Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 45905 JOB: 24-1169-R01 JOB NAME: LOT 0.0002 HONEYCUTT HILLS Wind Code: 37 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. *40 Truss Design(s)*

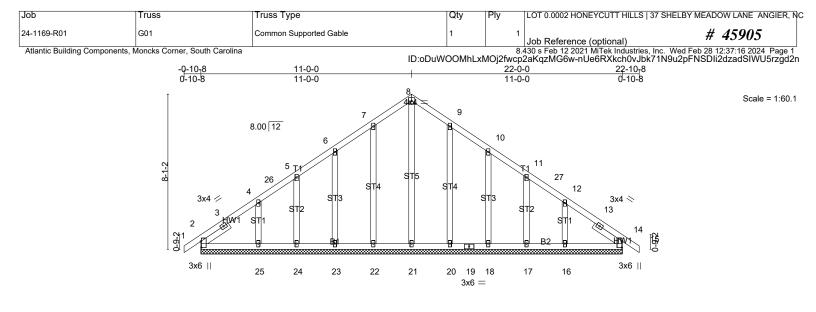
Trusses:

G01, G02, J01, J01A, J02, J04, J05, J06, J08, J09, J10, J11, J12, P02, R01, R02, R07, R08, R09, R10, R11, R12, R13, R14, R14A, R15, R16, R17, V01, V02, V03, V04, V05, V06, V07, V08, V09, V10, V11, V12



Warning !--- Verify design parameters and read notes before use.

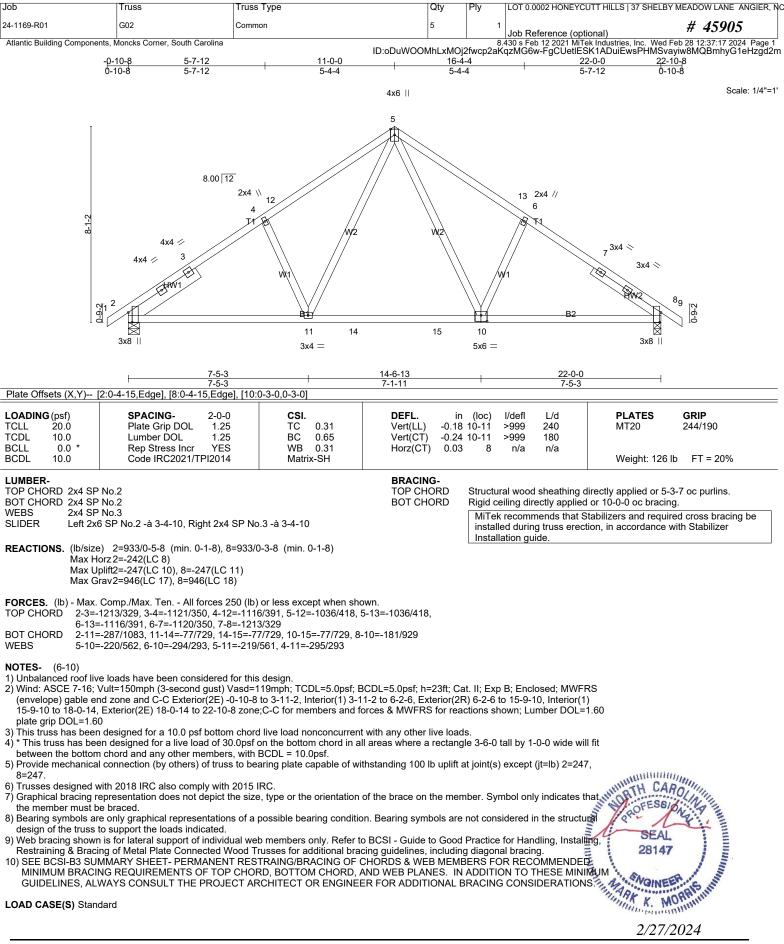
This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for*

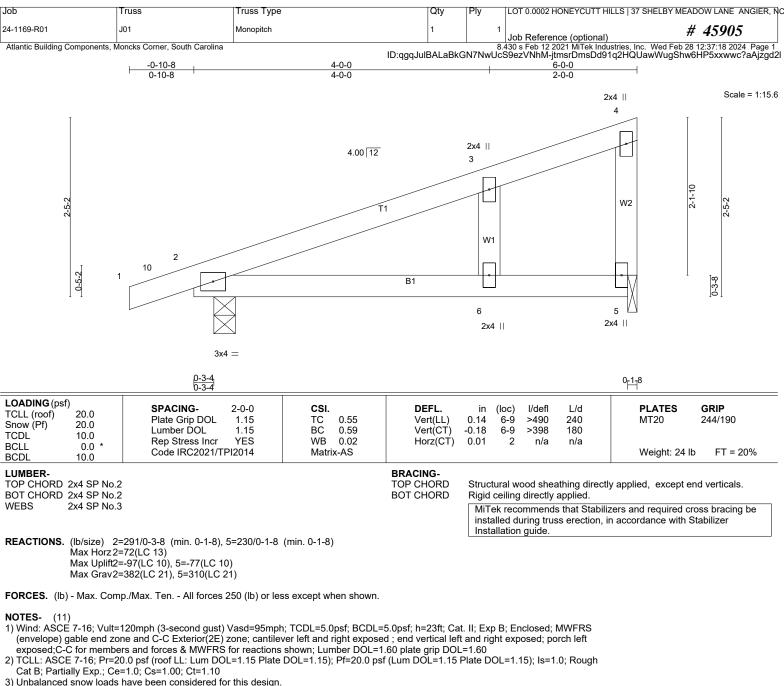


22-0-0

22-0-0 Plate Offsets (X,Y)-- [2:0-3-15,0-0-5], [14:0-3-15,0-0-5] LOADING (psf) SPACING-CSI. l /d PLATES GRIP 2-0-0 DEFL (loc) l/defl in 0.00 0.12 TCLL 20.0 Plate Grip DOL 1.25 тс Vert(LL) 14 n/r 180 **MT20** 244/190 TCDL 10.0 Lumber DOL 1.25 вс 0.13 Vert(CT) 0.00 15 n/r 80 BCLL 0.0 Rep Stress Incr YES WB 0.27 Horz(CT) 0.01 14 n/a n/a BCDL 10.0 Code IRC2021/TPI2014 Matrix-SH Weight: 142 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. OTHERS 2x4 SP No.3 Left 2x4 SP No.3 - 1-9-6, Right 2x4 SP No.3 -à 1-9-6 SI IDFR REACTIONS. All bearings 22-0-0. (lb) - Max Horz 2=-242(LC 8) Max Uplift All uplift 100 lb or less at joint(s) 2, 22, 24, 20, 17, 14 except 23=-107(LC 10), 25=-185(LC 10), 18=-107(LC 11), 16=-174(LC 11) Max Grav All reactions 250 lb or less at joint(s) 2, 24, 17, 14 except 21=277(LC 20), 22=272(LC 17), 23=280(LC 17), 25=269(LC 17), 20=269(LC 18), 18=280(LC 18), 16=257(LC 18) FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 7-8=-184/289, 8-9=-184/289 NOTES- (10-14) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 6-2-6, Corner(3R) 6-2-6 to 15-9-10, Exterior(2N) 15-9-10 to 18-0-14, Corner(3E) 18-0-14 to 22-10-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. 4) All plates are 2x4 MT20 unless otherwise indicated. Gable requires continuous bottom chord bearing. 6) Gable studs spaced at 2-0-0 oc. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads 8)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 22, 24, 20, 17, 14 except (jt=lb) 23=107, 25=185, 18=107, 16=174. 10) Trusses designed with 2018 IRC also comply with 2015 IRC. 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
12) Bearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representations of a possible bearing condition. Dearing symbols are only graphical representation.
13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Install the member must be braced MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS. L CARDING SE Ship of parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support

vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer of truss engineer. Bracing shown is to lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



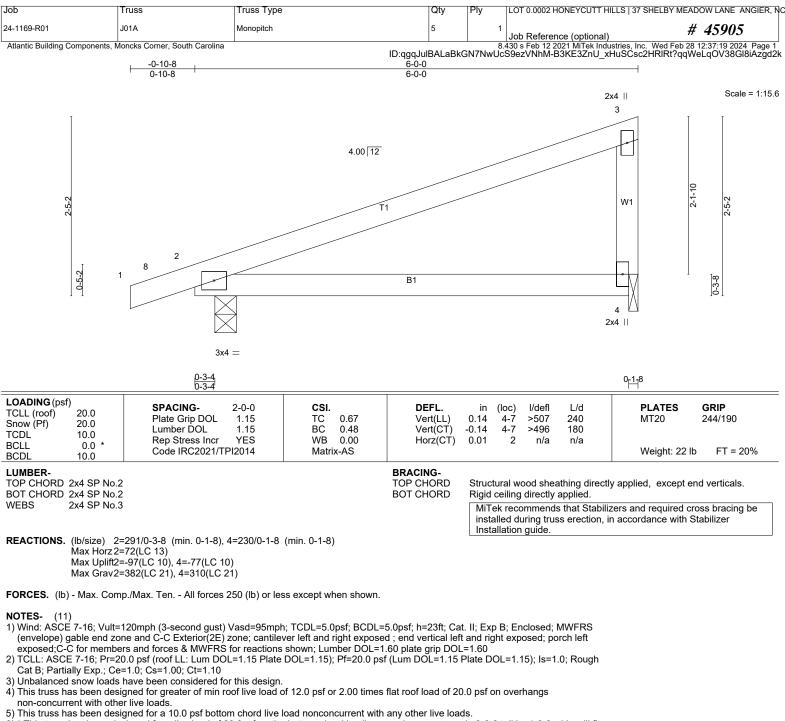


- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.

LOAD CASE(S) Standard

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.





6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

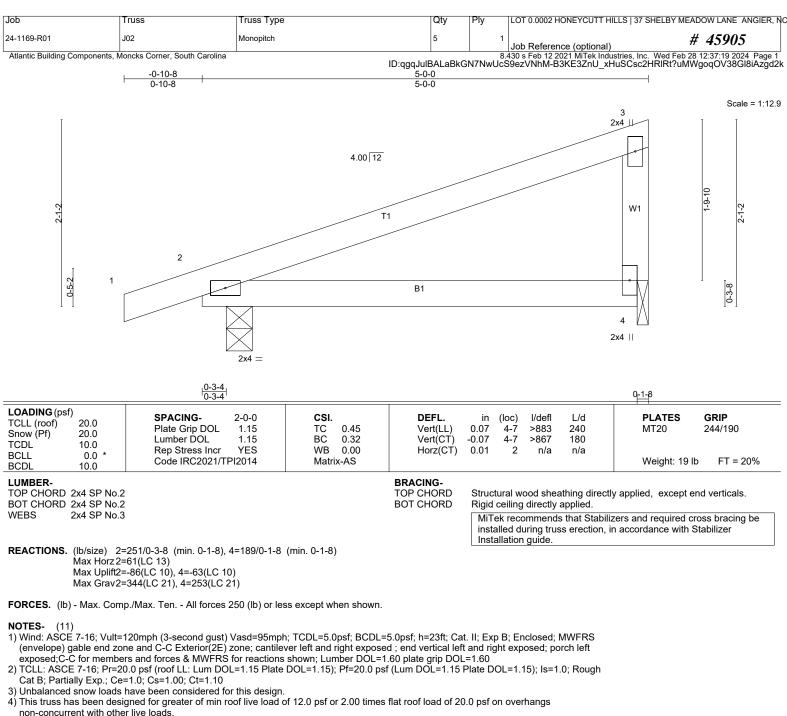
7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

LOAD CASE(S) Standard

- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



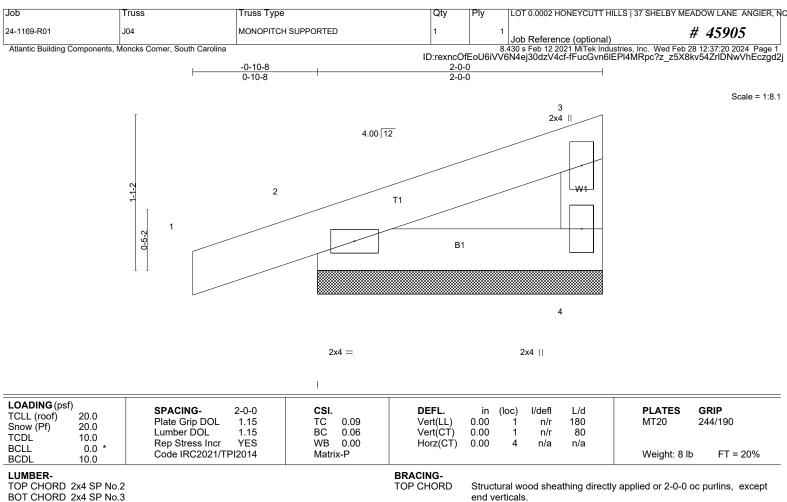


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LOAD CASE(S) Standard

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BOT CHORD

BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS. (lb/size) 4=62/2-0-0 (min. 0-1-8), 2=139/2-0-0 (min. 0-1-8) Max Horz 2=27(LC 11) Max Uplift4=-9(LC 14), 2=-41(LC 10) Max Grav4=76(LC 21), 2=180(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

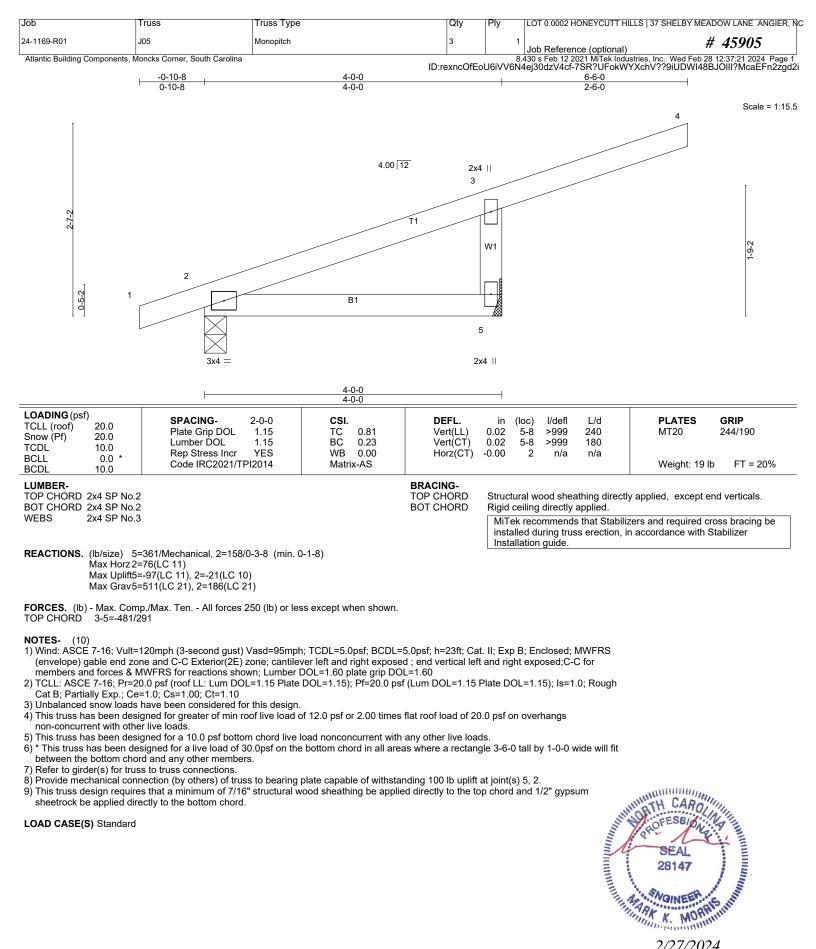
NOTES- (11)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B: Partially Exp.: Ce=1.0: Cs=1.00: Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads. 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 9) between the bottom chord and any other members.

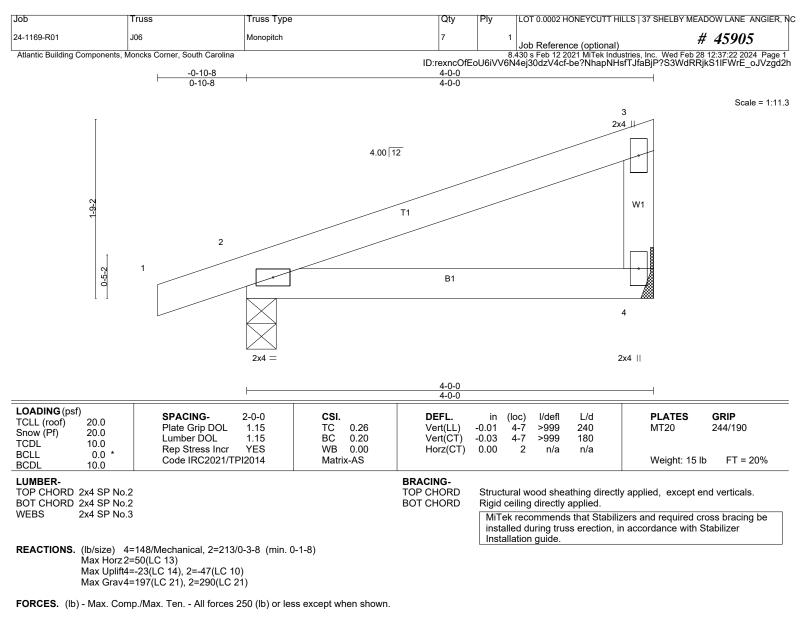
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

LOAD CASE(S) Standard





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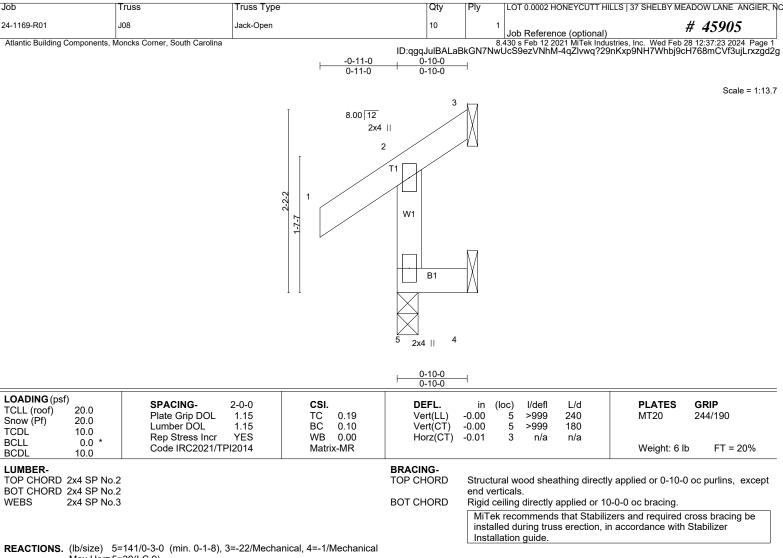


NOTES- (10)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
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- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard





Max Horz 5=39(LC 9)

Max Uplift3=-63(LC 18), 4=-29(LC 9) Max Grav5=203(LC 18), 3=12(LC 10), 4=33(LC 10)

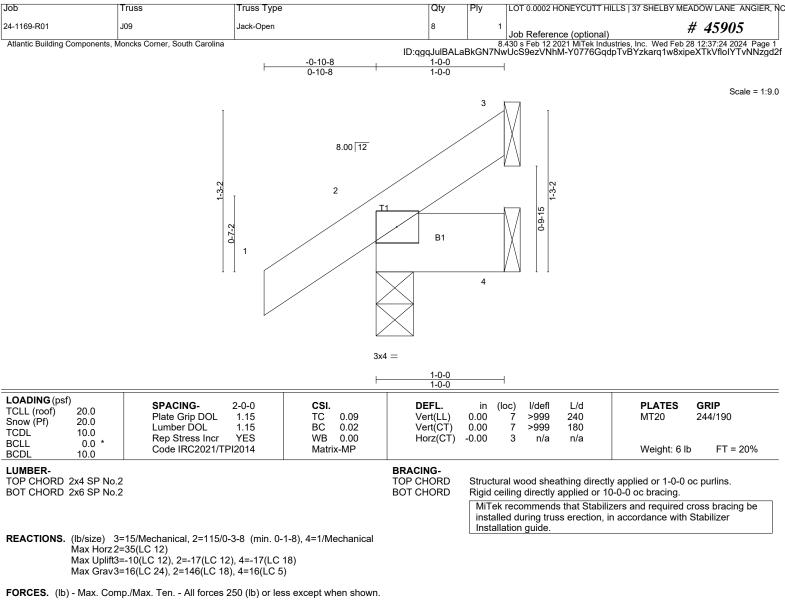
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
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- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.

LOAD CASE(S) Standard

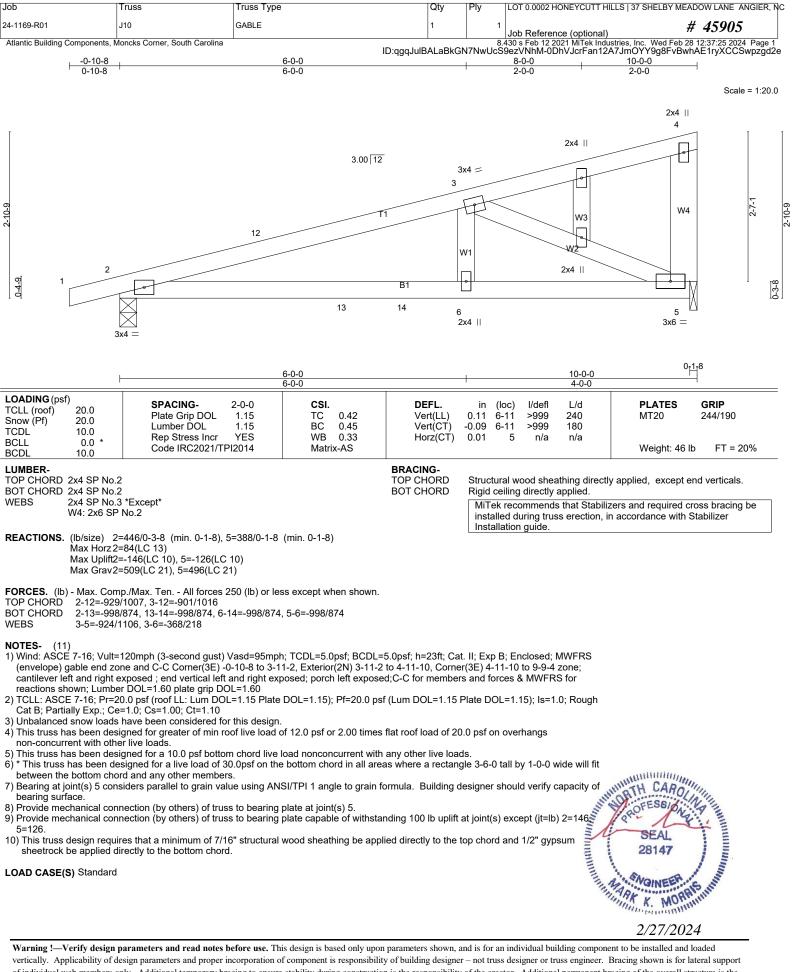




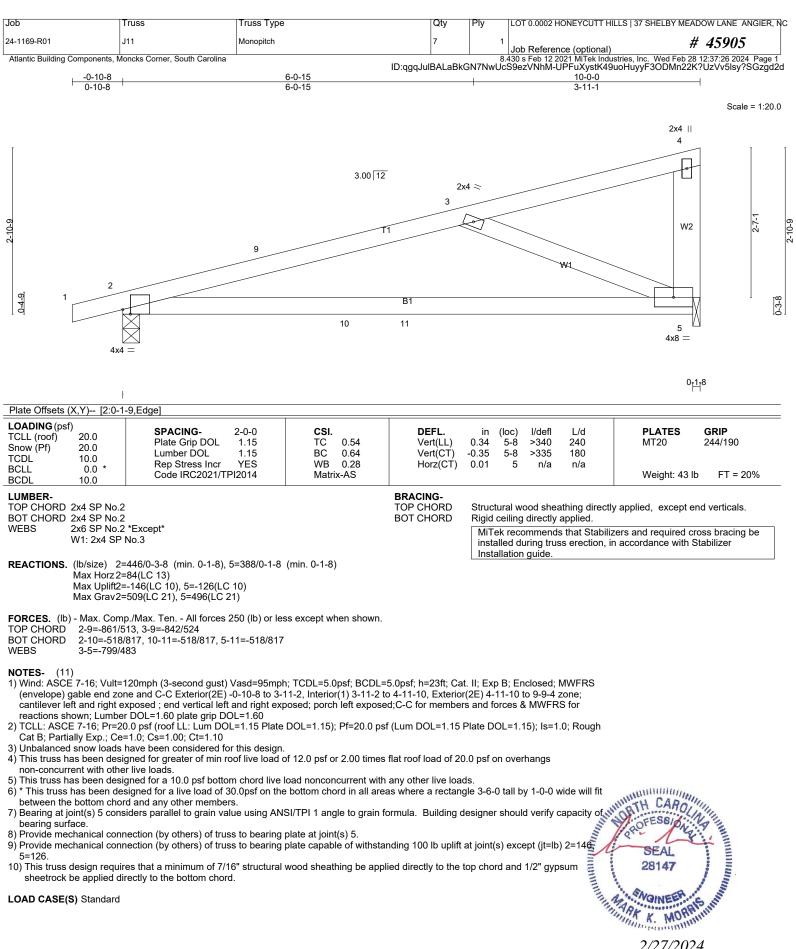
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- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.

LOAD CASE(S) Standard



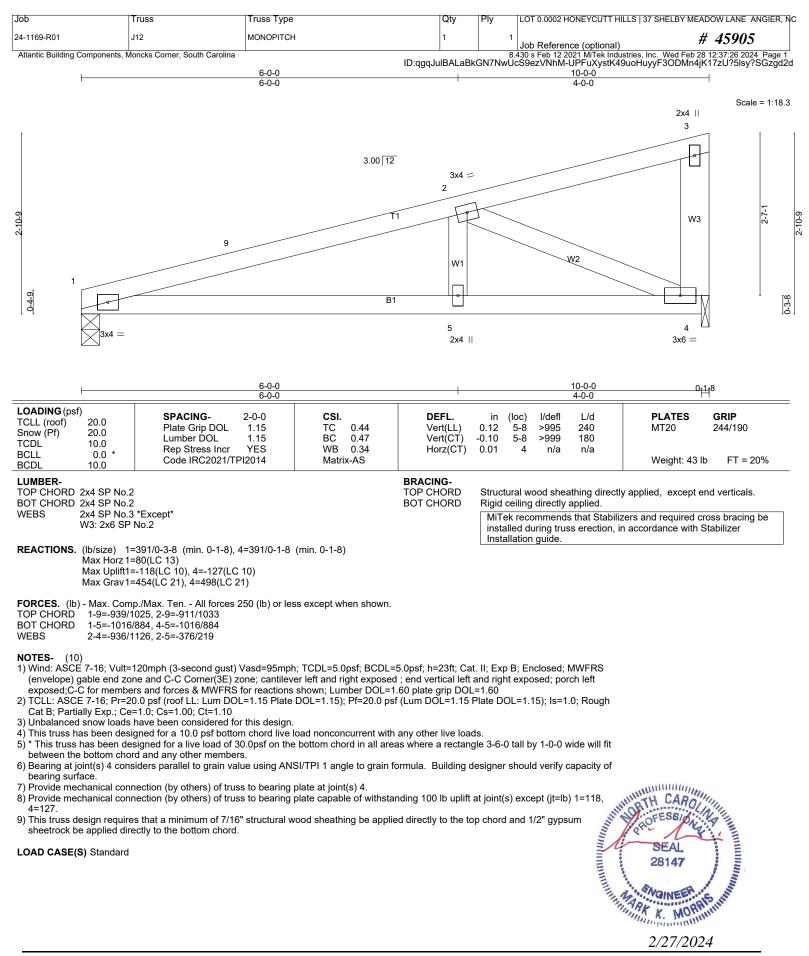


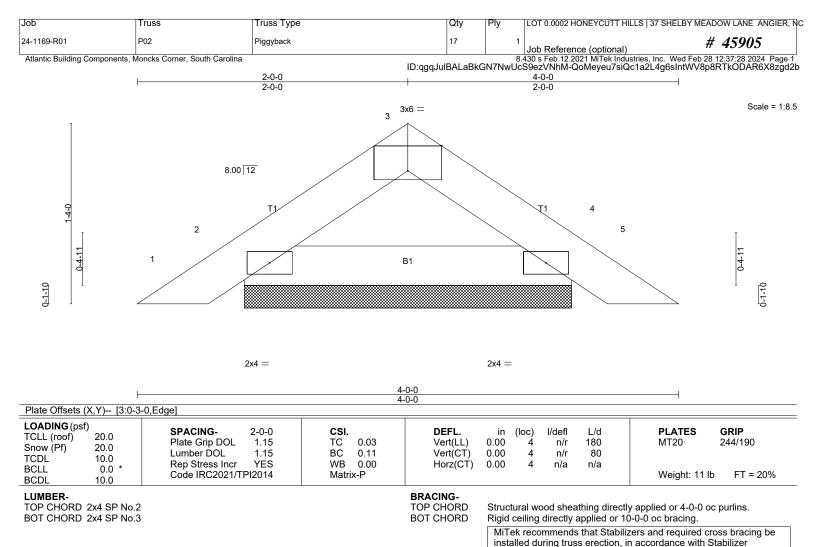
of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



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2/27/2024





Installation guide

REACTIONS. (lb/size) 2=128/2-5-0 (min. 0-1-8), 4=128/2-5-0 (min. 0-1-8) Max Horz 2=-23(LC 10) Max Uplift2=-16(LC 12), 4=-16(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

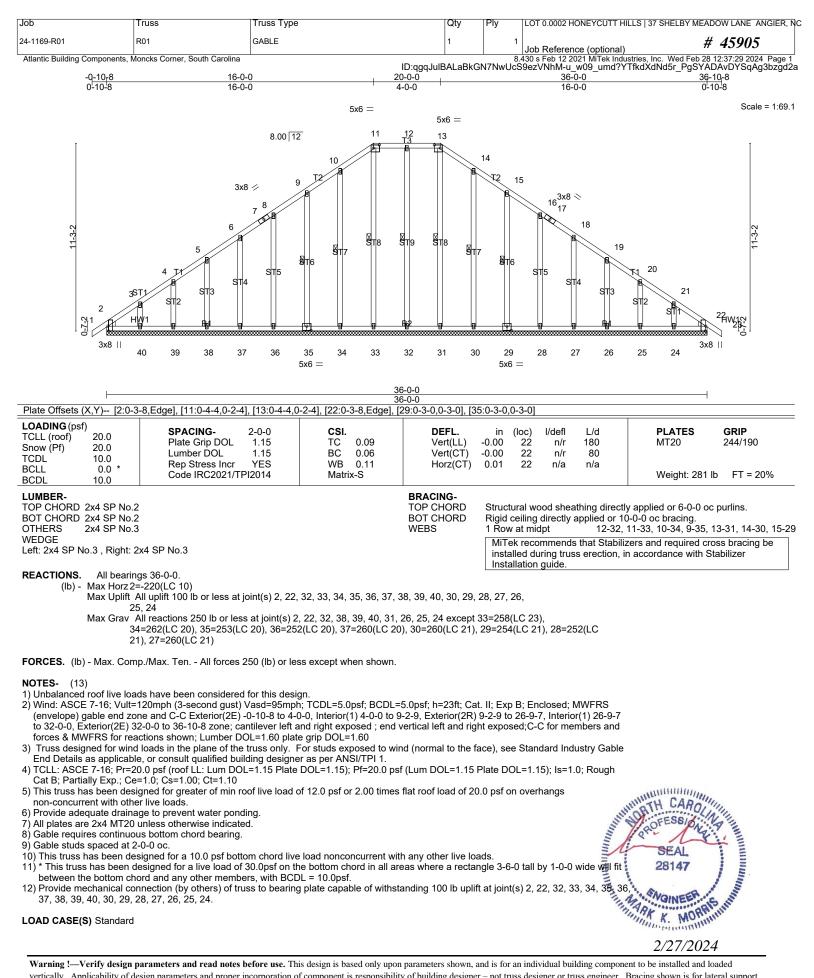
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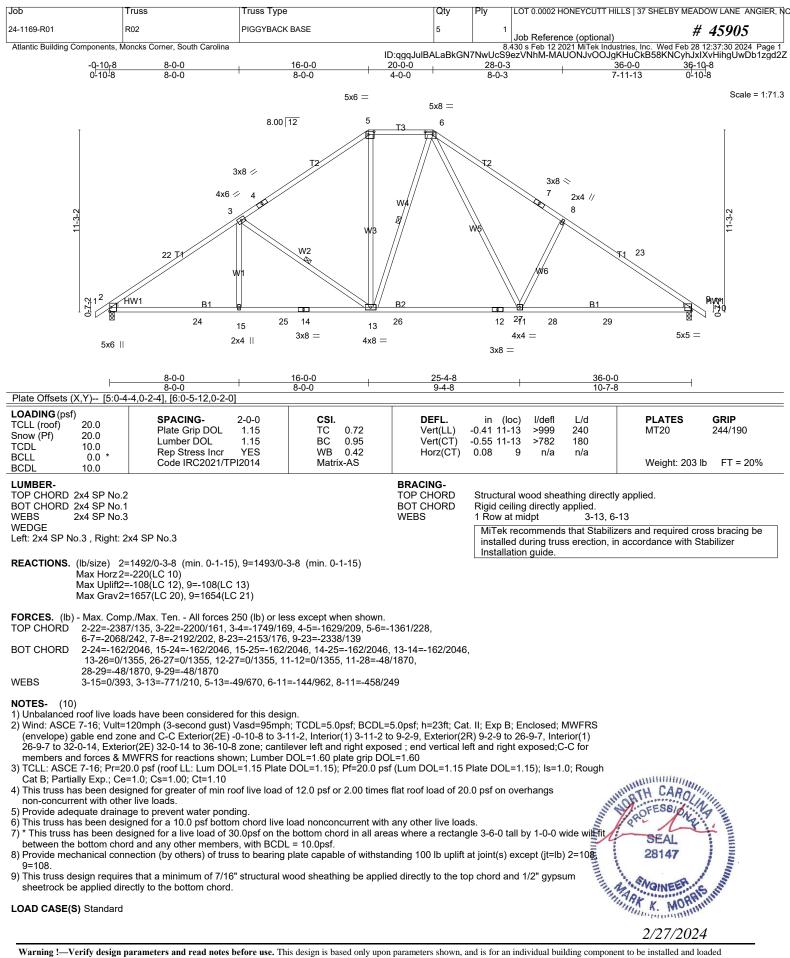
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

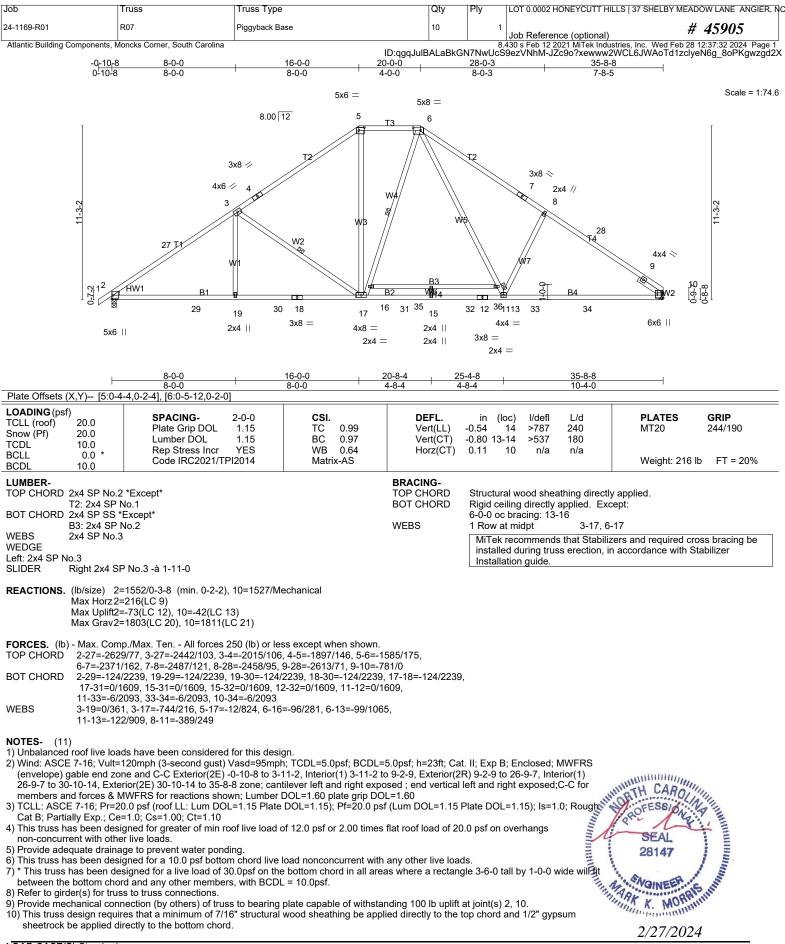
9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard









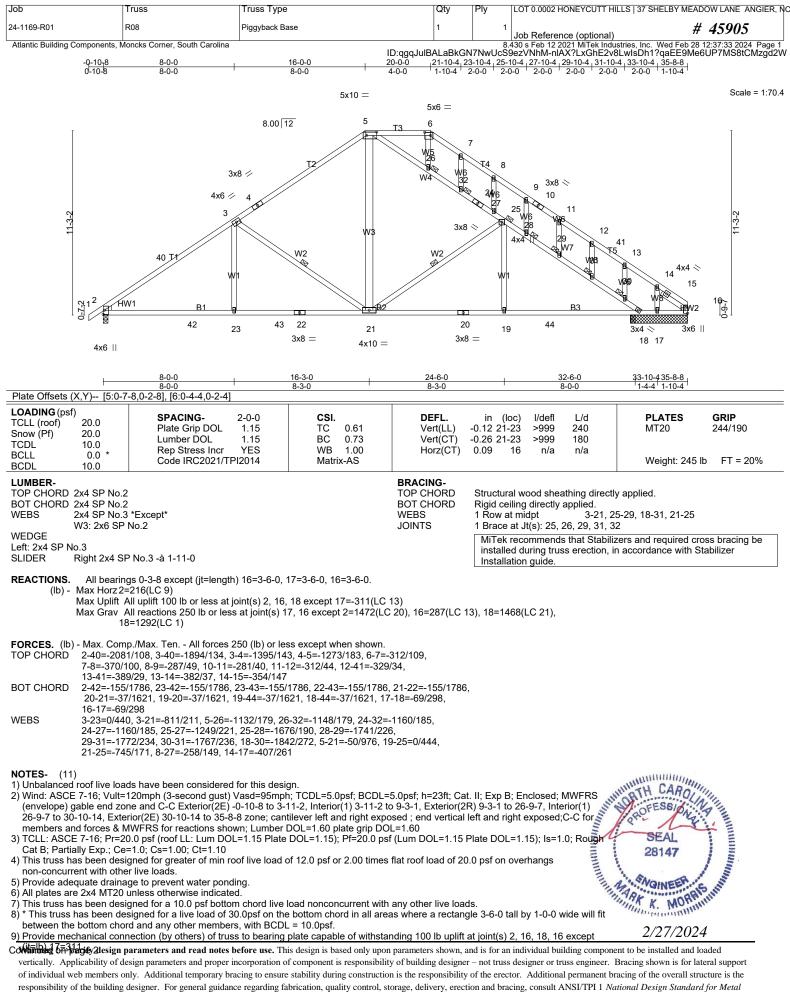


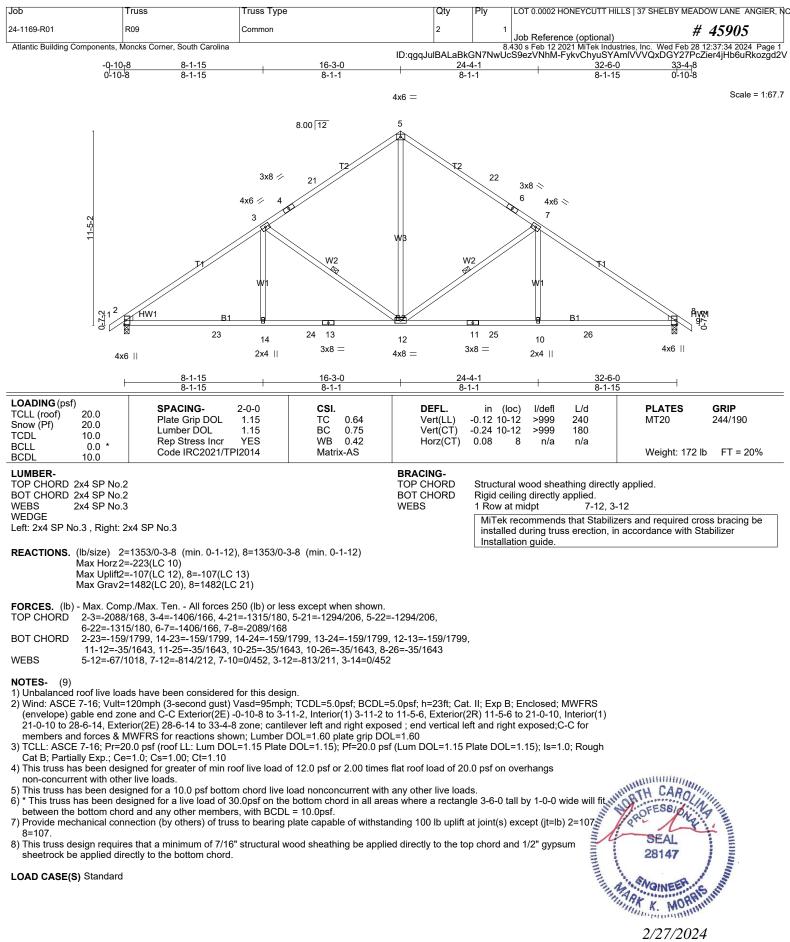
Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

	Job	Truss	Truss Type	Qty	Ply	LOT 0.0002 HONEYCUTT HILLS 37 SHELBY MEADOW LANE ANGIER, NC
	24-1169-R01	R08	Piggyback Base	1	1	Job Reference (optional) # 45905
Atlantic Building Components, Moncks Corner, South Carolina			ID:qgqJulB	ALaBkGN	8.4 I7NwUcS	430 s Feb 12 2021 MiTek Industries, Inc. Wed Feb 28 12:37:33 2024 Page 2 9ezVNhM-nIAX?LxGhE2v8LwIsDh1?qaEE9Me6UP7MS8tCMzgd2W

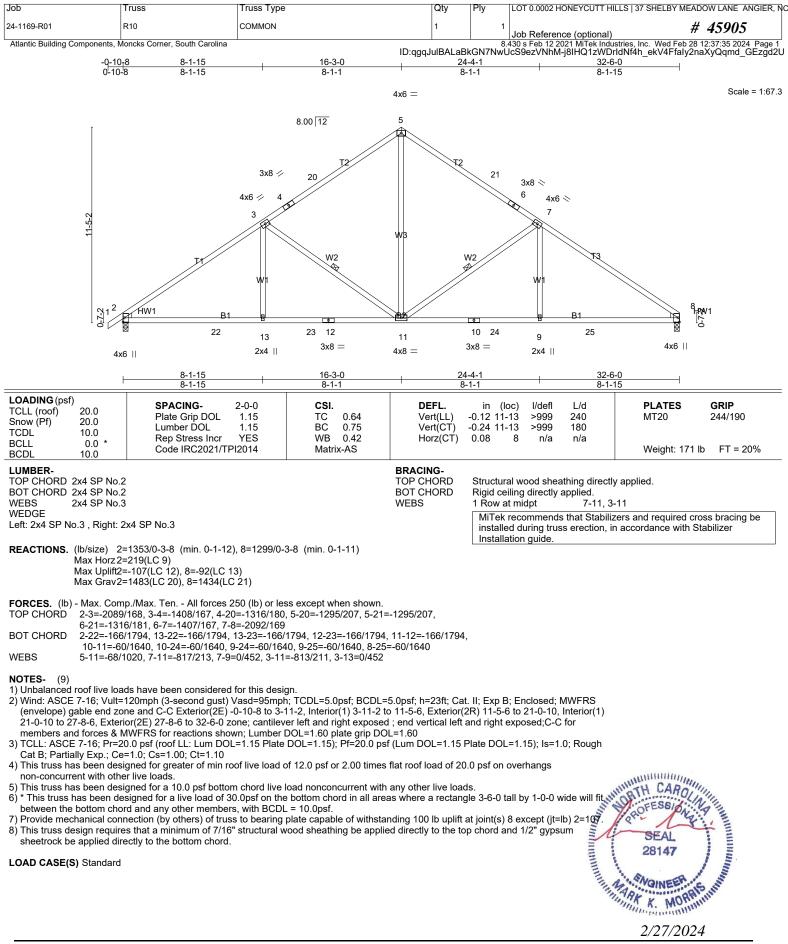
NOTES- (11)
 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

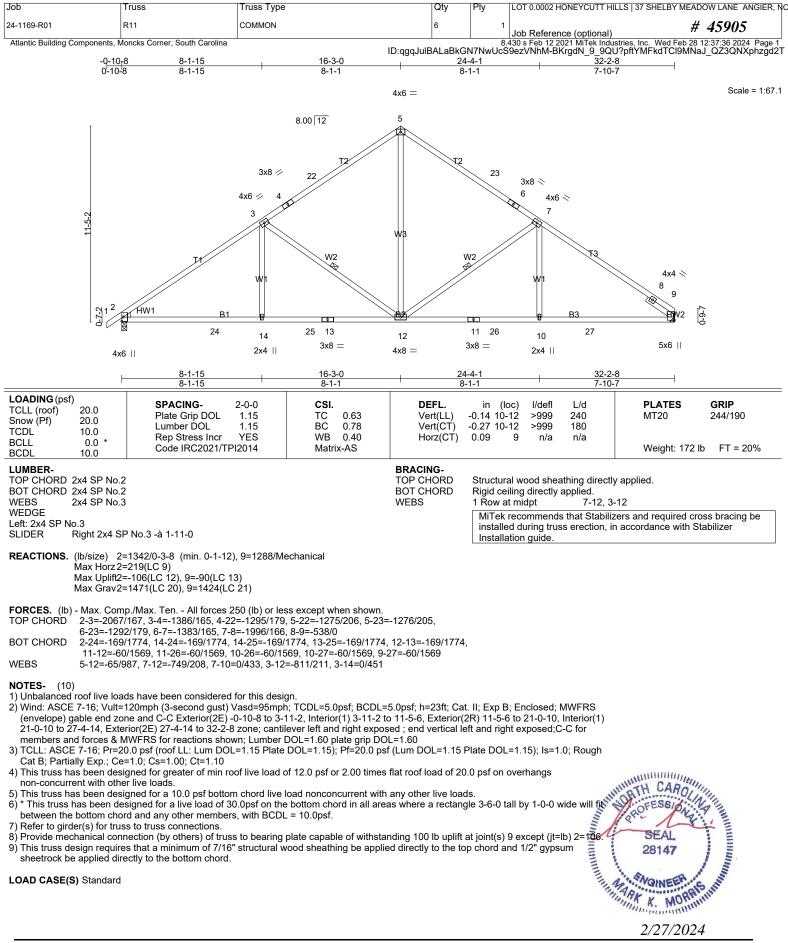
LOAD CASE(S) Standard

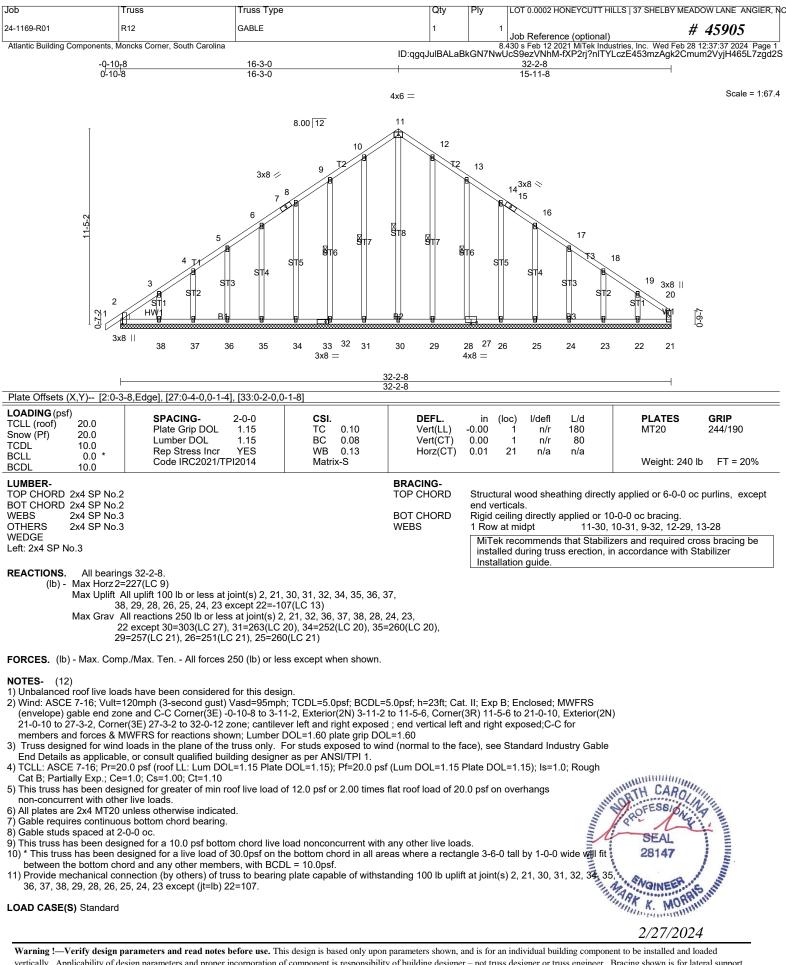


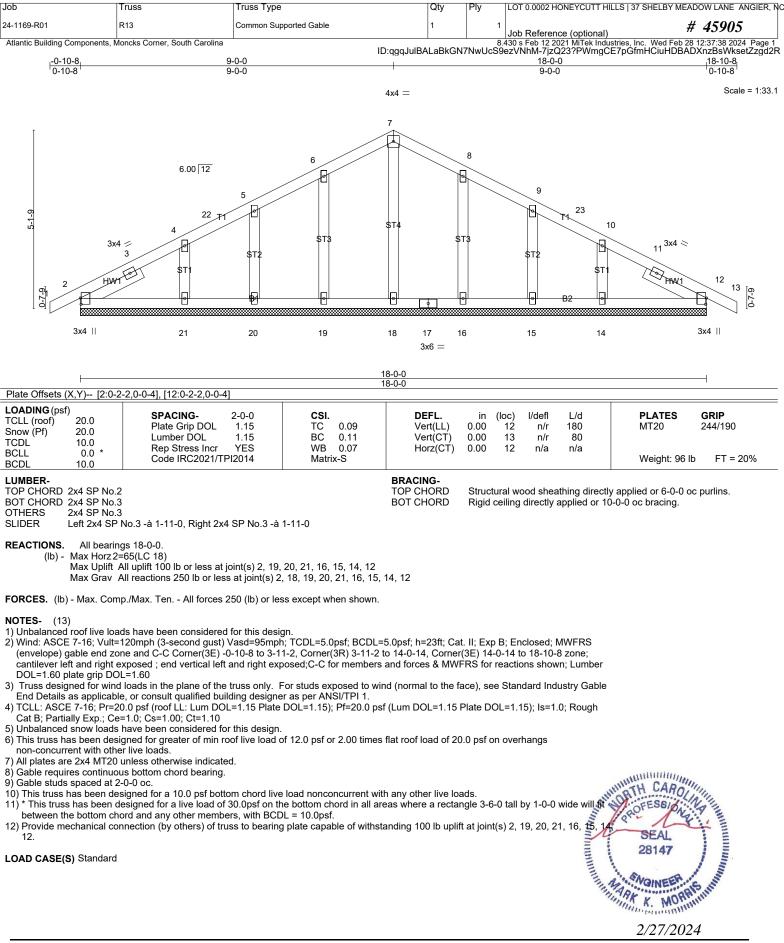


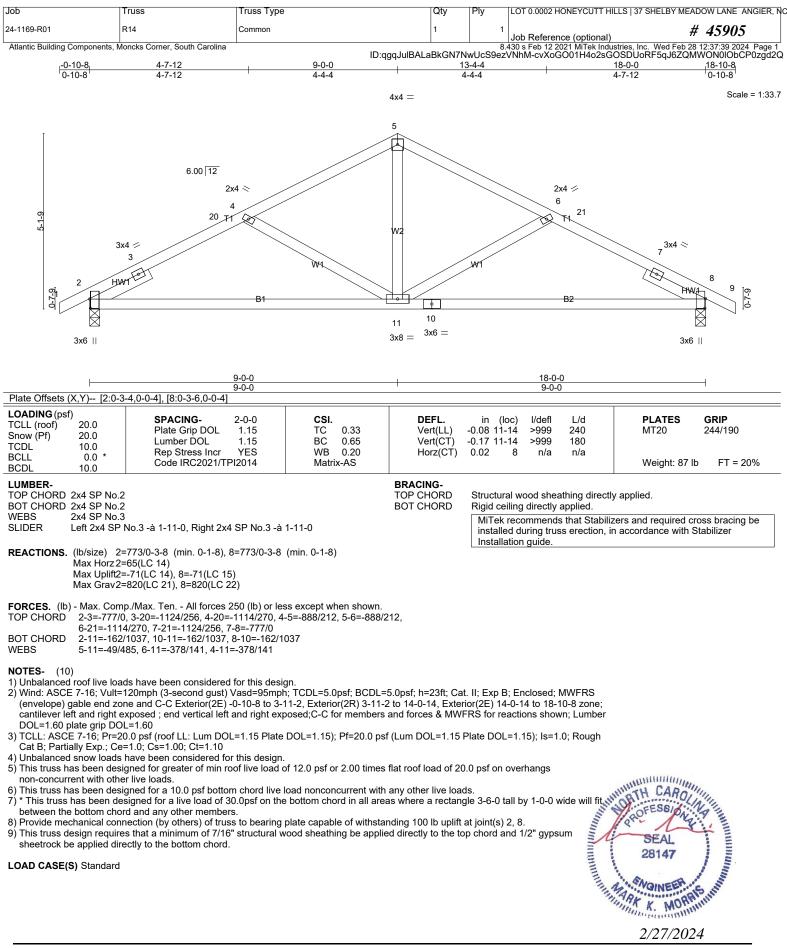
2/27/2024

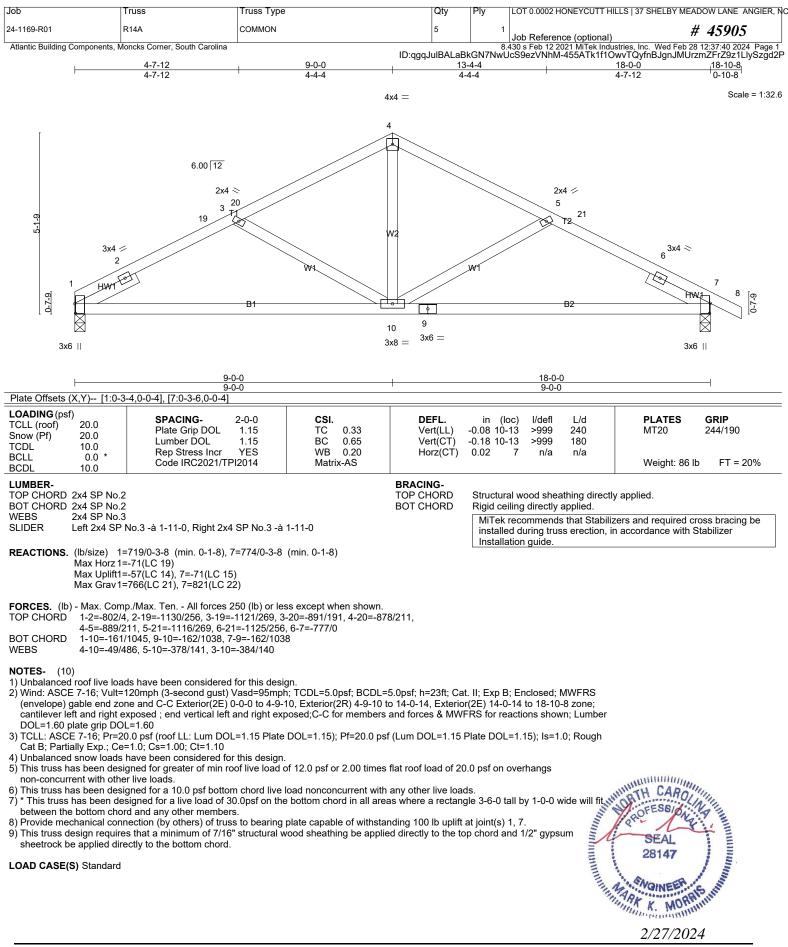












Job Truss 24-1169-R01 R15 Atlantic Building Components, Moncks Corner, South Carol	0-2-8 3-3-0 5-3-0 7-3-0	1 3 Job Reference 8.430 s Feb 12 3 JOB Reference ID:qgqJulBALaBkGN7NwUcS9ezVNM 9-3-0 11-3-0 13-3-0 15-3-0 18-3-8-16	ence (optional) 22021 MiTek Industries, Inc. We 1-NS0qx762OXpwpV??h9y, 8 ₁ 6-0	LBY MEADOW LANE ANGIER, NC # 45905 ed Feb 28 12:37:47 2024 Page 1 Jan9bMoCuOrTBadXdiYzgd21
	0-2-8 3-0-8 2-0-0 2-0-0	2-0-0 ' 2-0-0 ' 2-0-0 ' 2-0-0 ' 3-0-8 0 4x6)-2-8	Scale = 1:69.6
11:2-10	$6x10 = \begin{array}{c} 12.00 \overline{12} & 7 \\ 5 \\ 2 \\ 4 \\ 30 \\ 7 \\ 2 \\ 7 \\ 7 \\ 7 \\ 7 \\ 7 \\ 7 \\ 7 \\ 7$	$\begin{array}{c} 4x6 = & & & & & & & \\ 11 & 13 & & & & & \\ 11 & 13 & & & & & \\ 12 & 1 & 13 & & & & \\ 13 & 1 & 13 & & & \\ 14 & 13 & 1 & 13 & \\ 15 & 13 & 1 & 13 & \\ 15 & 13 & 1 & 13 & \\ 15 & 13 & 1 & 13 & \\ 15 & 13 & 1 & 13 & \\ 15 & 13 & 1 & 13 & \\ 15 & 13 & 1 & 13 & \\ 15 & 13 & 13 & \\ 15 & 13 & 13 & 13 & \\ 15 & 13 & 1$	$10 = \frac{19}{10 - 11 - 1}$	
	4-8-6 9-3-0 4-8-6 4-6-10) 4-6-10 4-8-6	-1	
Plate Offsets (X,Y) [1:0-6-8,0-1-7], [19:0-6-8,0-7], [19:0-6-8,0-7], [19:0-6-8,0-7], [19:0-6-8,0-7], [19:0-6-8,0-7], [19:0-6-8,0-7], [19:0-6-8,0-7], [19:0-6-8,0-7], [19:0-6-8,0-7], [19:0	2-0-0 CSI. - 1.15 TC 0.76 1.15 BC 0.48 r NO WB 0.96], [24:0-3-8,0-4-8] DEFL. in (loc) I/defl Vert(LL) -0.06 21-23 >999 Vert(CT) -0.12 21-23 >999 Horz(CT) 0.02 20 n/a	240 MT2 180 n/a	
LUMBER- TOP CHORD 2x4 SP No.2 *Except* T2,T3: 2x6 SP No.2 BOT CHORD 2x6 SP DSS WEBS 2x4 SP No.3	i	end verticals.	eathing directly applied or y applied or 10-0-0 oc bra , 16, 4, 13, 8	• • •
REACTIONS. (lb/size) 25=8257/0-3-8 (min. 0- Max Horz 25=226(LC 7) Max Uplift25=-460(LC 11), 20=-4 Max Grav25=9484(LC 4), 20=774	4(LC 10)			
12-14=-5186/366, 14-17=-5178/ 1-3=-4157/495, 3-4=-4157/495, 9-11=-1659/120, 11-13=-1659/1 18-19=-4147/503	5-7=-5186/364, 7-10=-5027/398, 10-12 316, 17-19=-5388/266, 1-25=-6814/38 4-6=-1659/120, 6-8=-1659/120, 8-9=- 20, 13-15=-1659/120, 15-16=-1659/12	2=-5026/400, 8, 19-20=-6825/388, 1659/120, 0, 16-18=-4147/503,		
28-29=-616/7748, 23-29=-616/7 21-30=-547/7776, 21-31=-92/13 WEBS 11-23=-393/6381, 16-23=-2659/	352, 24-27=-260/1352, 24-28=-616/77 748, 22-23=-547/7776, 22-30=-547/77 24, 31-32=-92/1324, 20-32=-92/1324 409, 16-21=-162/1508, 4-23=-2686/41 47, 17-18=-145/253, 10-11=-497/6758	76, 0, 4-24=-160/1495,		
 NOTES- (14) 1) 3-ply truss to be connected together with 10d together the form of the second sec	0.131"x3") nails as follows: at 0-9-0 oc, 2x6 - 2 rows staggered at ws staggered at 0-4-0 oc. -0 oc.	0-9-0 oc.	h. Ply to ply t; MWFRS p DOL=150 1.0; Rough SE 281 0 wide will fit d verify =lb) 25=460, 2/22 building component to be insta truss engineer. Bracing show	AROLUS BUCK STATES MORAL MORAL MORAL MORAL MORAL MINING MORAL MINING MORAL MINING MORAL MINING MORAL MINING MININA MININA MININA MININA MININA MININA MININA MININA MININA MININA MININA MININA MININA MININA MININA MININA MININI

Jo	b	Truss	Truss Type	Qty	Ply	LOT 0.0002 HONEYCUTT HILLS 37 SHELBY MEADO	W LANE ANGIER, NC
24	-1169-R01	R15	COMMON GIRDER	1	3	Job Reference (optional) #	45905
Atlantic Building Components, Moncks Corner, South Carolina		ID:qg	8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Feb 28 12:37:47 2024 Page 2 ID:qqqJuIBALaBkGN7NwUcS9ezVNhM-NS0qx762OXpwpV??h9yJan9bMoCuOrTBadXdiYzqd2I				

NOTES- (14)

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 27 lb down and 60 lb up at 0-4-11, and 27 lb down and 60 lb up at 18-1-5 on top chord, and 1779 lb down and 54 lb up at 0-1-12, 29 lb down and 31 lb up at 2-3-4, 1771 lb down and 62 lb up at 2-3-12, 29 lb down and 31 lb up at 4-3-4, 1771 lb down and 62 lb up at 4-3-12, 29 lb down and 31 lb up at 6-3-4, 1771 lb down and 62 lb up at 4-3-12, 29 lb down and 31 lb up at 6-3-12, 29 lb down and 31 lb up at 8-3-4, 1771 lb down and 62 lb up at 10-3-4, 1771 lb down and 62 lb up at 10-3-4, 1771 lb down and 62 lb up at 10-3-4, 1771 lb down and 62 lb up at 12-3-4, 1771 lb down and 62 lb up at 10-3-4, 1771 lb down and 62 lb up at 10-3-4, 1771 lb down and 62 lb up at 10-3-4, 1771 lb down and 62 lb up at 10-3-4, 1771 lb down and 62 lb up at 10-3-4, 1771 lb down and 31 lb up at 10-3-4, 1771 lb down and 31 lb up at 10-3-4, 1771 lb down and 31 lb up at 10-3-4, 1771 lb down and 31 lb up at 10-3-4, 1771 lb down and 31 lb up at 10-3-4, 1771 lb down and 31 lb up at 10-3-4, 1771 lb down and 31 lb up at 10-3-4, 1771 lb down and 31 lb up at 10-3-4, 1771 lb down and 31 lb up at 10-3-4, 1771 lb down and 31 lb up at 10-3-4, 1771 lb down and 31 lb up at 10-3-4, 1771 lb down and 62 lb up at 10-3-12, 29 lb down and 31 lb up at 10-3-4, 1771 lb down and 62 lb up at 10-3-12, 29 lb down and 31 lb up at 10-3-4, 1771 lb down and 62 lb up at 10-3-12, 29 lb down and 31 lb up at 10-3-4, 1771 lb down and 62 lb up at 10-3-12, 29 lb down and 31 lb up at 10-3-4, 1771 lb down and 62 lb up at 10-3-12, 29 lb down and 31 lb up at 10-3-4, 1771 lb down and 62 lb up at 10-3-12, 29 lb down and 31 lb up at 10-3-4, 1771 lb down and 62 lb up at 10-3-12, 20 lb down and 31 lb up at 10-3-4, and 1771 lb down and 62 lb up at 10-3-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

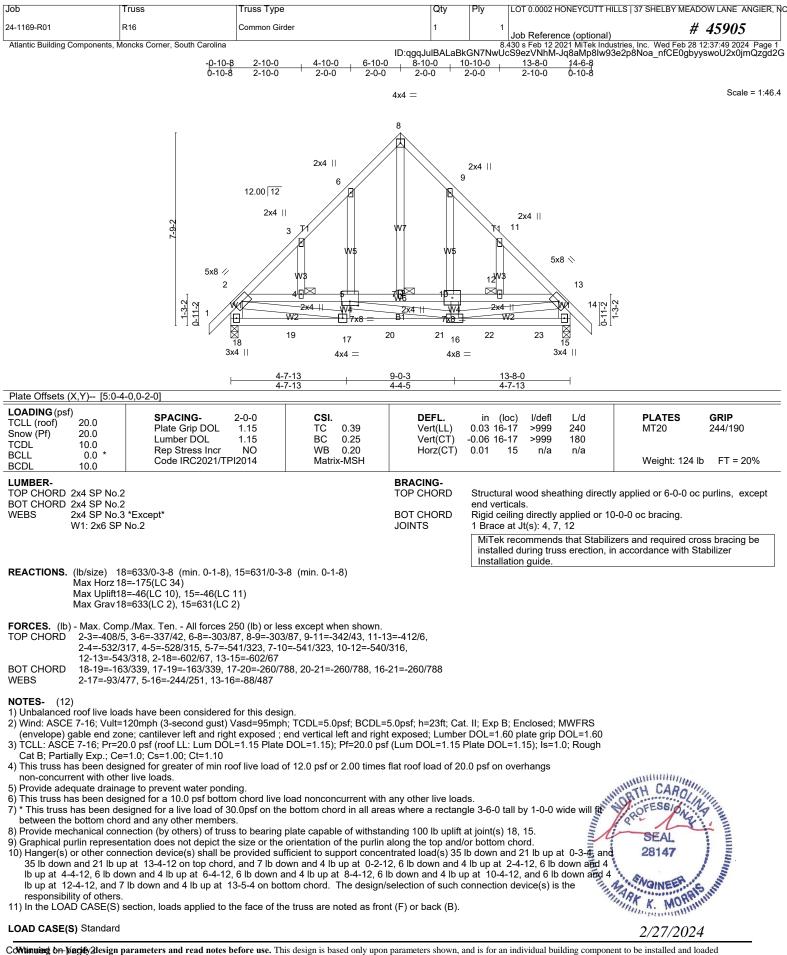
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-10=-60, 10-19=-60, 20-25=-20, 1-19=-6(F)

Concentrated Loads (lb)

Vert: 22=-1503(F=4, B=-1507) 1=20(F) 19=20(F) 25=-1515(B) 26=-1503(F=4, B=-1507) 27=-1503(F=4, B=-1507) 28=-1503(F=4, B=-1507) 29=-1503(F=4, B=-1507) 30=-1503(F=4, B=-1507) 31=-1503(F=4, B=-1507) 32=-1503(F=4, B=-1507) 32=-1503(F





vertically. Applicability of design parameters and read notes before user in sousce only upon parameters shown, and is for an individual bolicing component to be instanted and backed only upon parameters shown, and is for an individual bolicing component to be instanted and backed only upon parameters shown, and is for an individual bolicing component to be instanted and backed only upon parameters shown, and is for an individual bolicing component to be instanted and backed only upon parameters shown, and is for an individual bolicing component to be instanted and backed only upon parameters shown, and is for an individual bolicing component is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusses Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0002 HONEYCUTT HILLS 37 SH	IELBY MEADOW LANE ANGIER, NC
24-1169-R01	R16	Common Girder	1	1	Job Reference (optional)	# 45905
Atlantic Building Components, Moncks Corner, South Carolina				8.	.430 s Feb 12 2021 MiTek Industries, Inc.	Wed Feb 28 12:37:49 2024 Page 2

ID:qgqJulBALaBkGN7NwUcS9ezVNhM-Jq8aMp8lw93e2p8Noa_nfCE0gbyyswoU2x0jmQzgd2G

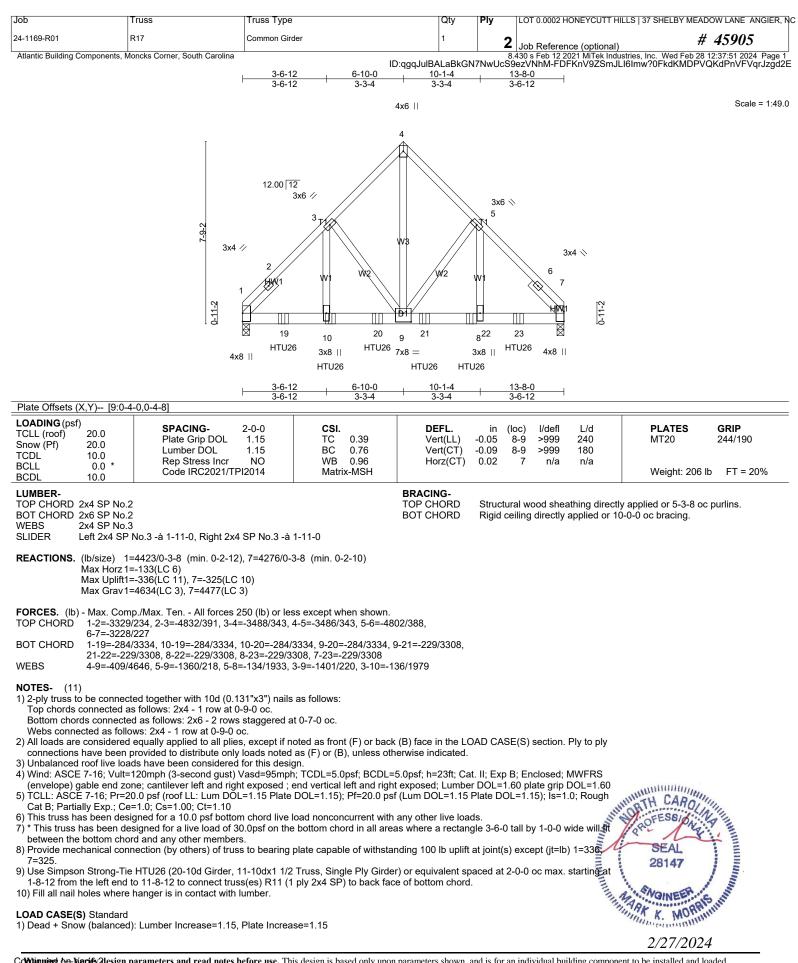
LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-8=-60, 8-13=-60, 13-14=-60, 15-18=-20, 2-13=-8(F) Concentrated Loads (lb)

Vert: 18=4(F) 15=4(F) 17=4(F) 19=4(F) 20=4(F) 21=4(F) 22=4(F) 23=4(F)





Job	Truss	Truss Type	Qty	Ply	LOT 0.0002 HONEYCUTT HILLS 37	7 SHELBY MEADOW LANE ANGIER, NC
24-1169-R01	R17	Common Girder	1	2	Job Reference (optional)	# 45905
Atlantic Building Componer	ts Moncks Corner, South Carolina				430 s Eeb 12 2021 MiTek Industries In	nc Wed Eeb 28 12:37:51 2024 Page 2

ID:qgqJulBALaBkGN7NwUcS9ezVNhM-FDFKnV9ZSmJLl6Imw?0FkdKMDPVQKdPnVFVqrJzgd2E

LOAD CASE(S) Standard

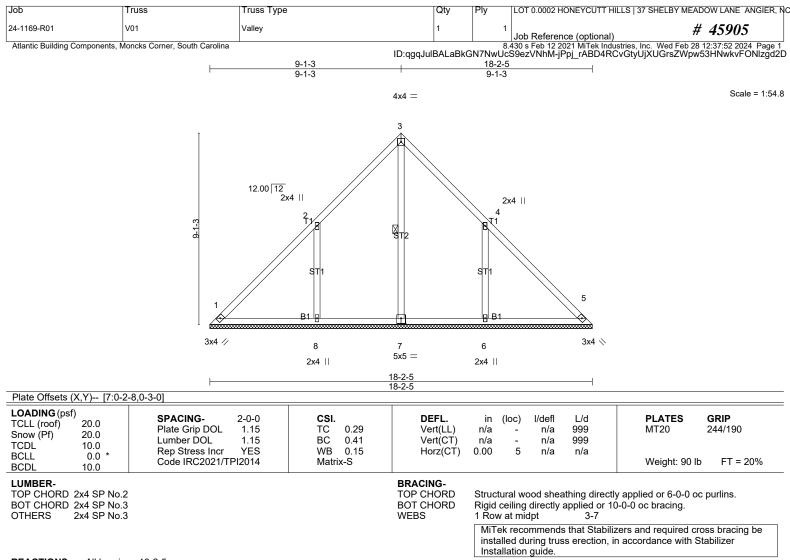
Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 11-15=-20

Concentrated Loads (lb)

Vert: 10=-1268(B) 19=-1268(B) 20=-1268(B) 21=-1268(B) 22=-1268(B) 23=-1268(B)



2/27/2024



REACTIONS. All bearings 18-2-5.

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-344/260, 4-6=-344/260

NOTES- (8)

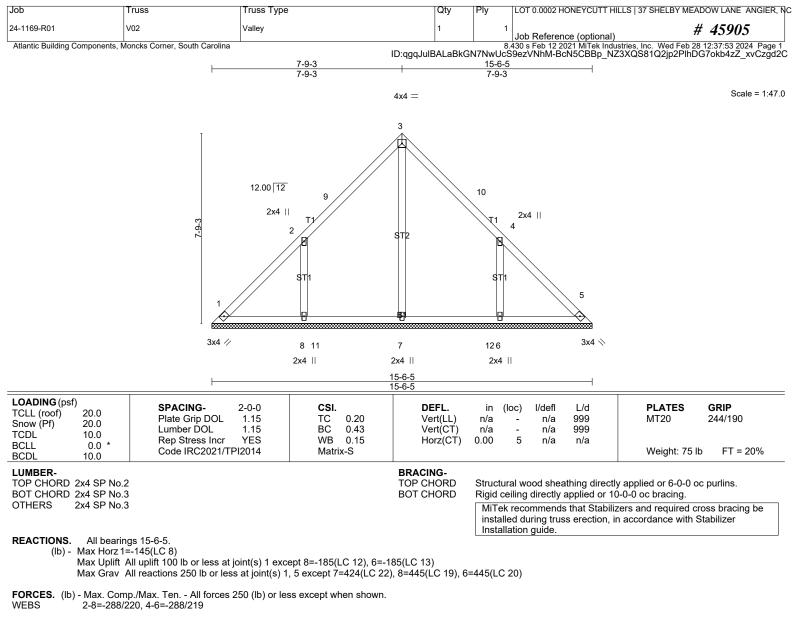
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-3, Exterior(2R) 5-1-3 to 13-1-3, Exterior(2E) 13-1-3 to 17-10-2 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=222, 6=222.





⁽lb) - Max Horz 1=-171(LC 8) Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-222(LC 12), 6=-222(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=424(LC 22), 8=542(LC 19), 6=541(LC 20)



NOTES- (8)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 10-4-8, Exterior(2E) 10-4-8 to 15-2-2 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

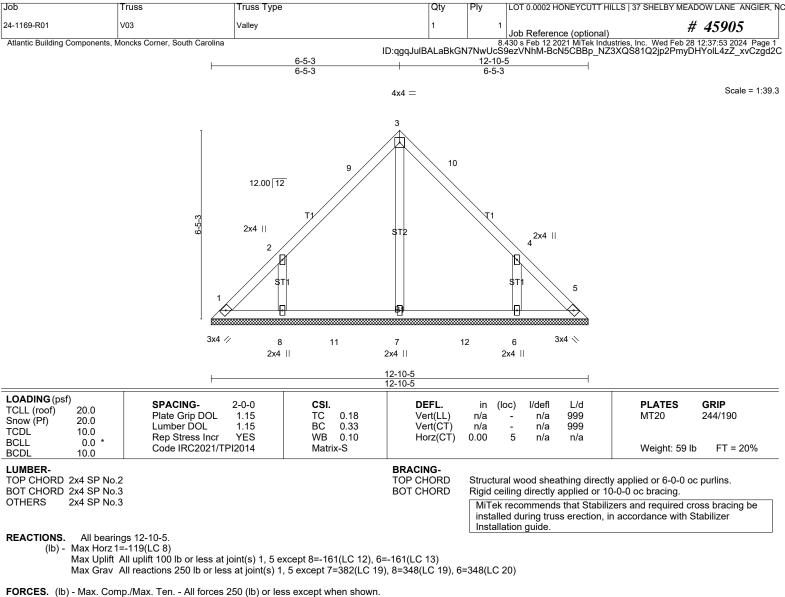
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=185, 6=185.

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LOAD CASE(S) Standard
```





WEBS 2-8=-254/196, 4-6=-254/196

NOTES- (8)

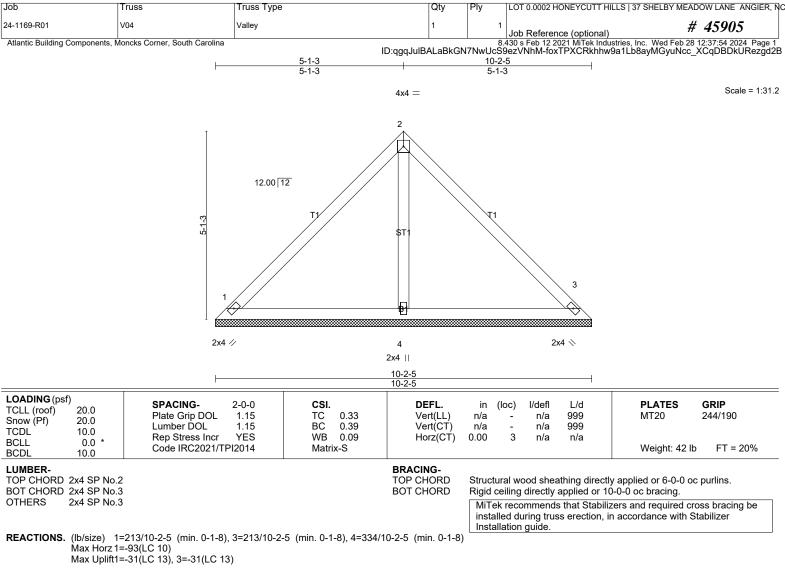
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 7-8-8, Exterior(2E) 7-8-8 to 12-6-2 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=161, 6=161.

```
LOAD CASE(S) Standard
```





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

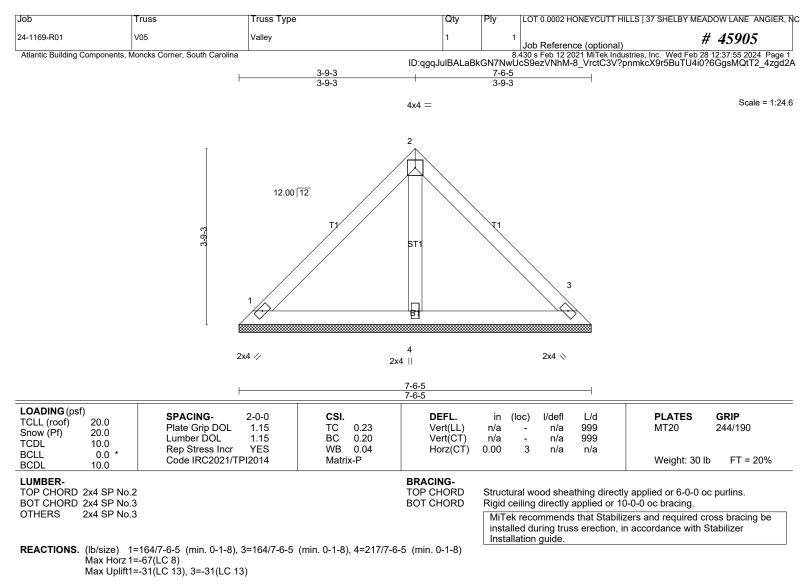
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

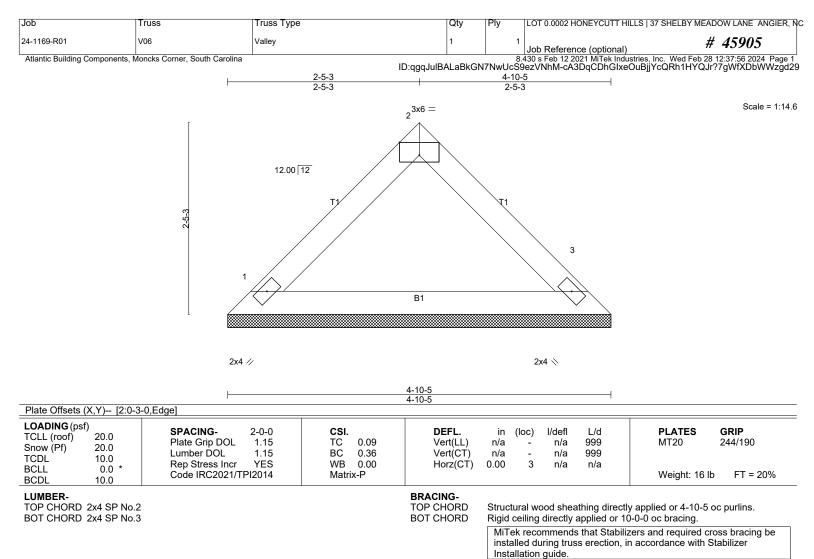
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 1=166/4-10-5 (min. 0-1-8), 3=166/4-10-5 (min. 0-1-8) Max Horz 1=-41(LC 10) Max Uplift1=-8(LC 12), 3=-8(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

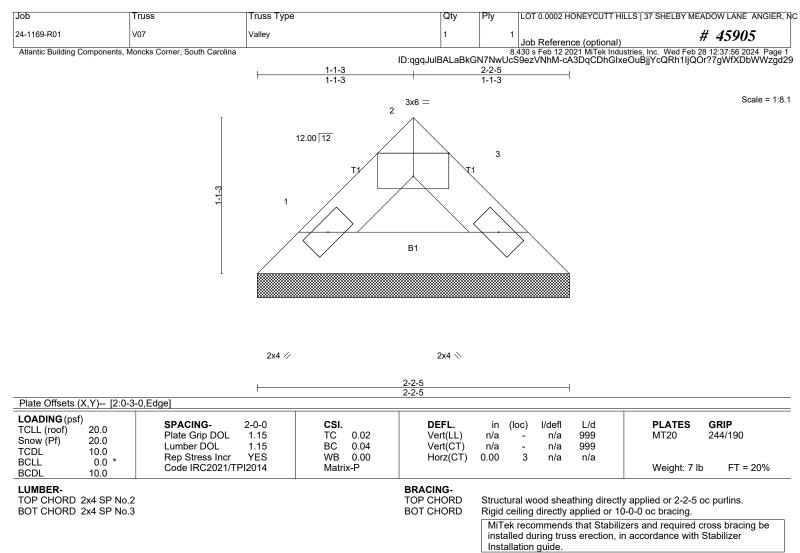
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 1=60/2-2-5 (min. 0-1-8), 3=60/2-2-5 (min. 0-1-8) Max Horz 1=15(LC 11) Max Uplift1=-3(LC 12), 3=-3(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

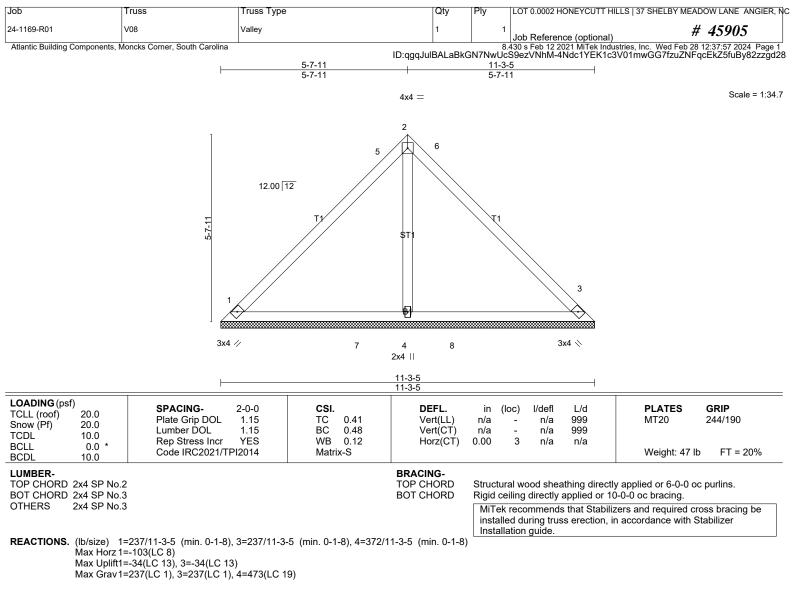
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 6-1-8, Exterior(2E) 6-1-8 to 10-11-2 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 and C-D and C-D

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

Gable requires continuous bottom chord bearing.

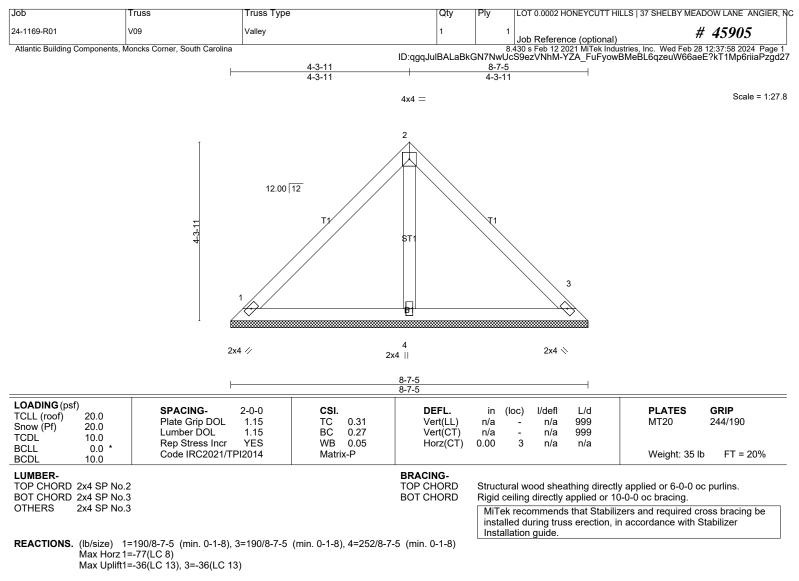
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

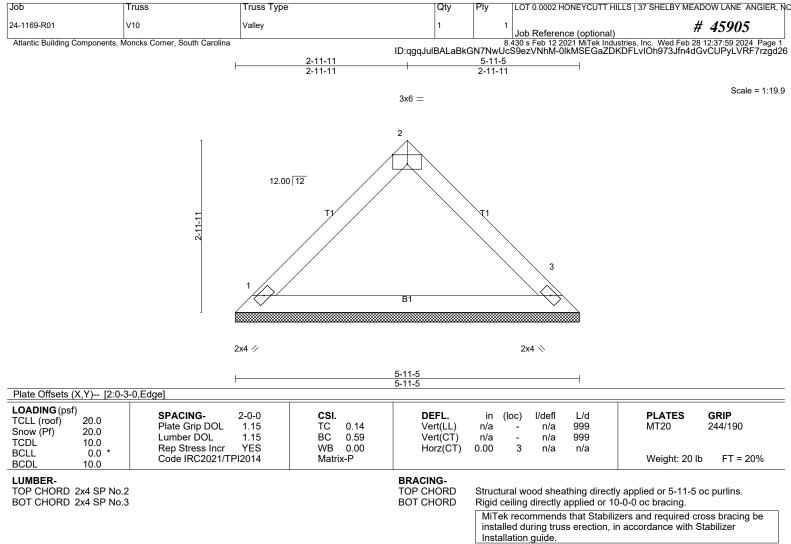
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 1=210/5-11-5 (min. 0-1-8), 3=210/5-11-5 (min. 0-1-8) Max Horz 1=-51(LC 8) Max Uplift1=-10(LC 12), 3=-10(LC 12)

NOTES- (8)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

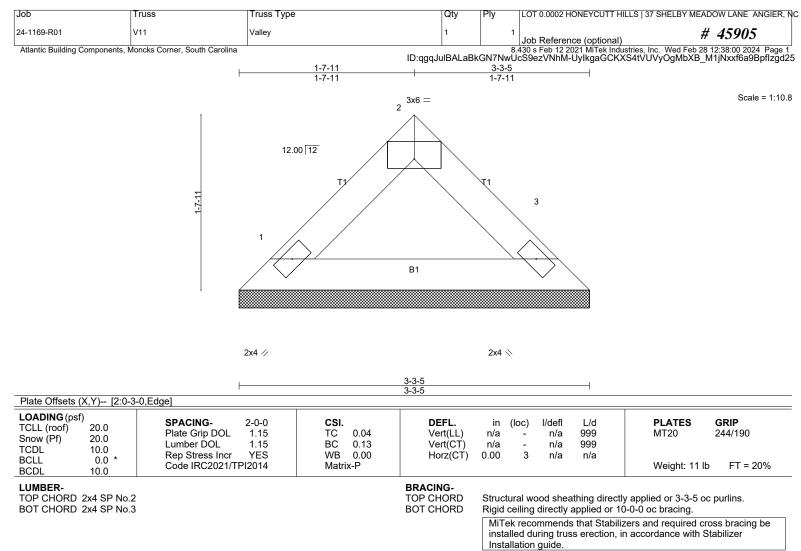
4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.



REACTIONS. (lb/size) 1=103/3-3-5 (min. 0-1-8), 3=103/3-3-5 (min. 0-1-8) Max Horz 1=25(LC 11) Max Uplift1=-5(LC 12), 3=-5(LC 13)

NOTES- (8)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

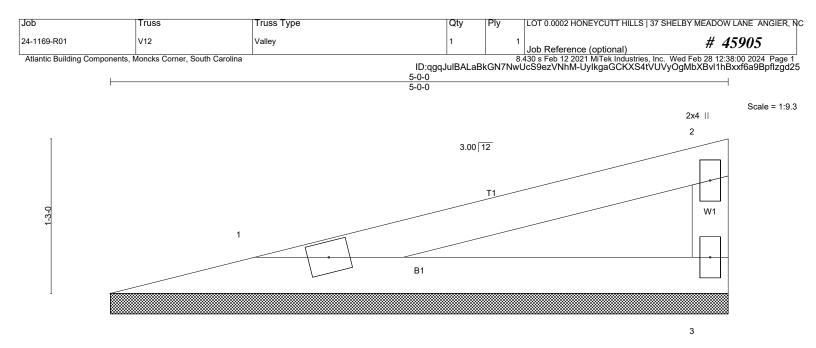
4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.



3x4 =

2x4 ||

LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.33 BC 0.27 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d n/a - n/a 999 n/a - n/a 999 0.00 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 14 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing direc end verticals. Rigid ceiling directly applied or 1	ly applied or 5-0-0 oc purlins, except 0-0-0 oc bracing.
				MiTek recommends that Stabil installed during truss erection, Installation guide.	izers and required cross bracing be in accordance with Stabilizer

REACTIONS. (lb/size) 1=147/5-0-0 (min. 0-1-8), 3=147/5-0-0 (min. 0-1-8) Max Horz 1=29(LC 11) Max Uplift1=-18(LC 10), 3=-21(LC 14) Max Grav1=184(LC 20), 3=184(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard

