

Client: WEAVER

Project: Address: Date:

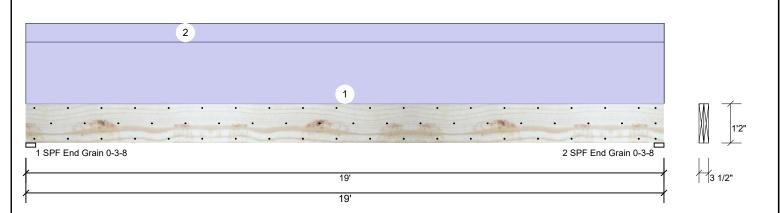
2/19/2024 Input by: LENNY NORRIS Job Name: SINCLAIR

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Project #:

Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED GDH 18' FL

Level: Level



Member Inf	formation						Reac	tions l	UNP/	ATTERN	ED I	b (Uplift)			
Type:	Girder		Applicati	on: F	loor		Brg	Direction	on	Live		Dead	Snow	Wind	Cons
Plies:	2		Design N	/lethod: A	SD		1	Vertical		0		2573	0	0	
Moisture Cond	lition: Dry		Building	Code: IF	RC 2018		2	Vertical		0		2573	0	0	
Deflection LL:	480		Load Sha	aring: N	0										
Deflection TL:	360		Deck:	N	ot Checked										
Importance:	Normal - II														
Temperature:	Temp <= 10	0°F													
							Beari	ings							
							Bear	ring Le	ngth	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb
							1 - S End		500"	Vert	25%	2573 / 0	2573	Uniform	D
Analysis Res	sults						Grai	n							
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2 - S		500"	Vert	25%	2573 / 0	2573	Uniform	D
Moment	11641 ft-lb	9'6"	24299 ft-lb	0.479 (48%) D	Uniform	End Grai								
Unbraced	11641 ft-lb	9'6"	11659 ft-lb	0.999 (100%)	D	Uniform									

Uniform

Uniform

Design Notes

Shear

1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.

0.233 (23%) D

0 999.000 (L/0) 0.000 (0%)

9'6 1/16" 0.618 (L/360) 0.772 (77%) D

17'6 1/2" 9408 lb

- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.

2191 lb

LL Defl inch 0.000 (L/999)

TL Defl inch 0.477 (L/466)

- 6 Top must be laterally braced at a maximum of 8'11 5/16" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

	Lateral Sicrider	ness ratio based on single	pry wiatri.									
II)	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1		Uniform			Тор	200 PLF	0 PLF	0 PLF	0 PLF	0 PLF	GABLE END	
2		Uniform			Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	DEAD WALL	
		Self Weight				11 PLF						

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used.

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info



This design is valid until 6/28/2026





Client: WEAVER

Application:

Design Method:

Building Code:

Load Sharing:

Deck:

Floor

ASD

No

IRC 2018

Not Checked

Project: Address: Date: 2/19/2024

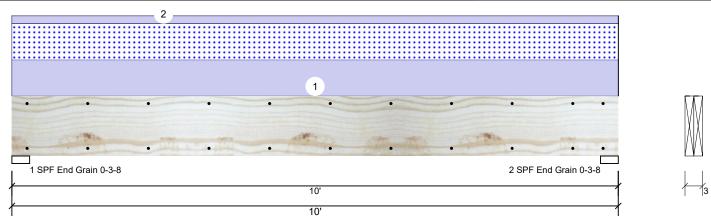
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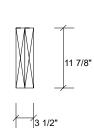
Project #:

GDH 9' FL **Kerto-S LVL** 1.750" X 11.875"

2-Ply - PASSED

Level: Level





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Member Information

Type: Girder Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature:

Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1746	1400	0	0
2	Vertical	0	1746	1400	0	0

Bearings

Grain

Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.500" 1746 / 1400 D+S Vert 3146 L End Grain 2 - SPF 3.500" 1746 / 1400 3146 L D+S Vert End

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	7161 ft-lb	5'	22897 ft-lb	0.313 (31%)	D+S	L
Unbraced	7161 ft-lb	5'	9721 ft-lb	0.737 (74%)	D+S	L
Shear	2349 lb	1'3 3/8"	10197 lb	0.230 (23%)	D+S	L
LL Defl inch	0.062 (L/1838)	5'	0.239 (L/480)	0.261 (26%)	S	L
TL Defl inch	0.140 (L/818)	5'	0.318 (L/360)	0.440 (44%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	280 PLF	0 PLF	280 PLF	0 PLF	0 PLF	C1 TRUSS
2	Uniform			Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	DEAD WALL
	Self Weight				9 PLF					

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