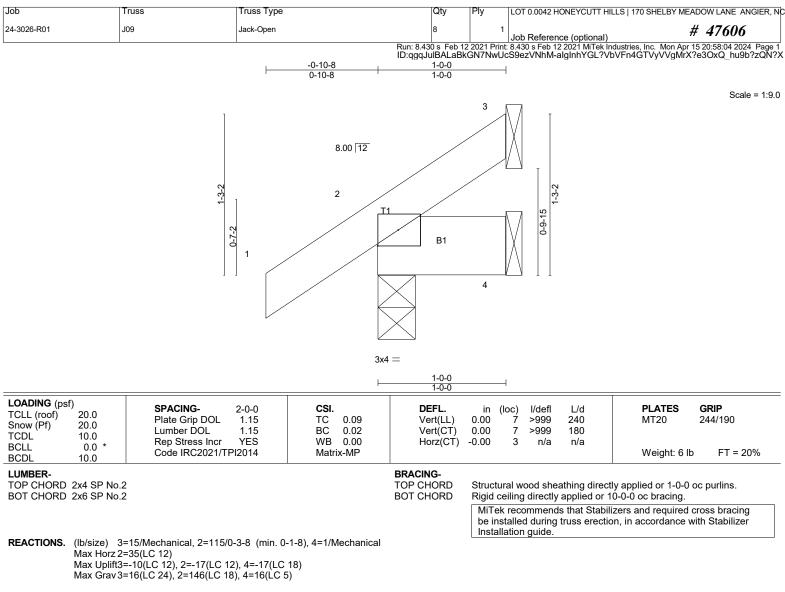
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 3 and 29 lb uplift at joint 4.

LOAD CASE(S) Standard





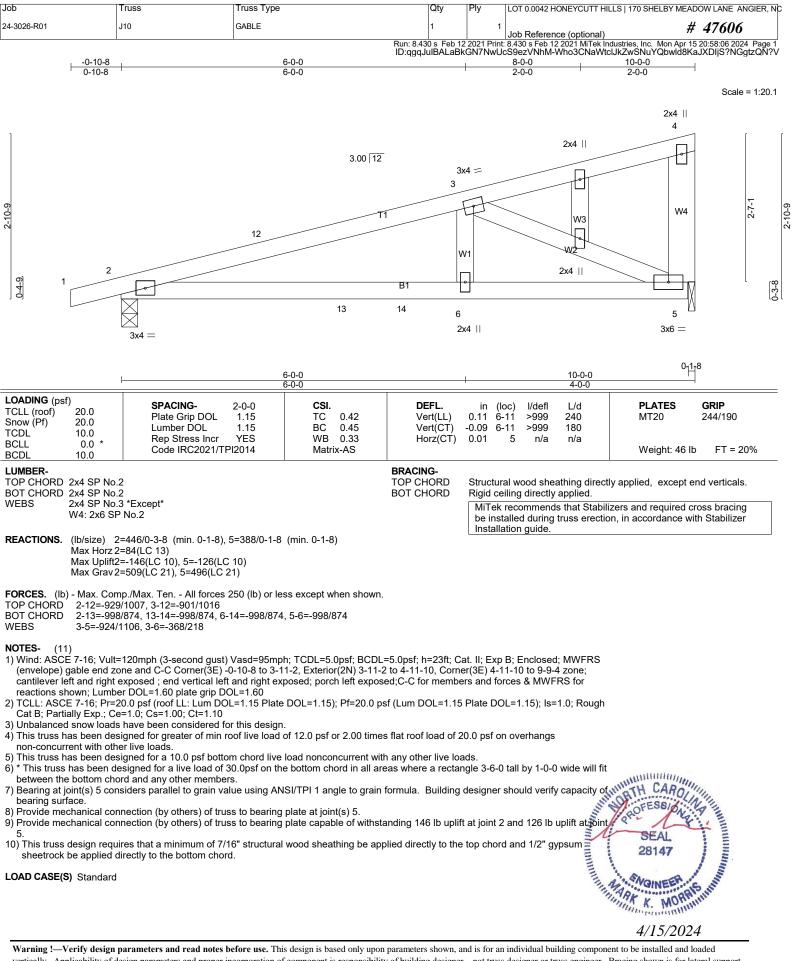
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

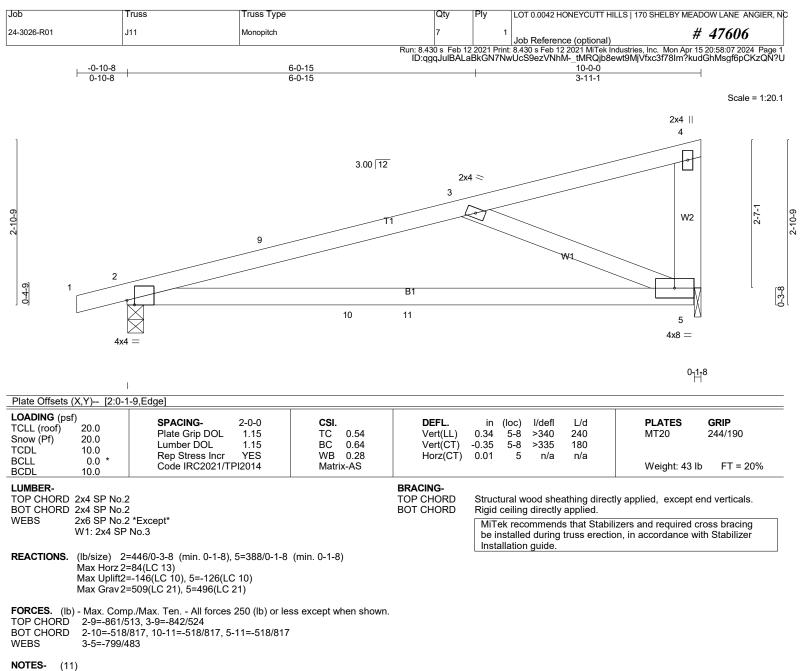
NOTES- (8)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 3, 17 lb uplift at joint 2 and 17 lb uplift at joint 4.

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LOAD CASE(S) Standard
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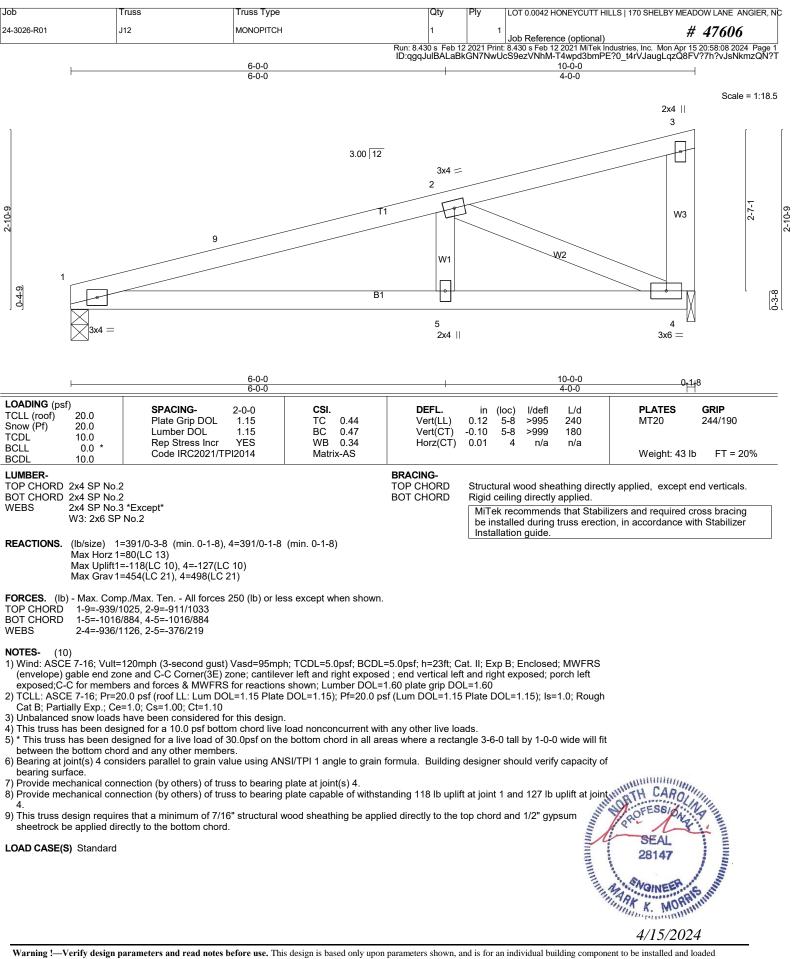


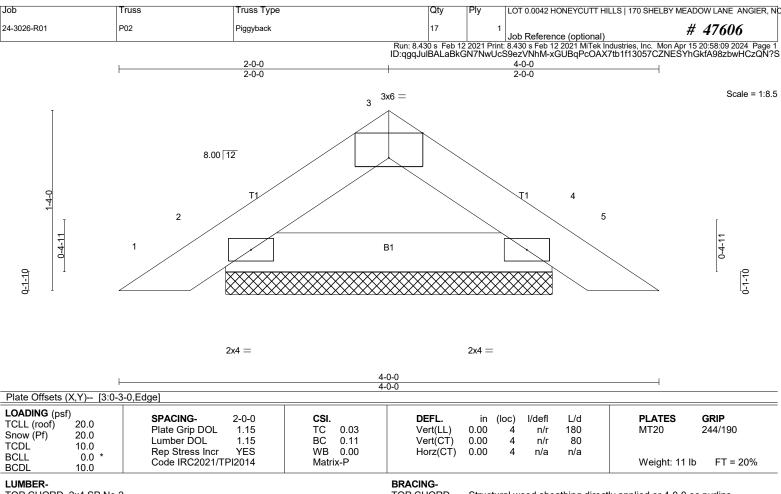




- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 4-11-10, Exterior(2E) 4-11-10 to 9-9-4 zone; cantilever left and right exposed; end vertical left and right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 7) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 2 and 126 lb uplift at joint 5.
 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
 LOAD CASE(S) Standard

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TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

- **REACTIONS.** (Ib/size) 2=128/2-5-0 (min. 0-1-8), 4=128/2-5-0 (min. 0-1-8) Max Horz 2=-23(LC 10) Max Uplift2=-16(LC 12), 4=-16(LC 13)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.

NOTES- (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

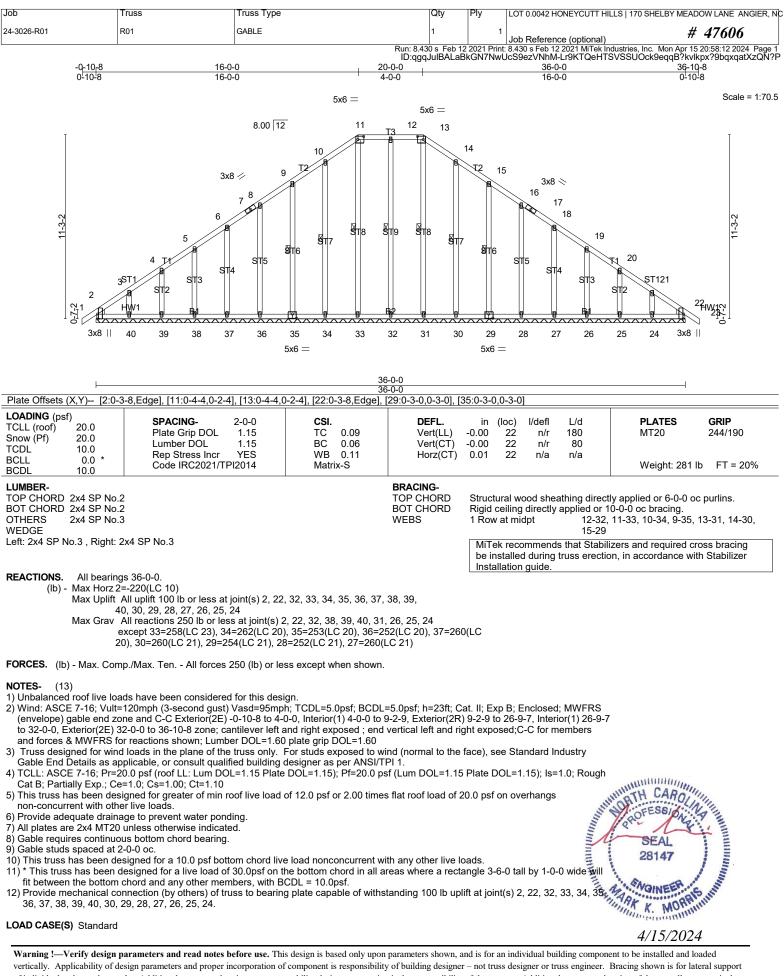
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 2 and 16 lb uplift at joint 4.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

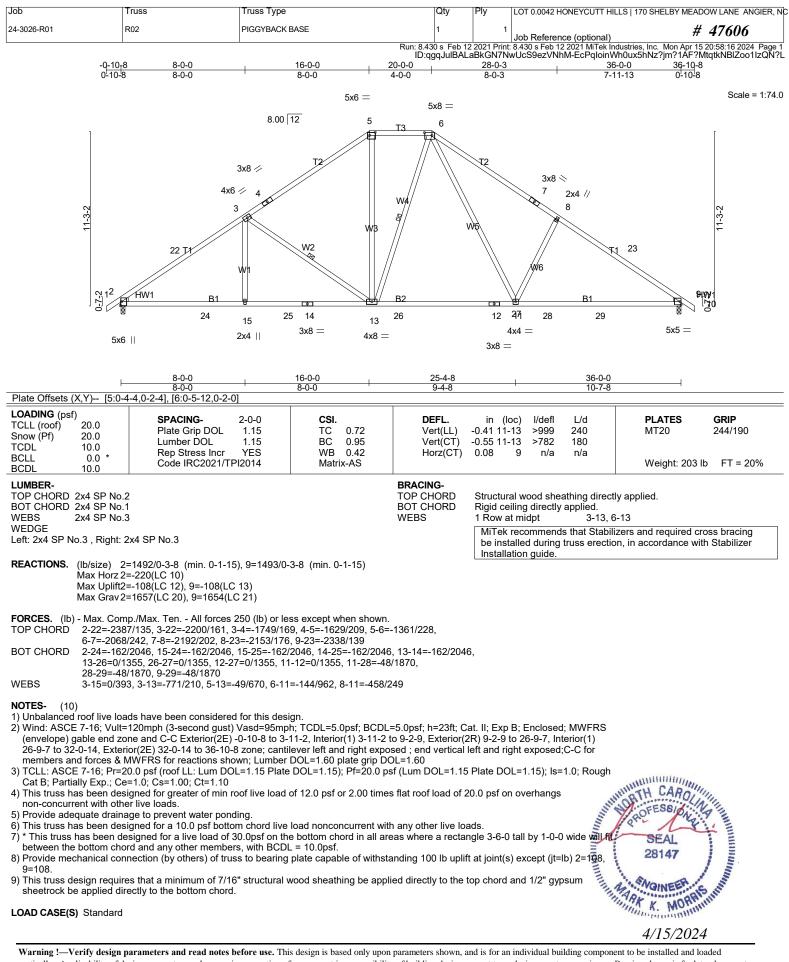
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LOAD CASE(S) Standard
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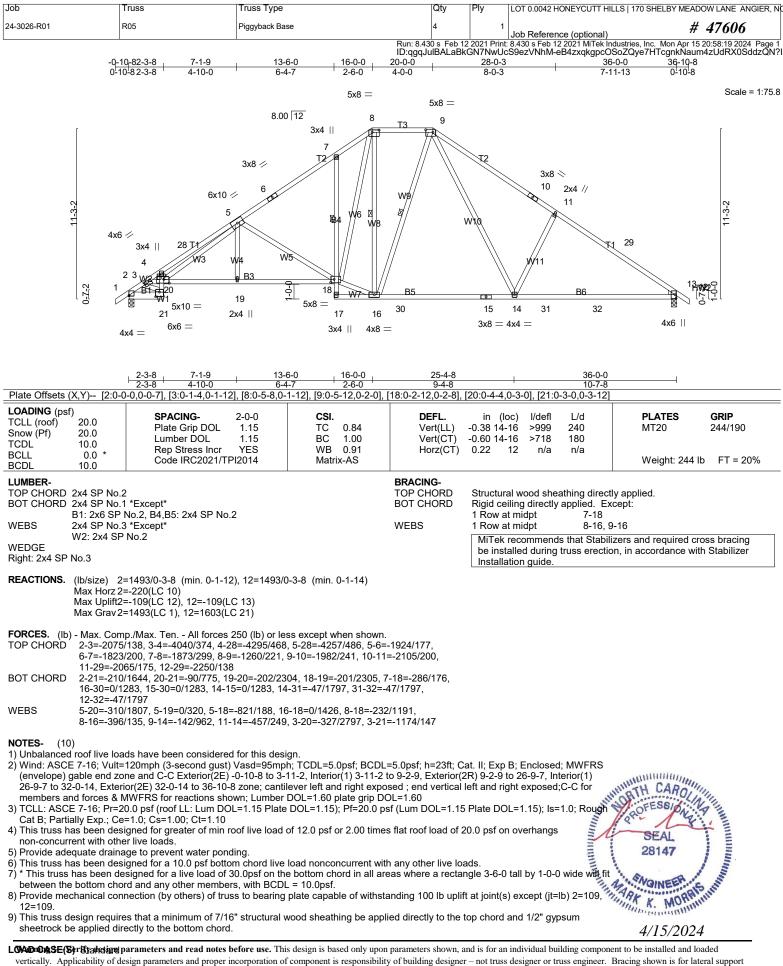


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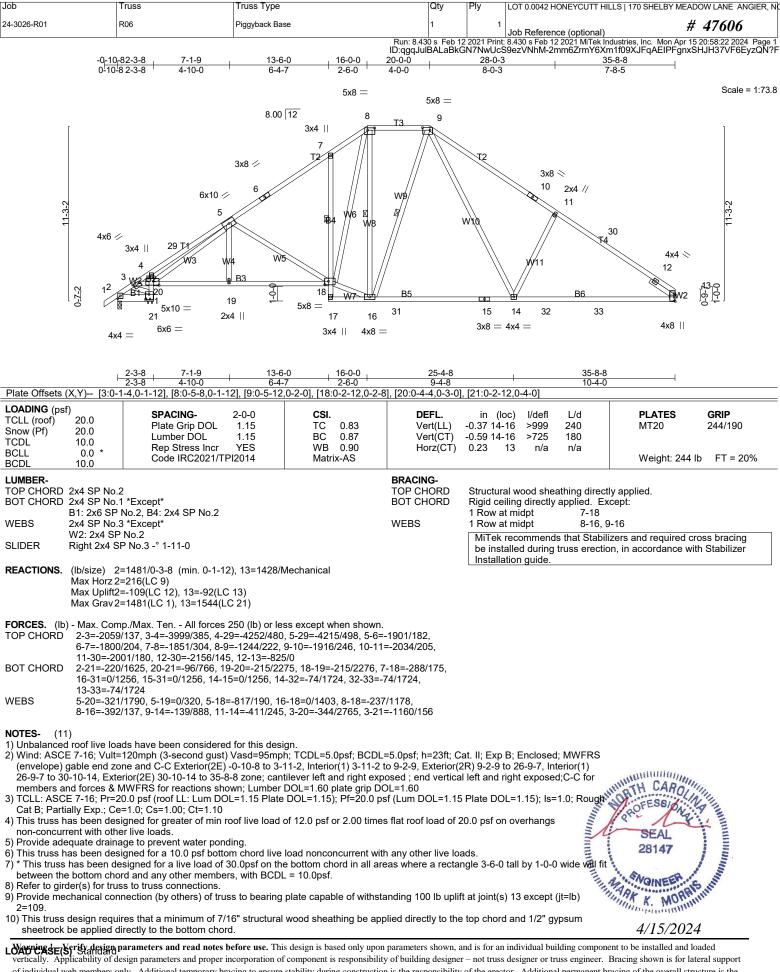


of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

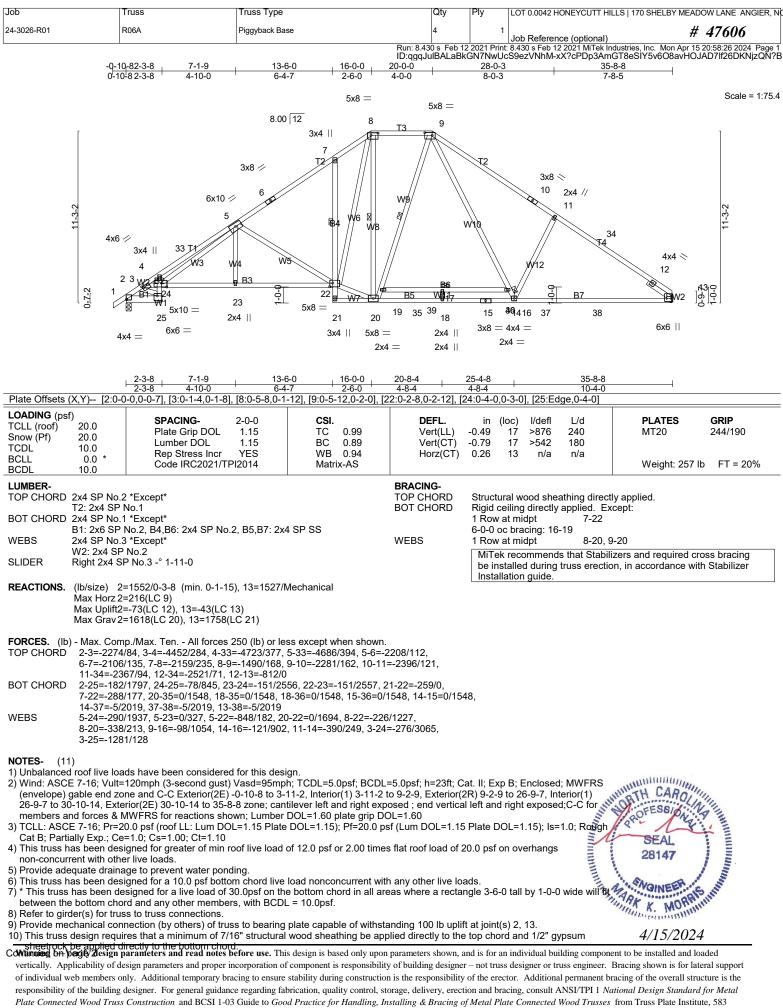




vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSUTPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

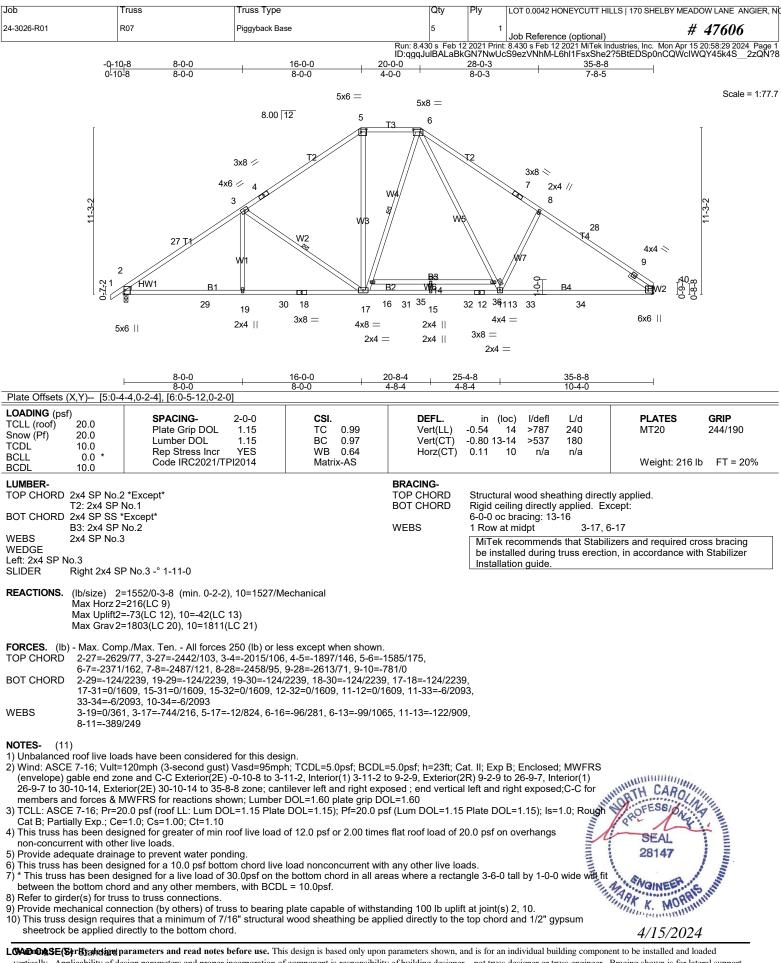


D'Onofrio Drive, Madison, WI 53719.

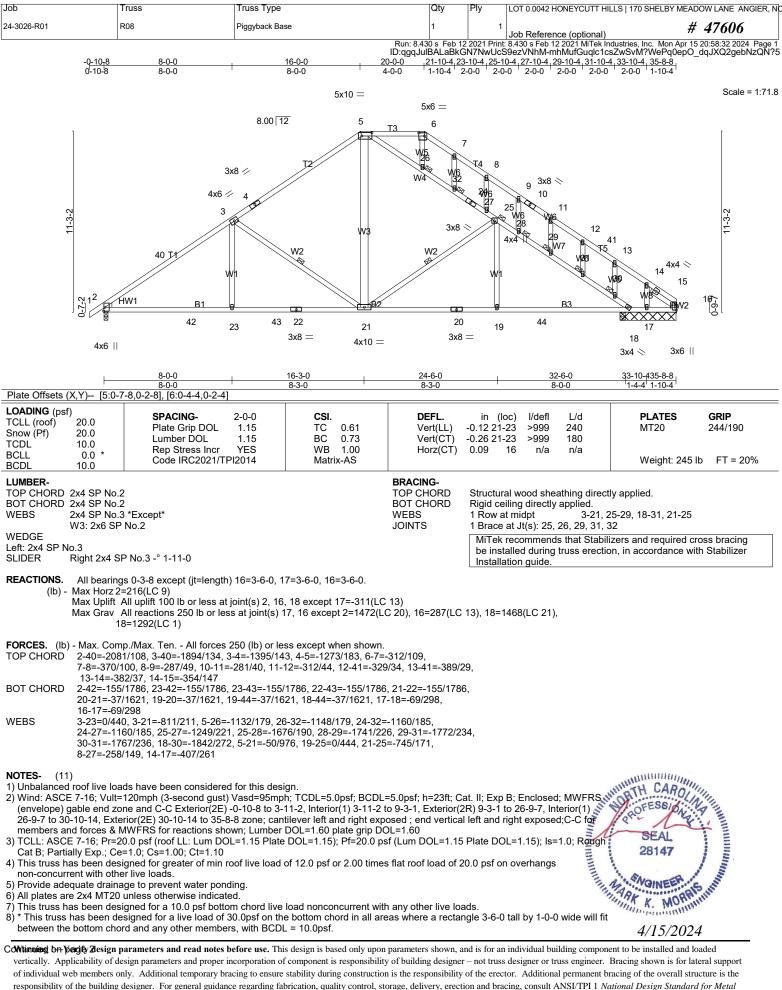
Job	Truss	Truss Type	Qty	Ply	LOT 0.0042 HONEYCUTT HILLS 170 SHELBY MEADOW LANE ANGIER, N	ЛС
24-3026-R01	R06A	Piggyback Base	4	1	Job Reference (optional) # 47606	
					t: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Apr 15 20:58:26 2024 Page cS9ezVNhM-xX?cPDp3AmGT8eSIY5v6O8avHOJAD7If26DKNjzQN?	

LOAD CASE(S) Standard





vertically. Applicability of design parameters and roter three orthore are incorporation of component is responsibility of building designer – not truss designer of truss designer of truss engines. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Trusse Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Met Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0042 HONEYCUTT HILLS 170 SHELBY MEADOW LANE ANGIER, NC
24-3026-R01	R08	Piggyback Base	1	1	Job Reference (optional) # 47606
					8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Apr 15 20:58:33 2024 Page 2 S9ezVNhM-EuwGtcvSWv9TUjVeT3XIBdNBODkDMHYhfiQB7pzQN?4

NOTES- (11)

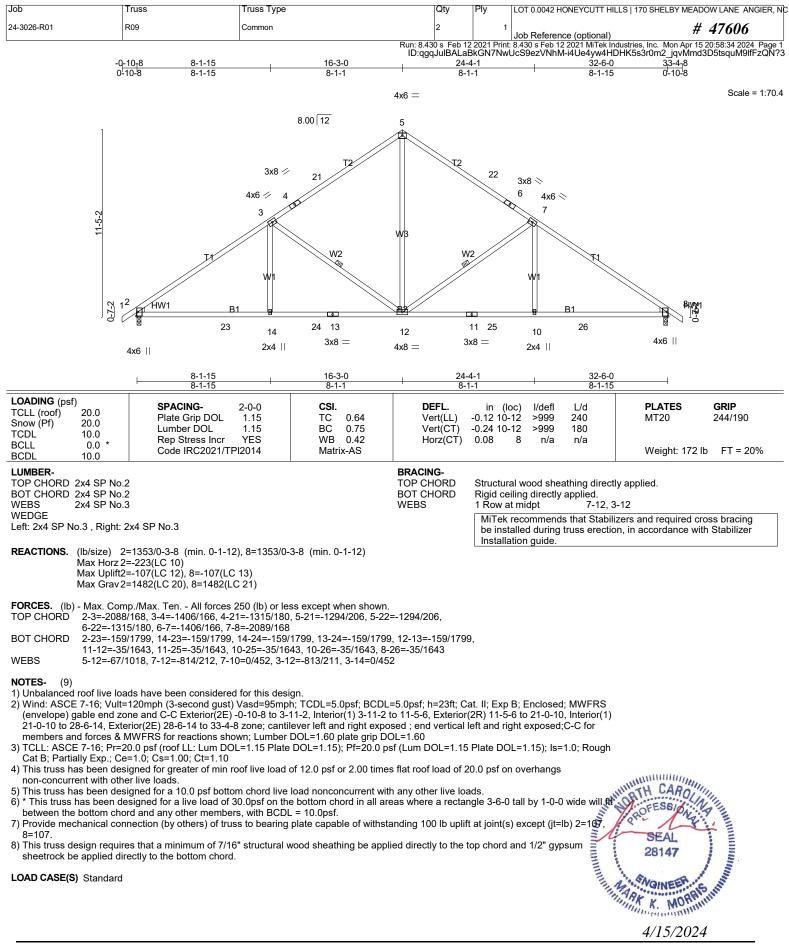
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 16, 18, 16 except (jt=lb) 17=311.

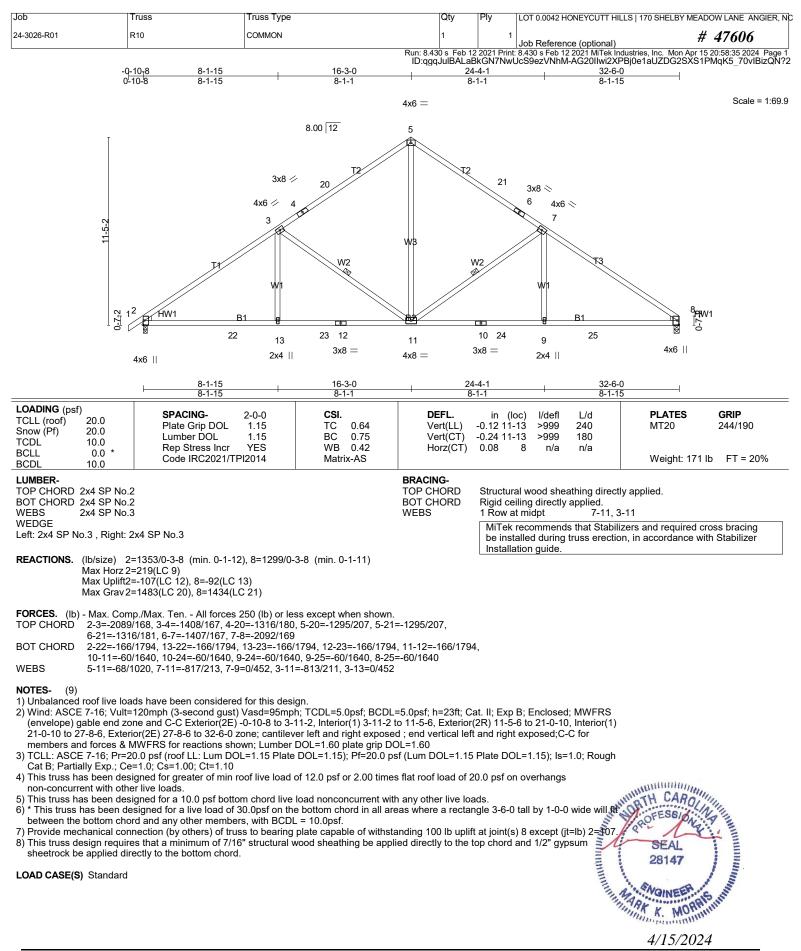
10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

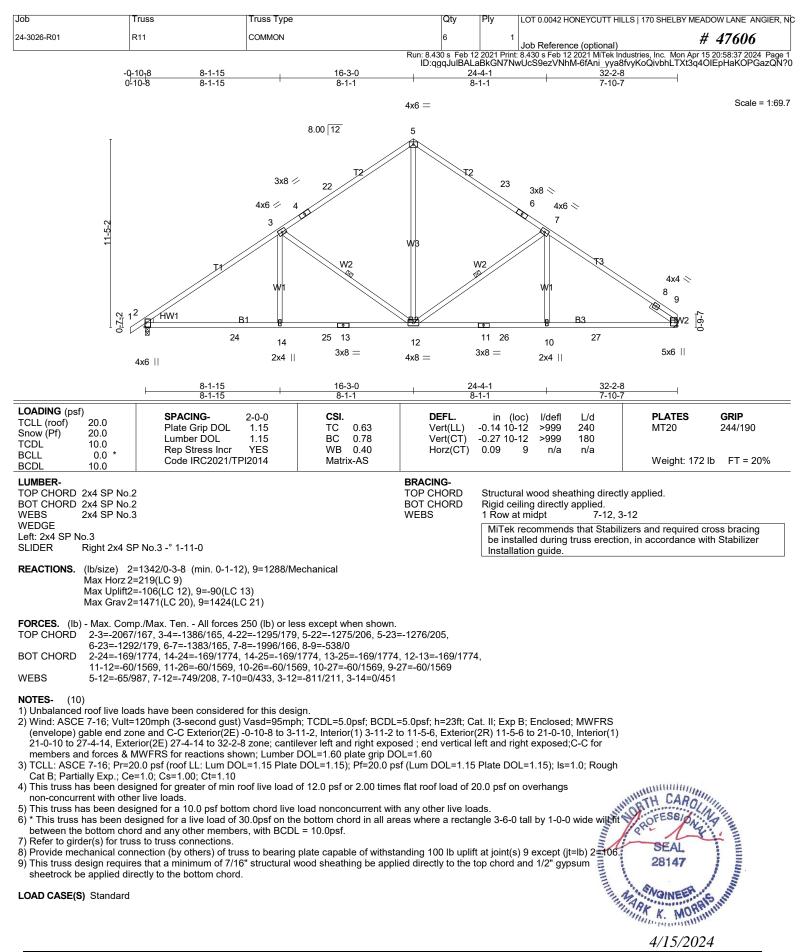
LOAD CASE(S) Standard

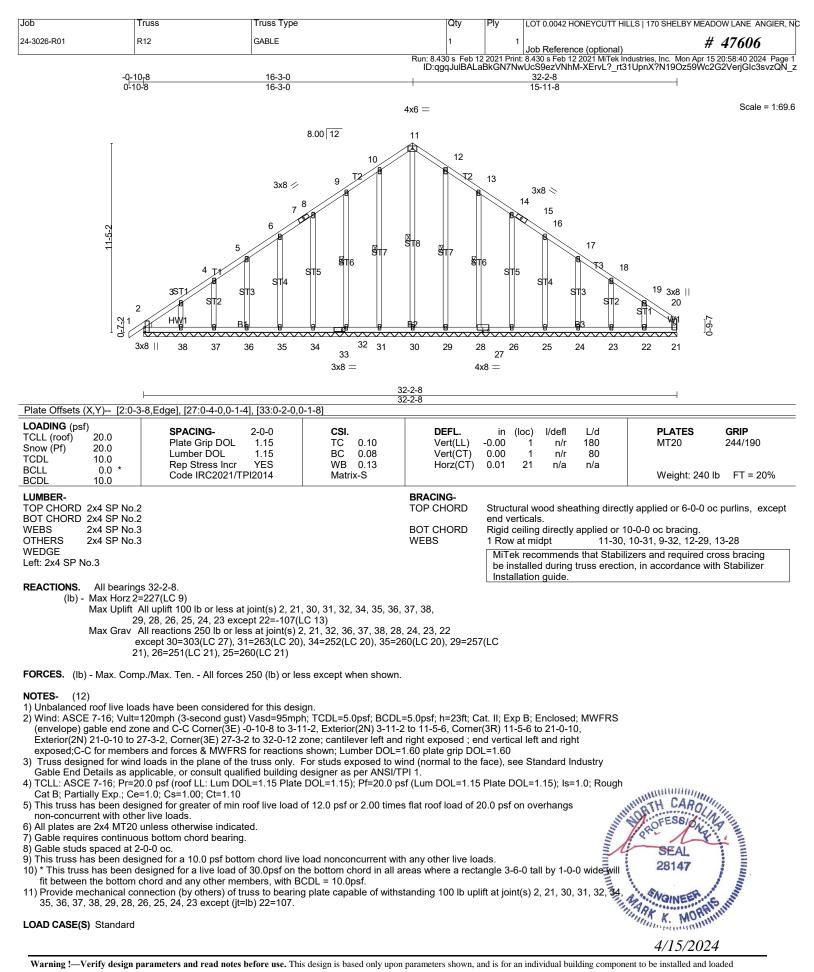


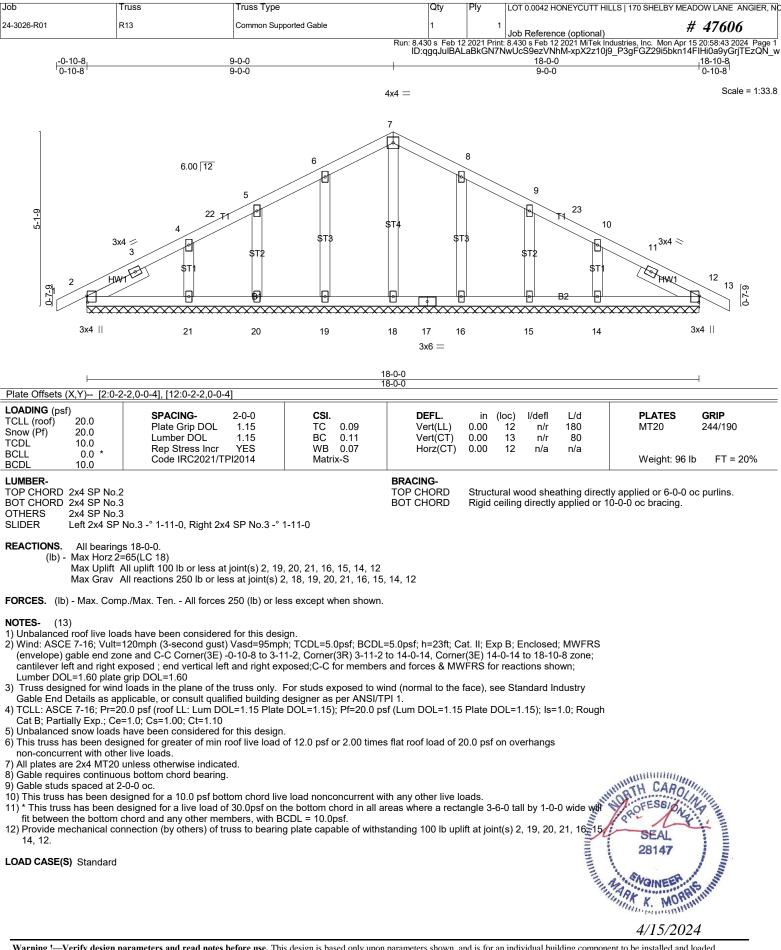
4/15/2024

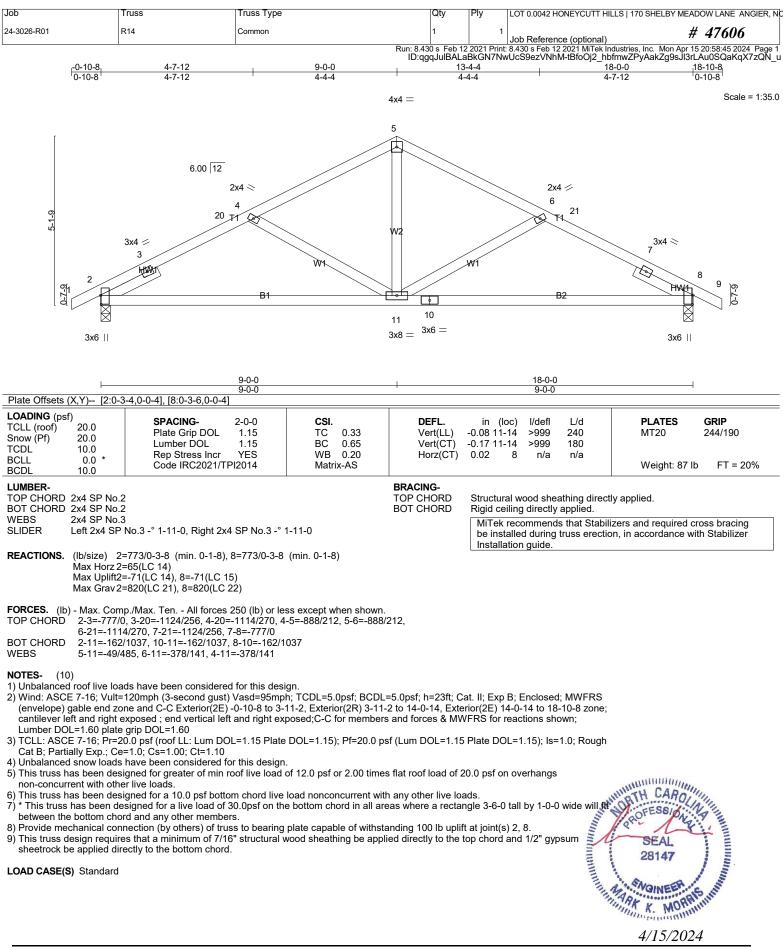


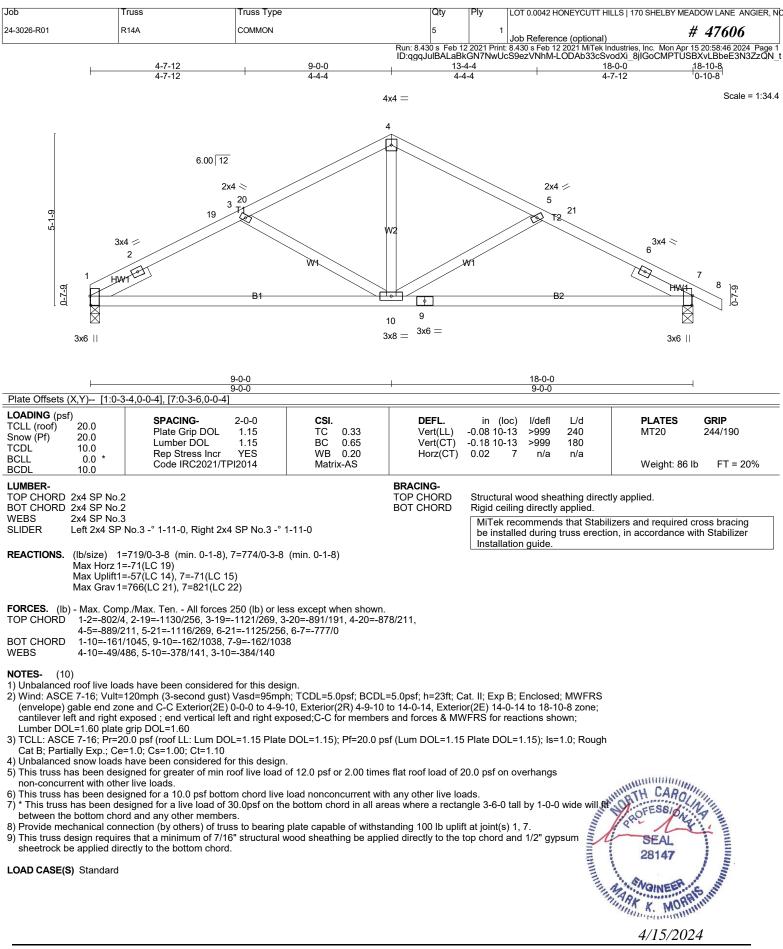












Job 24-3026-R01	Tr R1	uss Truss Typ 15 COMMON		Qty 1	2	0.0042 HONEYCUTT H Reference (optional)	ILLS 170 SHELBY MEADOW LANE ANGIER, # 47606
	I	0- <u>2-8 3</u> 0-2-8 3	-3-0 5-3-0 7-3-0	9-3-0 11-3-0 13-3-	2 2021 Print: 8.43 kGN7NwUcS96 -0 _ 15-3-0 _	0 s Feb 12 2021 MiTek 1 ezVNhM-iL03fm7kHR 8-3-8 18 _r 6-0	ndustries, Inc. Mon Apr 15 20:58:51 2024 Page QweUt6WrrzvQ6EaTxTaReKoVn8ImzQN
		0-2-8 3	-0-8 2-0-0 2-0-0	2-0-0 ¹ 2-0-0 ¹ 2-0- 4x6	0 ' 2-0-0 '	3-0-8 0-2-8	Scale = 1:69
		$ \begin{array}{c} 12\\ 01-2-11\\ 0-1-2-1\\ 0-1-1-\\ 0-1-2-5\\ 5x6 = \end{array} $	$\frac{4}{39} + \frac{6}{12} + \frac{6}{8} + $	$\begin{array}{c} 1 \\ 9 \\ 9 \\ -1 \\ 10 \\ 10 \\ 0 \\ 4x6 \\ 7x8 \\ = \end{array}$	W_{6} 4x4 = W3 16 1598 = 189 W_{4} 21 $318x8 =$	$\frac{1}{32} \qquad \begin{array}{c} 1 \\ 20 \\ 5x6 \end{array} =$	7-7-7
Plate Offsets	(X.Y) [1:0-6-		4-8-6 9-3-0 4-8-6 4-6-10 .0-4-8], [23:0-4-0.0-4-12		18- 4-8		
LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	,	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2021/TPI2014	CSI. TC 0.76 BC 0.47 WB 0.96 Matrix-MSH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.06 21-23 -0.12 21-23 0.02 20	l/defl L/d >999 240 >999 180 n/a n/a	PLATES GRIP MT20 244/190 Weight: 649 lb FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SP No.2 T2,T3: 2x6 SI 2x6 SP DSS 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD JOINTS	end verticals Rigid ceiling		
REACTIONS.	Max Horz 25 Max Uplift25	=8158/0-3-8 (min. 0-2-1), 20=6773 =226(LC 41) =-511(LC 11), 20=-414(LC 10) =9063(LC 3), 20=7682(LC 3)	/0-3-8 (min. 0-1-12)				
FORCES. (Ib TOP CHORD BOT CHORD	1-2=-5314/2 12-14=-5114 1-3=-4012/4 9-11=-1635 18-19=-4174 25-26=-2617 23-29=-6174	./Max. Ten All forces 250 (lb) or 264, 2-5=-5106/314, 5-7=-5115/365 4/367, 14-17=-5107/317, 17-19=-5 195, 3-4=-4012/495, 4-6=-1635/120 /120, 11-13=-1635/120, 13-15=-16 6/503 /1332, 26-27=-261/1332, 24-27=-2 /7553, 22-23=-547/7755, 22-30=-5 1315, 20-32=-92/1315	, 7-10=-4957/399, 10-12 314/266, 1-25=-6685/38 , 6-8=-1635/120, 8-9=-1 35/120, 15-16=-1635/12 61/1332, 24-28=-617/75	2=-4957/401, 3, 19-20=-6766/388 635/120, 0, 16-18=-4176/503 53, 28-29=-617/758	3, 53,		
WEBS	11-23=-394	/6288, 16-23=-2716/409, 16-21=-1 6742, 19-21=-490/6933, 17-18=-14					
Top chords Bottom cho Webs conn 2) All loads ar	to be connected connected as rds connected ected as follow e considered a	ed together with 10d (0.131"x3") na follows: 2x4 - 1 row at 0-9-0 oc, 2x l as follows: 2x6 - 2 rows staggered ws: 2x4 - 1 row at 0-9-0 oc. equally applied to all plies, except i rovided to distribute only loads not is have been considered for this de 20mph (3-second gust) Vasd=95m e; cantilever left and right exposed .0 psf (roof LL: Lum DOL=1.15 Pla 1.0; Cs=1.00; Ct=1.10 ge to prevent water ponding. inless otherwise indicated. gned for a 10.0 psf bottom chord liv igned for a live load of 30.0psf on f d and any other members. 0 considers parallel to grain value u tec. rameters and read notes before use. T ign parameters and proper incorporation y. Additional temporary bracing to ensu	6 - 2 rows staggered at at 0-4-0 oc.	ck (B) face in the l	OAD CASE(S at. II; Exp B; E er DOL=1.60 p Plate DOL=1 ads. ngle 3-6-0 tall iilding designe) section. Ply to ply inclosed; MWFRS plate grip DOL=1.6 (15); Is=1.0; Rough by 1-0-0 wide will fill er should verify	SEAL 28147
CADACITY O	n bearing surfa	100.					A/15/2022A

of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0042 HONEYCUTT HILLS 170 SHELE	BY MEADOW LANE ANGIER, NC
24-3026-R01	R15	COMMON GIRDER	1	3	Job Reference (optional)	# 47606
					8.430 s Feb 12 2021 MiTek Industries, Inc. Mc cS9ezVNhM-AXaRs67N2IYnFdSI4YMCS	

NOTES- (14)

- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 25=511, 20=414.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 27 lb down and 60 lb up at 0-4-11, and 27 lb down and 60 lb up at 18-1-5 on top chord, and 1512 lb down and 104 lb up at 0-1-12, 29 lb down and 31 lb up at 2-3-4, 1717 lb down and 63 lb up at 2-3-4, 1717 lb down and 31 lb up at 4-3-4, 1717 lb down an at 14-3-4, 1771 lb down and 62 lb up at 14-3-12, and 29 lb down and 31 lb up at 16-3-4, and 1771 lb down and 62 lb up at 16-3-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-10=-60, 10-19=-60, 20-25=-20, 1-19=-6(F)

Concentrated Loads (lb)

Vert: 22=-1503(F=4, B=-1507) 1=20(F) 19=20(F) 25=-1416(B) 26=-1503(F=4, B=-1507) 27=-1503(F=4, B=-1507) 28=-1503(F=4, B=-1507) 29=-1503(F=4, B=-1507) 29=-1503(F 30=-1503(F=4, B=-1507) 31=-1503(F=4, B=-1507) 32=-1503(F=4, B=-1507)



4/15/2024

Job	Trus	6	Truss Type	Qty	Ply I	LOT 0.0042 HONEYCUTT HI	LLS 170 SHELBY MEADOW LANE ANGIER, N
24-3026-R01	R16		Common Girder	1	1		# 47606
				Run: 8.430 s Feb 1	2 2021 Print: 8	Job Reference (optional) 3.430 s Feb 12 2021 MiTek In Soort (Neb 4 SwiChoodom	ndustries, Inc. Mon Apr 15 20:58:54 2024 Page 1 oVVxbhBzPgX2kqah0hmzAnUT?oL5zQN_
		- <u>0-10</u> 0-10-	-8 2-10-0 4-10-0 6-1	10-0 8-10-0 1	0-10-0	13-8-0 14-6-8	OVVXDIBZPGAZKQANUNITIZANU I ?OLOZQIN_
		0-10-	8 2-10-0 2-0-0 2-		2-0-0	2-10-0 0-10-8	
				4x4 =			Scale: 1/4"=1
				8			
		2657 2732 2713 2 2713 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	W3 40 2x4 W4 W2 7x8 =		2x4 12V3 2x4 12V3 2x4W2 22	5x8 \ 13 14 TY	72
		:	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	20 21 16 4x8 : 9-0-3 4-4-5		15 3x4 3-0	
Plate Offsets (X,	,Y) [5:0-4-0,0	0-2-0]	UI = 1-T				
Snow (Pf) 2 TCDL BCLL	20.0 20.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/TP	2-0-0 CSI. 1.15 TC 0.39 1.15 BC 0.25 NO WB 0.20 12014 Matrix-MSH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loo 0.03 16-1 -0.06 16-1 0.01 1	7 >999 240	PLATES GRIP MT20 244/190 Weight: 124 lb FT = 20%
BCDL	10.0			BRACING-			
TOP CHORD 23 BOT CHORD 23 WEBS 23				TOP CHORD BOT CHORD JOINTS	end vertio Rigid ceil		ly applied or 6-0-0 oc purlins, except 0-0-0 oc bracing.
		_			MiTek ro be insta	ecommends that Stabil	izers and required cross bracing on, in accordance with Stabilizer
Ñ N	/lax Horz 18=-1 /lax Uplift18=-4					Ĩ	
FORCES. (lb) -	Max. Comp./M	lax. Ten All forces 2	250 (lb) or less except when show	wn.			
TOP CHORD	2-3=-408/5, 3-	6=-337/42, 6-8=-303/	87, 8-9 [´] =-303/87, 9-11=-342/43, 1 541/323, 7-10=-541/323, 10-12=-	1-13=-412/6,			
	12-13=-543/31	8, 2-18=-602/67, 13-		,			
		5-16=-244/251, 13-10					
NOTES- (12)	oof live lesses		for this desire				
2) Wind: ASCE 7	7-16; Vult=120		Vasd=95mph; TCDL=5.0psf; BC				
3) TCLL: ÁSĆĔ :	7-16; Pr=20.0	psf (roof LL: Lum DŎI	nt exposed ; end vertical left and _=1.15 Plate DOL=1.15); Pf=20.0				
		; Cs=1.00; Ct=1.10 d for greater of min ro	oof live load of 12.0 psf or 2.00 ti	mes flat roof load of 2	20.0 psf on	overhangs	-
non-concurrer	nt with other liv	e loads.	ing			3	WHITH CARO
6) This truss has	s been designe	d for a 10.0 psf botto	m chord live load nonconcurrent	with any other live lo	ads. ngle 3-6-0 t	all by 1_0_0 wide will fit	OFESSION NUT
between the b	pottom chord a	nd any other member	s. S.	atonding 100 lb unlit	ft at iaint(a)	19.15	are the
and 35 lb do and 4 lb up a down and 4 l	wn and 21 lb u at 4-4-12, 6 lb lb up at 12-4-1	p at 13-4-12 on top o down and 4 lb up at 2, and 7 lb down and	oof live load of 12.0 psf or 2.00 til ing. m chord live load nonconcurrent 0.0psf on the bottom chord in all s. s to bearing plate capable of with e size or the orientation of the pu provided sufficient to support co chord, and 7 lb down and 4 lb up 6-4-12, 6 lb down and 4 lb up at 4 lb up at 13-5-4 on bottom cho	at 0-2-12, 6 lb down 8-4-12, 6 lb down an	and 4 lb up d 4 lb up at	chord. nd 21 lb up at 0-3-4, at 2-4-12, 6 lb down 10-4-12, and 6 lb h connection device(s)	SEAL 28147 4/15/2024
	nsibility of othe CASE(S) sect		he face of the truss are noted as	front (F) or back (B)			Mining K. MOHumm
LOAD CASE(S)	Standard						4/15/2024
-=(-)							.,

vertically. Applicability of design parameters and rotat notes before user into one of component is usered only upon parameters shown, and is for an individual building component to be instanted and totated of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

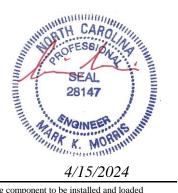
1	Job	Truss	Truss Type	Qty	Ply	LOT 0.0042 HONEYCUTT HILLS 170 SHELBY MEADOW LANE ANGIER, NC
	24-3026-R01	R16	Common Girder	1	1	Job Reference (optional) # 47606
						t: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Apr 15 20:58:54 2024 Page 2 IcS9ezVNhM-6wiCHo9daMoVVxbhBzPgX2kqah0hmzAnUT?oL5zQN_I

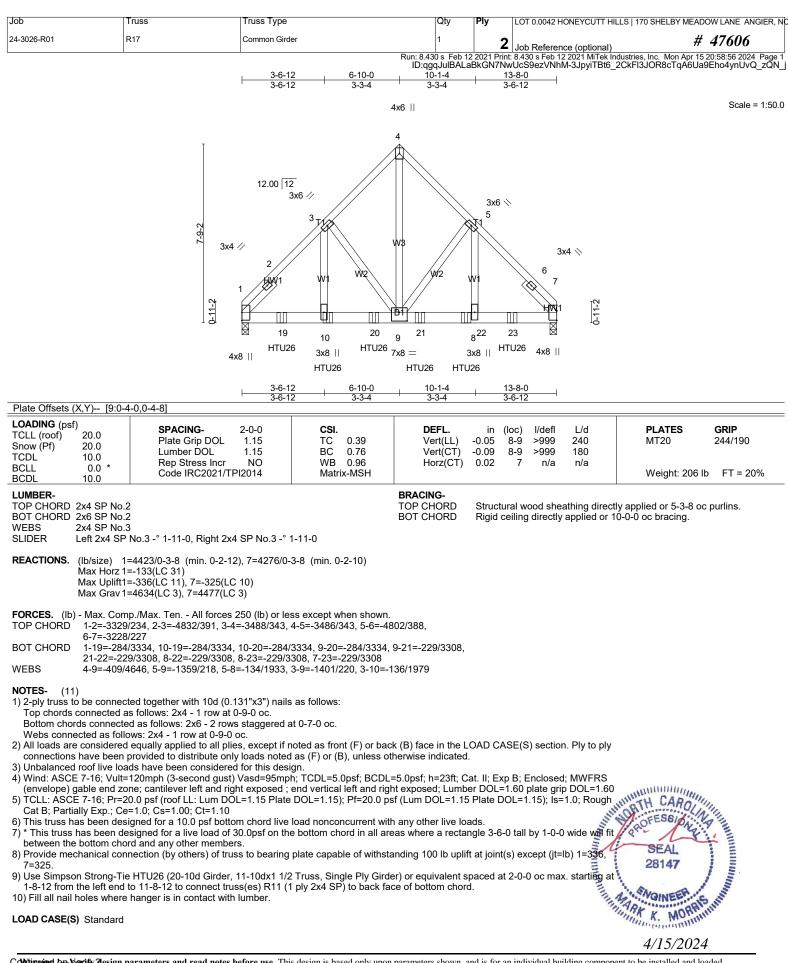
LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-8=-60, 8-13=-60, 13-14=-60, 15-18=-20, 2-13=-8(F) Concentrated Loads (lb)

Vert: 18=4(F) 15=4(F) 17=4(F) 19=4(F) 20=4(F) 21=4(F) 22=4(F) 23=4(F)





Job	Truss	Truss Type	Qty	Ply	LOT 0.0042 HONEYCUTT HILLS 170 S	HELBY MEADOW LANE ANGIER, NC
24-3026-R01	R17	Common Girder	1	2	Job Reference (optional)	# 47606
		Run: 8.4	30 s Feb 12	2 2021 Print	: 8.430 s Feb 12 2021 MiTek Industries. Industries.	c. Mon Apr 15 20:58:56 2024 Page 2

tun: 8.430 s_Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc._Mon Apr 15 20:58:56 2024 Page 2 ID:qgqJulBALaBkGN7NwUcS9ezVNhM-3JpyiTBt6_2CkFl3JOR8cTqA6Ua9Eho4ynUvQ_zQN_j

LOAD CASE(S) Standard

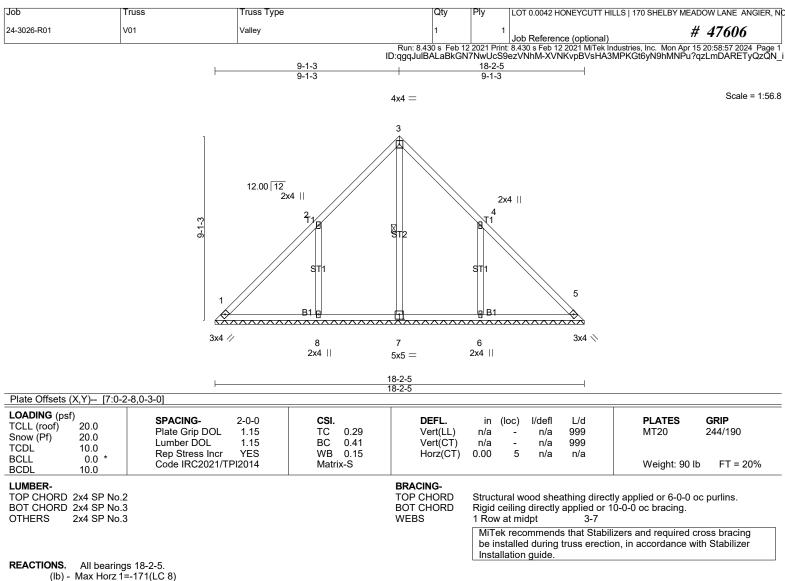
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-4=-60, 4-7=-60, 11-15=-20

Concentrated Loads (lb)

Vert: 10=-1268(B) 19=-1268(B) 20=-1268(B) 21=-1268(B) 22=-1268(B) 23=-1268(B)





Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-222(LC 12), 6=-222(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=424(LC 22), 8=542(LC 19), 6=541(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

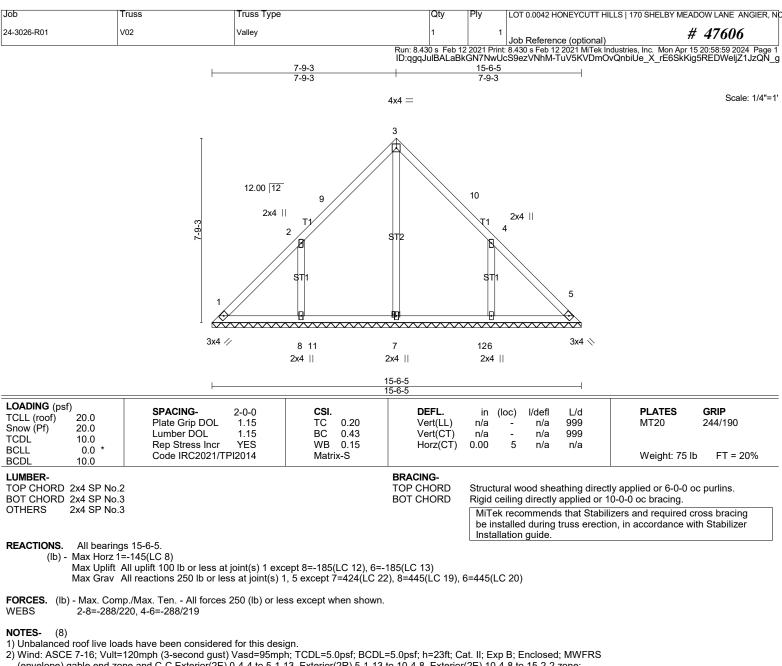
WEBS 2-8=-344/260. 4-6=-344/260

NOTES-(8)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-3, Exterior(2R) 5-1-3 to 13-1-3, Exterior(2E) 13-1-3 to 17-10-2 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=222, 6=222.

LOAD CASE(S) Standard





- (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 10-4-8, Exterior(2E) 10-4-8 to 15-2-2 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

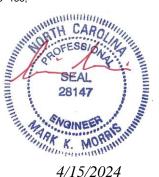
4) Gable requires continuous bottom chord bearing.

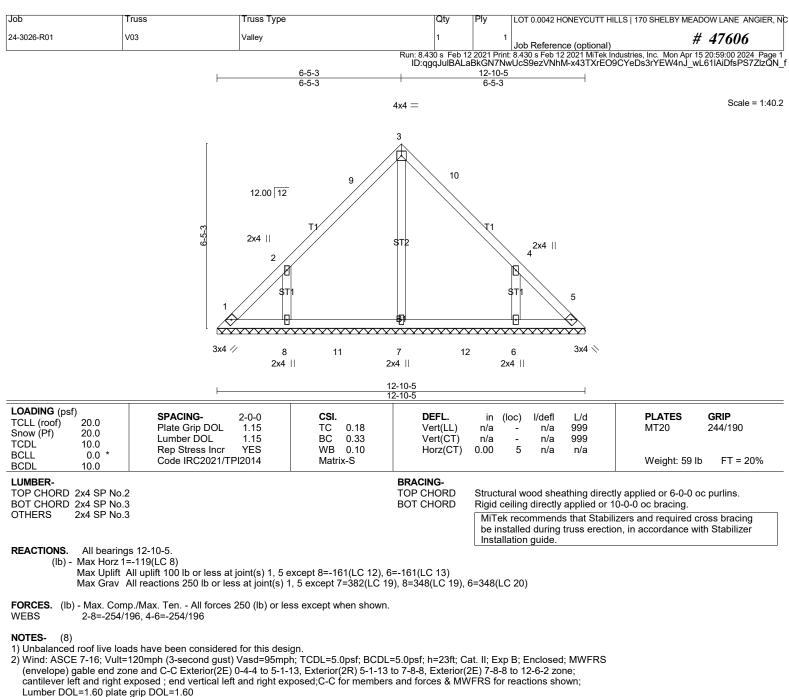
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=185, 6=185.

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LOAD CASE(S) Standard
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3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

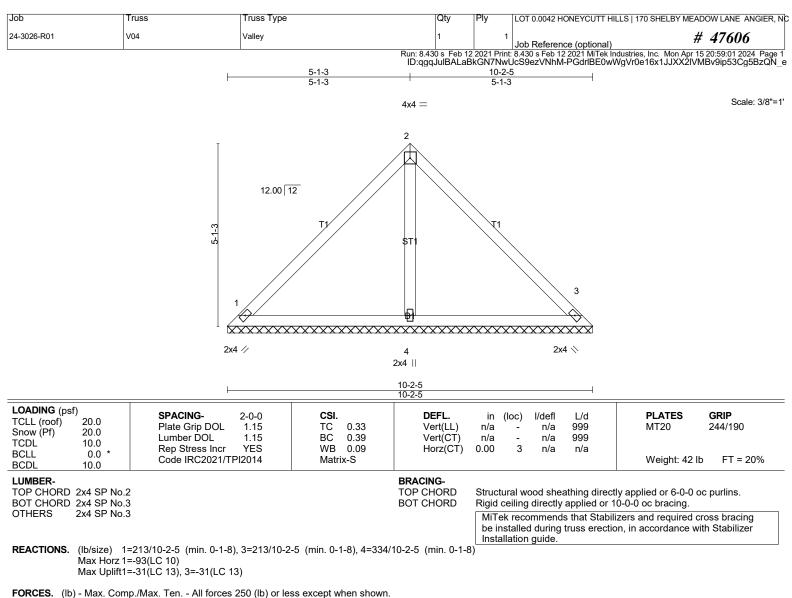
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=161, 6=161.

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LOAD CASE(S) Standard
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NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

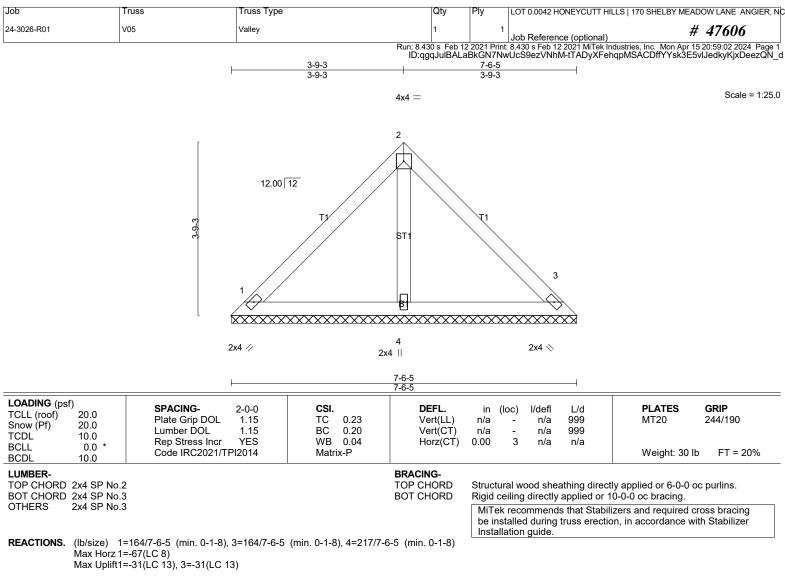
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

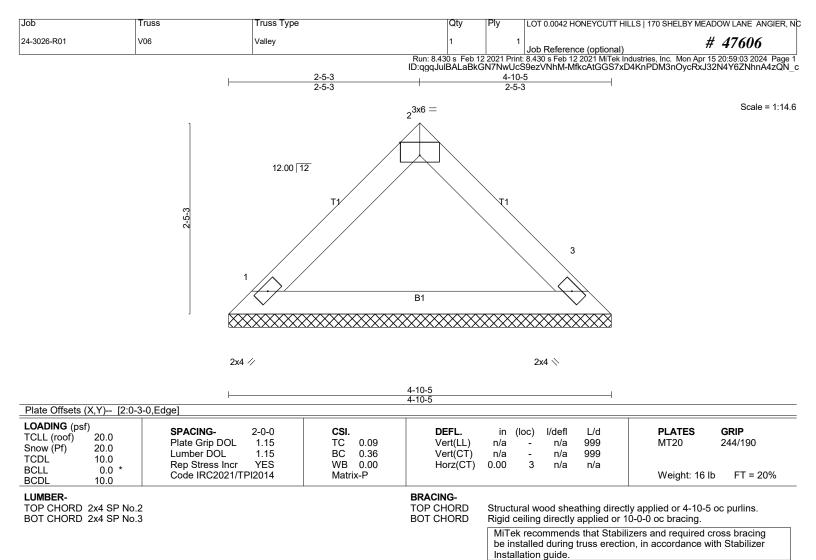
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 1=166/4-10-5 (min. 0-1-8), 3=166/4-10-5 (min. 0-1-8) Max Horz 1=-41(LC 10) Max Uplift1=-8(LC 12), 3=-8(LC 12)

NOTES- (8)

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard

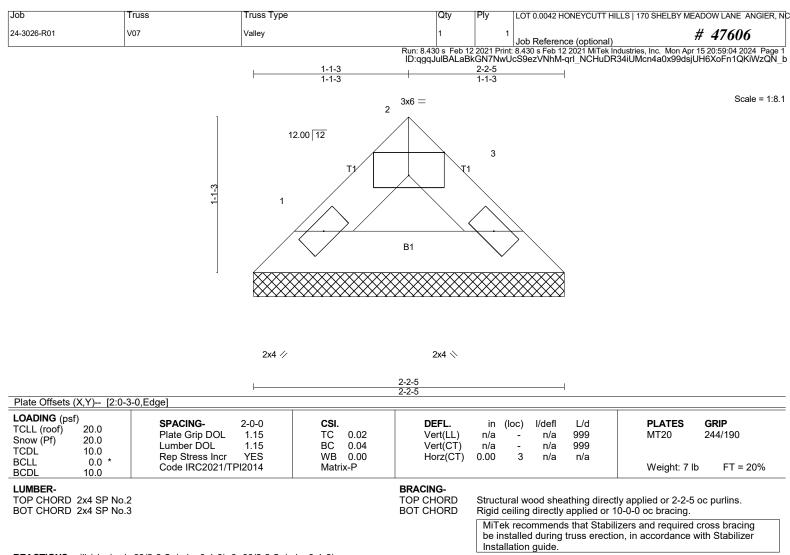


FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

¹⁾ Unbalanced roof live loads have been considered for this design.

²⁾ Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

³⁾ TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10



REACTIONS. (Ib/size) 1=60/2-2-5 (min. 0-1-8), 3=60/2-2-5 (min. 0-1-8) Max Horz 1=15(LC 11) Max Uplift1=-3(LC 12), 3=-3(LC 13)

NOTES- (8)

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

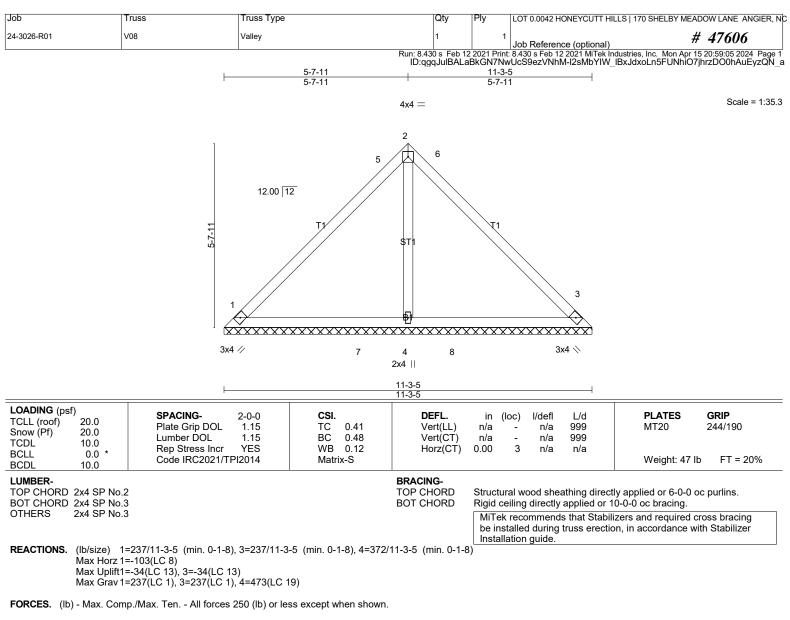
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

¹⁾ Unbalanced roof live loads have been considered for this design.



- NOTES-(8)
- 1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 6-1-8, Exterior(2E) 6-1-8 to 10-11-2 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Gable requires continuous bottom chord bearing.

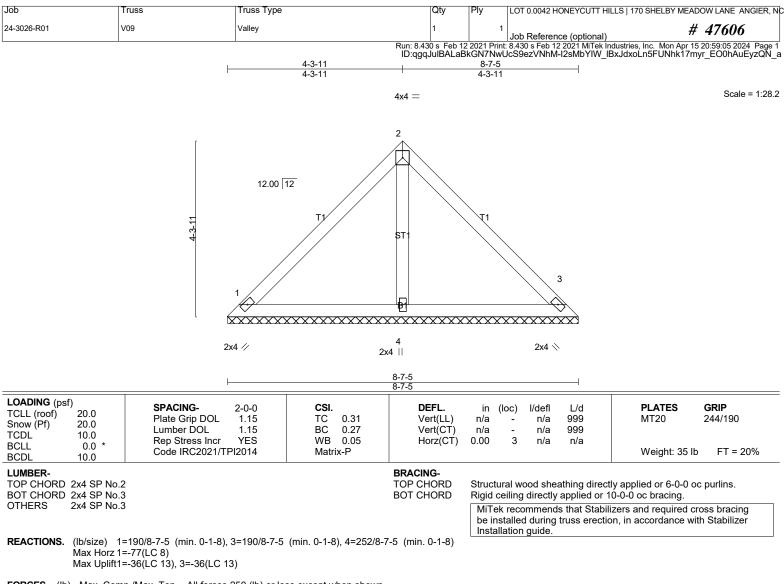
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

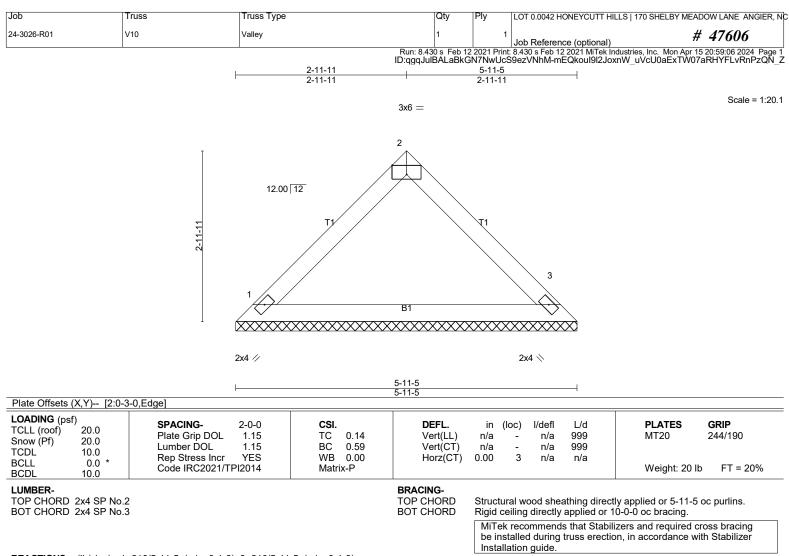
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





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REACTIONS. (lb/size) 1=210/5-11-5 (min. 0-1-8), 3=210/5-11-5 (min. 0-1-8)
Max Horz 1=-51(LC 8)
Max Uplift1=-10(LC 12), 3=-10(LC 12)
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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

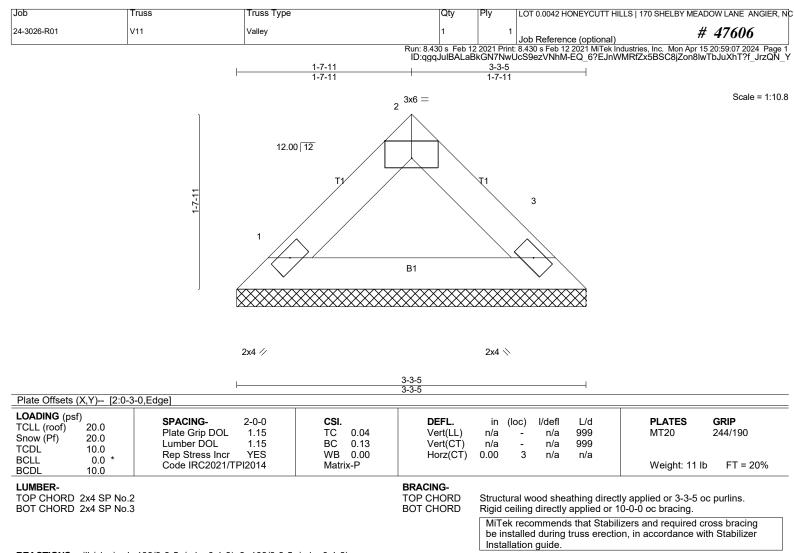
6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



¹⁾ Unbalanced roof live loads have been considered for this design.



REACTIONS. (lb/size) 1=103/3-3-5 (min. 0-1-8), 3=103/3-3-5 (min. 0-1-8) Max Horz 1=25(LC 11) Max Uplift1=-5(LC 12), 3=-5(LC 13)

NOTES- (8)

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

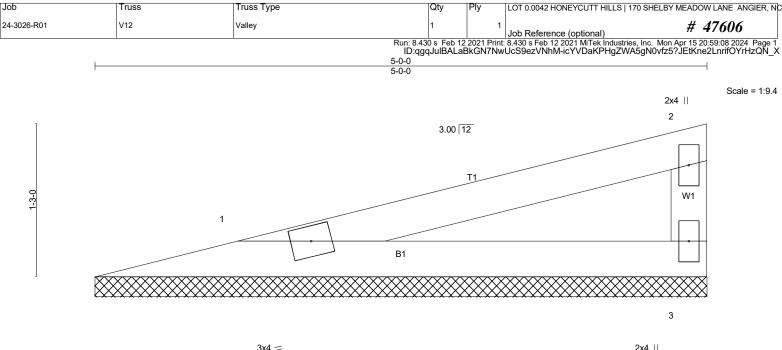
LOAD CASE(S) Standard



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

¹⁾ Unbalanced roof live loads have been considered for this design.

²⁾ Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



2x4 ||

LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2021/TPI2014	CSI. TC 0.33 BC 0.27 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 14 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD	end vertica	als.	U U	tly applied or 5-0-0 o 10-0-0 oc bracing.	c purlins, excep
					ed during		ilizers and required ci ion, in accordance wi	

REACTIONS. (lb/size) 1=147/5-0-0 (min. 0-1-8), 3=147/5-0-0 (min. 0-1-8) Max Horz 1=29(LC 11) Max Uplift1=-18(LC 10), 3=-21(LC 14) Max Grav 1=184(LC 20), 3=184(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard

