# Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 45873 JOB: 24-1218-R01 JOB NAME: LOT 0.0043 HONEYCUTT HILLS Wind Code: 37 Wind Speed: Vult= 115mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. *57 Truss Design(s)* 

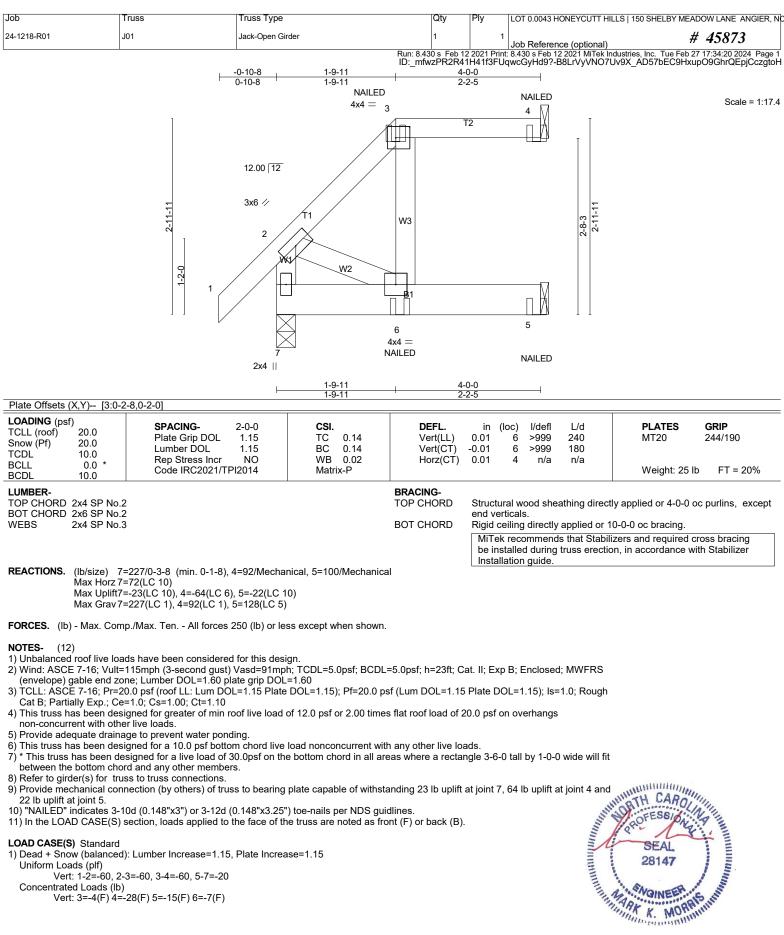
Trusses:

J01, J02, J03, J04, J05, J06, J07, J08, J09, J10, J11, J12, J13, J14, J15, PB01, PB02, PB03, PB04, R01, R02, R03, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, R14, R15, R16, R17, R18, R19, R20, R21, R22, R23, R24, R25, R26, R27, R28, R29, R30, R30A, R31, R32, R33, R34, R35, VT01, VT02



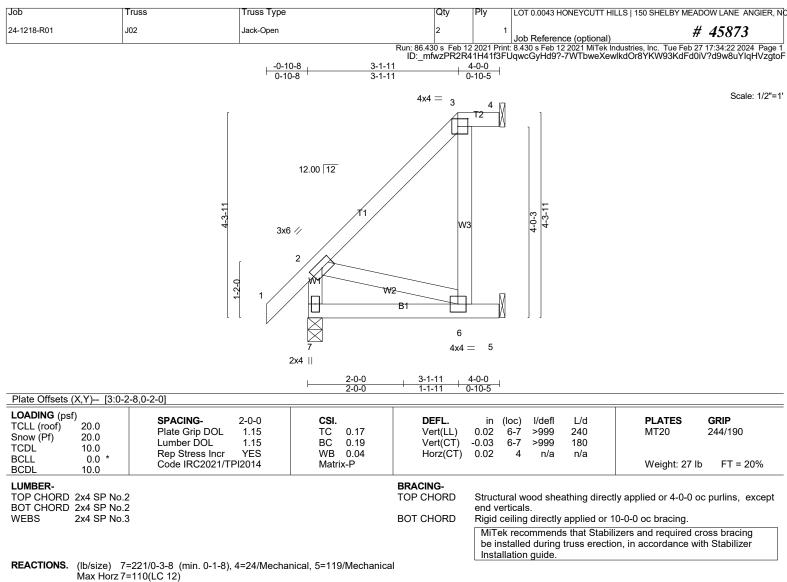
# Warning !--- Verify design parameters and read notes before use.

This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling*,



Vert: 3=-4(F) 4=-28(F) 5=-15(F) 6=-7(F)

<sup>2/26/2024</sup> 



Max Uplift4=-7(LC 8), 5=-61(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left exposed ; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads. 5) Provide adequate drainage to prevent water ponding.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

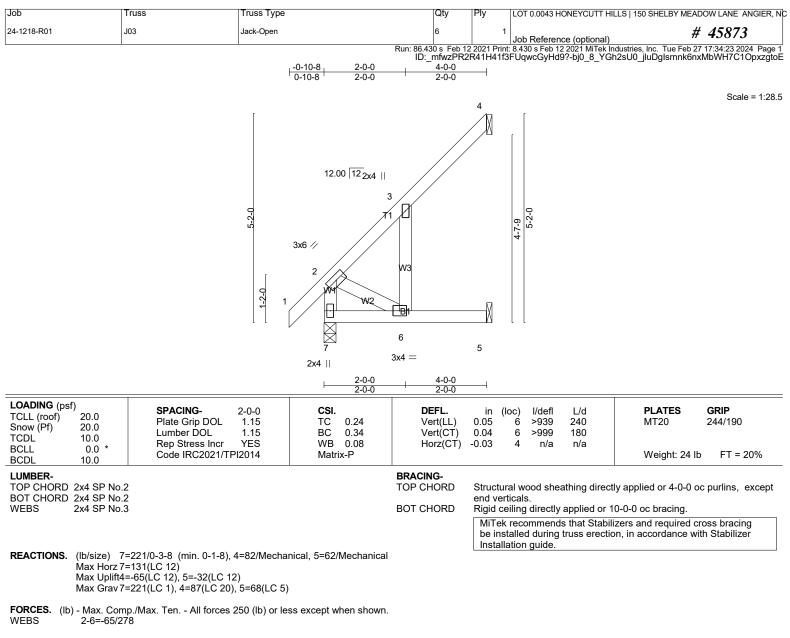
7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 4 and 61 lb uplift at joint 5.

# LOAD CASE(S) Standard





NOTES- (8)

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left exposed ; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

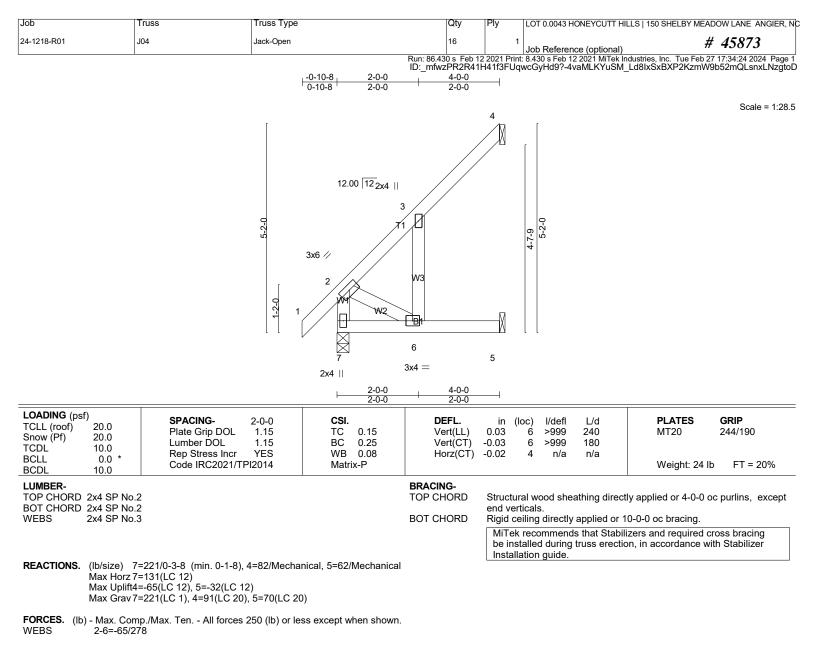
5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 65 lb uplift at joint 4 and 32 lb uplift at joint 5.

LOAD CASE(S) Standard





**NOTES-** (8)

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

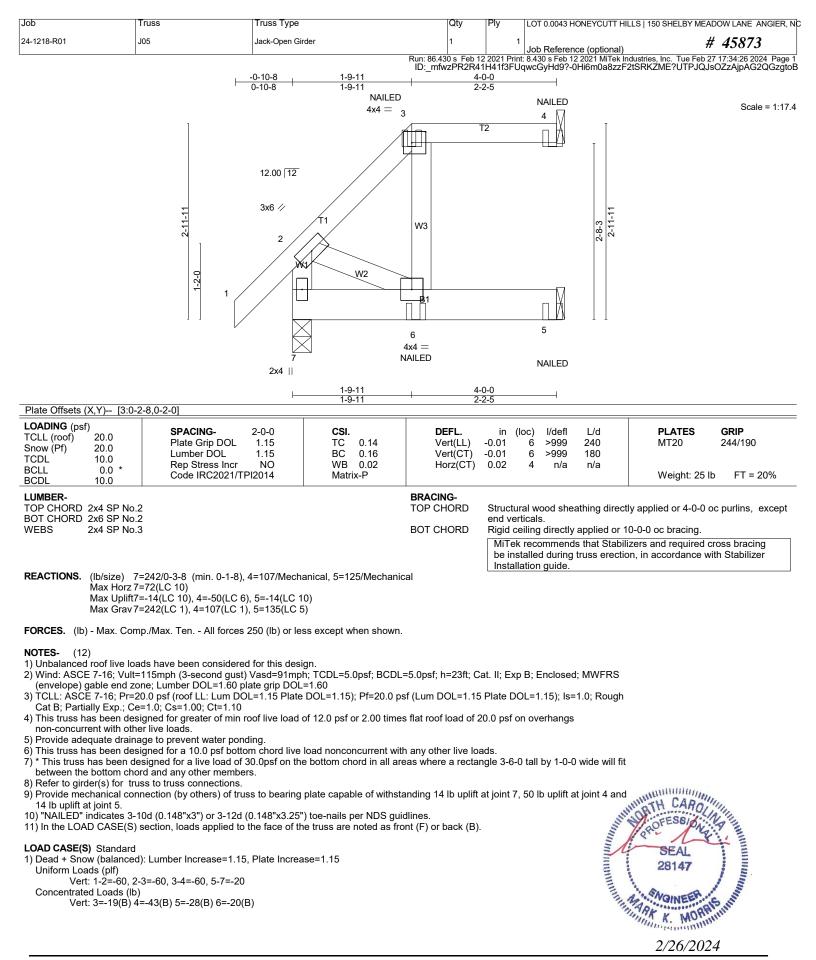
5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

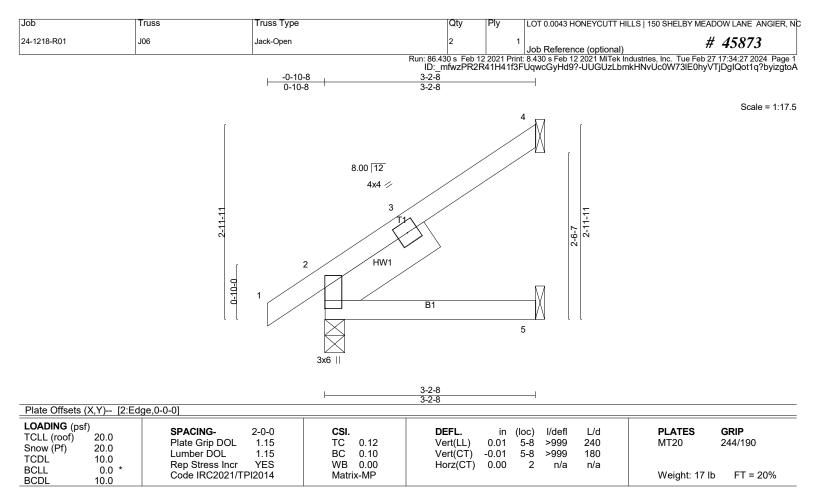
6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 65 lb uplift at joint 4 and 32 lb uplift at joint 5.

LOAD CASE(S) Standard







BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

SLIDER Left 2x6 SP No.2 - 1-11-0

REACTIONS. (lb/size) 4=79/Mechanical, 2=186/0-3-8 (min. 0-1-8), 5=40/Mechanical Max Horz 2=73(LC 12) Max Uplift4=-41(LC 12) Max Grav 4=82(LC 20), 2=186(LC 1), 5=55(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

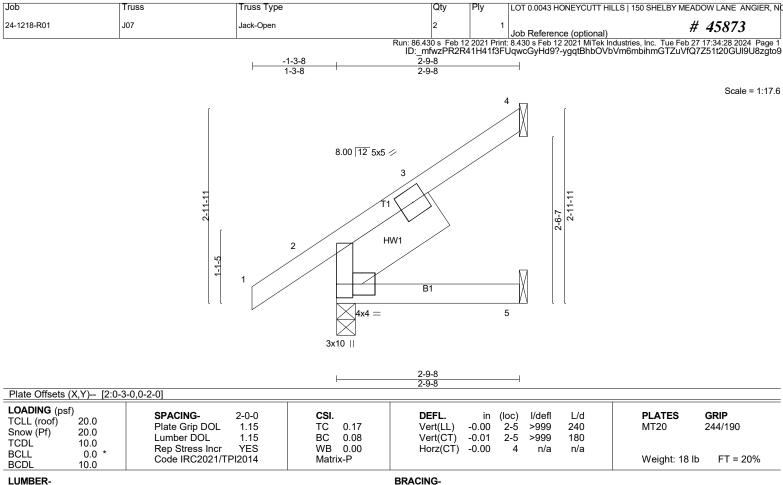
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 41 lb uplift at joint 4.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 3-2-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

Left 2x8 SP No.2 -Ø 1-11-4 SLIDER

REACTIONS. (Ib/size) 4=64/Mechanical, 2=205/0-3-8 (min. 0-1-8), 5=27/Mechanical Max Horz 2=75(LC 12) Max Uplift4=-55(LC 12) Max Grav 4=71(LC 20), 2=214(LC 18), 5=55(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

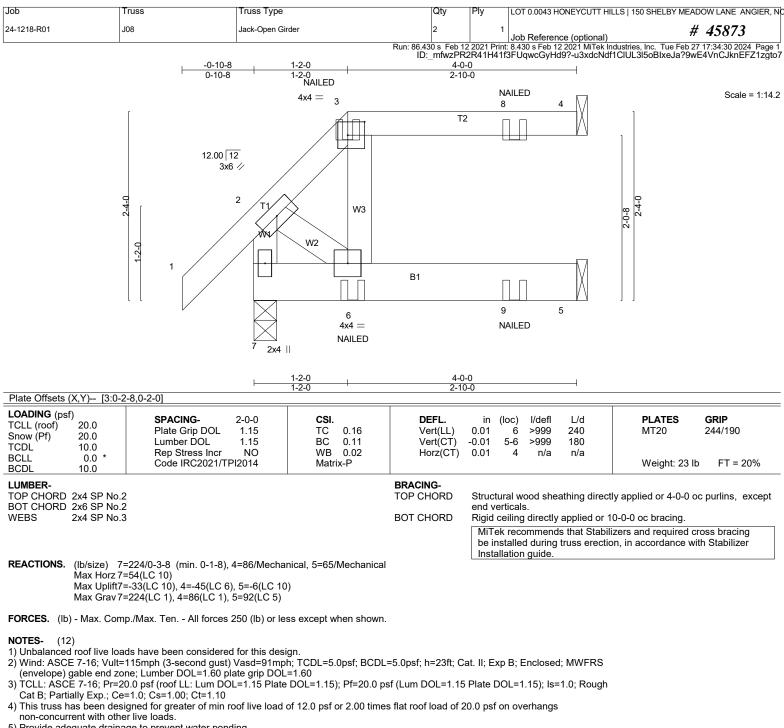
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 55 lb uplift at joint 4.

#### LOAD CASE(S) Standard



Structural wood sheathing directly applied or 2-9-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



5) Provide adequate drainage to prevent water ponding.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 7, 45 lb uplift at joint 4 and 6 lb uplift at joint 5.

10) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

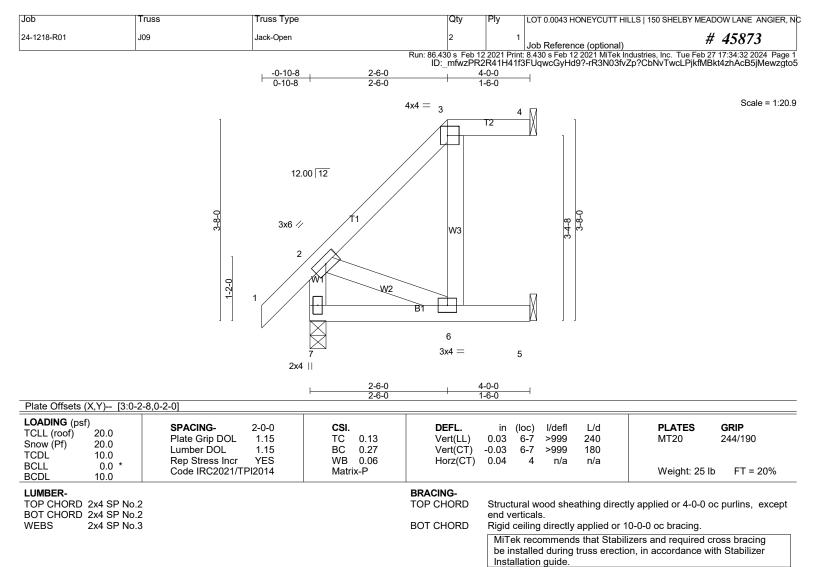
11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 3-4=-60, 5-7=-20 Concentrated Loads (lb)

Vert: 6=-2(B) 8=-4(B) 9=-5(B)





REACTIONS. (lb/size) 7=221/0-3-8 (min. 0-1-8), 4=43/Mechanical, 5=100/Mechanical Max Horz 7=92(LC 12) Max Uplift4=-13(LC 8), 5=-36(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

**NOTES-** (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads.

5) Provide adequate drainage to prevent water ponding.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

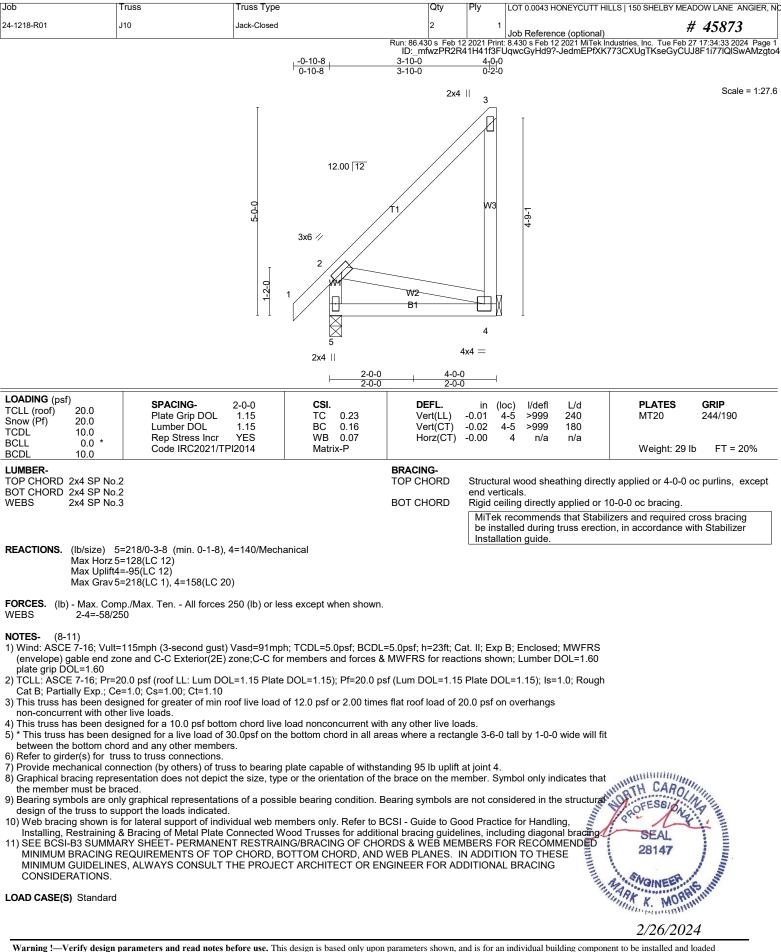
between the bottom chord and any other members.

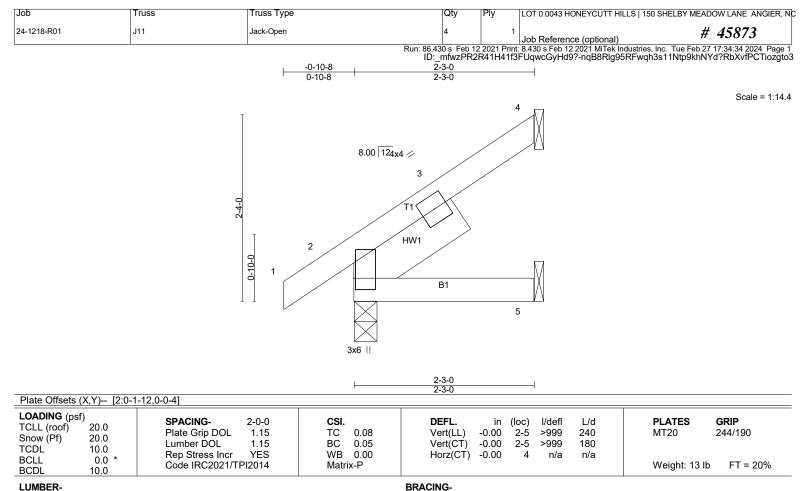
8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 4 and 36 lb uplift at joint 5.

#### LOAD CASE(S) Standard







TOP CHORD

BOT CHORD

### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

Left 2x6 SP No.2 -Ø 1-6-11 SLIDER

REACTIONS. (Ib/size) 4=55/Mechanical, 2=151/0-3-8 (min. 0-1-8), 5=22/Mechanical Max Horz 2=57(LC 12) Max Uplift4=-42(LC 12) Max Grav 4=60(LC 20), 2=151(LC 1), 5=44(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

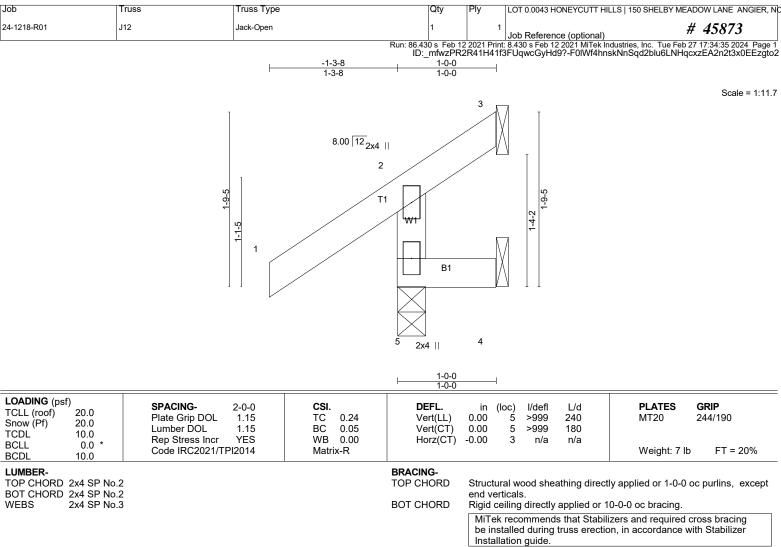
- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 4.

#### LOAD CASE(S) Standard



Structural wood sheathing directly applied or 2-3-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



REACTIONS. (lb/size) 5=194/0-3-8 (min. 0-1-8), 3=-32/Mechanical, 4=-8/Mechanical Max Horz 5=39(LC 12) Max Uplift5=-8(LC 12), 3=-86(LC 18), 4=-20(LC 18) Max Grav 5=283(LC 18), 3=3(LC 8), 4=11(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

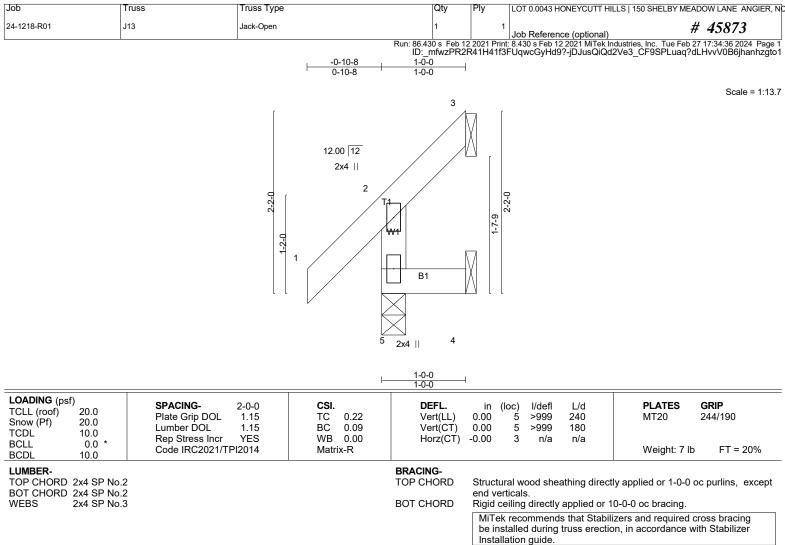
5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 5, 86 lb uplift at joint 3 and 20 lb uplift at joint 4.

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LOAD CASE(S) Standard
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REACTIONS. (lb/size) 5=132/0-3-8 (min. 0-1-8), 3=-4/Mechanical, 4=0/Mechanical Max Horz 5=48(LC 12) Max Uplift3=-38(LC 18), 4=-21(LC 12) Max Grav 5=181(LC 18), 3=9(LC 10), 4=14(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

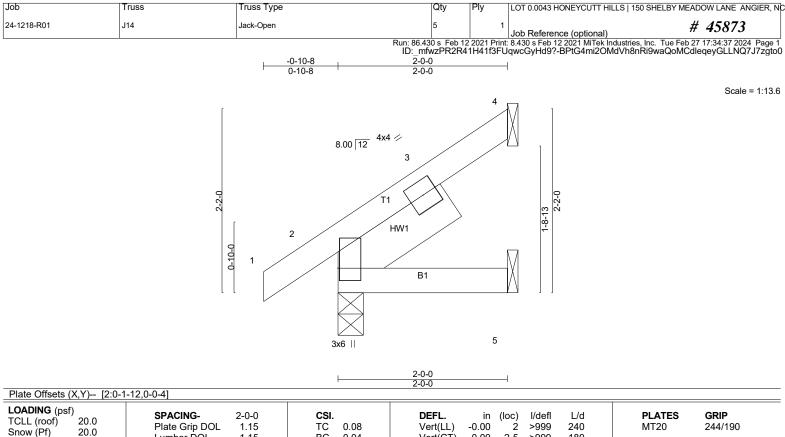
5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 3 and 21 lb uplift at joint 4.

LOAD CASE(S) Standard





TCLL (roof)         20.0           Snow (Pf)         20.0           TCDL         10.0           BCLL         0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.08 BC 0.04 WB 0.00 Matrix-P	DEFL. Vert(LL) -0.0 Vert(CT) -0.0 Horz(CT) -0.0	00 2-5	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 12 lb	<b>GRIP</b> 244/190 FT = 20%
BCDL 10.0			BRACING-	ructural w	rood sheathing direct	v applied or 2-0-0 or	nurlins

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

SLIDER Left 2x6 SP No.2 -Ø 1-6-10

REACTIONS. (lb/size) 4=48/Mechanical, 2=144/0-3-8 (min. 0-1-8), 5=20/Mechanical Max Horz 2=53(LC 12) Max Uplift4=-39(LC 12) Max Grav 4=53(LC 20), 2=147(LC 18), 5=40(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES- (8)

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

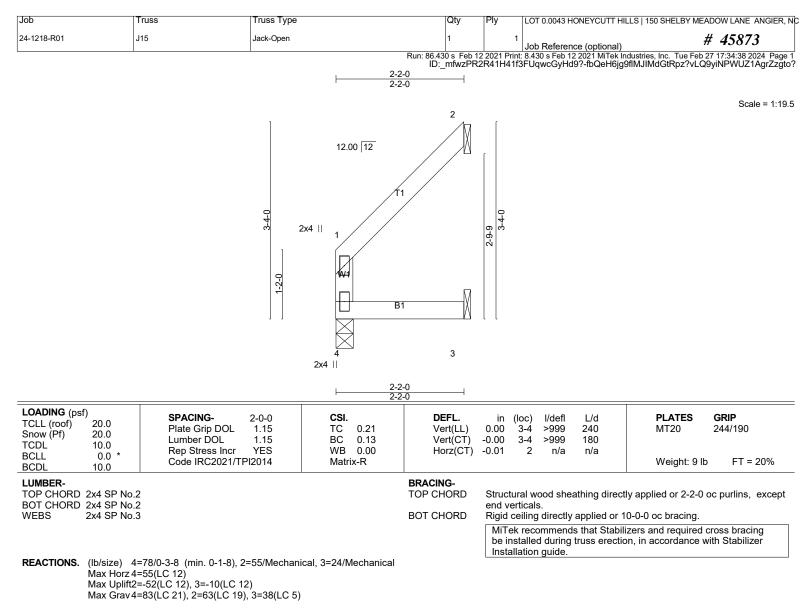
6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 4.

# LOAD CASE(S) Standard



Rigid ceiling directly applied or 10-0-0 oc bracing.



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

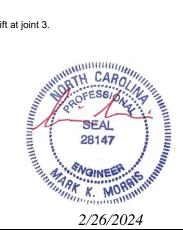
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

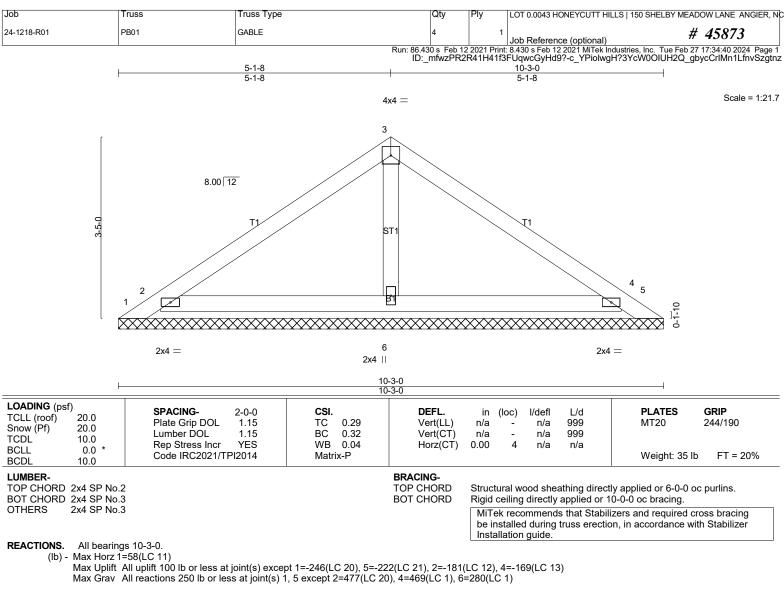
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 2 and 10 lb uplift at joint 3.

LOAD CASE(S) Standard





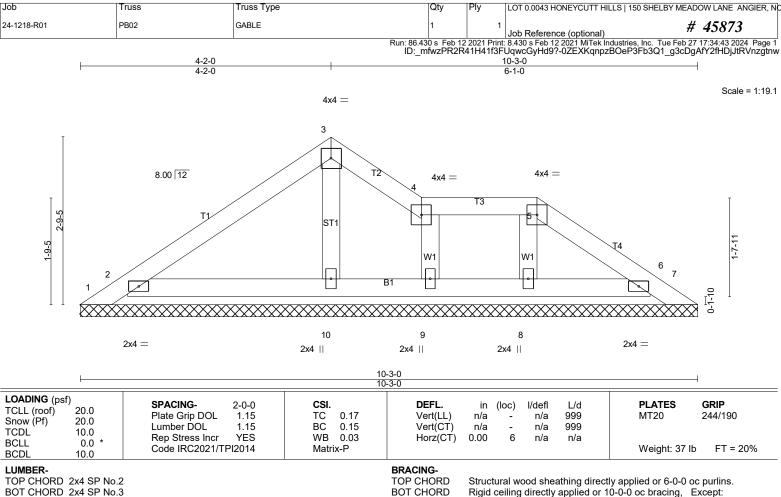
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (11)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 4-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 246 lb uplift at joint 1, 222 lb uplift at joint 5, 181 lb uplift at joint 2 and 169 lb uplift at joint 4.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard





BOT CHORD 2x4 SP No.3 WFBS 2x4 SP No.3 2x4 SP No.3 OTHERS

Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 8-9.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS. All bearings 10-3-0.

(lb) - Max Horz 1=47(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 7, 6, 9, 8 except 1=-141(LC 20), 2=-117(LC 12) Max Grav All reactions 250 lb or less at joint(s) 1, 7, 6, 10, 9, 8 except 2=330(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

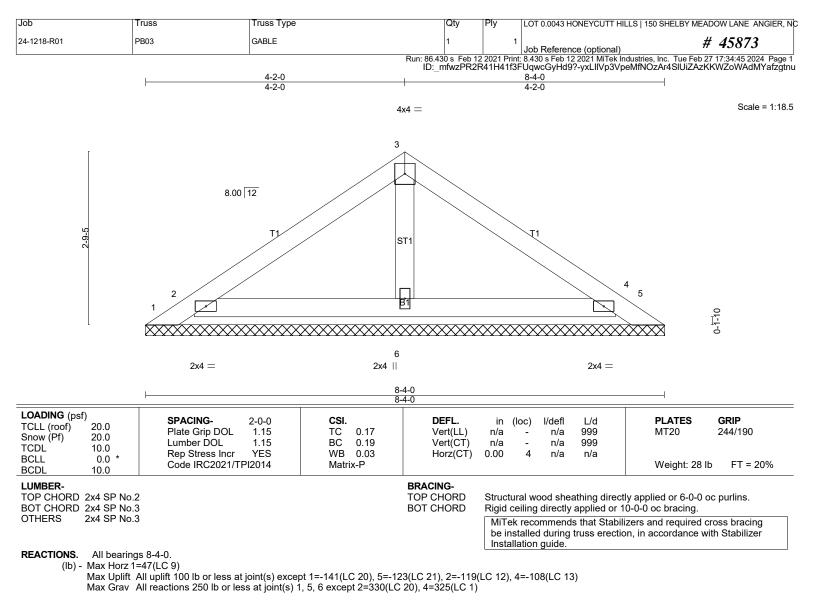
NOTES-(12)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) 0-3-2 to 5-8-0, Corner(3R) 5-8-0 to 7-7-0, Corner(3E) 7-7-0 to 9-11-14 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Provide adequate drainage to prevent water ponding.
- 6) Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc 7)
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 9)
- between the bottom chord and any other members. 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 6, 9, 8 except
- (jt=lb) 1=141, 2=117. 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard



2/26/2024



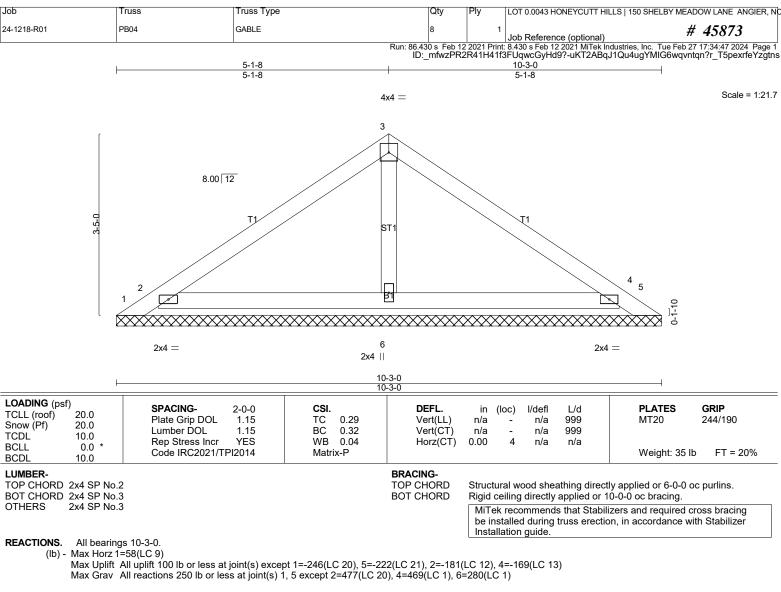
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

**NOTES-** (11)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 4-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 1, 123 lb uplift at joint 5, 119 lb uplift at joint 2 and 108 lb uplift at joint 4.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard





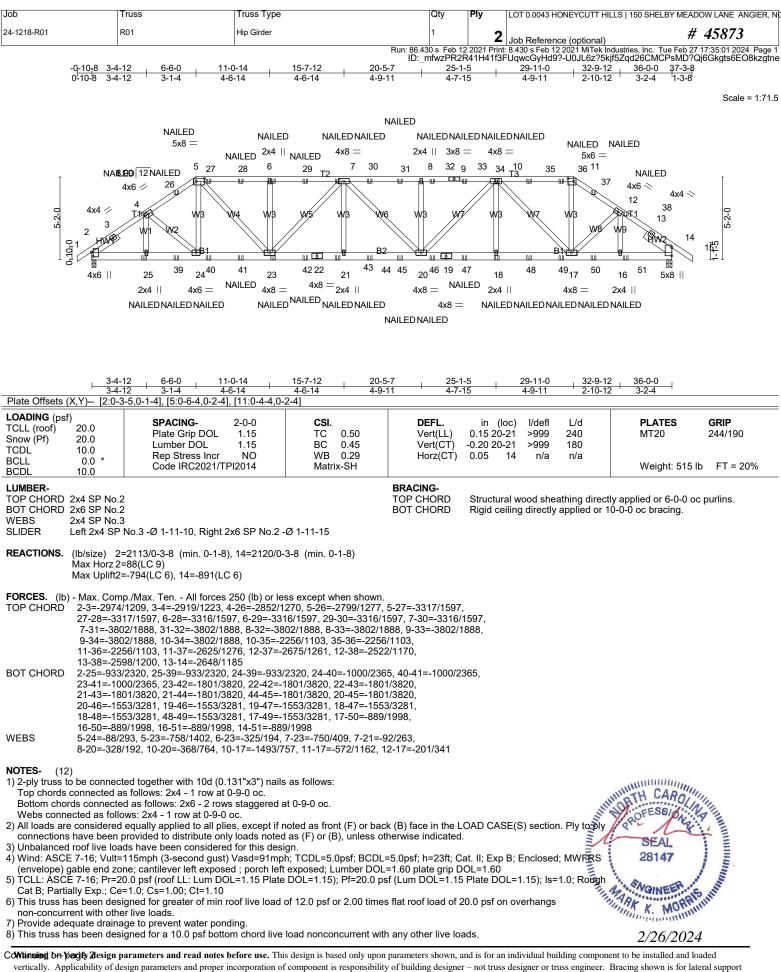
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

**NOTES-** (11)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 246 lb uplift at joint 1, 222 lb uplift at joint 5, 181 lb uplift at joint 2 and 169 lb uplift at joint 4.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard





of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusse Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0043 HONEYCUTT HILLS   150 SHELBY MEADOW LANE ANGIER, NC
24-1218-R01	R01	Hip Girder	1	2	Job Reference (optional) # 45873
Run: 86.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Feb 27 17:35:02 2024 Page 2 ID:_mfwzPR2R41H41f3FUqwcGyHd9?-yDtjJJ0jV1nyB_CEgvtRx3vOlq3L?Bw05m_ygAzgtn					

# NOTES- (12)

9) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 794 lb uplift at joint 2 and 891 lb uplift at joint 14.

11) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

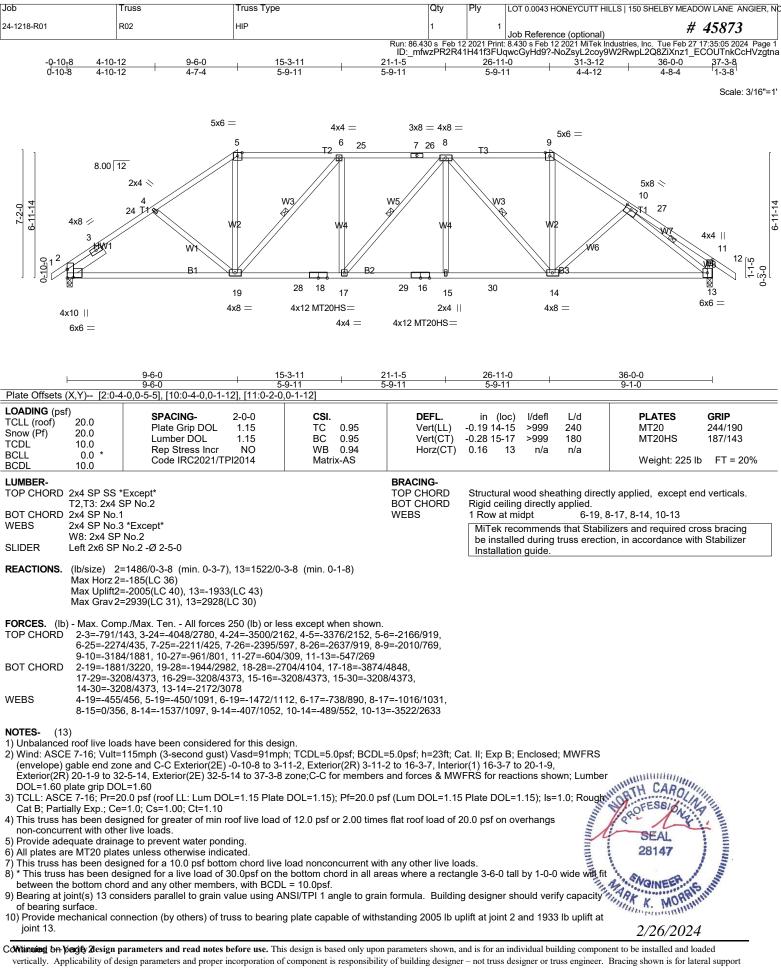
Uniform Loads (plf)

Vert: 1-ؾ=-60, 5-11=-60, 11-15=-60, 2-14=-20

Concentrated Loads (lb)

Vert: 4=-47(F) 25=-105(F) 23=-42(F) 6=-22(F) 18=-42(F) 10=-22(F) 27=-22(F) 28=-22(F) 29=-22(F) 30=-22(F) 31=-22(F) 32=-22(F) 33=-22(F) 33=-22(F) 35=-22(F) 3





of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0043 HONEYCUTT HILLS   150 SHELBY MEADOW LANE ANGIER, NC
24-1218-R01	R02	HIP	1	1	Job Reference (optional) # 45873
Run: 86.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Feb 27 17:35:06 2024 P ID:_mfwzPR2R41H41f3FUqwcGyHd9?-r_7E9h3EZGHNgbV0vlyN6v3yjRKTxrkc?Oy9px:					

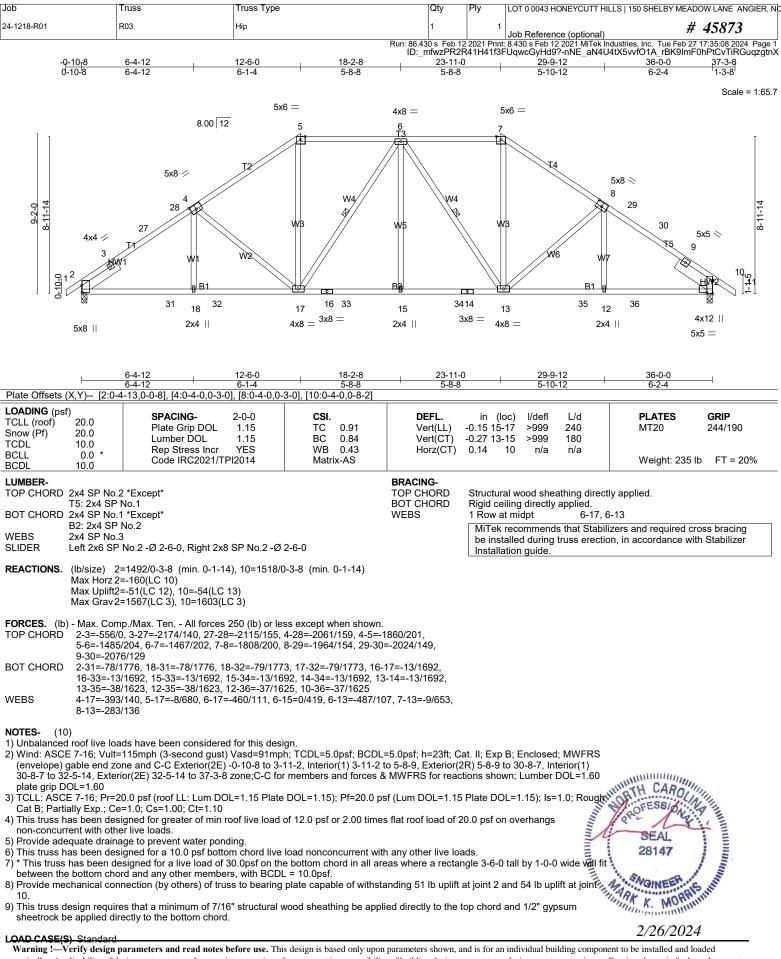
NOTES- (13)

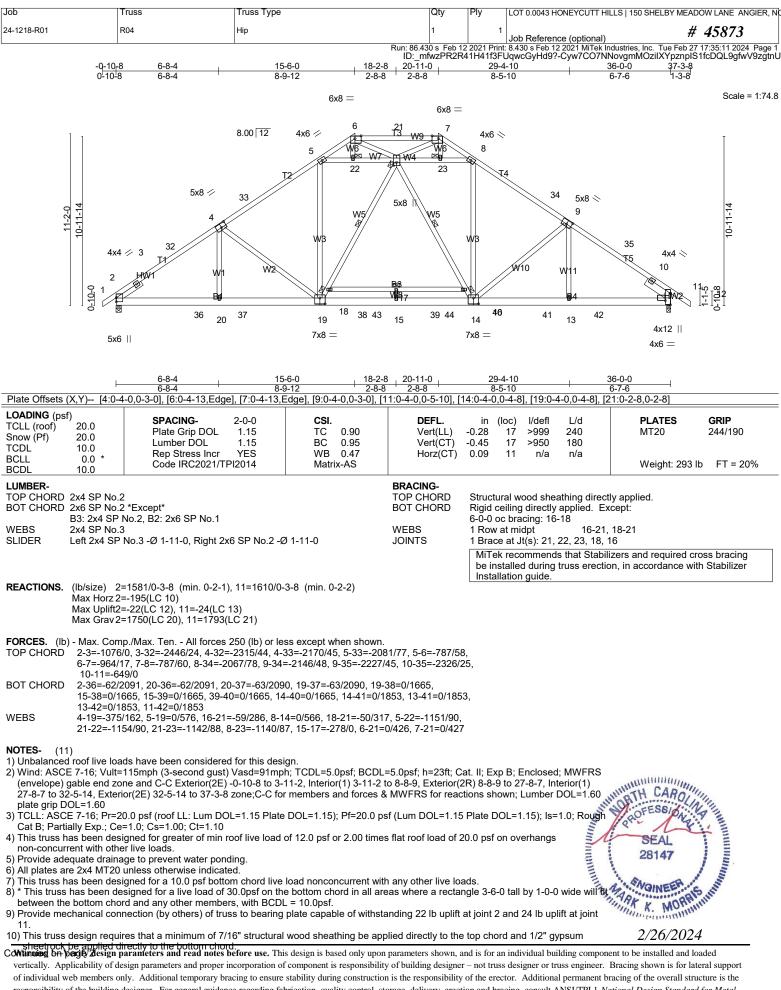
11) This truss has been designed for a total drag load of 200 plf. Lumber DOL=(1.33) Plate grip DOL=(1.33) Connect truss to resist drag loads along bottom chord from 0-0-0 to 15-6-0 for 464.5 plf.

12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard







responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

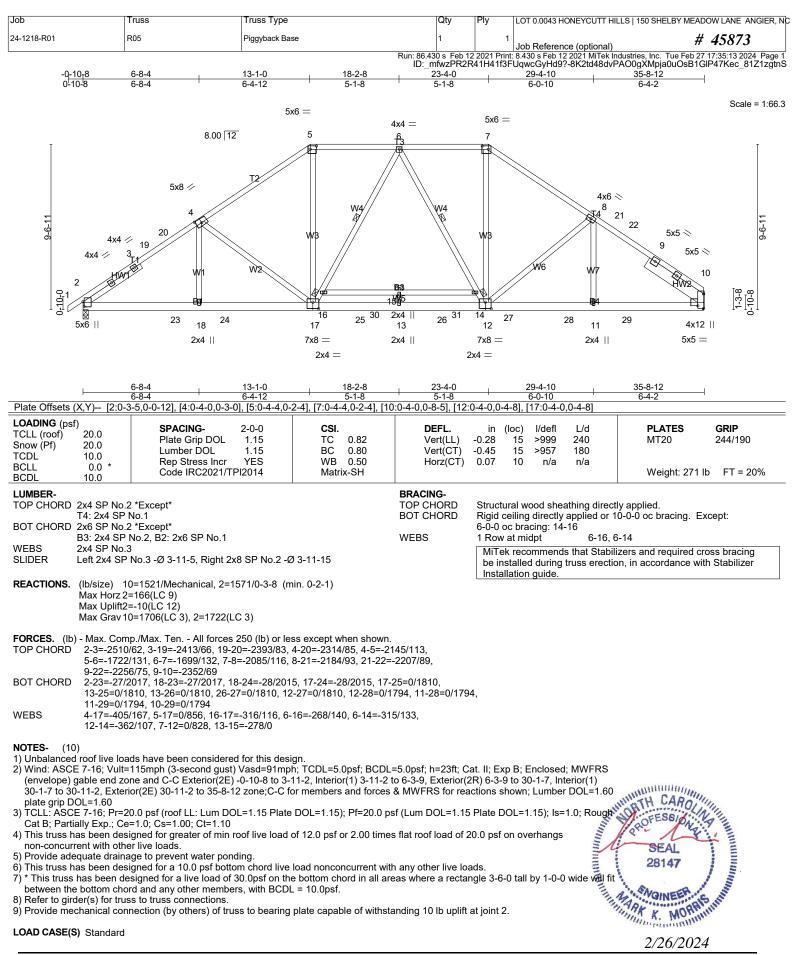
Job	Truss	Truss Type	Qty	Ply	LOT 0.0043 HONEYCUTT HILLS   150 SHELBY MEADOW LANE ANGIER, NC
24-1218-R01	R04	Нір	1		Job Reference (optional) # 45873

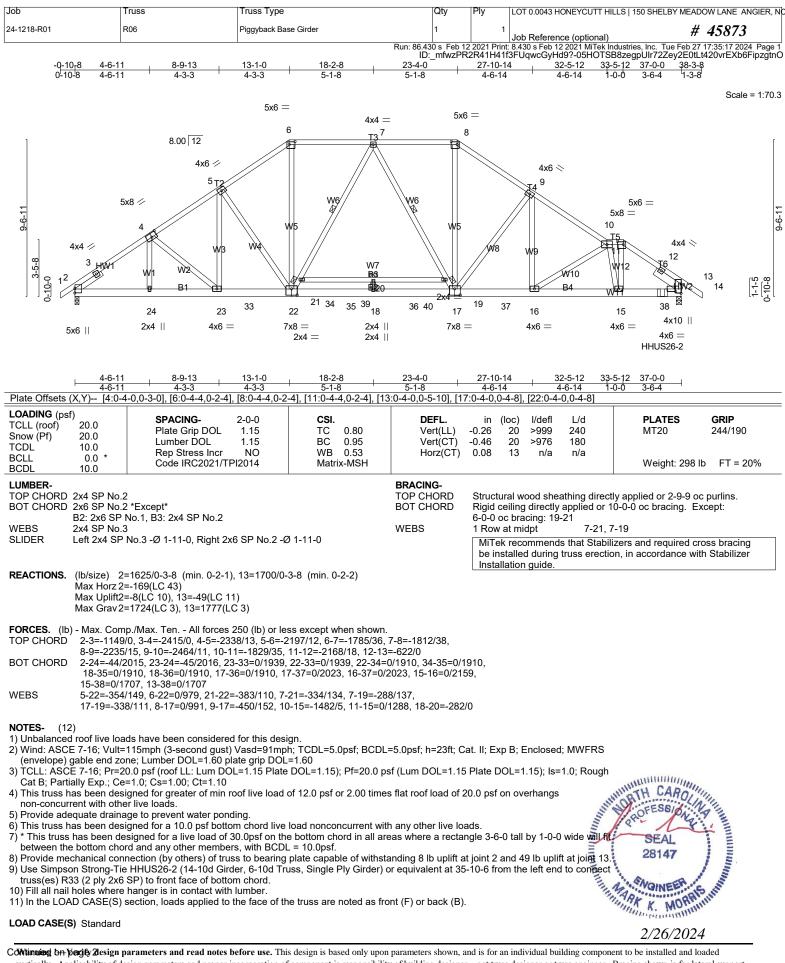
Run: 86.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Feb 27 17:35:12 2024 Page 2 ID:\_mfwzPR2R41H41f3FUqwcGyHd9?-g8UVQk7?861XOWy9G03nLAJ\_2sNtLggVOKPU1bzgtnT

LOAD CASE(S) Standard



2/26/2024





Job	Truss	Truss Type	Qty	Ply	LOT 0.0043 HONEYCUTT HILLS   150 SHELBY MEADOW LANE ANGIER, NC
24-1218-R01	R06	Piggyback Base Girder	1	1	Job Reference (optional) # 45873
Run: 86.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Feb 27 17:35:17 2024 Page ID:mfwzPR2R41H41f3FUqwcGyHd9?-05HOTSB8zegpUIr72Zey2E0tLt420vrEXb6Fipzgt					

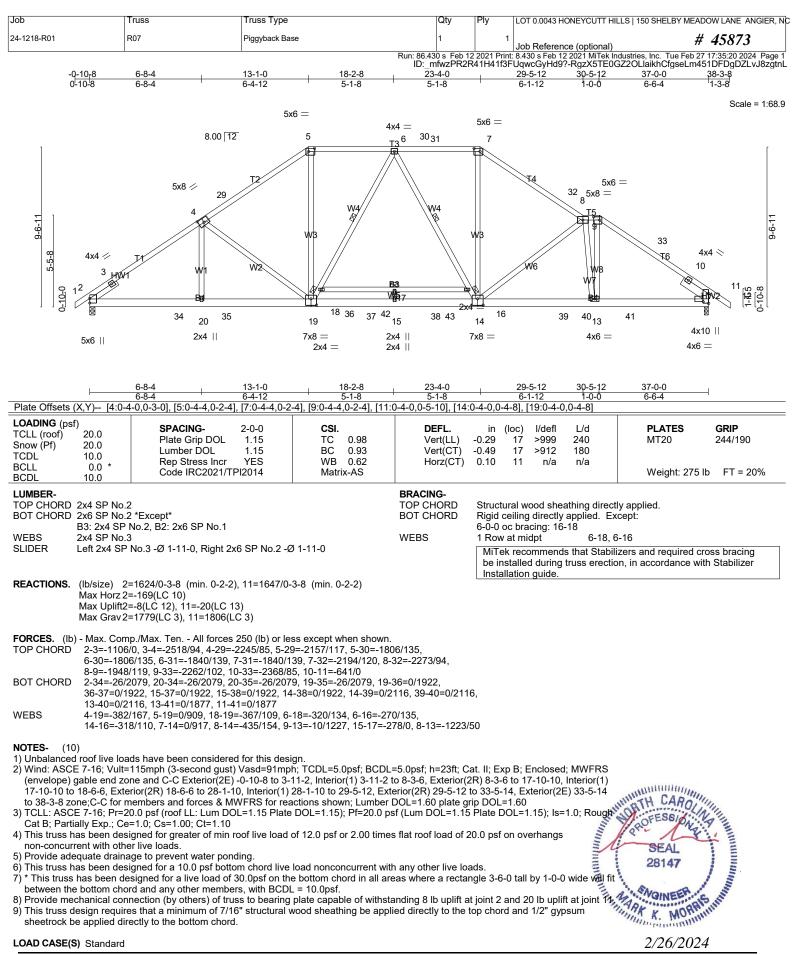
# LOAD CASE(S) Standard

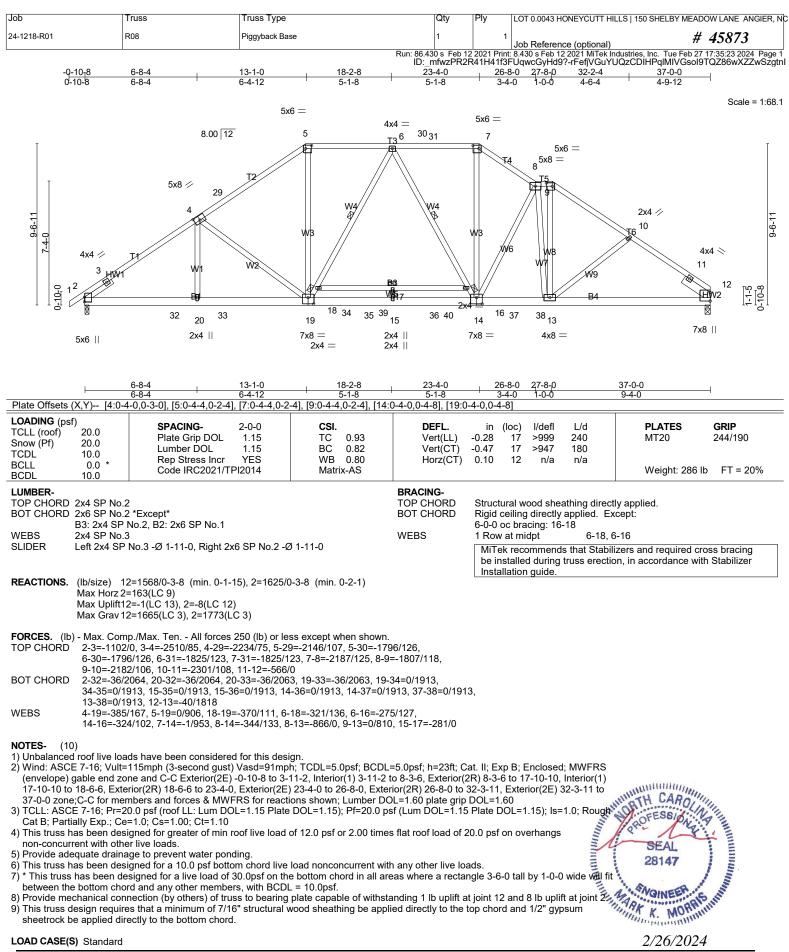
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

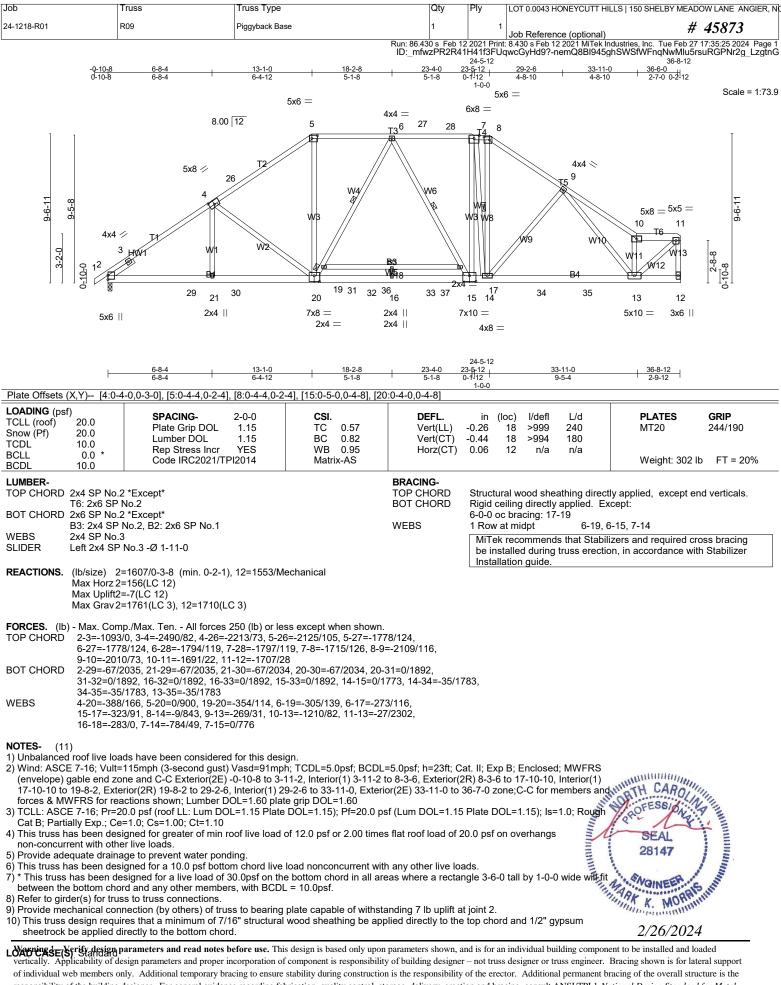
Vert: 1-6=-60, 6-8=-60, 8-10=-60, 10-11=-60, 11-14=-60, 25-29=-20, 19-21=-20 Concentrated Loads (lb)

Vert: 38=-54(F)

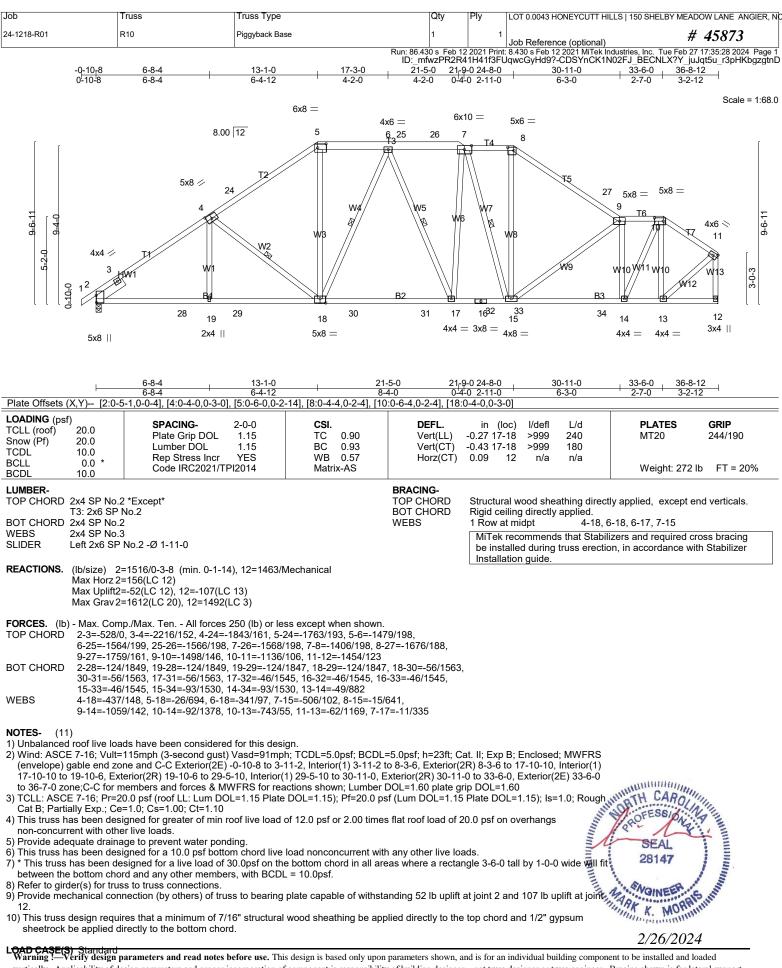


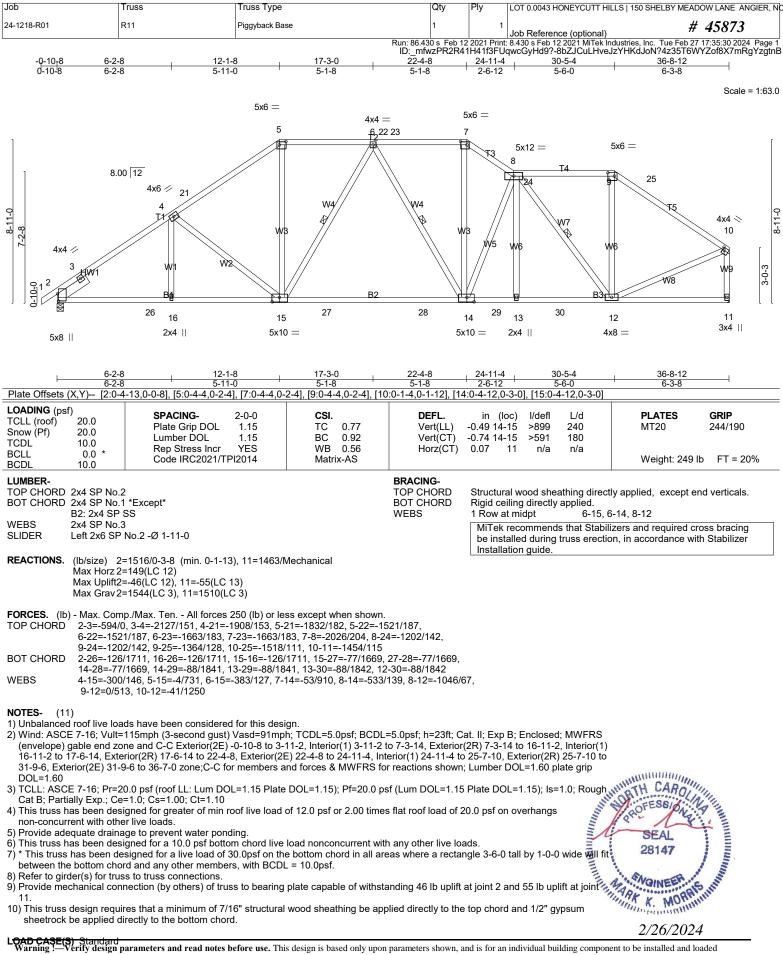


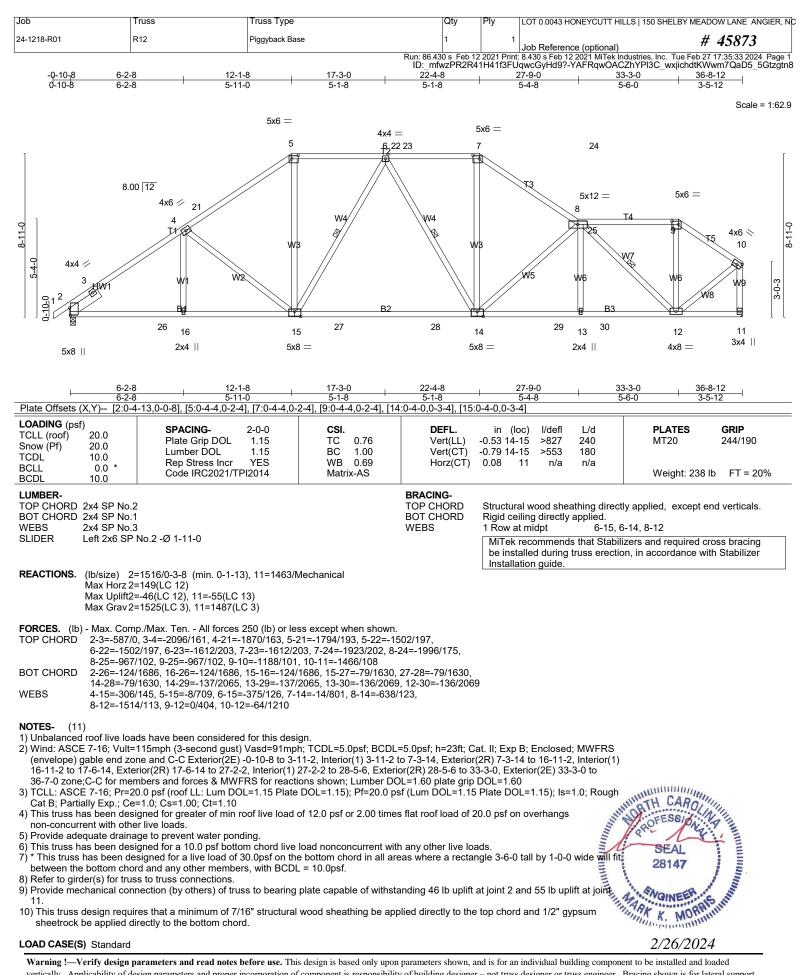


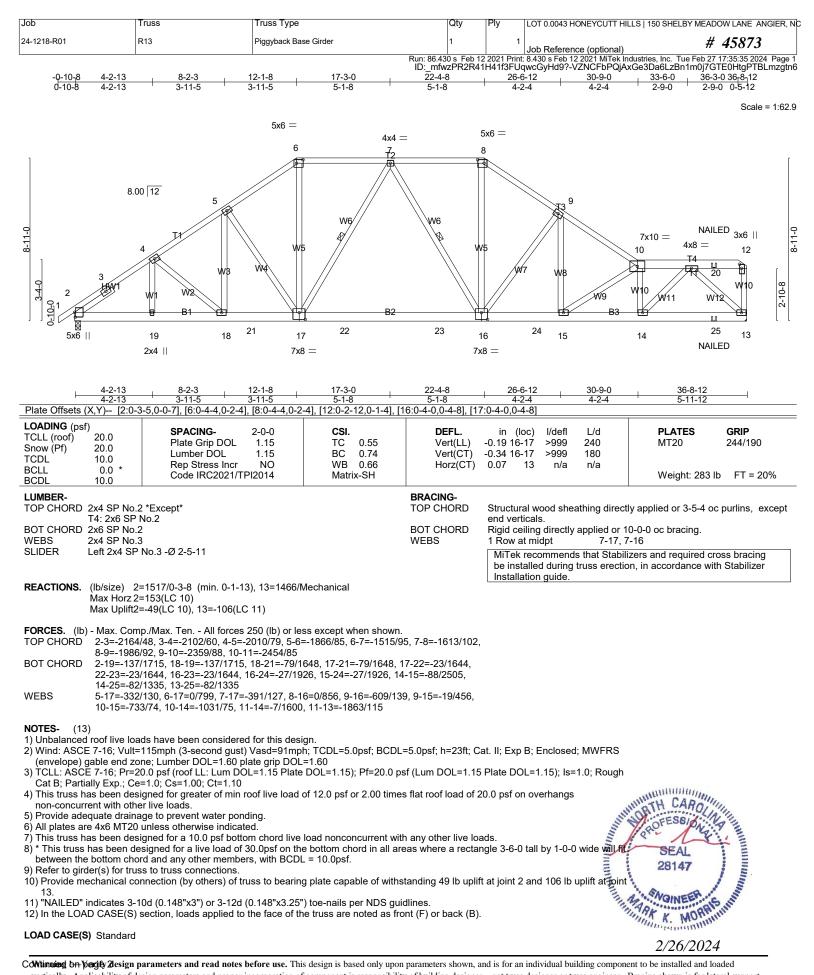


responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive. Madison, WI 53719.









Job	Truss	Truss Type	Qty	Ply	LOT 0.0043 HONEYCUTT HILLS   150	SHELBY MEADOW LANE ANGIER, NC
24-1218-R01	R13	Piggyback Base Girder	1	1	Job Reference (optional)	# 45873
		Run: 86	430 s Feb 12	2 2021 Print	: 8.430 s Feb 12 2021 MiTek Industries, I	nc. Tue Feb 27 17:35:36 2024 Page 2

86.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Feb 27 17:35:36 2024 Page 2 ID:\_mfwzPR2R41H41f3FUqwcGyHd9?-zIxaSxQ2UU37GCong2UQJEJBTXcizTX1v3DltCzgtn5

## LOAD CASE(S) Standard

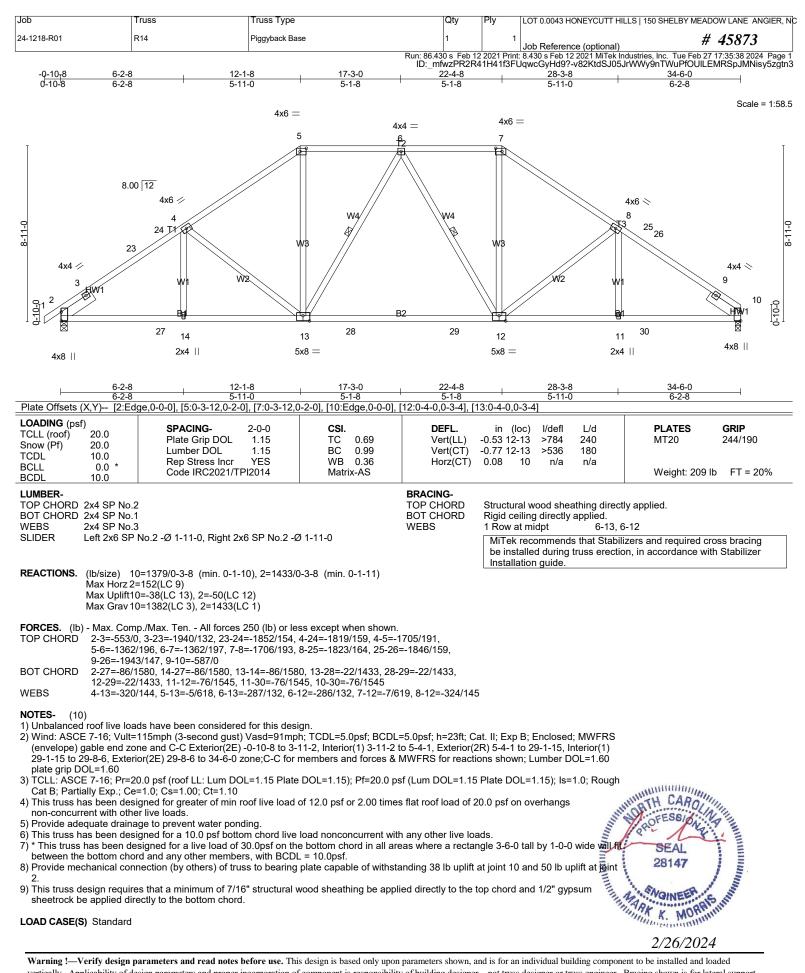
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

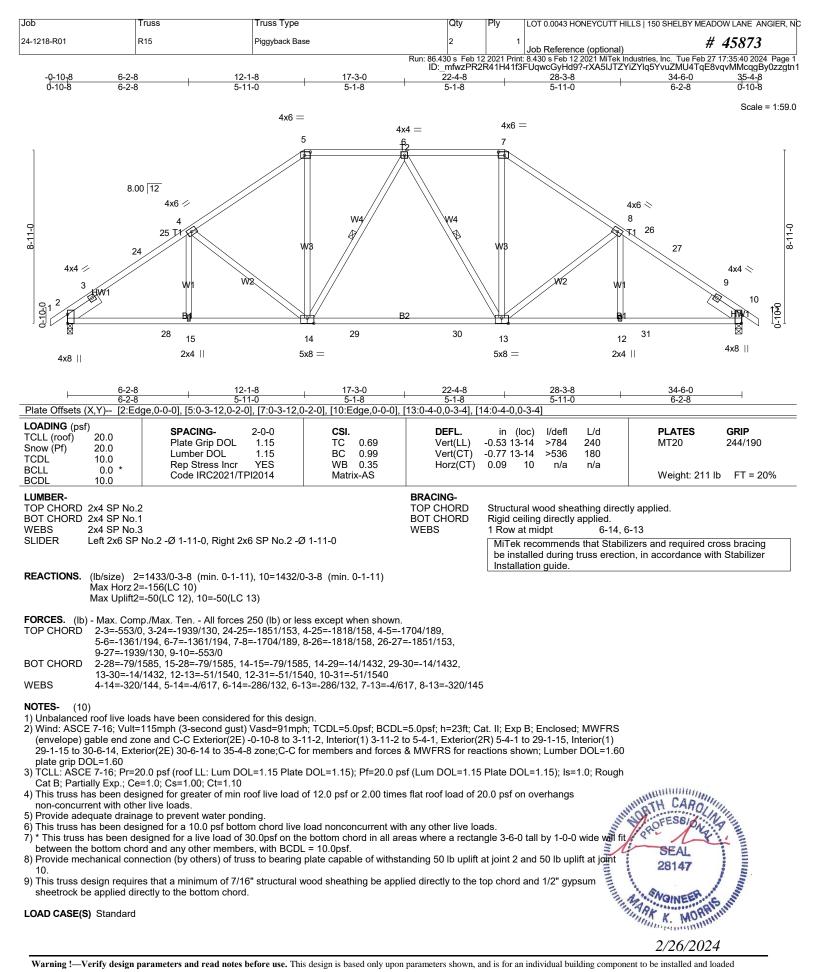
Vert: 1-6=-60, 6-8=-60, 8-10=-60, 10-12=-60, 2-13=-20 Concentrated Loads (lb)

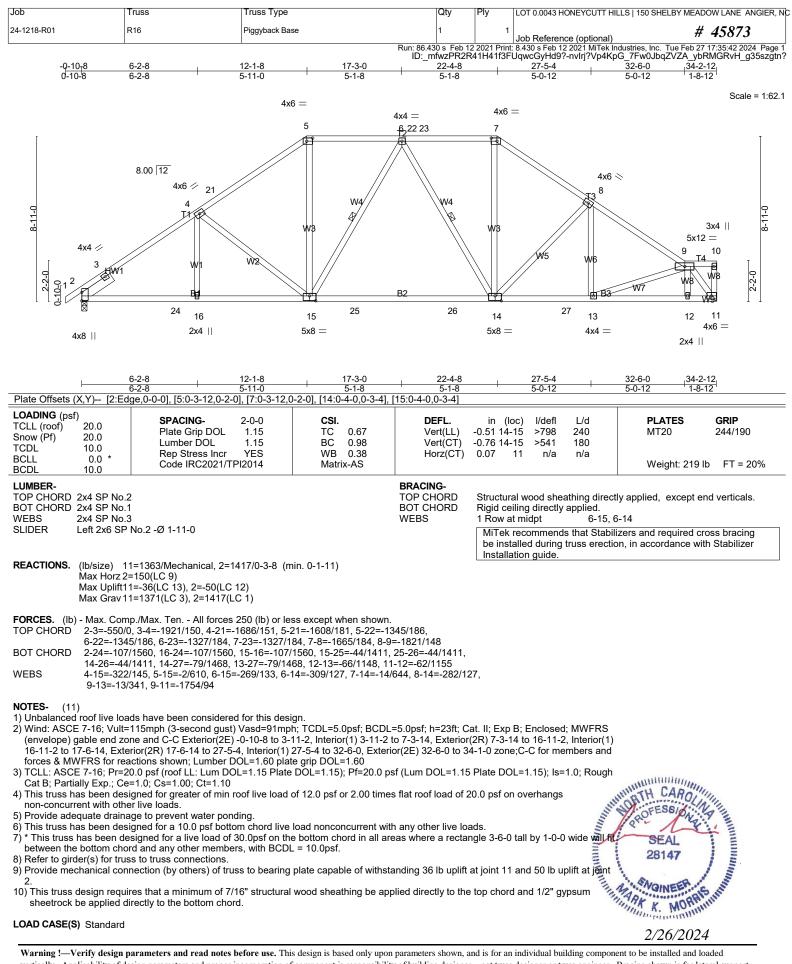
Vert: 25=-4(B)

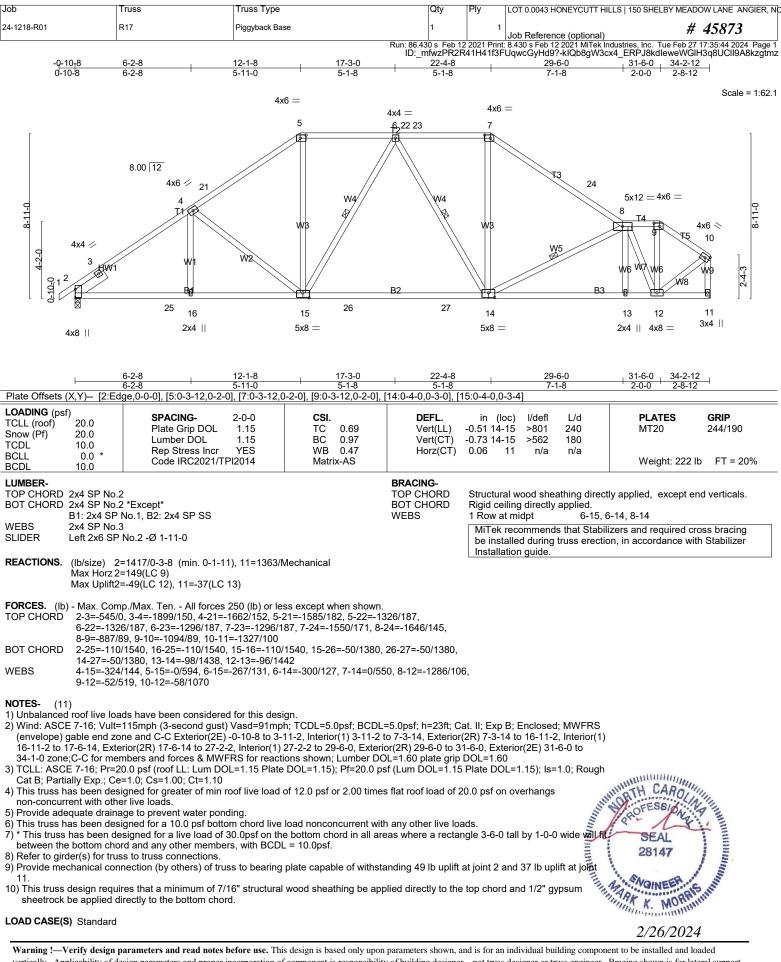


2/26/2024



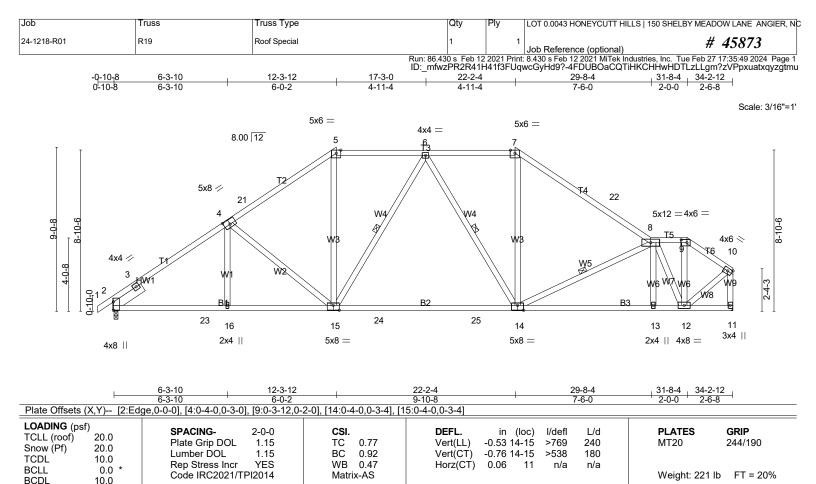






LOAD CASE(S) Standard

lob	Truss	Truss Type		Qty	Ply LO	T 0.0043 HONEYCUTT H	ILLS   150 SHELBY ME	
24-1218-R01	R18	Roof Special		1	1 Jo	b Reference (optional)		# 45873
	0.40.0 7.0.40	45	. 40	Run: 86.430 s Feb 12 ID:_mfwzPR2R4	2 2021 Print: 8.4 41H41f3FUqw	30 s Feb 12 2021 MiTek Ir cGyHd9?-8t5kmiZyvsS	ndustries, Inc. Tue Feb 25v8upsB?GYG_h	27 17:35:47 2024 Page zKX1UTeRGOql3zgtm
	-0 <u>-10-8 7-9-10</u> 0-10-8 7-9-10		3-12 5-2	19-2-4 3-10-8	<u>26-8-4</u> 7-6-0	28-8-4	34-2-12 5-6-8	
			6x8 =					Scale = 1:73.
		8.00 12	5	5x6 =				
		0.00   12	Ŕ					]
		5x8 ⁄/ 22	2		74	23 5x8 = <sup>5x8 =</sup>	=	
8-	9-0	4		104		7	24	9-
11-0-8	10-10-0		WЗ	VV3			\	10-10-6
		-    \v	2		W5	w7	16 4x6 ×	2
		vv1				ws// ws		1
	0,1 <sup>2</sup>						W9 W8	2-4-3
l		B1 g 25 17 26	16 15	B2 6 27 14 13		<b></b>	Ø	] ]
		25 17 26 2x4	15 3x8 =	<sup>27</sup> 14 <sup>13</sup> 4x8 =		12   11   4x4 = 4x4 = 11	10 3x4	1
	4x8	2x4	3x8 — 4x4 =		=	4x4 — 4x4 —	3X4	Ι
	7-9-10 7-9-10	7-	3-12 5-2	19-2-4 3-10-8	26-8-4 7-6-0	28-8-4	34-2-12 5-6-8	
	- [2:Edge,0-0-0], [4:0-4-0,0-3	8-0], [5:0-4-13,Edge], [8:	0-6-4,0-2-4]					
OADING (psf) CLL (roof) 20.			<b>SI.</b> C 0.87	DEFL. Vert(LL)	in (loc) -0.12 15-17	l/defl L/d >999 240	PLATES MT20	GRIP 244/190
now (Pf) 20. CDL 10.	Lumber DOL	1.15 E	BC 0.85	Vert(CT)	-0.26 15-17	>999 180	11120	211/100
CLL 0. CDL 10.			VB 0.52 /atrix-AS	Horz(CT)	0.07 10	n/a n/a	Weight: 23	85 lb FT = 20%
UMBER-	<u> </u>			BRACING-				
OP CHORD 2x4 OT CHORD 2x4				TOP CHORD BOT CHORD		vood sheathing direct g directly applied.	lly applied, excep	t end verticals.
	SP No.3 2x6 SP No.2 -Ø 1-11-0			WEBS	1 Row at m	· · · · · · · · · · · · · · · · · · ·	5-14, 7-14	
					be installe	commends that Stabil ed during truss erection		
REACTIONS. (Ib/s	ize) 2=1417/0-3-8 (min. 0-1	-13), 10=1363/Mechan	cal		Installation	n guide.		
	Horz 2=185(LC 9) Uplift2=-66(LC 12), 10=-53(I	C 13)						
	Grav 2=1534(LC 20), 10=13							
	x. Comp./Max. Ten All for							
6-2	=-544/0, 3-4=-2078/137, 4-2 3=-1396/182, 7-23=-1497/15							
	0=-1325/101 5=-136/1755, 17-25=-136/17	55, 17-26=-137/1752, 1	6-26=-137/1752	, 15-16=-137/1752	2,			
	27=-3/1220, 14-27=-3/1220, 12=-41/1138	13-14=-65/1486, 13-28	-65/1486, 12-28	3=-65/1486,				
/EBS 4-1	7=0/406, 4-15=-666/169, 5-1 2=-67/1071, 8-11=-401/36, 9		6, 7-14=-449/13	6, 7-12=-798/125,				
	207/1071, 0-11401/30, 9	-1134/1139						
	live loads have been conside							
	6; Vult=115mph (3-second gu end zone and C-C Exterior(2							
1.4.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1	44 to 00 0 4 E to 10 00 00	0 44.00 0 0 5.4.		10		d forces & MWFRS		-
TCLL: ASCE 7-1	6; Pr=20.0 psf (roof LL: Lum	DOL=1.15 Plate DOL=1	.15); Pf=20.0 ps	f (Lum DOL=1.15	Plate DOL=	1.15); ls=1.0; Rough	IN RTH CARO	11/1/1/
This truss has be	en designed for greater of m	in roof live load of 12.0	osf or 2.00 times	flat roof load of 2	0.0 psf on ov	/erhangs	ROFESSION	Allin
non-concurrent v Provide adequat	14 to 26-8-4, Exterior(2/K) 26 wn; Lumber DOL=1.60 plate 6; Pr=20.0 psf (roof LL: Lum xp.; Ce=1.0; Cs=1.00; Ct=1. en designed for greater of m ith other live loads. e drainage to prevent water p en designed for a 10.0 psf b been designed for a live load	onding.				in the	SEAL	
) This truss has be ) * This truss has	en designed for a 10.0 psf b been designed for a live load	ottom chord live load no of 30.0psf on the bottom	nconcurrent with the chord in all are	any other live loa	ads. Iale 3-6-0 tai	l by 1-0-0 wide will fit	28147	
between the bott	om chord and any other mem for truss to truss connection cal connection (by others) of	bers, with BCDL = $10.0$	psf.					
) Provide mechan	cal connection (by others) of	s. truss to bearing plate ca	apable of withsta	nding 66 lb uplift	at joint 2 and	53 lb uplift at joint	A NOINEER	ALS WITH
10.	n requires that a minimum of					nd 1/2" gypsum	MININ K. MOP	
	plied directly to the bottom c		5 11	,		071	(*iliviti.	
Sheellock be a							2/26/20	24



BRACING-

WFBS

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied, except end verticals.

6-15. 6-14. 8-14

MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer

Rigid ceiling directly applied.

1 Row at midpt

Installation guide. REACTIONS. (lb/size) 2=1417/0-3-8 (min. 0-1-11), 11=1363/Mechanical Max Horz 2=149(LC 9) Max Uplift2=-51(LC 12), 11=-38(LC 13) FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-550/0, 3-4=-1905/152, 4-21=-1677/155, 5-21=-1599/184, 5-6=-1330/188, 6-7=-1304/189. 7-22=-1566/171. 8-22=-1666/145. 8-9=-857/85. 9-10=-1056/86. 10-11 = -1328/100BOT CHORD 2-23=-110/1548, 16-23=-110/1548, 15-16=-111/1547, 15-24=-53/1391, 24-25=-53/1391,

14-25=-53/1391, 13-14=-101/1441, 12-13=-99/1445 4-15=-305/146, 5-15=-3/602, 6-15=-278/129, 6-14=-308/129, 7-14=0/555, 8-12=-1326/111, WEBS 9-12=-57/521, 10-12=-58/1062

#### NOTES-(11)

LUMBER-

WEBS

SLIDER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.1 \*Except\*

B2: 2x4 SP SS

Left 2x6 SP No.2 -Ø 1-11-0

2x4 SP No 3

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 7-6-2, Exterior(2R) 7-6-2 to 26-11-14, Interior(1) 26-11-14 to 29-8-4, Exterior(2R) 29-8-4 to 31-8-4, Exterior(2E) 31-8-4 to 34-1-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

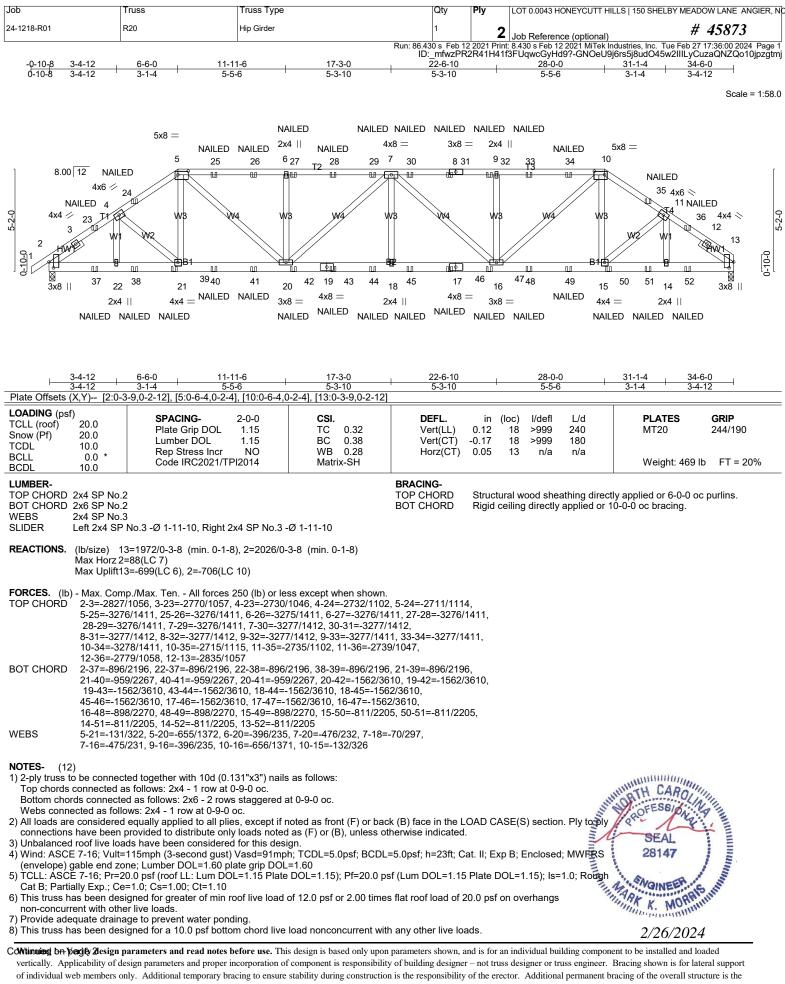
7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide 🖬 🕅 between the bottom chord and any other members, with BCDL = 10.0psf.

8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 2 and 38 lb uplift at joint 11

10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

# LOAD CASE(S) Standard



responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0043 HONEYCUTT HILLS   150 SHELBY MEADOW LANE ANGIER, NC
24-1218-R01	R20	Hip Girder	1	2	Job Reference (optional) # 45873
					8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Feb 27 17:36:01 2024 Page 2 3FUqwcGyHd9?-kZx1iUjkc9Dam2CadoRHqVrWicECJtdifSnaFFzgtmi

## NOTES- (12)

9) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 699 lb uplift at joint 13 and 706 lb uplift at joint 2.

11) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-60, 5-10=-60, 10-13=-60, 2-13=-20

Concentrated Loads (lb)

Vert: 23=-26(B) 25=-22(B) 26=-22(B) 27=-22(B) 28=-22(B) 29=-22(B) 30=-22(B) 31=-22(B) 32=-22(B) 33=-22(B) 34=-22(B) 36=-26(B) 37=-49(B) 38=-80(B) 39=-120(B) 40=-42(B) 41=-42(B) 42=-42(B) 43=-42(B) 44=-42(B) 46=-42(B) 46=-42(B) 47=-42(B) 49=-42(B) 50=-120(B) 51=-80(B) 52=-49(B)



lob	Truss	Truss Type		Qty	Ply	LOT 0.0043 HONEYCUTT H	ILLS   150 SHELBY MEADOW LANE ANGIER, N
24-1218-R01	R21	Half Hip Girder		1		1 Job Reference (optional)	# 45873
				Run: 86.430 s Feb 1 ID: mfwzPR2R41	2 2021 Pri H41f3FU	nt: 8.430 s Feb 12 2021 MiTek I	ndustries, Inc. Tue Feb 27 17:36:02 2024 Page 1 .ROCnmBWyWNjOky0f02Oxsu5W7nizgtmh
		-0-10-8 0-10-8		2-0-0 2-0-0		<u>3-0-0</u> 1-0-0	
				NAILE 4	4.0 -	5 <sup>2x4</sup>	Scale = 1:13.5
	l			4	<u> </u>		
					1д.,		
		8.0	0 12 3x4 ∅				
			3				
	9			$\times$		W1	
	2-2-0				W1	W2	
		2	HW1				
	0-10-0		$1 \angle$				
	6 1			B1		L j	
						\$	
			$\leq$		7 <sub>2x4</sub>	6	
		$\geq$	$\leq$	1	NAILED		
		3x4				4x4 =	
		<u> </u>		2-0-0		3-0-0	
Plate Offsets (X,Y) [4	:0-5-12,0-2-0]		2	2-0-0	·	1-0-0	
LOADING (psf) TCLL (roof) 20.0	SPACING-	2-0-0	CSI.	DEFL.	in (		PLATES GRIP
Snow (Pf) 20.0 TCDL 10.0	Plate Grip DOI Lumber DOL	1.15 1.15	TC 0.09 BC 0.02	Vert(LL) Vert(CT)	-0.00 -0.00	2 >999 240 2 >999 180	MT20 244/190
BCLL 0.0 *	Rep Stress Inc Code IRC2021		WB 0.02 Matrix-P	Horz(CT)	0.00	6 n/a n/a	Weight: 22 lb FT = 20%
BCDL 10.0 LUMBER-				BRACING-			
TOP CHORD 2x4 SP I BOT CHORD 2x6 SP I				TOP CHORD	Structu end ve		tly applied or 3-0-0 oc purlins, except
WEBS 2x4 SP I SLIDER Left 2x4	No.3 SP No.3 -Ø 1-4-13			BOT CHORD		eiling directly applied or	<u> </u>
					be ins	stalled during truss erecti	lizers and required cross bracing on, in accordance with Stabilizer
	6=90/Mechanical, 2=16	8/0-3-8 (min. 0-1-8)			Instal	lation guide.	
Max Up	rz 2=54(LC 48) lift6=-45(LC 10), 2=-19(L0	C 10)					
Max Gra	av 6=90(LC 2), 2=168(LC	2)					
FORCES. (Ib) - Max. (	Comp./Max. Ten All forc	es 250 (lb) or less exc	ept when show	'n.			
NOTES- (12)	loads have been conside	ared for this design					
2) Wind: ASCE 7-16; V	ult=115mph (3-second gu	ıst) Vasd=91mph; TC		0L=5.0psf; h=23ft; C	at. II; Ex	p B; Enclosed; MWFRS	
3) TCLL: ASCE 7-16; F	d zone; Lumber DOL=1.6 Pr=20.0 psf (roof LL: Lum	DOL=1.15 Plate DOL:		psf (Lum DOL=1.15	5 Plate D	OL=1.15); ls=1.0; Rough	
4) This truss has been	Ce=1.0; Cs=1.00; Ct=1. designed for greater of m		0 psf or 2.00 tim	nes flat roof load of 2	20.0 psf o	on overhangs	
non-concurrent with 5) Provide adequate dr	other live loads. ainage to prevent water p	onding.					
	designed for a 10.0 psf b n designed for a live load					0 tall bv 1-0-0 wide will fit	t
between the bottom	chord and any other men truss to truss connection	bers.			5		
9) Provide mechanical	connection (by others) of 3-10d (0.148"x3") or 3-12	truss to bearing plate	capable of with	standing 45 lb uplift	at joint 6	and 19 lb uplift at joint 2	WHITH CARO
	(S) section, loads applied					1	FESSIO NULL
LOAD CASE(S) Standa			45			(IIIII)	and the
Úniform Loads (plf)	ced): Lumber Increase=1	.15, Plate Increase=1.	15				SEAL
Vert: 1-4=-60 Concentrated Loads	), 4-5=-60, 2-6=-20 (lb)					11HI	2014/
Vert: 7=4(B)						Internet	15 ANGINEER S
						0	SEAL 28147
							2/26/2024
							2/26/2024

Job	Truss	Truss Type		Qty	Ply	LOT 0.0043 HONEYCUTT H	HILLS   150 SHELBY MEADOW LANE ANGIER,
24-1218-R01	R22	Roof Special		4		1	# 45873
				Run: 86.430 s Feb 1	2 2021 Pri	Job Reference (optional) nt: 8.430 s Feb 12 2021 MiTek	)
				ID:_mfwzPR 2-8-8	2R41H41	f3FUqwcGyHd9?-gy3n7Al	Industries, Inc. Tue Feb 27 17:36:03 2024 Page 7nTI?MMzIDTIwwwu6Q_TnrZ?6IGgJ8zgt
				2-8-8			
				0.4.11			Scale = 1:16
				2x4	3		
			8.00 12				
			8.00   12				
			5x5 🛷				
			2	T1			
		0-0		$\times$	W1		
		2-10-0		<u> </u>			
		l r	1				
			HW1				
		1-0-5					
		-1-					
				B1			
		l l	X		X		
					4		
		3x8	2		2x4		
		5.0	, 11		2,74		
				2-8-8			
				2-8-8			
Plate Offsets (X,Y	) [1:0-1-8,0-0-1]			1			
LOADING (psf) TCLL (roof) 20	0.0 SPACING		CSI.	DEFL.	in		PLATES GRIP
Snow (Pf) 20	0.0 Plate Gri Lumber D		TC 0.13 BC 0.07	Vert(LL) Vert(CT)	-0.00 -0.01	1-4 >999 240 1-4 >999 180	MT20 244/190
	0.0 * Rep Stree		WB 0.00	Horz(CT)	0.00	n/a n/a	
	0.0 Code IRC	2021/TPI2014	Matrix-P				Weight: 18 lb FT = 20%
LUMBER-				BRACING-			
TOP CHORD 2x4				TOP CHORD			ctly applied or 2-8-8 oc purlins, except
BOT CHORD 2x4 WEBS 2x4	SP No.2 SP No.3			BOT CHORD	end ve Rigid o	rticals. eiling directly applied or	10-0-0 oc bracing
	t 2x8 SP No.2 -Ø 1-9-14			201 01010	-		ilizers and required cross bracing
					be in	stalled during truss erect	ion, in accordance with Stabilizer
	(aiza) 1-102/Machanica				Insta	llation guide.	

REACTIONS. (Ib/size) 1=103/Mechanical, 4=103/Mechanical Max Horz 1=68(LC 12) Max Uplift4=-42(LC 12) Max Grav 1=103(LC 1), 4=108(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-** (7)

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

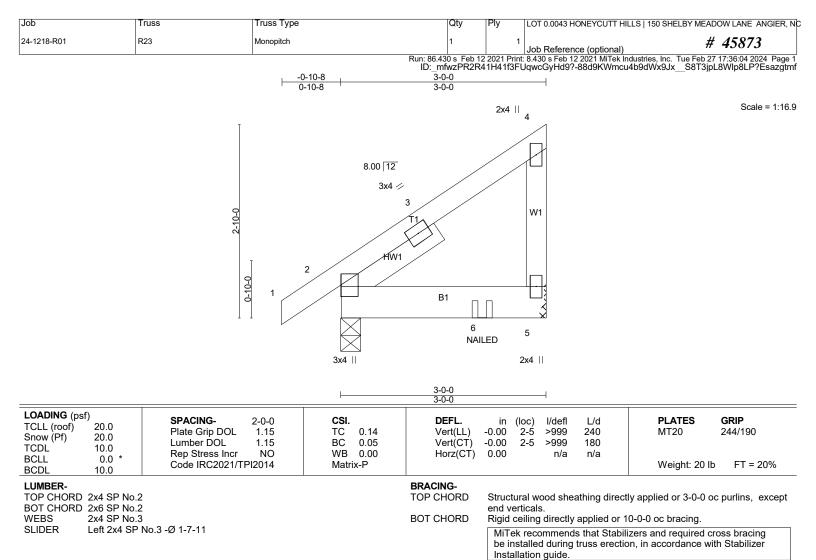
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 4.

LOAD CASE(S) Standard





REACTIONS. (Ib/size) 5=123/Mechanical, 2=181/0-3-8 (min. 0-1-8) Max Horz 2=69(LC 10) Max Uplift5=-62(LC 10), 2=-9(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (10)

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

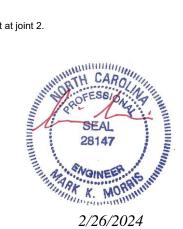
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 5 and 9 lb uplift at joint 2.

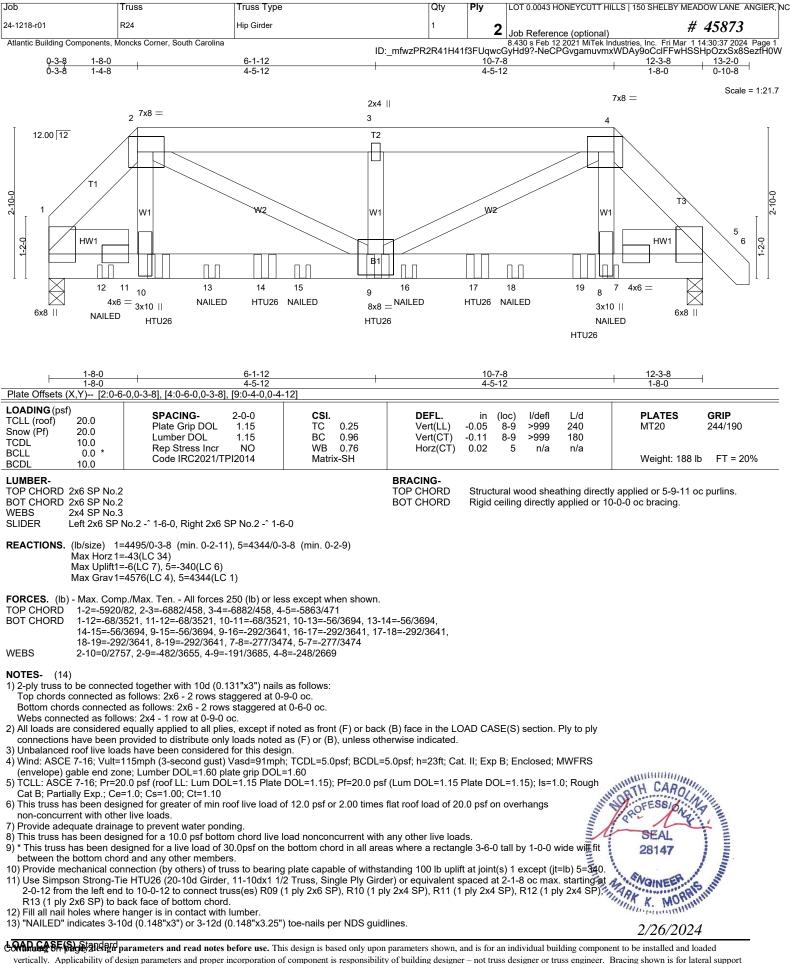
- 8) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 2-5=-20 Concentrated Loads (lb) Vert: 6=-24(F)





vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0043 HONEYCUTT HILLS   150	SHELBY MEADOW LANE ANGIER, NC
24-1218-r01	R24	Hip Girder	1	2	Job Reference (optional)	# 45873
Atlantic Building Components,	Moncks Corner, South Carolina				8.430 s Feb 12 2021 MiTek Industries,	Inc. Fri Mar 1 14:30:37 2024 Page 2

8.430 s Feb 12 2021 Millek Industries, Inc. Fri Mar 1 14:30:37 2024 Page 2 ID:\_mfwzPR2R41H41f3FUqwcGyHd9?-NeCPGvgamuvmxWDAy9oCclFFwHSSHpOzxSx8SezfH0W

#### LOAD CASE(S) Standard

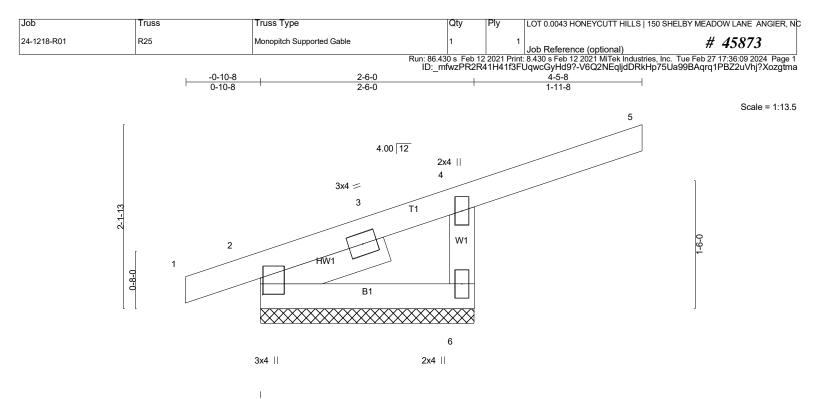
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-4=-60, 4-6=-60, 1-5=-20

Concentrated Loads (lb)

Vert: 10=-1533(B) 9=-1443(B) 8=-103(F) 12=-70(F) 13=-82(F) 14=-1443(B) 15=-82(F) 16=-82(F) 17=-1443(B) 18=-82(F) 19=-1446(B)





#### Plate Offecte (XX) [2:0.2.5.0.0.5]

LOADING (psf)           TCLL (roof)         20.0           Snow (Pf)         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	<b>CSI.</b> TC 0.50 BC 0.10 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.01 -0.01 0.00	(loc) 5 5	l/defl n/r n/r n/a	L/d 180 80 n/a	PLATES MT20 Weight: 16 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2			BRACING- TOP CHORD	Struct	tural w	ood she	athing direc	tly applied or 2-6-0 oc	purlins, excep

BOT CHORD

end verticals

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.3

2x4 SP No 3 WFBS

SLIDER Left 2x4 SP No.3 -Ø 1-6-7

REACTIONS. (Ib/size) 6=267/2-6-0 (min. 0-1-8), 2=100/2-6-0 (min. 0-1-8) Max Horz 2=47(LC 14) Max Uplift6=-72(LC 11), 2=-6(LC 10) Max Grav 6=378(LC 21), 2=135(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 4-6=-354/369

NOTES-(12)

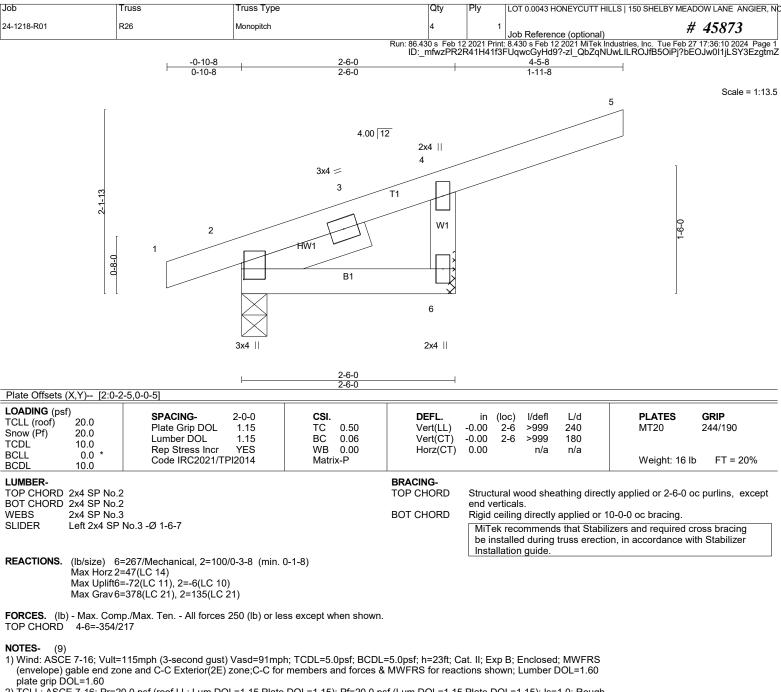
- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc. 7)

- 9) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
  10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 72 lb uplift at joint 6 and 6 lb uplift at joint 2.
  11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.

LOAD CASE(S) Standard

A PRIMARIA PRIMARIA And an and a second second SEAL 28147 VOINE K. MO ununu

2/26/2024



2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

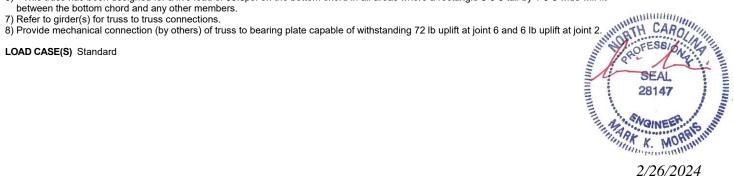
4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

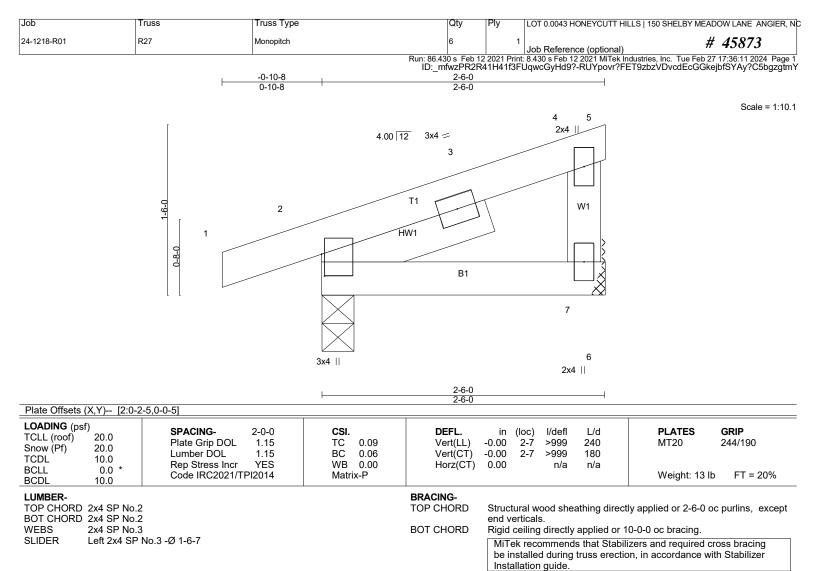
non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.





REACTIONS. (lb/size) 7=98/Mechanical, 2=154/0-3-8 (min. 0-1-8) Max Horz 2=32(LC 14) Max Uplift7=-16(LC 14), 2=-29(LC 10) Max Grav 7=125(LC 21), 2=204(LC 21)

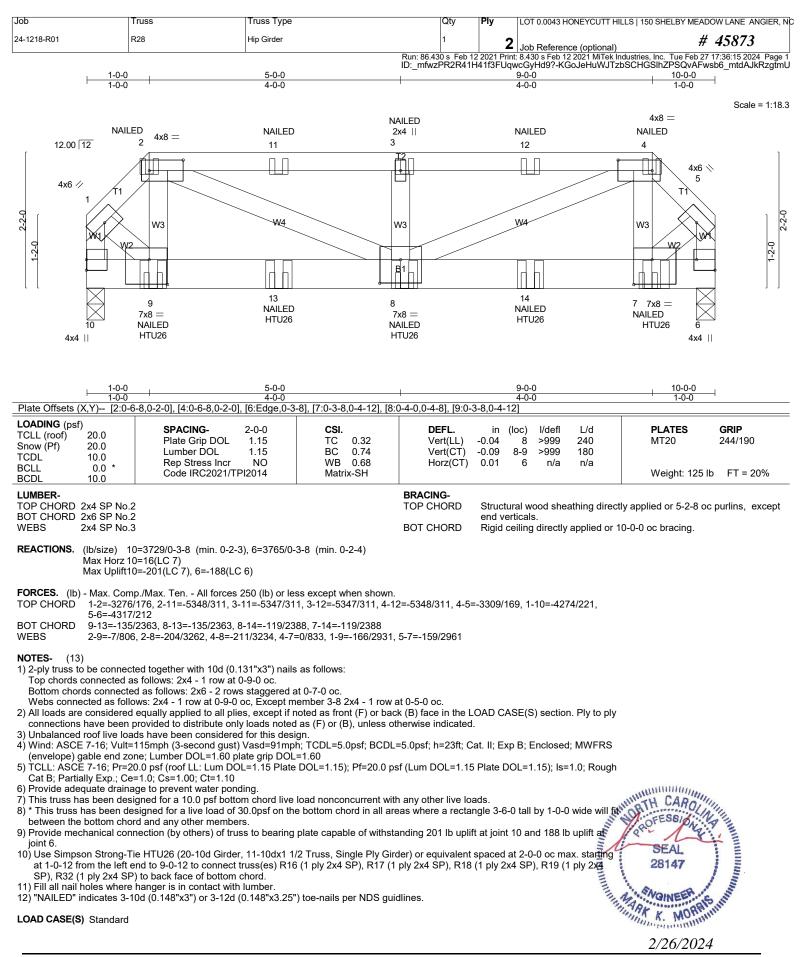
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

**NOTES-** (9)

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 7 and 29 lb uplift at joint 2.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0043 HONEYCUTT HILLS   150 SH	ELBY MEADOW LANE ANGIER, NC
24-1218-R01	R28	Hip Girder	1	2	Job Reference (optional)	# 45873
		Rup: 86.43	0 s Eeh 12	2021 Print	8 430 s Eeb 12 2021 MiTek Industries Inc.	Tue Feb 27 17:36:16 2024 Page 2

un: 86.430 s\_Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc.\_Tue Feb 27 17:36:16 2024\_Page 2 \_ID:\_\_mfwzPR2R41H41f3FUqwcGyHd9?-oSLirdv83m6S3MsT0SCoxgz4wfG5KZEv6HvsHuzgtmT

#### LOAD CASE(S) Standard

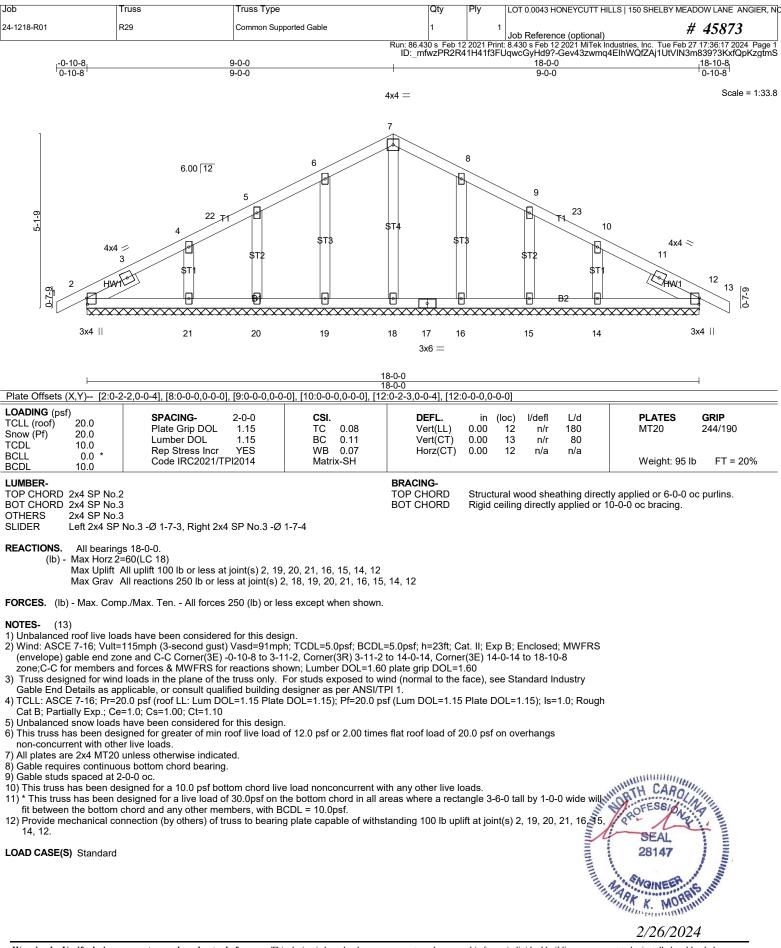
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

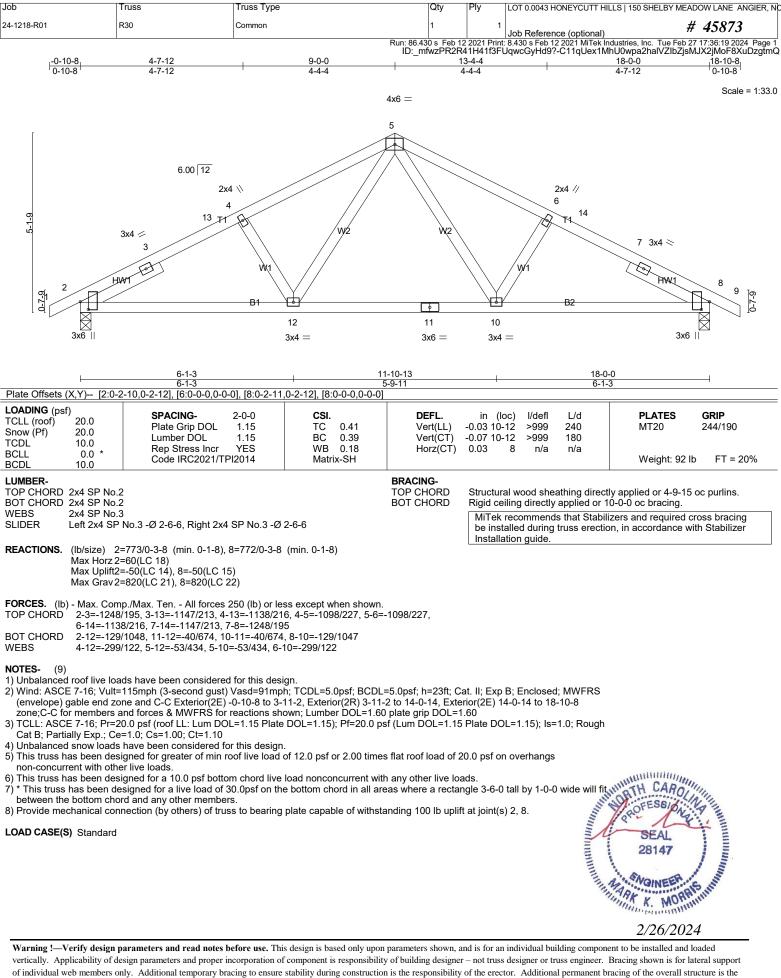
Vert: 1-2=-60, 2-4=-60, 4-5=-60, 6-10=-20

Concentrated Loads (lb)

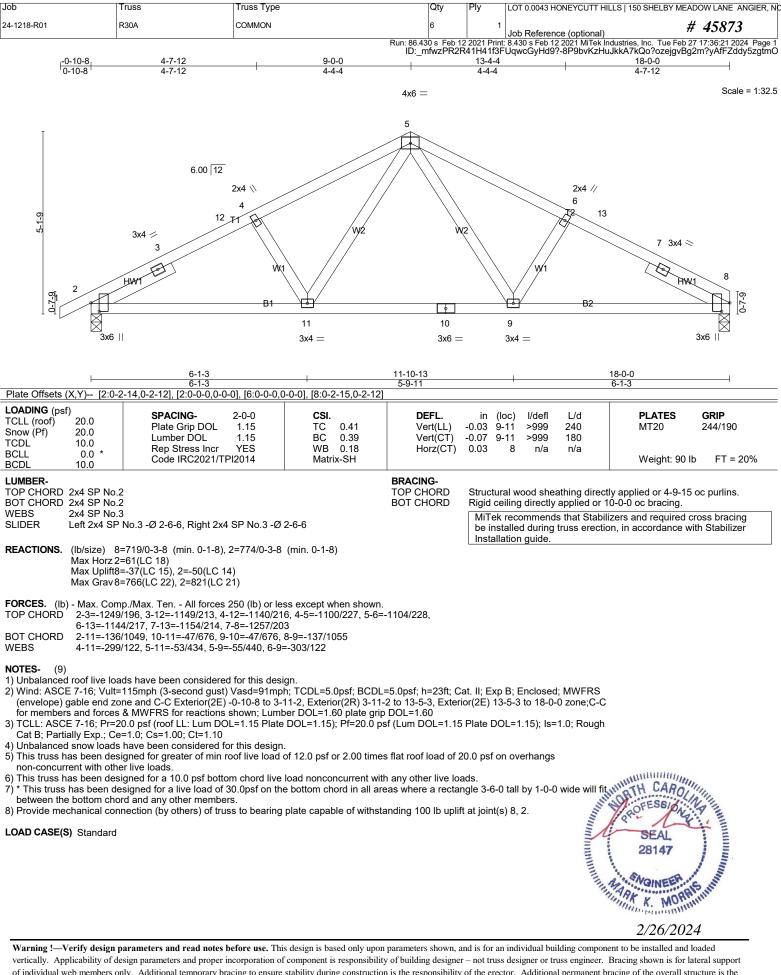
Vert: 9=-1344(F=-1, B=-1343) 8=-1343(B) 7=-1345(F=-1, B=-1345) 13=-1343(B) 14=-1343(B)



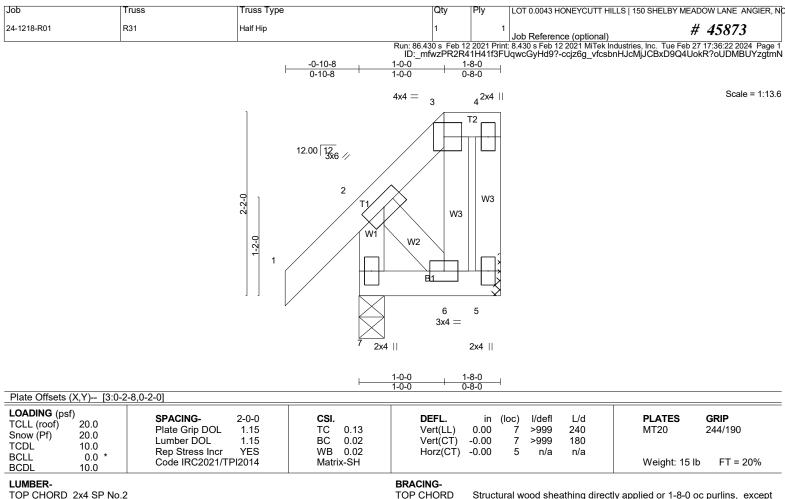




of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WFBS

2x4 SP No.3

Structural wood sheathing directly applied or 1-8-0 oc purlins, except end verticals BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing MiTek recommends that Stabilizers and required cross bracing

Installation guide.

be installed during truss erection, in accordance with Stabilizer

REACTIONS. (lb/size) 5=32/Mechanical, 7=139/0-3-8 (min. 0-1-8) Max Horz 7=50(LC 12) Max Uplift5=-29(LC 12) Max Grav 5=37(LC 28), 7=167(LC 18)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(9)

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 1-6-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads.

- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

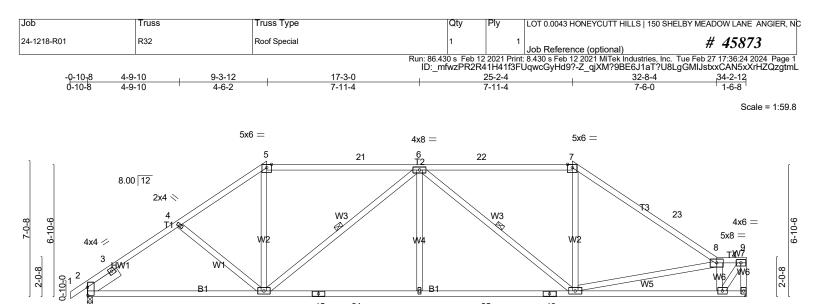
between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.

#### LOAD CASE(S) Standard





14

2x4 ||

25

13

3x8 =

12

4x8 =

10 11

4x6 =

3x4 ||

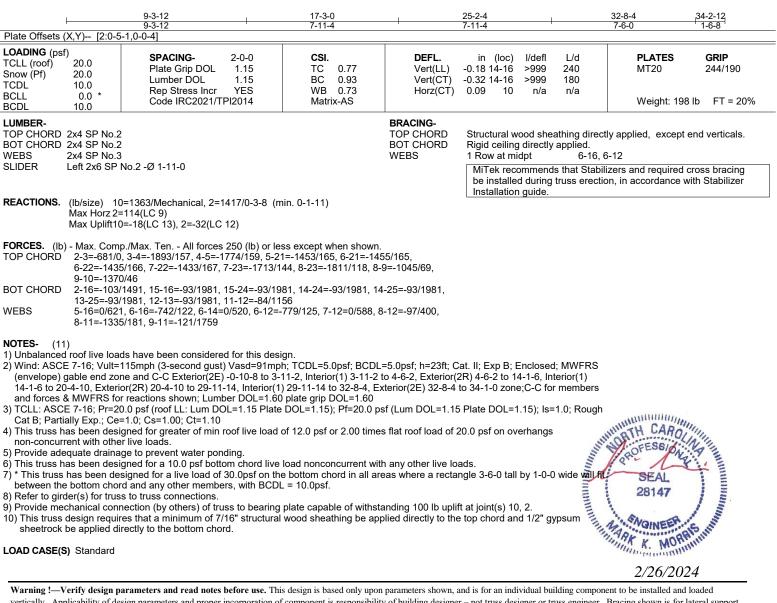
15

3x8 =

16

4x8 =

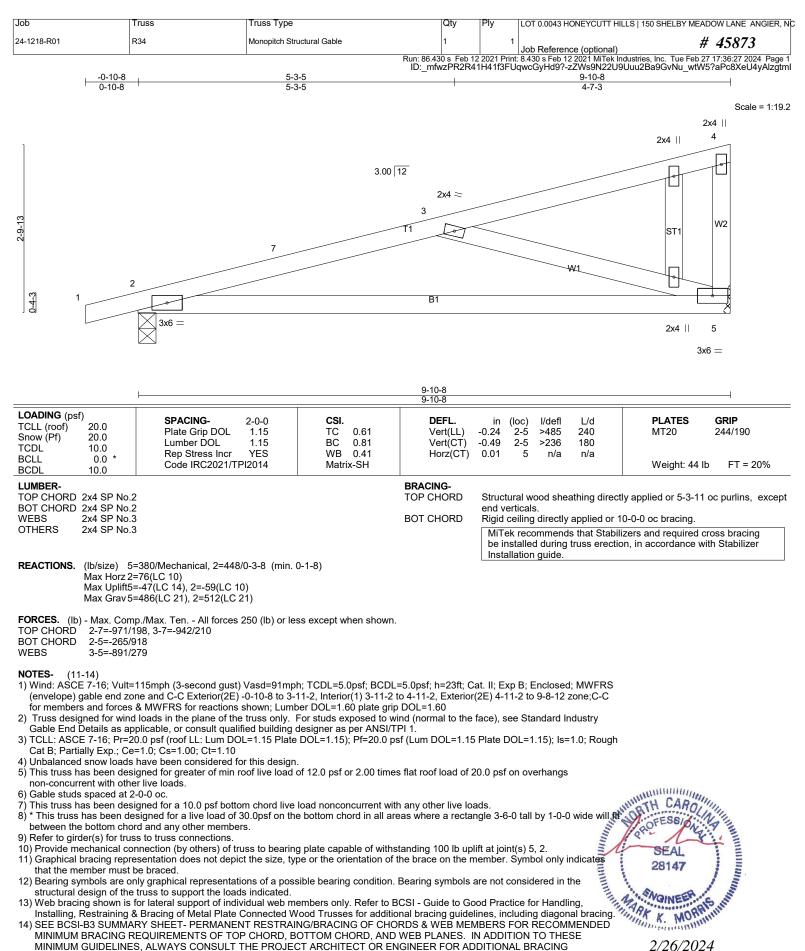
24



LOAD CASE(S) Standard

4x8 ||

Job		russ	Truss Type		Qty	Ply	LOT 0.0043 HONEYCUTT HI		
24-1218-R01		33	Half Hip Girder		1	<b>_</b>			45873
					Run: 86.430 s Feb 1	2 2021 Print:	Job Reference (optional) 8.430 s Feb 12 2021 MiTek Ir	dustries, Inc. Tue Feb 2	7 17:36:26 2024 Page 1
			0-7-5	1	ID:_mfwzPR2R4 2-3-8	1H41f3FU	qwcGyHd9?-VNyUy21QjrN	M1GucÓbYN8LnNr2hr	xgFBOPqKOdJzgtmJ
			0-7-5 NAILED	1	1-8-3	1			
			4x4 = 2			3			Scale = 1:11.6
		Ţ		1	T2	3x4 =			
			3x6 //						
			΄ _τı		/ L				
		I							
		-9-5		🔨 w3 🔪	W4	W5			
		`		w2					
		1-2-0							
				B1	1				
						X			
		1 1	7	5		X			
				AILED 3x8 =	=	4			
			6 2x4			2x4			
			HTU26	-12	2-3-8				
Plate Offsets (2	X Y) [2·0-2.	.8 0-2-01	1-1		1-1-12				
LOADING (psf)	., .	· •	2.0.0		DEEL	in (la			
TCLL (roof) Snow (Pf)	20.0 20.0	SPACING- Plate Grip DOL	2-0-0 <b>CSI.</b> 1.15 TC	0.02	<b>DEFL.</b> Vert(LL)	in (lo -0.00	5 >999 240	PLATES MT20	<b>GRIP</b> 244/190
TCDL	10.0	Lumber DOL Rep Stress Incr	1.15 BC NO WB	0.00 0.00	Vert(CT) Horz(CT)	-0.00 -0.00	5 >999 180 4 n/a n/a		
BCLL BCDL	0.0 * 10.0	Code IRC2021/TF	12014 Matr	ix-P				Weight: 34 It	• FT = 20%
LUMBER- TOP CHORD 2	214 SD No 2				BRACING- TOP CHORD	Structure	al wood sheathing direct	ly applied or 2.2.9 a	o purling oxeent
BOT CHORD	2x6 SP No.2					end verti	icals.		c purmis, except
	2x4 SP No.3				BOT CHORD	Rigia cei	iling directly applied or 1	0-0-0 oc bracing.	
	(lb/size) 4= Max Horz 6=	74/Mechanical, 6=1569/ 14(LC 10)	0-3-8 (min. 0-1-8)						
	Max Uplift4= Max Grav4=	-20(LC 7) 74(LC 1), 6=1745(LC 3)							
			250 (lb) or loop events	than about					
		o./Max. Ten All forces :	250 (ID) OF less except w	nen snown.					
NOTES- (13) 1) 2-ply truss to		ed together with 10d (0.1	31"x3") nails as follows						
		follows: 2x4 - 1 row at 0 as follows: 2x6 - 2 rows							
Webs conne	ected as follow	ws: 2x4 - 1 row at 0-9-0 equally applied to all plie	DC.	nt (E) or book	(P) face in the L		E(S) section Blute plu		
connections	have been p	rovided to distribute only	loads noted as (F) or (	B), unless othe	erwise indicated		()		
		15mph (3-second gust) ie; Lumber DOL=1.60 pl		Opsf; BCDL=	5.0psf; h=23ft; C	at. II; Exp	B; Enclosed; MWFRS		
		0.0 psf (roof LL: Lum DO :1.0; Cs=1.00; Ct=1.10	L=1.15 Plate DOL=1.15	); Pf=20.0 psf	(Lum DOL=1.15	Plate DO	L=1.15); ls=1.0; Rough		
5) Provide ade	quate draina	ge to prevent water pond gned for a 10.0 psf botto		ncurrent with	any other live lo	ade			
7) * This truss I	has been des	signed for a live load of 3	0.0psf on the bottom ch				•		
<ol><li>Refer to gird</li></ol>	ler(s) for truss	d and any other member s to truss connections.						MUMMINIA	
		ection (by others) of trus e HTU26 (20-10d Girder					) 4. from the left end to 🛛 🔬	IN ATH CAROL	11111
connect tru	ıss(es) R05 (*	1 ply 2x6 SP) to back fac nanger is in contact with	e of bottom chord.	0 ,	, ,		mm	ROFESSION	R III
		)d (0.148"x3") or 3-12d (		per NDS guidl	ines.		THIN	SEAL	Annu -
LOAD CASE(S							and the second se	28147	
1) Dead + Snov Uniform Loa		Lumber Increase=1.15	Plate Increase=1.15				11111		
	: 1-2=-60, 2-3	8=-60, 4-6=-20					inter	1 VOINEER	Bunnin
	: 2=26(F) 6=-	1509(B)						SEAL 28147	111.
								2/26/202	4
			for the second s	and or t	. 1		individual building compone		



CONSIDERATIONS. Consideration of the period of the interview of the interv

D'Onofrio Drive, Madison, WI 53719.

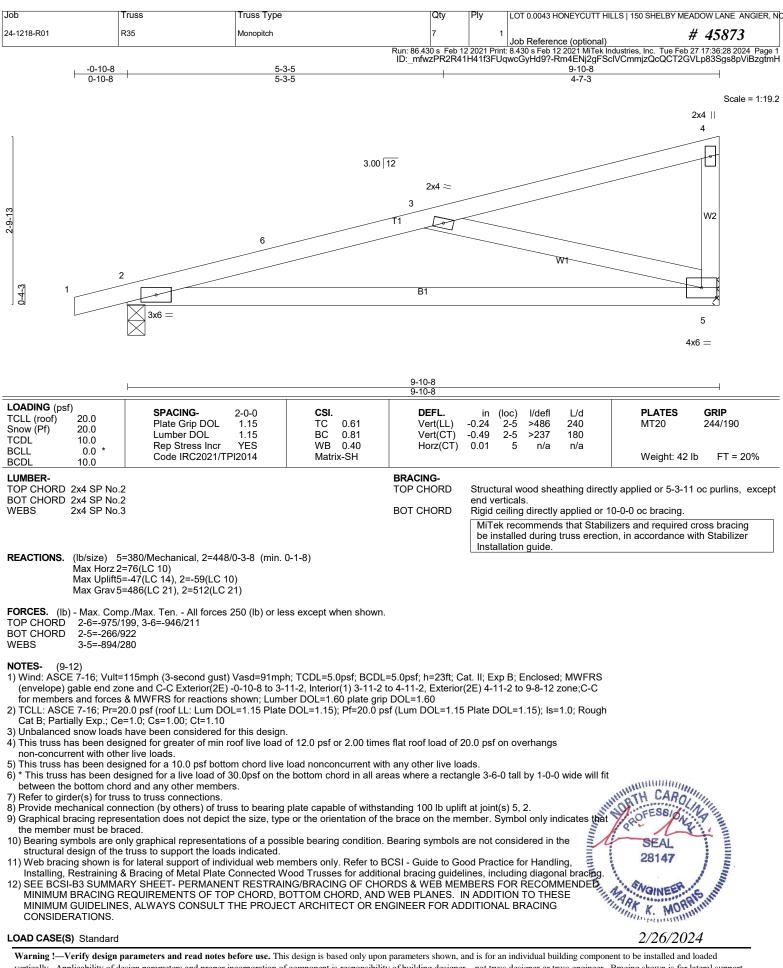
Job	Truss	Truss Type	Qty	Ply	LOT 0.0043 HONEYCUTT HILLS   150 SHELE	BY MEADOW LANE ANGIER, NC
24-1218-R01	R34	Monopitch Structural Gable	1	1	Job Reference (optional)	# 45873
		Dup: 96 /	20 a Eab 1	2021 Drint	· 9 420 a Eab 12 2021 MiTak Industrias Inc. Tu	a Eab 27 17:26:27 2024 Daga 2

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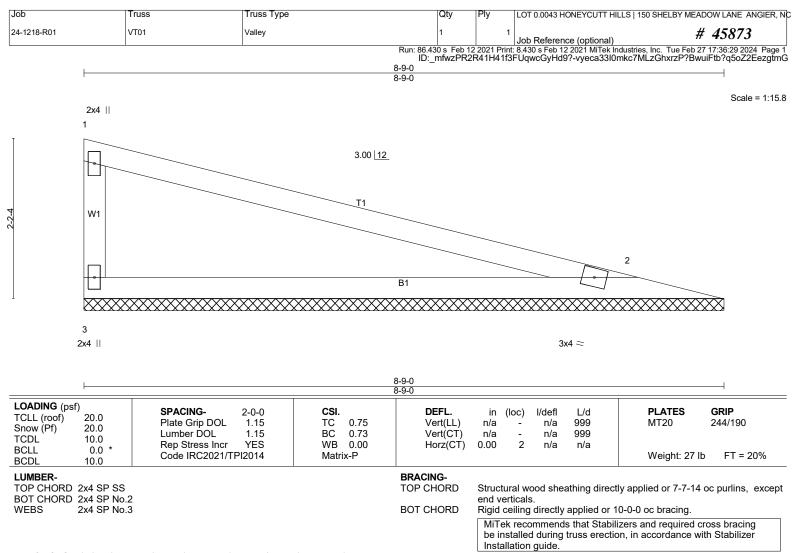
LOAD CASE(S) Standard



2/26/2024



vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



REACTIONS. (lb/size) 3=297/8-9-0 (min. 0-1-8), 2=297/8-9-0 (min. 0-1-8) Max Horz 3=-52(LC 11) Max Uplift3=-36(LC 11), 2=-23(LC 11) Max Grav 3=384(LC 21), 2=384(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 1-3=-310/119

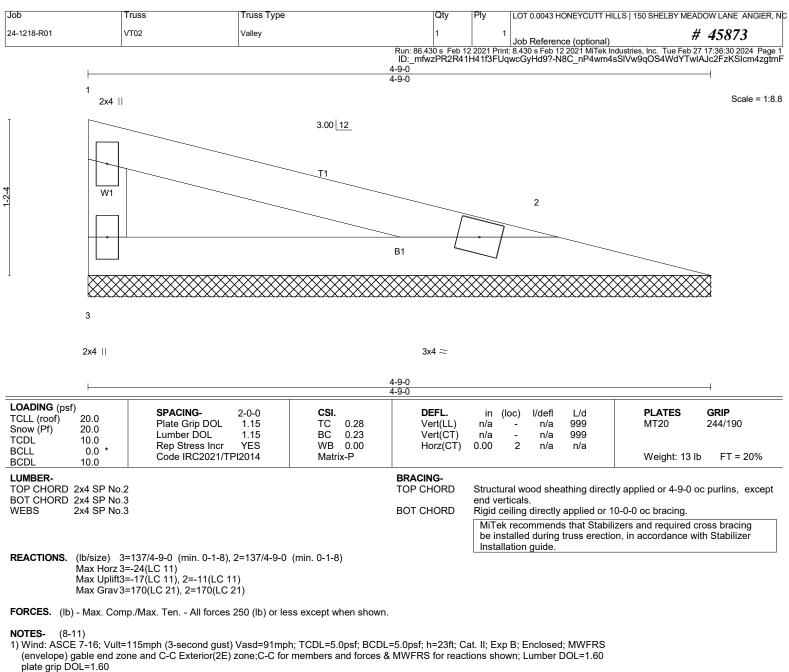
NOTES- (8-11)

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 8) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that

- B) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
  B) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
  (1) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
  (1) Web bracing SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED SEAL MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.
  LOAD CASE(S) Standard

Warning !--Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

2/26/2024



- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 8) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 9) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 10) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling,
- he member must be braced. Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated. Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. ISEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS. AD CASE(S) Standard 11) SEE BČŠI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR ŘECOMMENDED

LOAD CASE(S) Standard

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