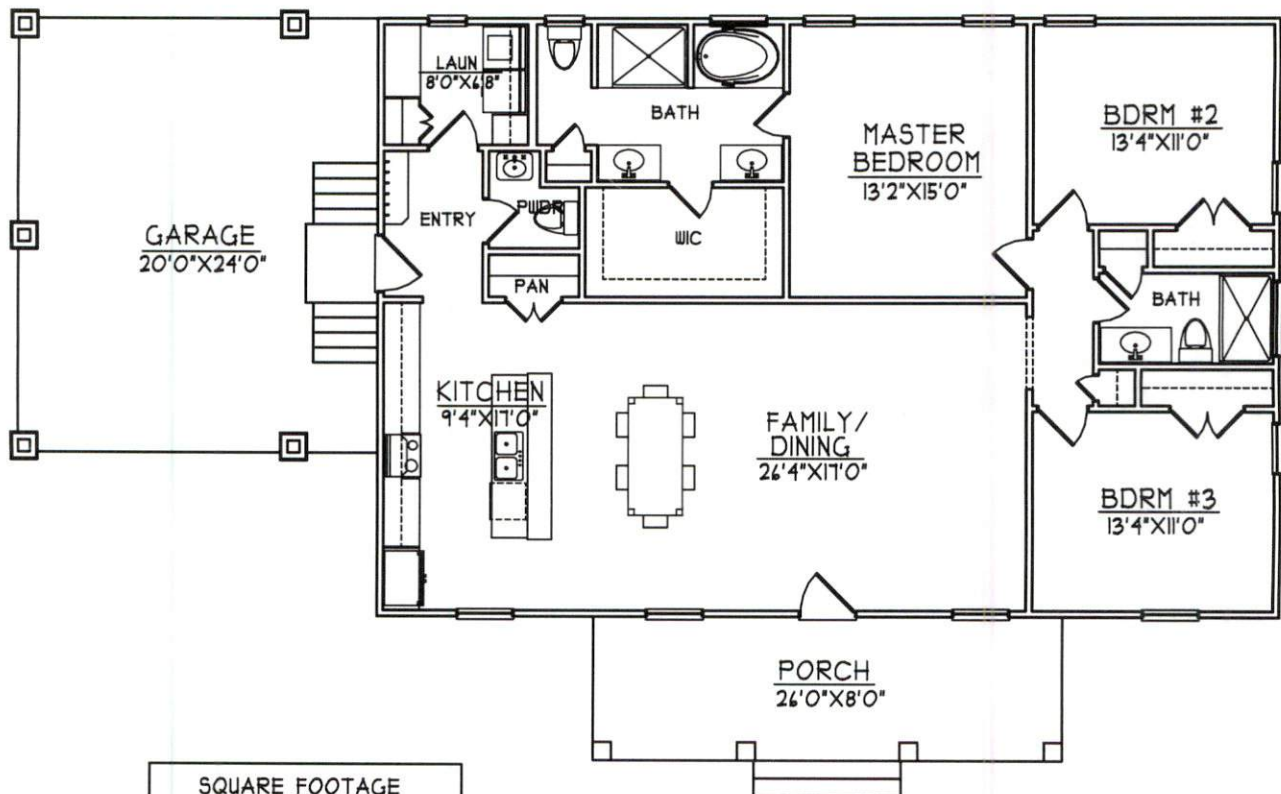


THE GARCIA

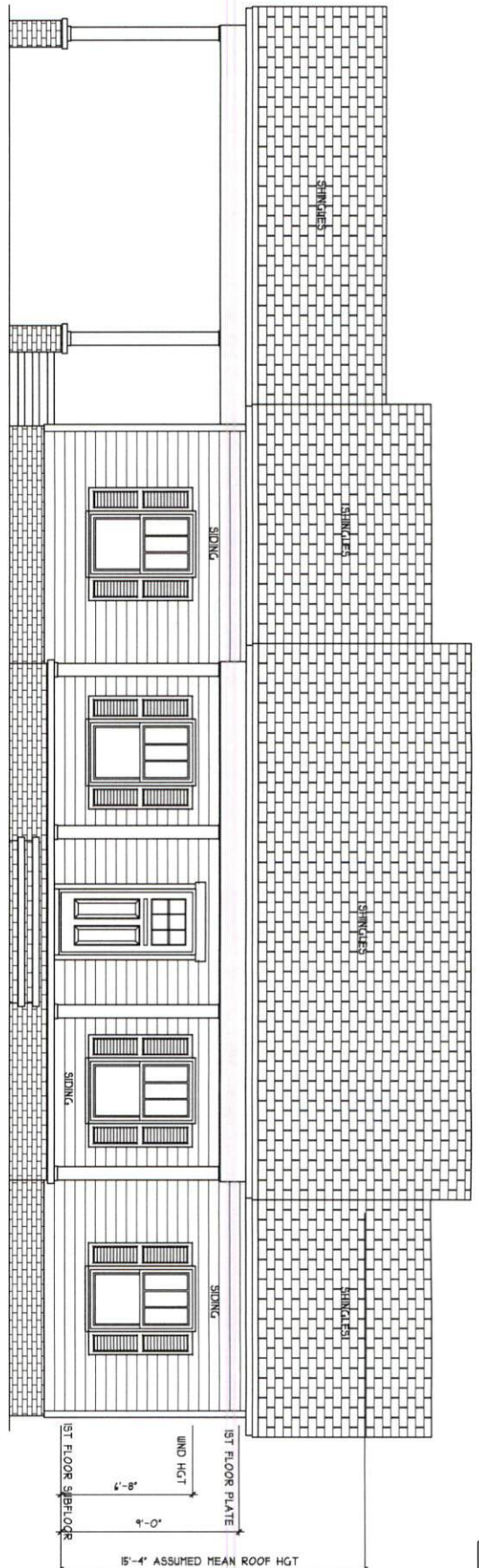
#1650



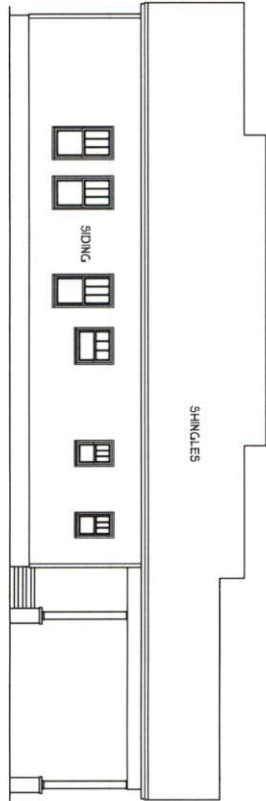
SQUARE FOOTAGE

FIRST FLOOR = 1676
CARPORT = 480

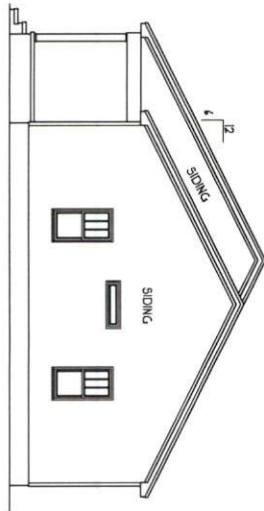
WIDTH = 10'-0"
DEPTH = 41'-0"



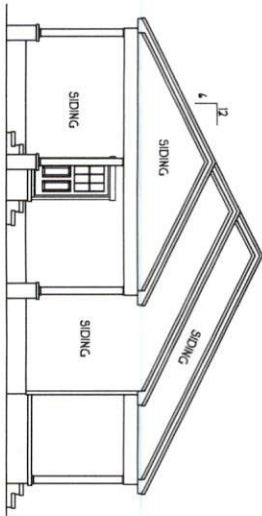
FRONT ELEVATION
SCALE 1/4" = 1'-0"



REAR ELEVATION
SCALE 1/8" = 1'-0"



RIGHT ELEVATION
SCALE 1/8" = 1'-0"



LEFT ELEVATION
SCALE 1/8" = 1'-0"

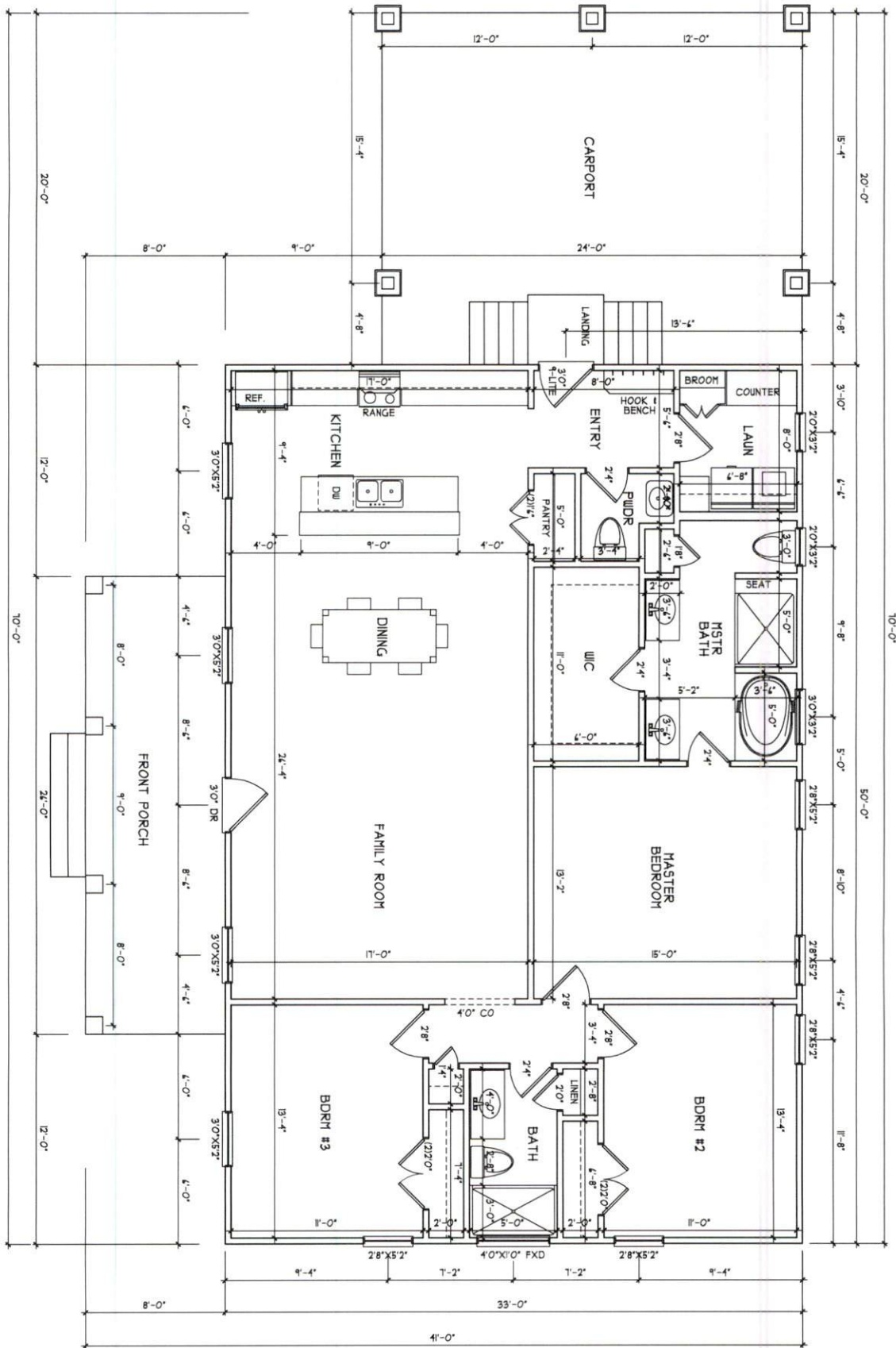
ENERGY COMPLIANCE
ZONE 3 = MAX GLAZING U-FACTOR .35
R-VALUE = CEILING R38, WALLS R15, FLOORS R11
FOR JOHNSTON, SAMPSON, WAYNE COUNTY
ZONE 4 = MAX GLAZING U-FACTOR .35
R-VALUE = CEILING R38, WALLS R15, FLOORS R11
FOR WAKE, DURHAM, ORANGE COUNTY

ATTIC VENTILATION:

THE NET FREE VENTILATING AREA SHALL BE NOT LESS THAN 1/60 OF THE AREA OF THE SPACE VENTILATED. AT LEAST 80 PERCENT OF THE REQUIRED VENTILATING AREA SHALL BE PROVIDED BY THE UPPER PORTION OF THE SPACE TO BE VENTILATED. AT LEAST 3 FEET ABOVE EAVE OR CORNICE VENTS SHALL BE PROVIDED BY EAVE OR CORNICE VENTS TO CROSS ATTIC AREA TO BE VENTILATED 2338 SQ.FT.

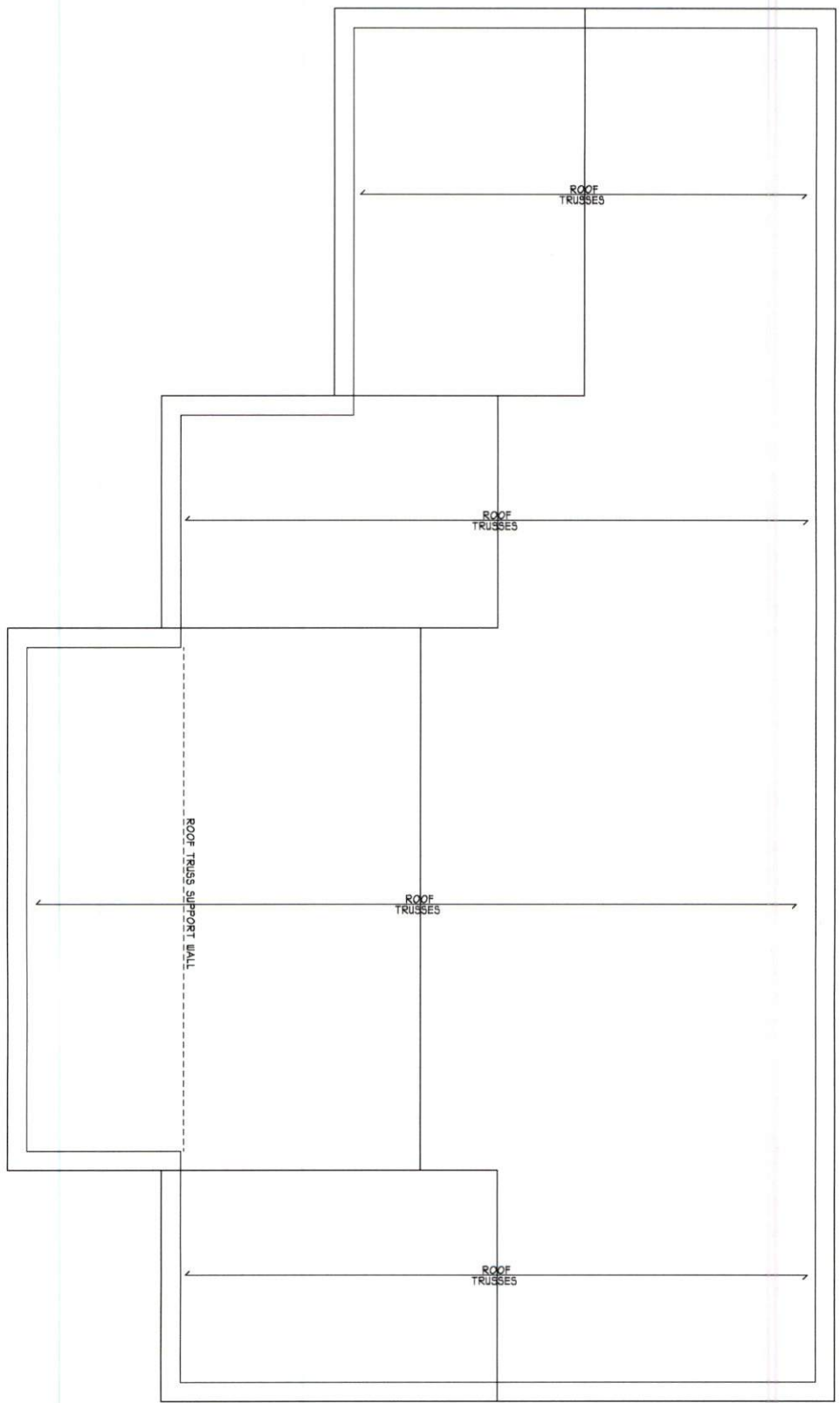
2338/SQ. = 15.2 SQ.FT. NET FREE AREA

FILE 042025	DATE 08/21/2025	ANY DEVIATION OF THE SPECIFIED MEASUREMENTS OR DIMENSIONS SHALL BE THE RESPONSIBILITY OF THE CLIENT. THIS PLAN HAS BEEN DRAWN IN ACCORDANCE WITH THE CAROLINA STATE RESIDENTIAL BUILDING CODES 2006 EDITION.	HEATHER HALL 185 HEATHERSTONE CT BENSON NC 27504 (919) 207-1403	SQUARE FOOTAGE: FIRST FLOOR = 1650 FRONT PORCH = 208 CARPORT = 480	HEATED FOOTAGE: #1650	SAM GARCIA PRIVATE RESIDENCE
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FIRST FLOOR PLAN
SCALE 1/4" = 1'-0"

FILE 042025	DATE 08/24/2025	ANY DEVIATION OF THE SPECIFIED REQUIREMENTS OR DIMENSIONS OF THIS PLAN SHALL BE THE RESPONSIBILITY OF THE CLIENT. THIS PLAN HAS BEEN DRAWN IN ACCORDANCE WITH NORTH CAROLINA STATE RESIDENTIAL BUILDING CODES 2008 EDITION.	 H SQUARED HOME DESIGN, INC.	HEATHER HALL 165 HEATHERSTONE CT BENSON NC 27504 (919) 207-1403	SQUARE FOOTAGE: FIRST FLOOR = 1650 FRONT PORCH = 208 CARPORT = 480	HEATED FOOTAGE: #1650	SAM GARCIA PRIVATE RESIDENCE
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TRUSS SYSTEM REQUIREMENTS
NO 2008 NCCCO.
1. TRUSS SYSTEM LAYOUTS (PLACEMENT PLANS)
SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW.
2. TRUSS SYSTEMS SHALL BE DESIGNED TO
RESIST ALL APPLIED LOADS AND BE PROPORTIONED
WITH THE TRUSS MANUFACTURER.
3. TRUSS SCHEMATICS (PROFILES) SHALL BE
PREPARED AND SEALED BY TRUSS
MANUFACTURER.
4. ALL TRUSSES SHALL BE DESIGNED FOR
LEADERS (MAX) 42 OR 43 PLATES OR
EQUivalent.
5. ALL REQUIRED ANCHORS FOR TRUSSES DUE
TO UPLIFT OR BEAMING SHALL MEET THE
REQUIREMENTS AS SPECIFIED ON THE TRUSS
SCHEMATICS.

SEE TO RISE DETAIL SHEET
FOR SCALED DETAIL, SPACING
DETAILS, AND STRUCTURAL NOTES

ROOF PLAN
SCALE 1/4" = 1'-0"

FILE 042025	DATE 08/24/2025	ANY DEVIATION OF THE SPECIFIED REQUIREMENTS OR DIMENSIONS Voids SHALL BE NOTED ON THE INC'S LIABILITY. THIS PLAN HAS BEEN DRAWN IN ACCORDANCE WITH NORTH CAROLINA STATE RESIDENTIAL BUILDING CODES 2008 EDITION.	 H SQUARED HOME DESIGN, INC.	HEATHER HALL 165 HEATHERSTONE CT BENSON NC 27504 (919) 207-1403	SQUARE FOOTAGE: FIRST FLOOR = 1650 FRONT PORCH = 208 CARPORT = 480	HEATED FOOTAGE: #1650	SAM GARCIA PRIVATE RESIDENCE
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STRUCTURAL NOTES

- 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF THE NORTH CAROLINA STATE RESIDENTIAL CODE - 2018 EDITION, PLUS ALL LOCAL CODES AND REGULATIONS. THE STRUCTURAL ENGINEER OR DESIGNER IS NOT RESPONSIBLE FOR, AND WILL NOT HAVE CONTROL OF, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, OR FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE CONSTRUCTION WORK, NOR WILL THE ENGINEER OR DESIGNER BE RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO CARRY OUT THE CONSTRUCTION WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. "CONSTRUCTION REVIEW" SERVICES ARE NOT PART OF OUR CONTRACT. ALL MEMBERS SHALL BE FRAMED, ANCHORED, TIED AND BRACED IN ACCORDANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BUILDING CODE.
- 2) DESIGN LOADS (R301.4)
- | | LIVE LOAD (PSF) | DEAD LOAD (PSF) | DEFLECTION (IN) |
|---|-----------------|-----------------|-----------------|
| ROOMS OTHER THAN SLEEPING ROOMS | 40 | 10 | L/360 |
| SLEEPING ROOMS | 30 | 10 | L/360 |
| ATTIC WITH PERMANENT STAIR | 40 | 10 | L/360 |
| ATTIC WITH OUT PERMANENT STAIR | 20 | 10 | L/360 |
| ATTIC WITH OUT STORAGE | 10 | 10 | L/240 |
| STAIRS | 40 | 10 | L/360 |
| EXTERIOR BALCONIES | 40 | 10 | L/360 |
| DECKS | 40 | 10 | L/360 |
| GUARDRAILS AND HANDRAILS | 200 | 10 | L/360 |
| PASSENGER VEHICLE GARAGES | 50 | 10 | L/360 |
| FIRE ESCAPES | 40 | 10 | L/360 |
| SNOW | 20 | 10 | --- |
| WIND LOAD (BASED ON 115/120 MPH WIND VELOCITY (EXPOSURE B)) | --- | --- | --- |
- 3) WALL BRACING: BRACED WALL PANELS SHALL BE CONSTRUCTED ACCORDING TO SECTION R602.10.3. THE AMOUNT AND LOCATION OF BRACING SHALL COMPLY WITH TABLE R602.10.1. THE LENGTH OF BRACED PANELS SHALL BE DETERMINED BY SECTION R602.10.4. LATERAL BRACING SHALL BE SATISFIED PER METHOD 3 BY CONTINUOUSLY SHEATHING WALLS WITH STRUCTURAL SHEATHING PER SECTION R602.10.3. NOTE THAT ANY SPECIFIC BRACED WALL DETAIL SHALL BE INSTALLED AS SPECIFIED.
- 4) CONCRETE SHALL HAVE A MINIMUM 28 DAY STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF 5 INCHES UNLESS NOTED OTHERWISE (UNO). AIR ENTRAINMENT PER TABLE 402.2. ALL CONCRETE SHALL BE PROPORTIONED, MIXED, HANDLED, SAMPLED, TESTED, AND PLACED IN ACCORDANCE WITH ACI STANDARDS. ALL SAMPLES FOR PUMPING SHALL BE TAKEN FROM THE EXIT END OF THE PUMP.
- 5) ALLOWABLE SOIL BEARING PRESSURE ASSUMED TO BE 2000 PSF. THE CONTRACTOR MUST CONTACT A GEOTECHNICAL ENGINEER AND THE STRUCTURAL ENGINEER IF UNSATISFACTORY SUBSURFACE CONDITIONS ARE ENCOUNTERED. THE SURFACE AREA ADJACENT TO THE FOUNDATION WALL SHALL BE PROVIDED WITH ADEQUATE DRAINAGE, AND SHALL BE GRADED SO AS TO DRAIN SURFACE WATER AWAY FROM FOUNDATION WALLS.
- 6) ALL FRAMING LUMBER SHALL BE SPF #2 (F_b = 875 PSI) UNLESS NOTED OTHERWISE (UNO). ALL TREATED LUMBER SHALL BE SYP #2 (F_b = 915 PSI). PLATE MATERIAL MAY BE SPF #3 OR SYP #3 (F_c = 425 PSI - MIN).
- 7) ALL WOODEN BEAMS AND HEADERS SHALL HAVE THE FOLLOWING END SUPPORTS: (1) 2x4 STUD COLUMN FOR 4'-0" MAX. BEAM SPAN (UNO), (2) 2x4 STUDS FOR BEAM SPAN GREATER THAN 4'-0" (UNO).
- 8) L.V.L. SHALL BE LAMINATED VENEER LUMBER: F_b=2400 PSI, F_v=285 PSI, E=1.9x10⁶ PSI. P.S.L. SHALL BE PARALLEL STRAND LUMBER: F_b=2400 PSI, F_v=240 PSI, E=2.0x10⁶ PSI. L.S.L. SHALL BE LAMINATED STRAND LUMBER: F_b=2250 PSI, F_v=400 PSI, E=1.55x10⁶ PSI. INSTALL ALL CONNECTIONS PER MANUFACTURER'S INSTRUCTIONS.
- 9) ALL ROOF TRUSS AND I-JOIST LAYOUTS SHALL BE PREPARED IN ACCORDANCE WITH ANY SEALED STRUCTURAL DRAWINGS. TRUSSES AND I-JOISTS SHALL BE INSTALLED ACCORDING TO THE MANUFACTURER'S SPECIFICATIONS. ANY CHANGE IN TRUSS OR I-JOIST LAYOUT SHALL BE COORDINATED WITH DESIGNER OR ENGINEER.
- 10) ALL STRUCTURAL STEEL SHALL BE ASTM A-36. STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3 1/2" INCHES AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO LAG SCREWS (1/2" DIAMETER x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOIST ARE TOE NAILED TO THE SOLE PLATE, AND SOLE PLATE IS NAILED OR BOLTED TO THE BEAM FLANGE @ 48" O.C. ALL STEEL TUBING SHALL BE ASTM A500.
- 11) REBAR SHALL BE DEFORMED STEEL, ASTM#6, GRADE 40.
- 12) FLITCH BEAMS SHALL BE BOLTED TOGETHER USING (2) ROWS OF 1/2" DIAMETER BOLTS (ASTM A307) WITH WASHERS PLACED UNDER THE THREADED END OF BOLT. BOLTS SHALL BE SPACED AT 24" O.C. (MAX), AND STAGGERED AT THE TOP AND BOTTOM OF BEAM (2" EDGE DISTANCE), WITH 2 BOLTS LOCATED AT 4" FROM EACH END.
- 13) BRICK LINTELS SHALL BE 3 1/2"x3 1/2"x4" STEEL ANGLE FOR UP TO 4'-0" SPAN AND 4"x4"x4" STEEL ANGLE WITH 4" LEG VERTICAL FOR SPANS UP TO 9'-0". SEE PLANS FOR SPANS OVER 9'-0".
- 14) THE POSITIVE AND NEGATIVE DESIGN PRESSURE FOR DOORS AND WINDOWS FOR A MEAN ROOF HEIGHT OF 35 FEET OR LESS SHALL BE 25 PSF.
- 15) THE POSITIVE AND NEGATIVE DESIGN PRESSURES REQUIRED FOR ANY ROOF OR WALL CLADDING APPLICATION NOT SPECIFICALLY ADDRESSED IN THE NORTH CAROLINA STATE RESIDENTIAL CODE - 2018 EDITION SHALL BE AS FOLLOWS:
- ROOF:
- 45.4 PSF - 2.25:12 PITCH OR LESS
 - 34.8 PSF - 2.25:12 TO 12:12 PITCH
 - 21 PSF - 12:12 TO 12:12 PITCH
- WALLS:
- 24.1 PSF - WALLS
- SEE ALSO SECTION R103.13 LINTELS

FOUNDATION STRUCTURAL NOTES:

- NC (2018 NRC):
- 1) (B) 240 SYP #2 OR SPF#2 GIRDER, TYPICAL UNO.
- 2) CONCRETE BLOCK PIER SIZE SHALL BE:
- | SIZE | HOLLOW MASONRY | SOLID MASONRY |
|-----------|----------------|-------------------|
| 8" x 8" | UP TO 32" HIGH | UP TO 5'-0" HIGH |
| 12" x 12" | UP TO 48" HIGH | UP TO 9'-0" HIGH |
| 16" x 16" | UP TO 64" HIGH | UP TO 12'-0" HIGH |
| 24" x 24" | UP TO 96" HIGH | UP TO 12'-0" HIGH |
- WITH 30" x 30" x 12" CONCRETE FOOTING, UNO.
- 3) WALL FOOTING AS FOLLOWS:
- DEPTH: 8" - UP TO 2-1/2 STORY
12" - 3 STORY
- WIDTH: SIDING (OR EQUAL)
- 12" - UP TO 2-1/2 STORY
 - 20" - 3 STORY
 - 24" - 3 STORY
- BRICK VENEER
- 12" - 1 STORY
 - 20" - 2 STORY
 - 24" - 3 STORY
- FOR FOUNDATION WALL HEIGHT AND BACKFILL REQUIREMENTS, REFER TO NORTH CAROLINA RESIDENTIAL CODE TABLE R401.1 (1) THRU (4). NOTE: ASSUMED SOIL BEARING CAPACITY = 2000 PSF. CONTRACTOR MUST VERIFY SITE CONDITIONS AND CONTACT SOILS ENGINEER IF MARGINAL OR UNSTABLE SOILS ARE ENCOUNTERED.

- 4) (A) 240 SYP#2 OR SPF#2 GIRDER.
- 5) (C) 15X12 LVL OR LSL GIRDER
- 6) (B) 15X12 LVL OR LSL GIRDER

7. "■" DESIGNATES A SIGNIFICANT POINT LOAD TO HAVE SOLID BLOCKING TO PIER. SOLID BLOCK ALL BEAR BEARING POINTS NOTED TO HAVE THREE OR MORE STUDS TO PND, TYPICAL.

8. ABBREVIATIONS:
- "S" = SINGLE JOIST
 - "D" = DOUBLE JOIST
 - "T" = TRIPLE JOIST

TRUSS SYSTEM REQUIREMENTS

- NC (2018 NRC):
1. TRUSS SYSTEM LAYOUTS (PLACEMENT PLANS) SHALL BE DESIGNED IN ACCORDANCE WITH SEALED TRUSS PROFILES. ANY NEED TO CHANGE TRUSSES SHALL BE COORDINATED WITH THE TRUSS MANUFACTURER.
2. TRUSS SCHEMATICS (PROFILES) SHALL BE PREPARED AND SEALED BY TRUSS MANUFACTURER.
3. ALL TRUSSES SHALL BE DESIGNED FOR BEARING ON SPF #2 OR #3 PLATES OR LEDGERS (UNO).
4. ALL REQUIRED ANCHORS FOR TRUSSES DUE TO UPLIFT OR BEARING SHALL MEET THE REQUIREMENTS AS SPECIFIED ON THE TRUSS SCHEMATICS.

HEADER/BEAM & COLUMN NOTES

1. ALL EXTERIOR AND LOAD BEARING HEADERS SHALL BE MIN. (2) 2x12 (1" BALL) OR (3) 2x10 (1" BALL) WITH (1) SUPPORT STUD, UNLESS NOTED OTHERWISE.
2. THE NUMBER SHOWN AT BEAM AND HEADER SUPPORTS INDICATES THE NUMBER OF SUPPORT STUDS REQUIRED IN STUD POCKET OR COLUMN. THE NUMBER OF KING STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS SHALL BE ACCORDING TO TABLE R602.15 OR AS BELOW:
- UP TO 3' SPAN: (1) KING STUD
 - OVER 3' UP TO 4' SPAN: (2) KING STUDS
 - OVER 4' UP TO 5' SPAN: (3) KING STUDS
 - OVER 5' UP TO 6' SPAN: (4) KING STUDS
 - OVER 6' UP TO 7' SPAN: (5) KING STUDS
 - OVER 7' UP TO 8' SPAN: (6) KING STUDS
 - OVER 8' UP TO 9' SPAN: (7) KING STUDS
 - OVER 9' UP TO 10' SPAN: (8) KING STUDS
 - OVER 10' UP TO 11' SPAN: (9) KING STUDS
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Comtech

Reilly Road Industrial Park, PO Box 40408
Fayetteville, NC 28309
Phone: (910) 864-8787

Reaction Summary

Job #: 250897-A

Product: Roof

Sch Del On:

Customer Information

Cash-Lenny

Address & Phone

Builder

Contact

Phone:

Job Information

Garcia Residence

Address

Garcia Residence
TBD
Benson NC

Lot

Sub-Division

Sales Person

Linwood Norris

Estimator

Customer P.O. No.

Designer

Delivery Notes:

Notes:

Loading

TCLL	TCDL	BCLL	BCDL
20	10	0	10

Spacing: 2-00-00 O.C.

Loading				Building Code	Wind Design Method	Velocity	Exp Cat	Wind Max	
TCLL	TCDL	BCLL	BCDL				Occ Cat	TCDL	BCDL
20	10	0	10	IRC2021/TPI2014	MWFRS (Envelope)/C-C hybrid Wind ASCE 7-16	130 mph	C II	6	6

Roof Trusses

Label	Profile	Qty	Span	TC Pitch	TC	Reactions							
		Ply	Height	BC Pitch	BC								
A01GE		1	40-11-00	6 /12	2 x 6	Joint 2	Joint 28	Joint 33	Joint 34	Joint 35	Joint 18	Joint 39	
		1-ply	11-00-10		2 x 6	1509	1494	171	262	184	256	256	
A02		12	40-11-00	6 /12	2 x 6	-299	-538	-43	-64	-91	0	0	
		1-ply	11-00-08		2 x 6	1511	2392						
A03GE		1	23-05-08	6 /12	2 x 6	-313	-446						
		1-ply	11-00-08	6 /12	2 x 6	Joint 2	Joint 21	Joint 20	Joint 19	Joint 18	Joint 37	Joint 28	
B01GE		1	32-11-00	6 /12	2 x 6	143	881	18	166	195	143	668	
		1-ply	9-00-08		2 x 6	0	-280	-8	-70	-92	0	-152	
B02		5	32-11-00	6 /12	2 x 6	Continuous Support							
		1-ply	9-00-08		2 x 6	Joint 2	Joint 8						
B03GE		1	32-11-00	6 /12	2 x 6	1511	1511						
		1-ply	9-00-08		2 x 6	-293	-293						
B04		3	32-11-00	6 /12	2 x 6	Continuous Support							
		1-ply	9-00-08		2 x 6	Joint 2	Joint 8						
B05		2	32-11-00	6 /12	2 x 6	1511	1511						
		1-ply	9-00-08		2 x 6	-293	-293						
B06		1	32-11-00	6 /12	2 x 6	Joint 2	Joint 8						
		1-ply	9-00-08		2 x 6	1711	1725						
B07		1	32-11-00	6 /12	2 x 6	-350	-355						
		1-ply	9-00-08		2 x 6	Joint 2	Joint 8						
C01GE		1	23-11-00	6 /12	2 x 6	1711	1685						
		1-ply	6-09-10		2 x 6	-350	-332						
C02		9	23-11-00	6 /12	2 x 6	Continuous Support							
		1-ply	6-09-08		2 x 6	Joint 2	Joint 8						
						1179	1179						
						-274	-274						



Comtech

Reilly Road Industrial Park, PO Box 40408
Fayetteville, NC 28309
Phone: (910) 864-8787

Quote

Job #: 250897-A

Product: Roof

Sch Del On:

Customer Information

Cash-Lenny

Address & Phone

Phone:

Builder

Contact

Job Information

Garcia Residence

Address

Garcia Residence
TBD
Benson NC

Lot

Sub-Division

Sales Person

Linwood Norris

Estimator

Customer P.O. No.

Designer

Notes:

Loading

TCLL	TCDL	BCLL	BCDL
20	10	0	10

Spacing: 2-00-00 O.C.

Rectangular EWP

QTY	LABEL	SIZE	LENGTH
6	BM1	1-3/4"x 11-7/8" LVL Kerto-S	21-00-00

Hangers and Fasteners

QTY	SIZE
1	JUS24
2	MSH422

Roof Trusses

Qty	Label	Profile	Span	Height	TC Pitch	TC	L-OH	L-Stub	L-Heel	Wt.	
Ply					BC Pitch	BC	R-OH	R-Stub	R-Heel	Tot. Wt.	
1	A01GE		40-11-00	11-00-10	6 /12	2 x 6	11-00	-	6-06	348	
						2 x 6	11-00	-	6-06	348	
12	A02		40-11-00	11-00-08	6 /12	2 x 6	11-00	-	6-06	269	
						2 x 6	11-00	-	6-06	3224	
1	A03GE		23-05-08	11-00-08	6 /12	2 x 6	11-00	-	6-06	210	
					6 /12	2 x 6	-	-	1-08-02	210	
1	B01GE		32-11-00	9-00-08	6 /12	2 x 6	11-00	-	6-06	260	
						2 x 6	11-00	-	6-06	260	
5	B02		32-11-00	9-00-08	6 /12	2 x 6	11-00	-	6-06	210	
						2 x 6	11-00	-	6-06	1051	
1	B03GE		32-11-00	9-00-08	6 /12	2 x 6	11-00	-	6-06	260	
						2 x 6	11-00	-	6-06	260	
3	B04		32-11-00	9-00-08	6 /12	2 x 6	11-00	-	6-06	229	
						2 x 6	11-00	-	6-06	687	
2	B05		32-11-00	9-00-08	6 /12	2 x 6	11-00	-	6-06	229	
						2 x 6	11-00	-	6-06	458	
1	B06		32-11-00	9-00-08	6 /12	2 x 6	11-00	-	6-06	229	
						2 x 6	11-00	-	6-06	229	
1	B07		32-11-00	9-00-08	6 /12	2 x 6	11-00	-	6-06	227	
						2 x 6	-	-	6-06	227	
1	C01GE		23-11-00	6-09-10	6 /12	2 x 6	11-00	-	6-06	169	
						2 x 6	11-00	-	6-06	169	
9	C02		23-11-00	6-09-08	6 /12	2 x 6	11-00	-	6-06	139	
						2 x 6	11-00	-	6-06	1247	

Roof Truss Totals:

250897-A
Accepted by Seller

Garcia Residence
Accepted by Buyer

BY: _____
TITLE: _____
DATE OF ACCEPTANCE: _____

PURCHASER: _____
BY: _____ TITLE: _____
ADDRESS: _____
PHONE: _____ DATE: _____

Sub-Total	\$7,388.31
Delivery	\$0.00
Subtotal:	\$7,388.31
Taxes:	\$498.71
TOTAL:	\$7,887.02





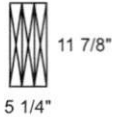
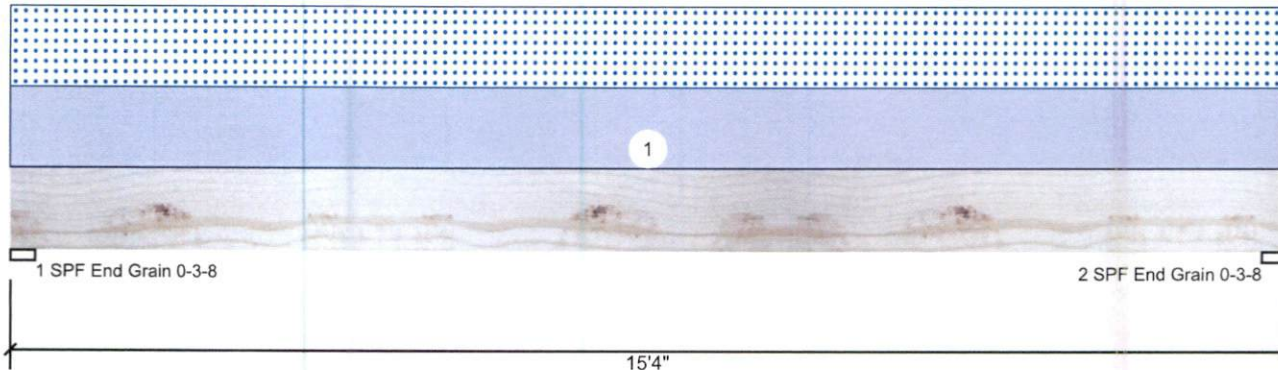
Client:
Project:
Address:

Date: 8/28/2025
Input by: Johnnie Baggett
Job Name: Garcia Residence
Project #: 250897 - A

Page 1 of 1

BM1 Kerto-S LVL 1.750" X 11.875" 3-Ply - PASSED

Level: Level



Member Information

Type: Girder
Plies: 3
Moisture Condition: Dry
Deflection LL: 480
Deflection TL: 240
Importance: Normal - II
Temperature: Temp <= 100°F

Application: Floor
Design Method: ASD
Building Code: IBC/IRC 2015
Load Sharing: Yes
Deck: Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	2368	2262	0	0
2	Vertical	0	2368	2262	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	30%	2368 / 2262	4630	L	D+S
2 - SPF End Grain	3.500"	Vert	30%	2368 / 2262	4630	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	16702 ft-lb	7'8"	35719 ft-lb	47%	D+S	L
Unbraced	16702 ft-lb	7'8"	16723 ft-lb	100%	D+S	L
Shear	3856 lb	1'3 3/8"	15295 lb	25%	D+S	L
LL Defl inch	0.237 (L/754)	7'8 1/16"	0.372 (L/480)	64%	S	L
TL Defl inch	0.485 (L/368)	7'8 1/16"	0.744 (L/240)	65%	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings.
- 2 Girders are designed to be supported on bottom edge only and across their full width.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 8' 1/4" o.c.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	295 PLF	0 PLF	295 PLF	0 PLF	0 PLF	C02
	Self Weight				14 PLF					

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

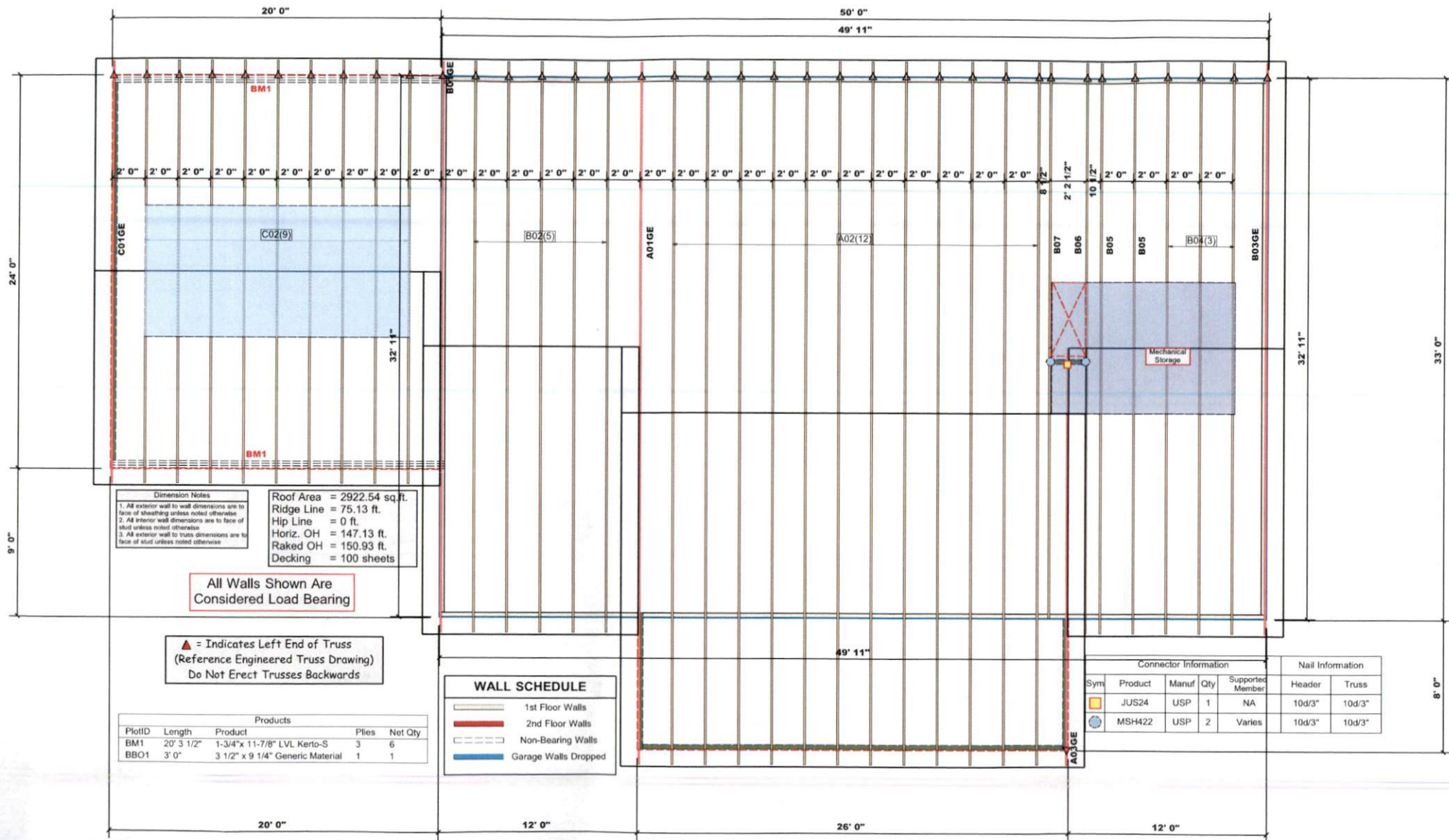
1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/28/2028

Manufacturer Info

Metsä Wood
301 Merritt 7 Building, 2nd Floor
Norwalk, CT 06851
(800) 622-5850
www.metsawood.com/us



Truss Placement Plan
SCALE: N.T.S.



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

CUSTOMER (ACCOUNT)	Cash-Lenny	STREET	TBD
(BUILDER)		CITY	Benson, NC
JOB NAME - LEVEL	Garcia Residence - Roof	TAX AUTH.	NC - Johnston
PLAN NAME		SALES REP.	Linwood Norris ()
PLAN SEAL DATE (EOR)	7/28/2025	DESIGNER (& ASST.)	Johnnie Baggett
JOB # (OT REF)	250897 - A	PLAN REV. DATE	7/30/2025

PLEASE NOTE:

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

LOAD CHART FOR JACK STUDS

