# Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 46668

JOB: 24-1484-R01

JOB NAME: LOT 0.0096 BLAKE POND

Wind Code: 37

Wind Speed: Vult= 115mph

Exposure Category: B

Mean Roof Height (feet): 23

These truss designs comply with IRC 2015 as well as IRC 2018.

20 Truss Design(s)

## Trusses:

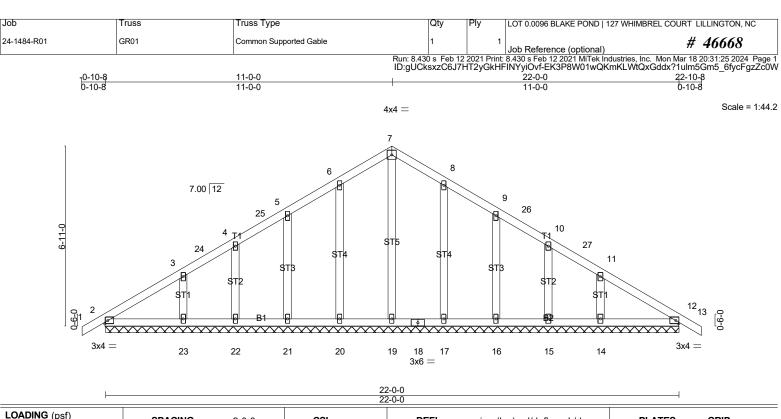
GR01, GR02, PB01, PB02, PB03, PB03A, PB04, R01, R02, R03, R05, R06, R07, R08, R09, R10, R11, R12, R13, R14



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LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 OTHERS 2x4 SP No.3

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

All bearings 22-0-0. REACTIONS.

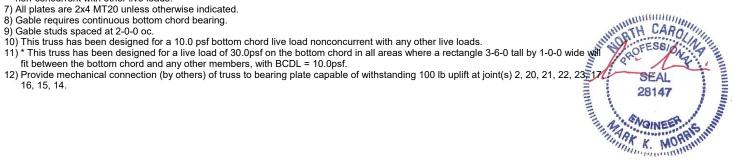
(lb) - Max Horz 2=-135(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 20, 21, 22, 23, 17, 16, 15, 14

Max Grav All reactions 250 lb or less at joint(s) 2, 19, 21, 22, 23, 16, 15, 14, 12 except 20=302(LC 5), 17=302(LC

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 6-2-6, Corner(3R) 6-2-6 to 15-9-10, Exterior(2N) 15-9-10 to 18-0-14, Corner(3E) 18-0-14 to 22-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.



Continuing by period assign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0096 BLAKE POND   127 WHIMBREL COURT LILLINGTON, NC
24-1484-R01	GR01	Common Supported Gable	1	1	Job Reference (optional) # 46668

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Mar 18 20:31:26 2024 Page 2 ID:gUCksxzC6J7HT2yGkHFINYyiOvf-iXdnMs1fhkSdxV53\_ensA8YCe86K?DL8LJi9n7zZc0V

- 13) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 15) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

  16) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS
- OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.



Joh Truss Truss Type LOT 0.0096 BLAKE POND | 127 WHIMBREL COURT LILLINGTON, NC 24-1484-R01 GR02 Common # 46668 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Mar 18 20:31:27 2024 Page 1 ID:gUCksxzC6J7HT2yGkHFINYyiOvf-AjB9ZC2HS2aUZegFXLI5iM4G?YJqkezHazRjJZzZc0U 11-0-0 16-4-11 22-0-0 22-10-8 0-10-8 5-4-11 Scale = 1:43.1 4x6 || 7.00 12 2x4 \\ 2x4 // 12 5 3 10 13 9 14 8 3x4 = 3x4 3x4 =3x6 =3x4 =22-0-0 7-4-14 7-4-14 7-2-4 Plate Offsets (X,Y)-- [2:0-0-0,0-0-8], [6:0-0-0,0-0-8] LOADING (psf) SPACING-DEFL. **PLATES** GRIP 2-0-0 CSI. in (loc) I/defl I/d TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.56 Vert(LL) -0.16 8-10 >999 240 MT20 244/190 Snow (Pf) 20.0 Lumber DOL 1.15 вс 0.60 Vert(CT) -0.22 8-10 >999 180 **TCDL** 10.0 WB 0.22 Rep Stress Incr YES Horz(CT) 0.03 6 n/a n/a **BCLL** 0.0 Code IRC2021/TPI2014 Weight: 106 lb Matrix-SH FT = 20% **BCDL** 10.0 LUMBER-BRACING-Structural wood sheathing directly applied or 4-5-5 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. TOP CHORD 2x4 SP No.2 TOP CHORD BOT CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No 3 WFBS MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide **REACTIONS.** (lb/size) 2=930/0-3-8 (min. 0-1-8), 6=930/0-3-8 (min. 0-1-8) Max Horz 2=-135(LC 12) Max Uplift2=-82(LC 14), 6=-82(LC 15) Max Grav 2=958(LC 21), 6=958(LC 22) FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1384/112, 3-11=-1244/121, 4-11=-1121/146, 4-12=-1121/146, 5-12=-1244/122,

5-6=-1384/112 **BOT CHORD** 

2-10=-115/1159, 10-13=-2/764, 9-13=-2/764, 9-14=-2/764, 8-14=-2/764, 6-8=-24/1130

WFBS 4-8=-78/547, 5-8=-357/158, 4-10=-78/547, 3-10=-357/158

NOTES-(9-12)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 6-2-6, Exterior(2R) 6-2-6 to 15-9-10, Interior(1) 15-9-10 to 18-0-14, Exterior(2E) 18-0-14 to 22-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- 9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

  This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

  This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

  Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.

  Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

  Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

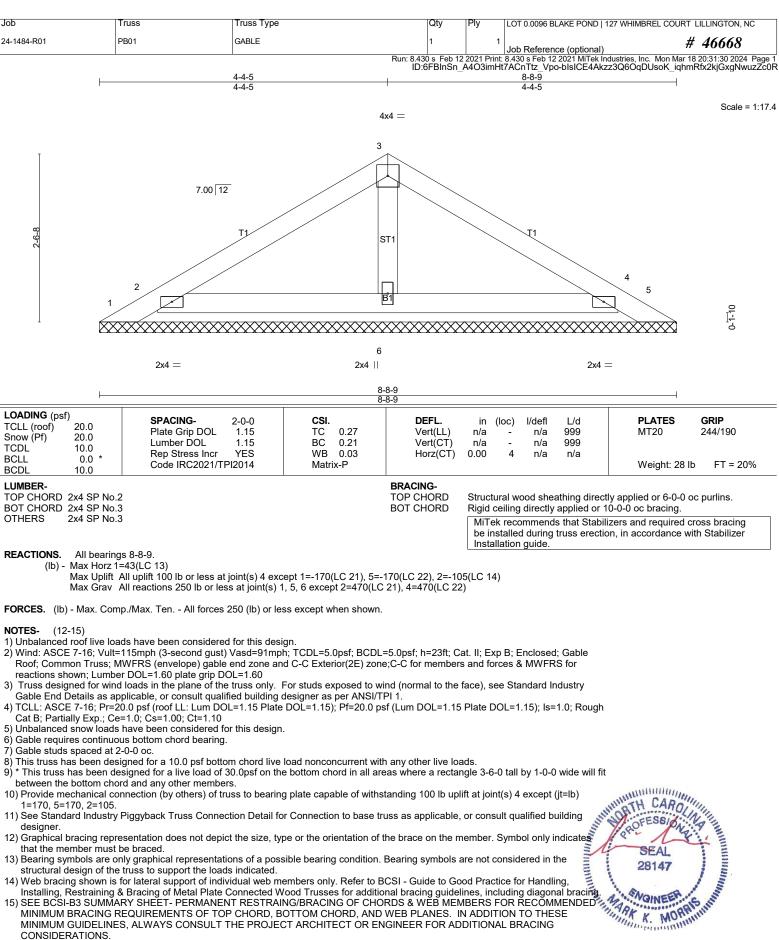
  Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing and MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING the member must be braced.

  10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

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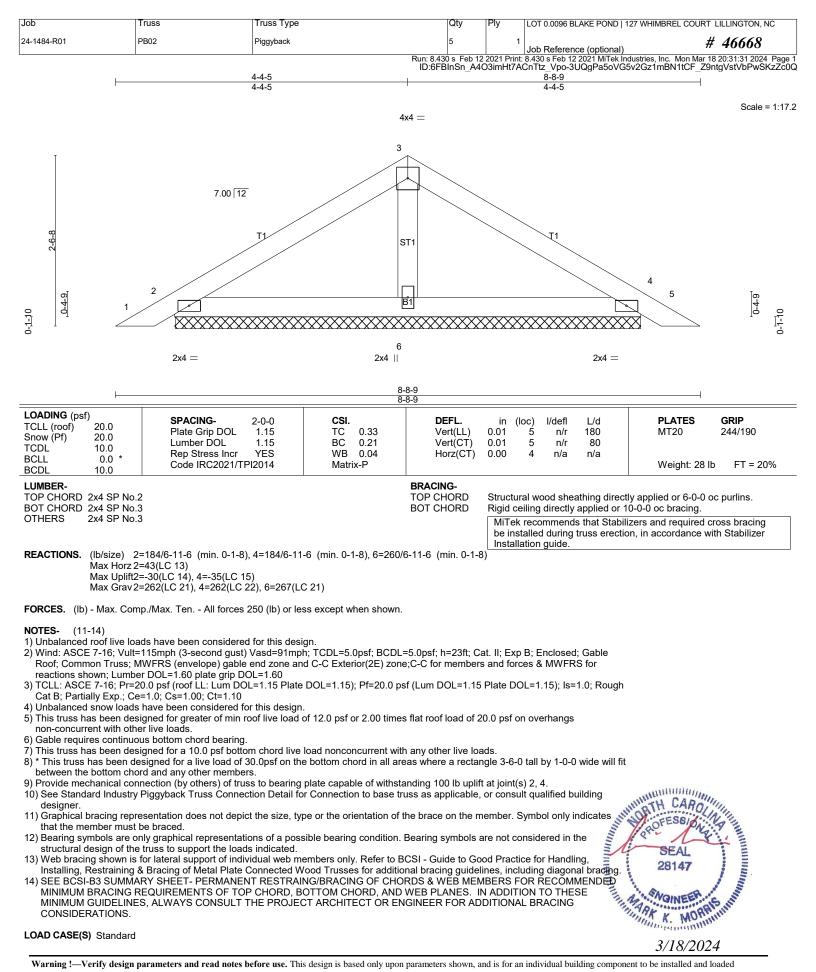
  \*\*ANNIALINA BRACING RECUIREMENTS OF TOP CHORD. BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

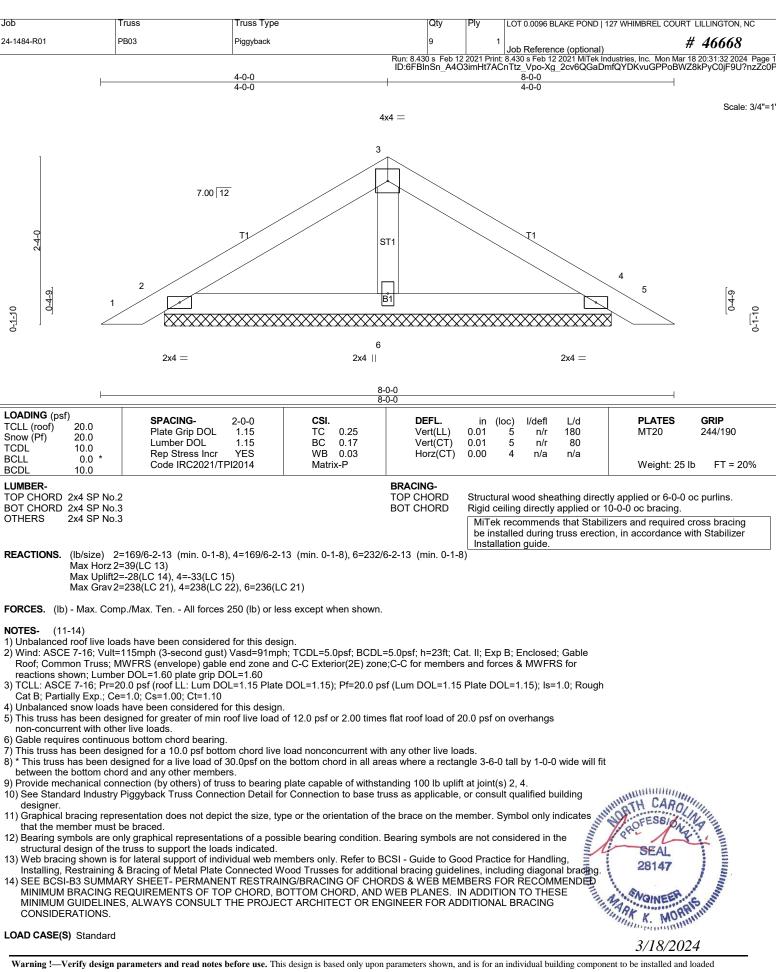
LOAD TO CASE SCIENCE TO PARAMETERS and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



CONSIDERATIONS.

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12) Bearing symbols are only graphics.

structural design of the truss to support the loads indicated.

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Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

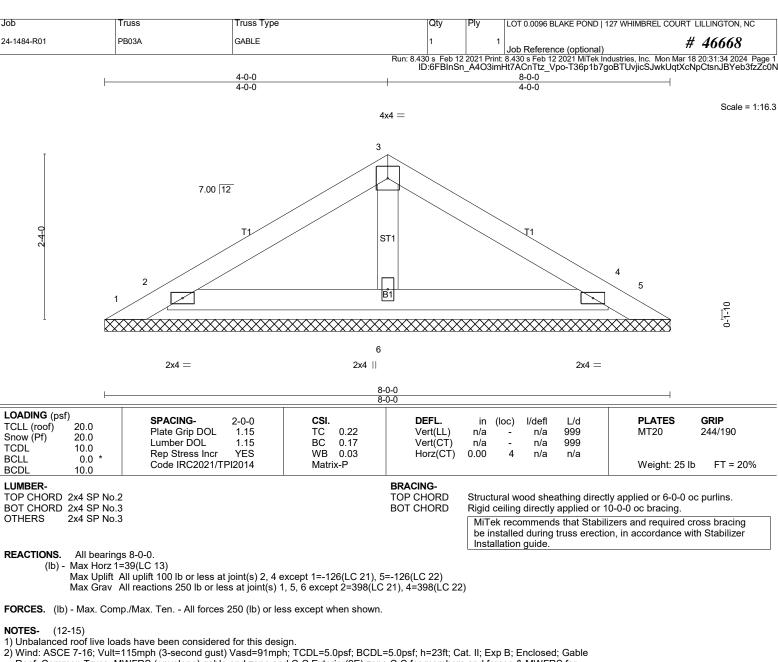
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MINIMI IM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE

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- Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

- between the bottom chord and any other members.

  10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4 except (jt=lb) 1=126, 5=126.

  11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

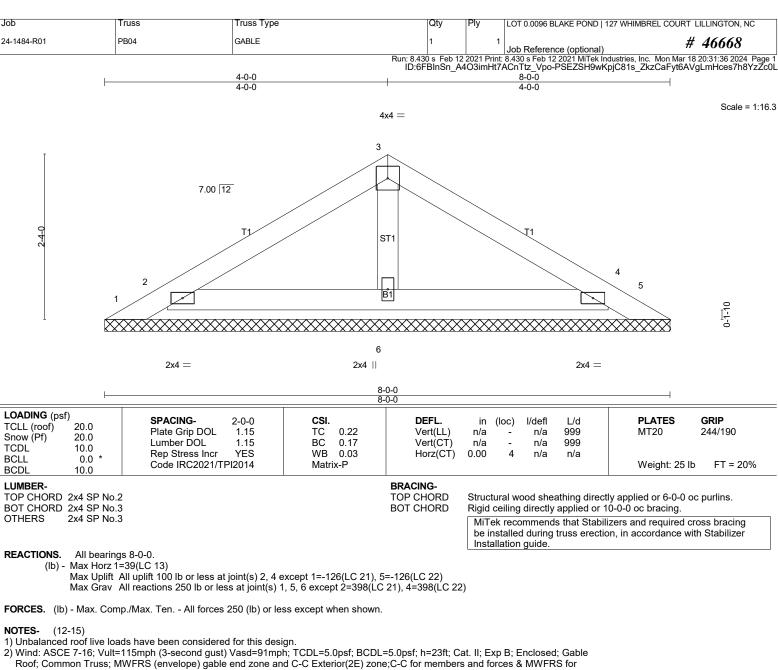
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  14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

  15) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS. CONSIDERATIONS.

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- reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.

CONSIDERATIONS.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

- between the bottom chord and any other members.

  10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4 except (jt=lb) 1=126, 5=126.

  11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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Joh Truss Truss Type LOT 0.0096 BLAKE POND | 127 WHIMBREL COURT LILLINGTON, NC 24-1484-R01 R01 GABLE # 46668 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Mar 18 20:31:37 2024 Page 1 ID:6FBInSn\_A4O3imHt7ACnTtz\_Vpo-uenxgdAZ56r3mBRA7SUR6TVtqai04DyltWsFg\_zZc0K -0-10-8 0-10-8 6-0-8 Scale = 1:16.5 2x4 || 3 2x4 || 4.00 12 W1 ST1 0-9-0 В1 3x6 || 2x4 || 2x4 || Plate Offsets (X,Y)-- [2:0-0-0,0-0-6], [2:0-0-13,0-9-1] LOADING (psf) DEFL. **PLATES** GRIP SPACING-2-0-0 CSI. (loc) I/defl I/d in TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.92 Vert(LL) -0.072-4 >977 240 MT20 244/190 Snow (Pf) 20.0 Lumber DOL 1.15 вс 0.73 Vert(CT) -0.14 2-4 >489 180 **TCDL** 10.0 WB 0.00 Rep Stress Incr YES Horz(CT) 0.00 n/a n/a **BCLL** 0.0 Code IRC2021/TPI2014 Weight: 26 lb Matrix-P FT = 20%**BCDL** 10.0 LUMBER-BRACING-Structural wood sheathing directly applied or 2-2-0 oc purlins, except TOP CHORD 2x4 SP No.2 TOP CHORD BOT CHORD 2x4 SP No.3 end verticals 2x4 SP No 3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WFBS 2x4 SP No 3 OTHERS MiTek recommends that Stabilizers and required cross bracing

WEDGE

Left: 2x4 SP No.3

REACTIONS. (lb/size) 4=225/Mechanical, 2=297/0-3-8 (min. 0-1-8)

Max Horz 2=64(LC 10)

Max Uplift4=-33(LC 14), 2=-42(LC 10) Max Grav 4=302(LC 21), 2=390(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-(11-14)

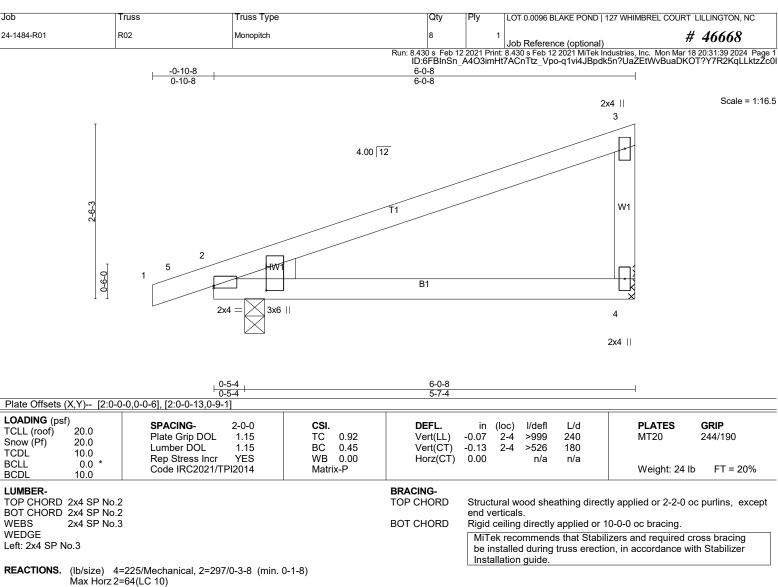
- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B: Partially Exp.: Ce=1.0: Cs=1.00: Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Gable studs spaced at 2-0-0 oc.

- 6) Gable studs spaced at 2-0-0 oc.
  7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
  9) Refer to girder(s) for truss to truss connections.
  10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
  11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
  12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
  13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
  14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS

be installed during truss erection, in accordance with Stabilizer

Installation guide.

LOAD DASE(S) r Stranging parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Max Uplift4=-33(LC 14), 2=-42(LC 10) Max Grav 4=302(LC 21), 2=390(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(9-12)

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections

7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
11) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
12) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.
LOAD CASE(S) Standard

Joh Truss Truss Type Qtv LOT 0.0096 BLAKE POND | 127 WHIMBREL COURT LILLINGTON, NC 24-1484-R01 R03 Piggyback Base Structural Gable COMMON I I Gable I Gable # 46668 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Mar 18 20:31:42 2024 Page 1 ID:6FBInSn\_A4O3imHt7ACnTtz\_Vpo-EcbqjKDhvfTMsyJ8w?4cpWCkObM8IEWU0oa0LBzZc0F 22-6-0 32-6-13 38-7-11 -0<sub>-</sub>10<sub>-</sub>8 18-6-0 26-6-0 45-0-0 45-10-8 0-10-8 0-10-8 6-4-5 6-0-13 4-0-0 4-0-0 6-0-13 6-0-13 6-4-5 Scale = 1:82.2 5x6 =5x6 = 4x4 = 6 838 37 7.00 12 3x6 =4x4 / 4x4 < 3x8 / 39 5x8 < 7x8 < 11-9-8 10 5x6 40 3 35 11 W2 W2 4x6 || 4x6 || 12 9 B1 B2 42 21 20 45 46 16 44 17 19 22 18 14 4x4 =4x4 =4x12 MT20HS= 3x8 MT20HS= 4x8 = 4x8 = 4x8 =3x8 MT20HS= 43-0-0 9-4-12 9-1-4 8-0-0 9-1-4 7-4-12 2-0-0 Plate Offsets (X,Y)-- [2:0-3-0,Edge], [6:0-4-0,0-2-4], [8:0-4-0,0-2-4], [9:0-3-0,0-1-4], [10:0-4-0,Edge], [11:0-4-0,0-2-0], [12:0-3-0,Edge] LOADING (psf) SPACING-**PLATES** GRIP 2-0-0 CSI DEFI in (loc) I/defl I/d TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.93 Vert(LL) -0.45 19-21 >999 240 MT20 244/190 Snow (Pf) 20.0 Lumber DOL 1.15 вс 0.93 Vert(CT) -0.69 16-18 >742 180 MT20HS 187/143 TCDL 10.0 WB Rep Stress Incr YES 1.00 Horz(CT) 0.16 14 n/a n/a **BCLL** 0.0 Code IRC2021/TPI2014 Matrix-SH Weight: 355 lb FT = 20%**BCDL** 10.0 LUMBER-BRACING-Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing, Except: TOP CHORD 2x4 SP No.2 TOP CHORD BOT CHORD 2x4 SP No.1 \*Except\* BOT CHORD B2: 2x4 SP SS 2-2-0 oc bracing: 16-18. WERS WFBS 5-19, 7-19, 7-18, 9-18, 3-22, 11-14 2x4 SP No 3 1 Row at midpt 2x4 SP No.3 **OTHERS** MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer REACTIONS. (lb/size) 22=1843/0-3-8 (min. 0-2-11), 14=1719/2-3-8 (min. 0-2-9), 15=137/0-3-8 (min. 0-1-8) Max Horz 22=-203(LC 12) Max Uplift22=-84(LC 14), 14=-131(LC 15) Max Grav 22=2256(LC 39), 14=2173(LC 49), 15=243(LC 7) FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-584/123, 3-35=-3332/115, 4-35=-3320/118, 4-36=-3223/130, 5-36=-3107/146,

5-6=-2687/168, 6-37=-2206/168, 7-37=-2206/168, 7-38=-2205/169, 8-38=-2205/169,

8-9=-2686/169, 9-39=-3082/161, 10-39=-3198/145, 10-40=-3295/133, 11-40=-3307/130,

11-12=-510/167, 2-22=-497/122, 12-14=-457/144

**BOT CHORD** 21-22=-172/2935, 21-41=-79/2721, 41-42=-79/2721, 20-42=-79/2721, 19-20=-79/2721 19-43=0/2213, 43-44=0/2213, 18-44=0/2213, 17-18=0/2638, 17-45=0/2638, 45-46=0/2638,

16-46=0/2638, 15-16=-49/2787, 14-15=-49/2787

**WEBS** 5-21=-20/429, 5-19=-832/171, 6-19=-19/973, 7-19=-286/166, 7-18=-293/158, 8-18=-19/972,

9-18=-815/181, 9-16=-36/402, 3-22=-2989/31, 11-14=-3012/2

#### NOTES-(14-17)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Hip Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 13-8-6, Exterior(2R) 13-8-6 to 31-3-10, Interior(1) 31-3-10 to 41-0-14, Exterior(2E) 41-0-14 to 45-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
7) Provide adequate drainage to prevent water ponding.
8) All plates are MT20 unless otherwise indicated.
9) All plates are 2x4 MT20 unless otherwise indicated.
10) Gable studs spaced at 2-0-0 oc. 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable

- 10) Gable studs spaced at 2-0-0 oc
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continuing by period assign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0096 BLAKE POND   127 WHIMBREL COURT LILLINGTON, NC
24-1484-R01	R03	Piggyback Base Structural Gable COMMON I I Gable I G	<b>á</b> ble	1	Joh Reference (ontional) # 46668

Run: 8430 s Feb 12 2021 Print: 8430 s Feb 12 2021 MiTek Industries, Inc. Mon Mar 18 20:31:43 2024 Page 2 ID:6FBInSn\_A4O3imHt7ACnTtz\_Vpo-io9CwgEJgybCU6uKTibrMklv7?iNUhmeFSKZuezZc0E

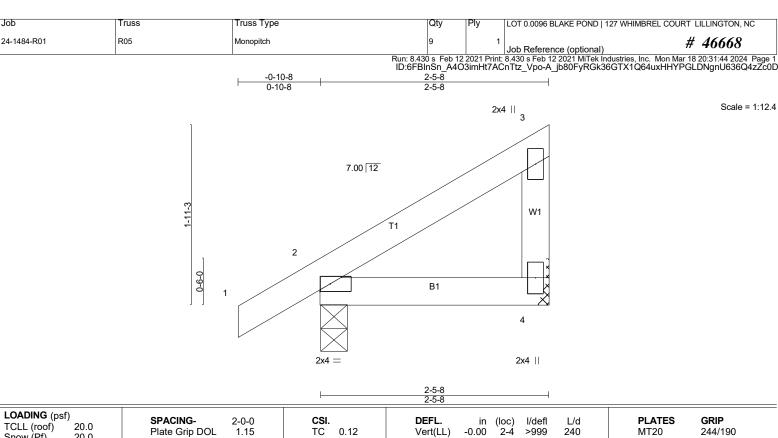
### **NOTES-** (14-17)

- 12) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 22 except (jt=lb) 14=131.
- 14) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

  15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 16) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

  17) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS
- OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.





Snow (Pf) 20.0 **TCDL** 10.0 0.0 **BCLL** BCDL 10.0

Lumber DOL 1.15 BC 0.05 Rep Stress Incr YES WB 0.00 Code IRC2021/TPI2014 Matrix-P

Vert(CT) -0.002-4 >999 180 Horz(CT) 0.00 n/a n/a

Weight: 12 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WFBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 2-5-8 oc purlins, except end verticals

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

> MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

**REACTIONS.** (lb/size) 4=72/Mechanical, 2=162/0-3-8 (min. 0-1-8)

Max Horz 2=51(LC 14)

Max Uplift4=-20(LC 14), 2=-12(LC 14) Max Grav 4=97(LC 21), 2=230(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed: Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

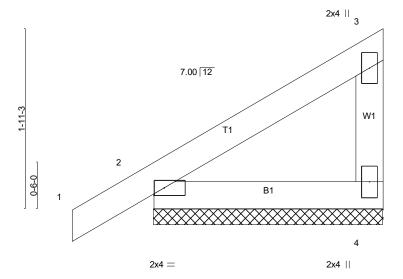
9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

11) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing

12) SEE BČŠI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR ŘECŎMMENDĒĎ MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS

.lob Truss Truss Type LOT 0.0096 BLAKE POND | 127 WHIMBREL COURT LILLINGTON, NC 24-1484-R01 R06 Monopitch Supported Gable # 46668 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Mar 18 20:31:45 2024 Page 1 ID:6FBInSn\_A4O3imHt7ACnTtz\_Vpo-fBGzLMGaCaswjP2jb7dJR9qRbpbpyqwxjmpgyWzZc0C -0-10-8 0-10-8 2-5-8



LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.10 BC 0.10 WB 0.00	DEFL.         in (loc)         l/defl         L/d           Vert(LL)         -0.00         1         n/r         180           Vert(CT)         0.00         1         n/r         80           Horz(CT)         0.00         n/a         n/a	<b>PLATES GRIP</b> MT20 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2021/TPI2014	Matrix-P	(* )	Weight: 12 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 **WEBS** 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 2-5-8 oc purlins, except end verticals

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

> MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

Scale = 1:12.4

**REACTIONS.** (lb/size) 4=83/2-5-8 (min. 0-1-8), 2=155/2-5-8 (min. 0-1-8)

Max Horz 2=51(LC 14)

Max Uplift4=-21(LC 14), 2=-10(LC 14) Max Grav 4=111(LC 21), 2=219(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

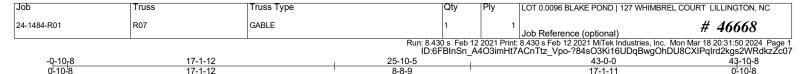
- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed: Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.

LOAD CASE(S) Standard

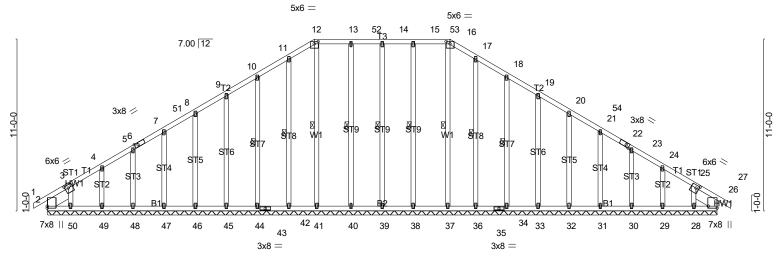
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  9) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
  10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
  11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
  12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
  13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
  14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

Warning !--Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Scale = 1:73.9



	11-1-12	20-10			70-0-		
'	17-1-12	' 8-8-	-9		17-1-1	1	'
Plate Offsets (X,Y) [3:0-2	2-8,0-2-4], [6:0-2-10,0-1-8], [12:0-3-0,0-	1-12], [16:0-3-0,0-1-12],	[22:0-2-10,0-1-8	8], [25:0-2-8,0-	-2-4], [26:Edge,0-6	6-12]	
LOADING (psf)   TCLL (roof)   20.0   Snow (Pf)   20.0   TCDL   10.0   BCLL   0.0 * BCDL   10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.10 BC 0.11 WB 0.22 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.00 26 -0.00 26 0.01 26	I/defl L/d n/r 180 n/r 80 n/a n/a	PLATES MT20 Weight: 356 lb	<b>GRIP</b> 244/190 FT = 20%

BRACING-

WFBS

TOP CHORD

**BOT CHORD** 

1 Row at midpt

25-10-5

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WFBS 2x4 SP No 3

**OTHERS** 2x4 SP No 3

SLIDER Left 2x8 SP No.2 -° 1-9-1, Right 2x8 SP No.2 -° 1-9-1

All bearings 43-0-0. (lb) - Max Horz 2=-191(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 39, 40, 42, 44, 45, 46, 47, 48, 49, 38, 36, 34, 33, 32, 31, 30,

29, 28, 26, 41 except 50=-108(LC 14)

17\_1\_12

Max Grav All reactions 250 lb or less at joint(s) 2, 48, 49, 50, 30, 29, 28, 26 except 39=279(LC 44), 40=339(LC

44), 42=298(LC 47), 44=323(LC 47), 45=317(LC 47), 46=315(LC 47), 47=327(LC 47), 38=339(LC 44), 36=296(LC

49), 34=324(LC 49), 33=316(LC 49), 32=315(LC 49), 31=327(LC 49), 41=273(LC 52), 37=264(LC 52)

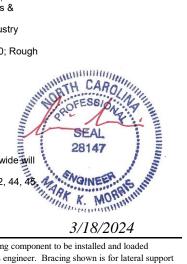
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(14-17)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph, TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 12-4-2, Corner(3R) 12-4-2 to 30-7-14, Exterior(2N) 30-7-14 to 39-0-14, Corner(3E) 39-0-14 to 43-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding. 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) This truss has been designed for a live load of 30.0psf on the bottom cnord in all areas much sit between the bottom chord and any other members, with BCDL = 10.0psf.

  13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 39, 40, 42, 44, 45, 47, 49, 40, 38, 36, 34, 33, 32, 31, 30, 29, 28, 26, 41 except (jt=lb) 50=108.



43-0-0

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

18-34, 12-41, 16-37

14-39, 13-40, 11-42, 10-44, 15-38, 17-36,

Continuing by period assign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0096 BLAKE POND   127 WHIMBREL COURT LILLINGTON, NC
24-1484-R01	R07	GABLE	1	1	Job Reference (optional) # 46668

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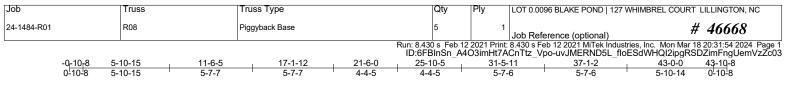
- 14) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

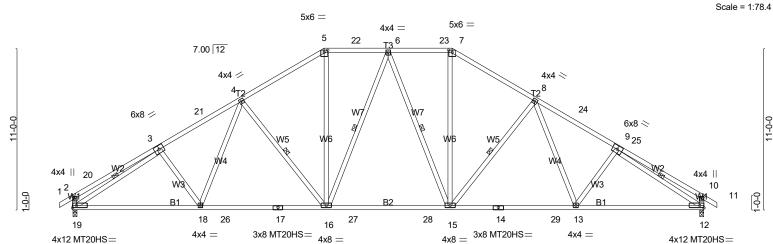
16) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

17) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.







<u> </u>	8-8-10 8-8-10	17-1-12 8-5-2			25-10-5 8-8-9	34- 8-5			43-0-0 8-8-10	<b>—</b>
Plate Offsets (X,Y) [2:0-2-	0,0-1-12], [5:0-3-0,0	-1-12], [7:0-3-0,0	)-1-12], [10	:0-2-0,0-1-12	2]					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- Plate Grip DOI Lumber DOL Rep Stress Inc Code IRC2021	1.15 r YES	CSI TC BC WB Mat	0.78 0.91	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.32 16-18 -0.49 15-16 0.15 12	I/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 MT20HS Weight: 289 lb	<b>GRIP</b> 244/190 187/143 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.1 2x4 SP No 3 WFBS

BRACING-

TOP CHORD

Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals

Rigid ceiling directly applied or 10-0-0 oc bracing.

Installation guide

BOT CHORD WFBS

1 Row at midpt 4-16, 6-16, 6-15, 8-15, 3-19, 9-12

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

REACTIONS. (lb/size) 19=1770/0-3-8 (min. 0-2-9), 12=1770/0-3-8 (min. 0-2-9)

Max Horz 19=-189(LC 12)

Max Uplift19=-76(LC 14), 12=-76(LC 15)

Max Grav 19=2172(LC 39), 12=2172(LC 39)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-20=-533/108, 3-20=-364/110, 3-21=-3135/105, 4-21=-2920/131, 4-5=-2557/153, TOP CHORD

5-22=-2103/153, 6-22=-2103/153, 6-23=-2103/153, 7-23=-2103/153, 7-8=-2557/153,

8-24=-2920/131, 9-24=-3135/105, 9-25=-364/110, 10-25=-533/107, 2-19=-464/112,

10-12=-464/111

**BOT CHORD** 18-19=-154/2744, 18-26=-68/2556, 17-26=-68/2556, 16-17=-68/2556, 16-27=0/2119,

27-28=0/2119, 15-28=0/2119, 14-15=0/2501, 14-29=0/2501, 13-29=0/2501, 12-13=-26/2640 4-18=-21/373, 4-16=-766/160, 5-16=-14/935, 6-16=-296/145, 6-15=-296/145, 7-15=-14/935,

8-15=-766/160, 8-13=-21/373, 3-19=-2827/30, 9-12=-2827/30

### NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Hip Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 12-4-2, Exterior(2R) Roof; Hip Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 12-4-2, Exterior(2R) 12-4-2 to 30-7-14, Interior(1) 30-7-14 to 39-0-14, Exterior(2E) 39-0-14 to 43-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) Provide adequate drainage to prevent water ponding.
7) All plates are MT20 plates unless otherwise indicated.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will the between the bottom chord and any other members, with BCDL = 10.0psf.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 19, 12.

Continuing by period assign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0096 BLAKE POND   127 WHIMBREL COURT LILLINGTON, NC
24-1484-R01	R08	Piggyback Base	5	1	Job Reference (optional) # 46668

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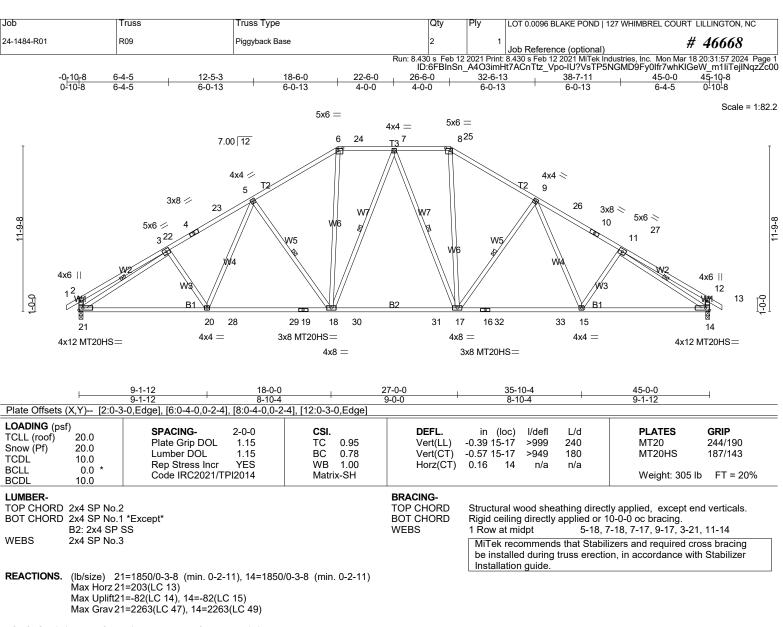
- 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-587/124, 3-22=-3355/116, 4-22=-3343/119, 4-23=-3246/132, 5-23=-3130/147,

5-6=-2770/165, 6-24=-2221/164, 7-24=-2221/165, 7-25=-2221/165, 8-25=-2221/164

8-9=-2770/165, 9-26=-3130/148, 10-26=-3246/132, 10-27=-3343/120, 11-27=-3355/117,

11-12=-587/124, 2-21=-498/122, 12-14=-498/122

**BOT CHORD** 20-21=-168/2953, 20-28=-75/2743, 28-29=-75/2743, 19-29=-75/2743, 18-19=-75/2743,

18-30=0/2230, 30-31=0/2230, 17-31=0/2230, 16-17=0/2672, 16-32=0/2672, 32-33=0/2672,

15-33=0/2672, 14-15=-31/2828

WEBS 5-20=-30/400, 5-18=-811/176, 6-18=-17/1015, 7-18=-295/165, 7-17=-295/165,

8-17=-17/1015, 9-17=-811/176, 9-15=-30/400, 3-21=-2998/26, 11-14=-2998/26

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Hip Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 13-8-6, Exterior(2R) Roof; Hip Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 13-8-6, Exterior(2R) 13-8-6 to 31-3-10, Interior(1) 31-3-10 to 41-0-14, Exterior(2E) 41-0-14 to 45-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) Provide adequate drainage to prevent water ponding.
7) All plates are MT20 plates unless otherwise indicated.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will the between the bottom chord and any other members, with BCDL = 10.0psf.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 21, 14.

Continuing by period assign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0096 BLAKE POND   127 WHIMBREL COURT LILLINGTON, NC
24-1484-R01	R09	Piggyback Base	2	1	Job Reference (optional) # 46668

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- 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

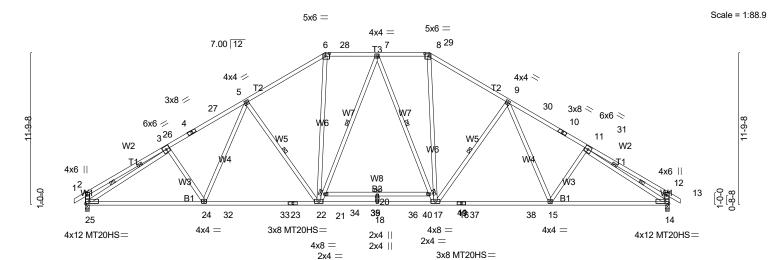
14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.





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	<b>—</b>	9-1-12	8-10-4		4-6-0	+ <del>27-0-0</del> + 4-6-0	8-10-4	•	+	9-1-12	
Plate Offsets (X,Y)	[2:0-3-	-0,Edge], [6:0-4-0,0-2	-4], [8:0-4-0,0-2	2-4], [12:0-3	3-0,Edge]						
LOADING (psf)       TCLL (roof)     20.0       Snow (Pf)     20.0       TCDL     10.0       BCLL     0.0       BCDL     10.0	*	SPACING- Plate Grip DOL Lumber DOL Rep Stress Inci Code IRC2021	1.15 YES	CSI TC BC WB Mat	0.80 0.78	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.49 20 -0.78 20 0.18 14	l/defl >999 >692 n/a	L/d 240 180 n/a	PLATES MT20 MT20HS Weight: 318 lb	<b>GRIP</b> 244/190 187/143  FT = 20%

LUMBER-TOP CHORD 2x4 SP No.2 \*Except\*

T2: 2x4 SP No.1

6-4-5

BOT CHORD 2x4 SP No.1 \*Except\*

B3: 2x4 SP No.2, B2: 2x4 SP SS

WFBS 2x4 SP No.3 BRACING-

WFBS

TOP CHORD **BOT CHORD** 

Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals

MiTek recommends that Stabilizers and required cross bracing

Rigid ceiling directly applied or 10-0-0 oc bracing. Except:

6-0-0 oc bracing: 19-21

1 Row at midpt 5-22, 7-21, 7-19, 9-17

2 Rows at 1/3 pts 3-25, 11-14

be installed during truss erection, in accordance with Stabilizer Installation guide REACTIONS. (lb/size) 25=1932/0-3-8 (min. 0-2-14), 14=1932/0-3-8 (min. 0-2-14)

Max Horz 25=-203(LC 12)

Max Uplift25=-41(LC 14), 14=-41(LC 15) Max Grav 25=2449(LC 47), 14=2449(LC 49)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-609/118, 3-26=-3669/46, 4-26=-3657/49, 4-27=-3561/61, 5-27=-3444/77

5-6=-3123/87, 6-28=-2518/99, 7-28=-2517/99, 7-29=-2517/99, 8-29=-2518/99, 8-9=-3123/87, 9-30=-3444/77, 10-30=-3561/61, 10-31=-3657/49, 11-31=-3669/46,

11-12=-609/117, 2-25=-510/119, 12-14=-510/118

**BOT CHORD** 24-25=-110/3209, 24-32=-11/3032, 32-33=-11/3032, 23-33=-11/3032, 22-23=-11/3032,

22-34=0/2563, 34-35=0/2563, 18-35=0/2563, 18-36=0/2563, 36-41=0/2563, 17-41=0/2563,

16-17=0/2961, 16-37=0/2961, 37-38=0/2961, 15-38=0/2961, 14-15=0/3084 5-24=-39/354, 5-22=-800/182, 6-22=0/1192, 21-22=-367/124, 7-21=-293/165,

7-19=-293/165, 17-19=-366/122, 8-17=0/1192, 9-17=-800/182, 9-15=-39/354, 3-25=-3275/0,

11-14=-3275/0

**WEBS** 

1) Unbalanced roof live loads have been considered for this design.

vilid: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Hip Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 13-8-6, Exterior(2R) 13-8-6 to 31-3-10, Interior(1) 31-3-10 to 41-0-14, Exterior(2E) 41-0-14 to 45-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.45 DOL=1.60) 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph, TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

7) All plates are MT20 plates unless otherwise indicated.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members, with BCDL = 10.00st.

Continuing by period and any other members, with BCDL = 10.00st.

Continuing by period and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0096 BLAKE POND   127 WHIMBREL COURT LILLINGTON, NC
24-1484-R01	R10	Piggyback Base	7	1	Job Reference (optional) # 46668

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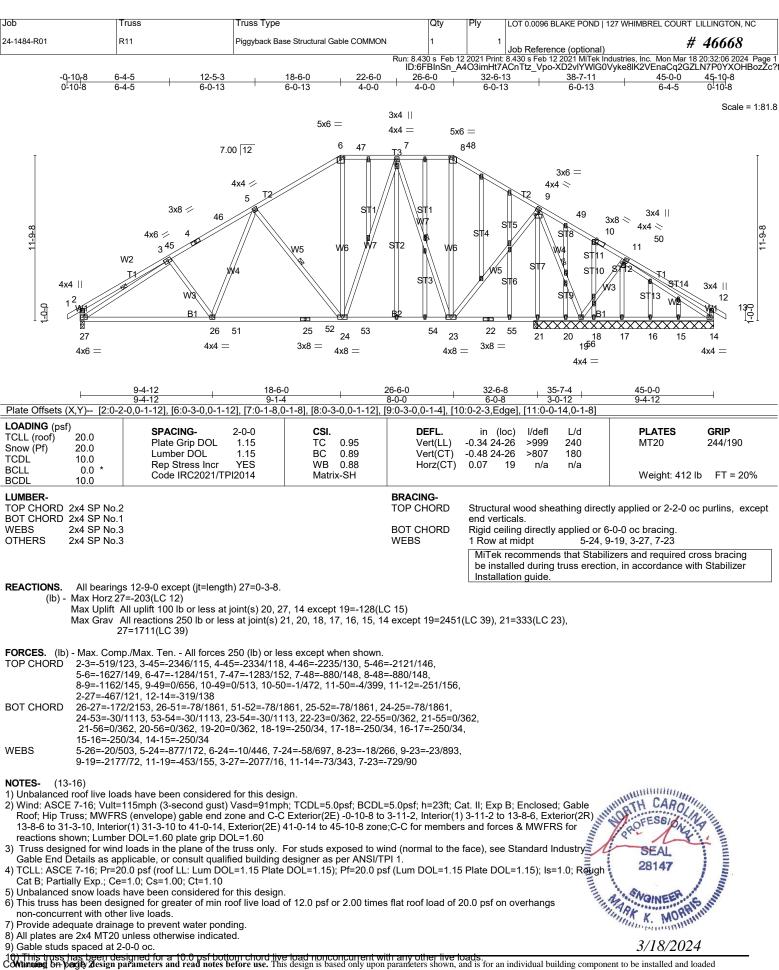
#### NOTES-(11-14)

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 14.

- 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

  14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.





15-16=-250/34, 14-15=-250/34

**WEBS** 5-26=-20/503, 5-24=-877/172, 6-24=-10/446, 7-24=-58/697, 8-23=-18/266, 9-23=-23/893,

9-19=-2177/72, 11-19=-453/155, 3-27=-2077/16, 11-14=-73/343, 7-23=-729/90

NOTES-

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph, TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof, Hip Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 13-8-6, Exterior(2R) 13-8-6 to 31-3-10, Interior(1) 31-3-10 to 41-0-14, Exterior(2E) 41-0-14 to 45-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.

9) Gable studs spaced at 2-0-0 oc

10) This dust has local designed for a 10.0 bit bottom choice live load nonconcurrent with any other live loads.

Continuing 6-peoply 2 design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0096 BLAKE POND   127 WHIMBREL COURT LILLINGTON, NC
24-1484-R01	R11	Piggyback Base Structural Gable COMMON	1	1	Job Reference (optional) # 46668

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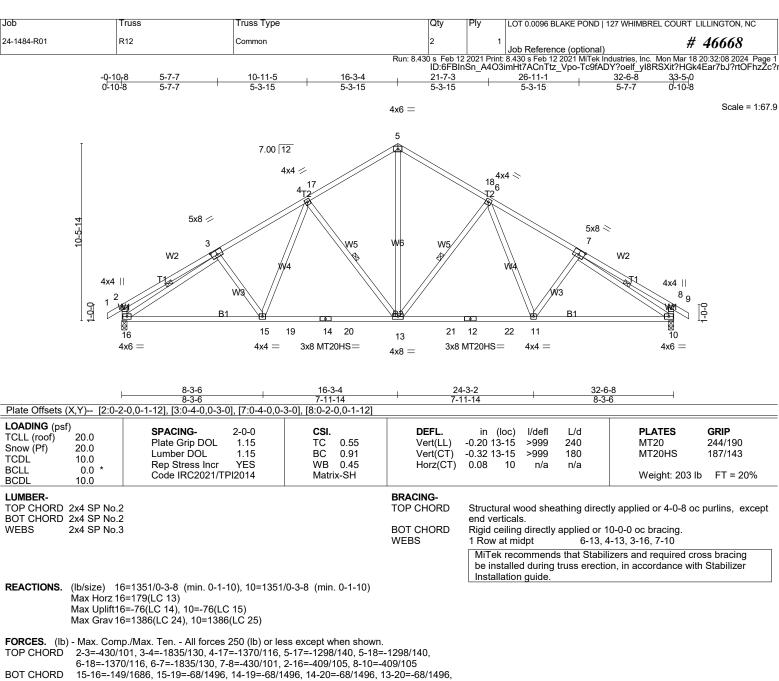
### **NOTES-** (13-16)

- 11) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20, 27, 14 except (jt=lb) 19=128.
- 13) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

  14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 15) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate
- Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

  16) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.





13-21=0/1411, 12-21=0/1411, 12-22=0/1411, 11-22=0/1411, 10-11=-28/1552

WEBS 5-13=-63/1050, 6-13=-556/151, 6-11=-20/368, 4-13=-556/151, 4-15=-20/368,

3-16=-1591/21, 7-10=-1590/20

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 11-5-10, Exterior(2R) 11-5-10 to 21-0-14, Interior(1) 21-0-14 to 28-7-6, Exterior(2E) 28-7-6 to 33-5-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

6) All plates are MT20 plates unless otherwise indicated.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide 嫡 lfit between the bottom chord and any other members, with BCDL = 10.0psf.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10.

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SEAL 28147

SEAL 28147

Continuing by period assign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0096 BLAKE POND   127 WHIMBREL COURT LILLINGTON, NC
24-1484-R01	R12	Common	2	1	Job Reference (optional) # 46668

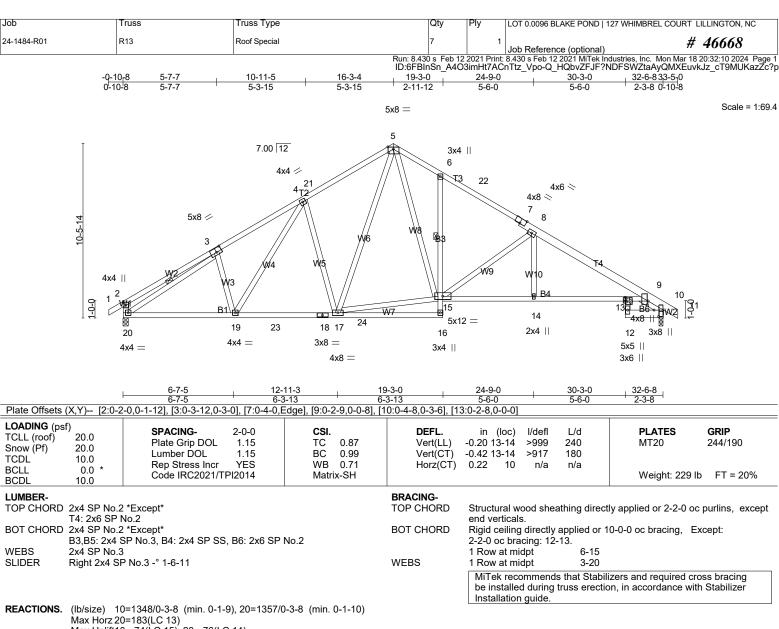
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Mar 18 20:32:09 2024 Page 2 ID:6FBInSn\_A4O3imHt7ACnTtz\_Vpo-yoj1NZYdZxtWb5tK?A3xPDqRUUapaarTEVdxn7zZc?q

- 10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 11) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

12) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 13) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

13) SEE BCSI-B3 SUMMARY SHEET- PERMANEŇŤ RESTRAING/BRĂCINĞ OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENT OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.





Max Uplift10=-74(LC 15), 20=-76(LC 14)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-417/115, 3-4=-1786/164, 4-21=-1500/148, 5-21=-1410/172, 5-6=-1591/179,

6-22=-1562/116, 7-22=-1659/91, 7-8=-1694/92, 8-9=-2315/102, 9-10=-1301/110,

2-20=-404/112

**BOT CHORD** 19-20=-138/1600, 19-23=-64/1393, 23-24=-64/1393, 18-24=-64/1393, 17-18=-64/1393,

6-15=-296/110, 14-15=0/1970, 13-14=0/1970, 9-13=0/1573, 10-12=-52/398 4-19=-72/338, 4-17=-521/178, 5-17=-114/543, 15-17=0/1024, 5-15=-118/1015,

8-15=-775/134, 8-14=0/367, 3-20=-1519/0

#### NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 11-5-10, Exterior(2R) 11-5-10 to 21-0-14, Interior(1) 21-0-14 to 28-7-6, Exterior(2E) 28-7-6 to 33-5-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10, 20. Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 11-5-10,

Continuing by period assign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0096 BLAKE POND   127 WHIMBREL COURT LILLINGTON, NC
24-1484-R01	R13	Roof Special	7	1	Job Reference (optional) # 46668

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- 9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 11) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
   12) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS
- 12) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.



Job Truss Type LOT 0.0096 BLAKE POND | 127 WHIMBREL COURT LILLINGTON, NC Truss 24-1484-R01 R14 Common Supported Gable # 46668 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Mar 18 20:32:13 2024 Page 1 ID:6FBInSn\_A4O3imHt7ACnTtz\_Vpo-qZzYDxb8cANy4jB5E07tZ3\_En58KWSZ297b9wuzZc?m 32-6-8 16-3-4 16-3-4

4x6 =

12 7.00 12 13 11 14 42 10 41 15 16 8 3x8 / 17 3x8 < 18 19 STI5 5x5 < 20 HW 21 23 9 3x8 || 3x8 II 34 29 40 39 38 37 36 35 33 32 31 30 28 27 26 25 24 3x8 =3x8 =

32-6-8 [2:0 5 45 0 0 6] [22:0 5 45 0 0 6] [20:0 2 4 0 4 9] [25:0

Plate Offsets (A, f) [2.0-	<u>5-15,0-0-6], [22.0-5-15,0-0-6], [29.0-2-</u>	4,0-1-0], [35.0-2-4,0-1-6]		
LOADING (psf)   TCLL (roof)   20.0   Snow (Pf)   20.0   TCDL   10.0   BCLL   0.0 *   BCDL   10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.09 BC 0.10 WB 0.21 Matrix-SH	DEFL.         in (loc)         l/defl         L/d           Vert(LL)         -0.00         22         n/r         180           Vert(CT)         -0.00         22         n/r         80           Horz(CT)         0.01         22         n/a         n/a	PLATES GRIP MT20 244/190 Weight: 244 lb FT = 20%

BRACING-

**WEBS** 

TOP CHORD

**BOT CHORD** 

1 Row at midpt

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 2x4 SP No 3 OTHERS SLIDER

Left 2x8 SP No.2 -° 1-9-8, Right 2x8 SP No.2 -° 1-9-8

REACTIONS. All bearings 32-6-8

(lb) - Max Horz 2=-182(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 33, 34, 36, 37, 38, 39, 40, 31, 30, 28, 27, 26, 25, 24, 22 Max Grav All reactions 250 lb or less at joint(s) 2, 36, 38, 39, 40, 28, 26, 25, 24, 22 except 32=268(LC 27), 33=296(LC 5), 34=262(LC 5), 37=256(LC 24), 31=296(LC 6), 30=262(LC 6), 27=256(LC 25)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(13-16)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 11-5-10, Corner(3R) 11-5-10 to 21-0-14, Exterior(2N) 21-0-14 to 28-7-6, Corner(3E) 28-7-6 to 33-5-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide 🔊 🛚 fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 33, 34, 36, 37 39, 40, 31, 30, 28, 27, 26, 25, 24, 22.

SEAL 28147

MONES SEAL 28147

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

12-32, 11-33, 13-31

Scale = 1:64.9

Continuing by period assign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0096 BLAKE POND   127 WHIMBREL COURT LILLINGTON, NC
24-1484-R01	R14	Common Supported Gable	1	1	Job Reference (optional) # 46668

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- 13) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

15) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

16) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

