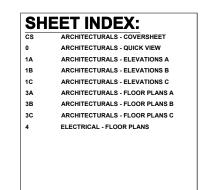
WILMINGTON -A, B, C

PLAN ID: 2800 - RIGHT HAND - NORTH CAROLINA

DATE: **REVISION:** 09/18/2017 **INITIAL RELEASE OF PLANS** 10/20/2017 CLIENT REVISIONS 11/01/2017 REMOVED PORCH RAILING FROM ELEVATION 'C' FLATTENED BAR TOP AT KITCHEN REVISED SIZE OF WINDOW AT BASE OF STAIRS REVISED MASTER BEDROOM TO OWNER'S BEDROOM 02/07/2018 **ELECTRICAL REVISIONS** 06/11/2018 CLIENT REVISIONS 11/14/2018 **CLIENT REVISIONS** 01/09/2019 **REVISED CODE REFERENCES** 07/23/2019 **CLIENT REVISIONS** 12/13/2019 **CLIENT REVISIONS** 02/28/2020 CLIENT REVISIONS



REVIEWERS STAMP	LOCATION	

MODEL 'WILMI	NGTON' SQUARE FOC	DTAGES
AREA		ELEV 'C'
lst FL <i>OO</i> R		1225 SF
2nd FLOOR		1595 SF
TOTAL LIVING		2824 SF
GARAGE		411 SF
PORCH	· .	72 SF

Woodgrove Lot 81 92 Pecan Grove Lane Fuquay Varina, NC 27526



WILMINGTON

COVERSHEET

PLAN REV DATE

COPYRIGHT PROPERTY OF DR
HORTON NOT TO BE REPRODUCE
SHEET NUMBER

CS





Front Elevation 'B' scale. 1/4'=1-0' at 22"x34' LAYOUT 1/8'=1-0' at 11"x17" LAYOUT

Front Elevation C

SCALE: I/4"=I'-O" AT 22"X34" LAYOUT I/8"=I'-O" AT II"XIT" LAYOUT

QUICK VIEW
"WILMINGTON"

America's Builder

D-R-HORTON

PLAN REV DATE 02.28.20

COPPERINT PROPERTY OF DE HUNCOUNCE!

SHEET NUMBER

N.C ATTIC VENT CALCULATION FOR MODEL 'WILMINGTON': 1:150 RATIO B" TYP EAVE HE NET FREE VENTILATING AREA SHALL NOT DE LESS THAN 150 OF THE AREA OF THE SPACE VISHTILATED, PROVIDED WITH AT LECATE OF PRECISIT AND WITH MADE THAN BO FRECISIT WITH AT LECATE OF THE UPPER PORTION OF THE SPACE OF WITH LATORS LOCATED IN THE UPPER PORTION OF THE SPACE OF WITH LATOR AT LEAST 3 THEFT ADON'T THE EARLY OR CRINICE VISTO SWITH THE BALLANCE OF THE REALE OR ORNICE VISTO SWITH THE BALLANCE OF THE REALE OR REPULLATION FROM THE SPACE OF THE REPULLATION OF THE SPACE OF THE SWITH THE SPACE OF THE SPACE OF THE SWITH THE PRINCIPLE OF THE SWITH THE SWITH THE SWITH THE PRINCIPLE OF THE SWITH THE SWITH THE PRINCIPLE OF THE SWITH THE SWITH THE PRINCIPLE OF THE PRINCIPLE OF THE SWITH THE PRINCIPLE OF THE PRINCIPLE O (PER NCRC SECTION R806.2) 1 SQUARE INCH VENT FOR EVERY ISO SQUARE INCHES OF CEILING 144 SQ. IN. = I SQ. FT. BLDG. CEILING (SF) X 144 = BLDG (SQ. IN.) 9LDG, (5Q, IN) / 150 = 5Q, IN, OF VENT REQUIRED 5Q, IN, OF VENT REQUIRED / 2 = 50% AT HIGH & 50% AT LOW. EXCEPTIONS I. EXCLOSED ATTIC/RAFTER SPACES REQUIRING LESS THAN I. SQ FT OF VENTILATION MAY BE VENTED WITH CONTINUOUS SOFFIT VENTILATION ONLY. ROOF AREA Is = 1787 SF 1636 SQ, FT, X 144 = 235584 SQ, IN, 235584 SQ, IN, / ISO = ISTO.56 SQ, IN, OF VENT REQID P. ENGLOSED ATTIC/RAFTER SPACES OVER UNCONDITIONED SPACE MAY BE VENTED WITH CONTINUOUS SOFFIT VENT ON 1570.56 SQ. IN. / 2 = 785.28 SQ. IN 785.28 SQ. IN. 0F VENT AT HIGH & 785.28 SQ. IN. 0F VENT AT LOW REGUIRED. SPACE PAY BE VERIFOR SHALL VERIFY THE NET PREE WHITLAND OF THE VERIT PRODUCT SELECTED BY OWNER WERFIY WHIT MAPACHIERS OF HICH AND LOW VERIFS TO BE USED FOR HIMMAN CALCULATED VERIFS REGUIRED. THE REGUIRED VERIFICATION SHALL BE MAINTAINED. PROVIDE INSULATION STOP SUCH THAT INSULATION DOCUMENT THE AND AND VERY BE ADMINISTRATION. ROOF AREA 2: = 12 SF 72 SQ. FT. X 144 = 10368 SQ. IN. 10368 SQ. IN. / 150 = 64.12 SQ. IN. OF VENT REQ'D 69.12 5Q. IN. / 2 = 34.56 5Q. IN 34.56 5Q. IN. OF VENT AT HIGH & 34.56 5Q. IN. OF VENT AT LOW REQUIRED. ZES NOT OBSTRUCT FREE AIR MOVEMENT AS REQUIRED. THE BULLIONS OFFICIAL. L OVERLAP FRAMED ROOF AREAS SHALL HAVE REPRINGS BETREEN THE ADJACENT ATTICS IN THE ROOF BATHING IN ALLOWED BY THE STRUCTURAL ENGINEER! ALLOH PASSACE AND ATTIC SYNILATION ALLOH PASSACE AND ATTIC SYNILATION VENTED INDEPENDENTLY TO GE REQUIREPENTS. ER DEVELOPER, AT ALL CANTILEVERED FLOORS, ANTILEVERED ARCHITECTURAL POP-OUTS, AND ANY DOUBLE RAMING PROJECTIONS THAT ARE SEPARATED FROM THE ENTING CALCULATIONS SHOWN ABOVE, PROVIDE A OWTHINGUS 2" CORROSION RESISTANT SOFFIT VENT AT WERSIDE OF FRANKED LEIDHENT. Left Elevation 'C' OTFS. AREA I ALL ROOF DRAINAGE SHALL BE PIPED TO STREET OR APPROVED DRAINAGE FACILITY. TRIJSS MANUFACTURER SHALL SUBMIT STRUCTURAL CALCS AND SHOP DR TO THE BUILDIES'S SENERAL CONTRACTOR AND BUILDING DEPARTMENT FOR REVIEW PRIOR TO FABRICATIONS. drainage facility. Dashed Lines indicate Wall Below. Locate gutter and downspouts per builder Pitched Roofs as Noted. ALL PLIMBING VENTS SHALL BE COMBINED INTO A MINIMA AMOUNT OF ROOF PENETRATIONS, ALL ROOF PENETRATIONS SHALL OCCUR TO THE REAR OF THE MAIN RIDGE. N.C ATTIC VENT CALCULATION FOR MODEL 'WILMINGTON': 1:300 RATIC AS AN ALTERNATE TO THE I/ISO RATIO LISTED ABOVE, HE NET FREE CROSS-VENTILATION AREA MAY BE REDUCED TO I/300 NEMA CLASS I OF II VAPOR RETARDER IS INSTALLE ON THE WARM - IN - WINTER SIDE OF THE CEILING. (PER NCRC SECTION R806.2) I SQUARE INCH VENT FOR EVERY 300 SQUARE INCHES OF CEILING *144 SQ. IN. = 1 SQ. FT. BLDG. CEILING (SF) X 144 = BLDG (SQ. IN.) SENERAL CONTRACTOR SHALL VERIET THE INT PREE WHITH LATION OF THE VIPOT THE INST PREE WHITH LATION OF THE VIPOT PRODUCT SELECTED BY ONER, WHITH LATION OF THE LATION OF THE LESS PRODUCT SELECTED BY ONER FOR LESS PRODUCT SELECTED THE VIPOT SEGUIFED. THE REQUIRED, VERIFICATION SHALL BE HARMATICALLY DOES INTO OBSTRICT FREE ARE NOVEMENT AS REQUIRED BY THE BUILDING SOFTICAL. ALL OVERLAP FRAMED ROOF AREAS SHALL HAVE PROPINGE SETURED HE ROJACHET ATTOS IN THE ROOF EACH THIN OF A LAUGHD BY THE STRICTURAL DEVINEED OF LIVOT FOR SHALL MAY BE SHALL WAS AND AND THE STRICTURAL DEVINEED OF LIVOT FOR SHALL MAY BE SHALL WAS AND AND THE STRICTURAL DEVINEED OF LIVOT FOR SHALL MAY BE SHALL WAS AND AND THE STRICTURAL DEVINEED OF LIVOT FOR SHALL MAY BE SHALL WAS AND AND THE STRICTURAL DEVINEED OF LIVOT FOR SHALL MAY BE SHALL WAS AND AND THE STRICTURAL DEVINEED OF LIVOT FOR SHALL MAY BE SHALL WAS AND AND THE SHALL WAS SHALL BE SHALL WAS AND AND AND THE STRICTURAL SHALL BE SHALL WAS AND AND AND AND THE SHALL WAS SHALL BE SHALL WAS AND AND AND AND THE SHALL WAS SHALL BE SHALL WAS AND AND AND AND THE SHALL WAS SHALL BE SHALL WAS AND AND AND AND AND AND AND AND THE SHALL WAS SHALL WAS BELIEVED BY DOORS. BLDG. (SQ. IN.) / 300 = SQ. IN. OF VENT REQUIRED SQ. IN. OF VENT REQUIRED / 2 = 50% AT HIGH & 50% AT LOW. ROOF AREA I: = 1636 SF 6:12 SLOPE 1636 Sc. Ft. X 144 = 295564 Sc. IN. 235564 Sc. IN. 300 = 185.26 Sc. IN. OF VENT REQD 185.26 Sc. IN. 2 = 392.64 Sc. IN. OF VENT AT LOW REQUIRED. 342.64 Sc. IN. OF VENT AT HIGH & 342.64 Sc. IN. OF VENT AT LOW REQUIRED. SLOPE ROOF AREA 2: = 72 SF 72 5Q. FT. X 144 = 10368 5Q. IN. 10368 5Q. IN. / 300 = 34:56 5Q. IN. 0F VENT REQTO 34:56 5Q. IN. / 2 = 17:28 5Q. IN SET FORM THE PROPERTY AT ALL CAMILLEVERED FLOORS, CAMILLEVERED ACCHITECTURAL POP-JOTS, AND ANY DOUBLE FROM THE CHARLEVERED ACCHITECTURAL POP-JOTS, AND ANY DOUBLE FROM THE CHARLEVER ACCITIONS THAT LARE SEPARATED FROM THE VEHING CALCULATIONS SHOWN ABOVE, PROVIDE A CONTINUOUS 2° CORROSION RESISTANT SOFFIT VEHIT AT MOREPOSIC OF FRAMED ELEMENT. 17.28 SQ. IN. OF VENT AT HIGH & 17.28 SQ. IN. OF VENT AT LOW REQUIRED. Right Elevation 'C' OTES: GRADE CONDITIONS MAY VARY FOR INDIVIDUAL SITE FROM THAT SHOWN. BUILDER SHALL VERIFY AND COORDINATE PER ACTUAL SITE CONDITIONS. AVAILABLE WITH OPTIONAL WINDOW HEAD HEIGHTS: IST FLOOR = 6-8" U.N.O. ON ELEVATIONS. 2ND FLOOR = 7'-0" U.N.O. ON ELEVATIONS. 9'-1" FIRST FLOOR PLATE AT SINGLE FAMILY DETACHED PLANS: PREFINISHED VENIED SOFFIT AT EAVE PER MANUFACTURER. (VERIEY FIRE SEPARATION DISTANCE FOR SOFFIT PROTECTION PER NORC SECTION R302.1.1 AND TABLE R302.1) ROOFING: PITCHED SHINGLES PER DEVELOPER NOTES AT OPT 9'-1" PLT: WINDOWS: MANUFACTURER PER DEVELOPER, DIVIDED LITES AS SHOWN ON THE EXTERIOR ELEVATIONS - WDW HT SET AT 7'-6' ENTRY DOOR: AS SELECTED BY DEVELOPER. GARAGE DOORS: AS SELECTED BY DEVELOPER, RAISED PANEL AS SHOWN. - INTERIOR SOFFITS AT 8'-0" ALL EXTERIOR MATERIALS TO BE INSTALLED PER MANUFACTURER'S WRITTEN INSTRUCTIONS. - EXTERIOR SOFFITS AT 8'-0' PROTECTION AGAINST DECAY: (ALL PORTIONS OF A PORCH, SCREEN PORCH OR DECK FROM THE BOTTOM OF THE HEADER DOWN, INCLIDING POST, RAILS, PICKETS, STEPS AND FLOOR STRUCTURE.) INGULATION: PER TABLE NIO2.1.2. EXTERIOR WALLS: CELING WITH ATTIC ABOVE: FLOOR OVER GARAGE: R-49 BATTS MINIMM. VERIFY R-49 BATTS MINIMM. VERIFY AREA 2 . 12:12 PITCH ATTIC KNEEWALL: R-I9 BATTS MINIMUM, VERIFY CRAWL SPACE FLOORING: R-I9 BATTS MINIMUM, VERIFY Roof Plan 'C' **KEY NOTES:** MASONRY: ADHERED STONE VENEER AS SELECTED BY DEVELOPER, HEIGHT AS NOTED MASONRY FULL BRICK AS SELECTED BY DEVELOPER, HEIGHT AS NOTED. TRUSS MANUFACTURE TO MASONRY FULL STONE AS SELECTED BY DEVELOPER, HEIGHT AS NOTED. VERIFY HEFI S PER COMMUNITY STANDARDS, BUILDER TO VERIFY PRIOR TO CONSTRUCTION 8" SOLDIER COURSE. ROWLOCK COURSE TYPICALS: MDM HD CORROSION RESISTANT SCREEN LOWERED VENTS, SIZE AS NOTED. 6" PEDIMENT CODE APPROVED TERMINATION CHIMNEY CAP. 17 12X6O CORROSION RESISTANT ROOF TO WALL FLASHING, CODE COMPLIANT FLASHING PER NCRC R405.2.8.3 16 IX4 O STANDING SEAM METAL ROOF, INSTALL PER MANUFCATURER'S WRITTEN INSTRUCTIONS. DECORATIVE WROUGHT IRON, SEE DETAILS. SIDING: VINYL SHAKE SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. 9-/ (AT SPECIFIED LOCATIONS: FIBER CEMENT SHAKE SIDING PER DEVELOPER W/ IX4 CORNER TRIM BOARD.) VINYL LAP SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT LAP SIDING PER DEVELOPER W IX4 CORNER TRIM BOARD.) WDW HD VINYL WAVY SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT WAVY SIDING PER DEVELOPER W IX4 CORNER TRIM BOARD.) VINYL BOARD AND BATT SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS; FIBER CEMENT PANEL SIDING W IX3 BATTS AT 12° O.C. PER DEVELOPER W IX4 CORNER TRIM BOARD.) VINYL TRIM SIZE AS NOTED (AT SPECIFIC LOCATIONS: IX FIBER CEMENT TRIM OR EQUAL, UN.O. SIZE AS NOTED PYPON SHUTTERS, TYPE AS SHOWN, SIZE AS NOTED. (AT SPECIFIC LOCATIONS: FALSE VINYL SHUTTERS, TYPE AS SHOWN, SIZE AS NOTED.) Rear Elevation 'C' ALL MINDOMS MHOSE OPENING IS LESS THAN 24" ABOVE HE FINISH FLOOR AND WHOSE OPENING IS GREATER THAN 12" ABOVE THE CUTSIDE WALKING SURFACE MUST HAVE WINDOM OPENING LIMITING DEVICES COMPLYING WITH THE ICRC SECTION R312.21 AND R312.22. Front Elevation

N ш Σ 0 I

> WILMINGTON ELEVATIONS

IX6 RAKE

FASCIA

-[13]

IX6 16

FRIEZE 16

–[12]

OPTIONAL 2868

4XI2 BRACKETS

DATE REV AN PL,

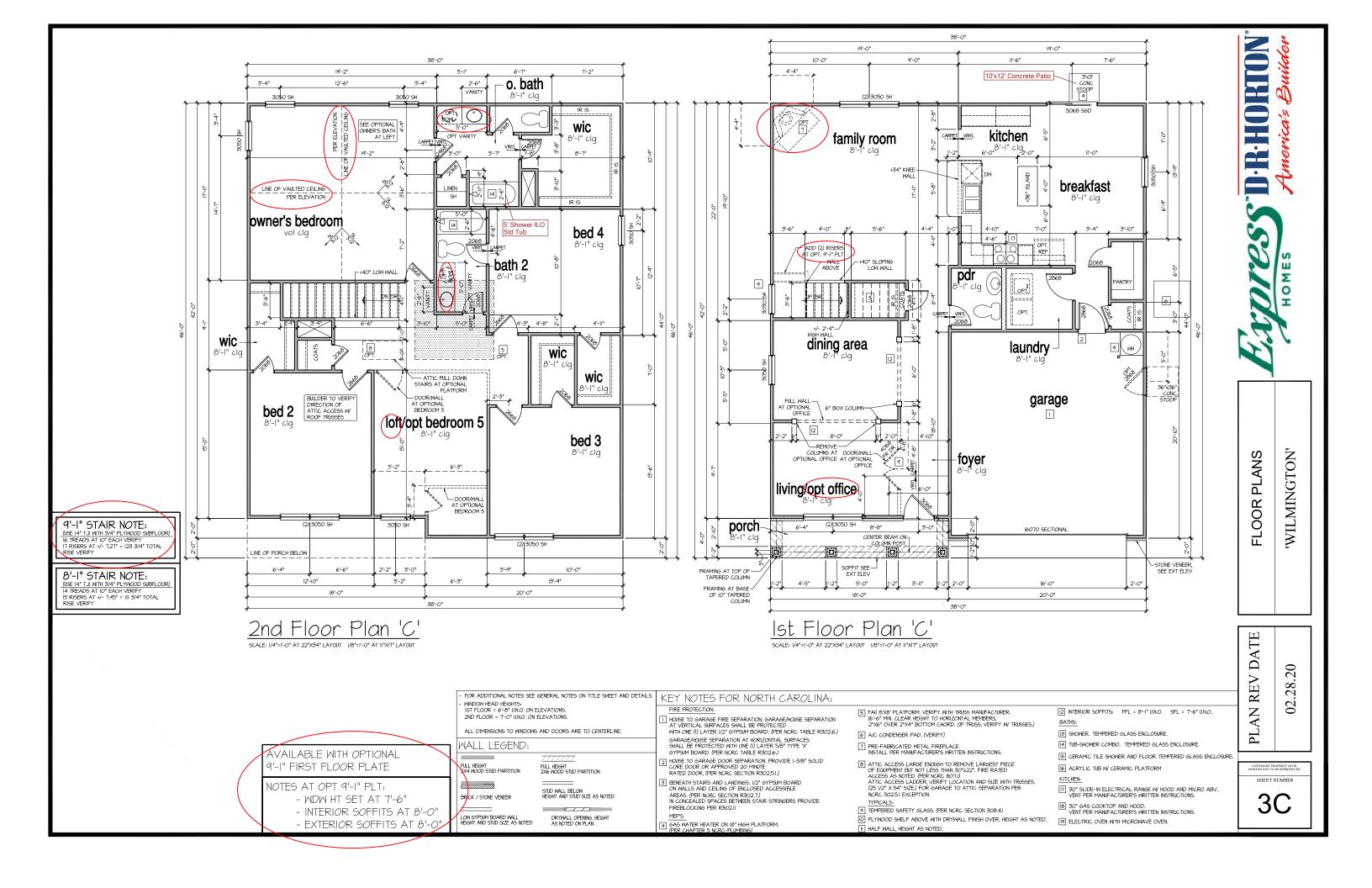
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02

SHEET NUMBER





'WILMINGTON' FLOOR PLANS

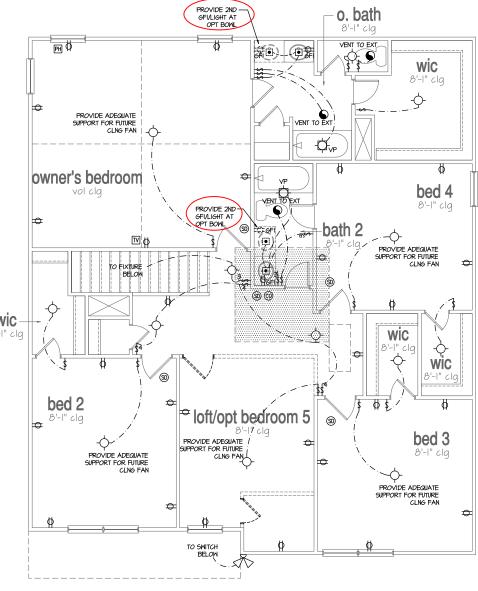
PLAN REV DATE .28.20 02.

SHEET NUMBER

4

ALL ELEVATIONS

ARE SIMILAR



PH O GFI kitchen family room Defi BELOW FOR DEFN breakfast PROVIDE ADEQUATE SUPPORT FOR FUTURE CLNG FAN T<u>₽</u>6FI | D ABOVE FOR HOOD/ MICRO pdr 50 **⊕** 220V A/C DISCONNECT, 30" MIN. CLEAR \$\$ & **b** PROVIDE WP/GFI PER LOCAL CODE WP/GFI dining area laundry 8'-1" clq garage KEYLESS -KEYLESS O PREWIRE ONLY foyer living/opt office NOTE: SIZE SERVICE PANEL PER BUILDERS SPECIFICATIONS AND LOCAL CODES ф \$ \$\$\$ ф -wP/6Fi **→** COACH LIGHT, CENTERLINE 6'-0" A.F.F. COACH LIGHT TO FLOOD ABOVE

2nd Floor Plan 'A'
scale. 1/4'=1'-0" AT 22"x34" LAYOUT 1/8'-1'-0" AT 11"x17" LAYOUT

<u>Ist Floor Plan 'A'</u>

porch-8'-1" clg

wic - PROVIDE AND INSTALL ARC FAULT CIRCUIT-INTERRUPTERS (AFCI) AS REQUIRED BY NATIONAL ELECTRICAL CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.

ALL EXHAUST FANS SHALL HAVE BACKDRAFT DAMPERS. FAWLIGHTS IN WET/DAMP LOCATIONS SHALL BE LABLED "SUITABLE FOR WET OR DAMP LOCATIONS. ELECTRICAL SYSTEMS ARE SHOWN FOR INTENT ONLY. THESE SYSTEMS SHALL BE ENGINEERED BY OTHERS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER INSTALLATION AND PLACEMENT PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. PROVIDE AND INSTALL GROUND FAULT CIRCUIT-INTERRUPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRICAL CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.

ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS. HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS.

PROVIDE GROUNDING ELECTRICAL ROD PER LOCAL CODES.

ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FURNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUMP PITS, DRAIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS.

PROVIDE POWER, LIGHT AND SWITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S WRITTEN INSTRUCTIONS.

LEGEND:

DUPLEX OUTLET	
### GROUND-FAULT CIRCUIT-INTERRIPTER DIPLEX OUTLET HALF-SHITCHED DUPLEX OUTLET P220V 220 VOLT OUTLET RENFORCED JUNTION BOX WALL SHITCH SHITCH	
GROUD-PAULT CIRCUT-INTERRIPTER GROUD-PAULT CIRCUT-INTERRIPTER HALF-SHITCHED DUPLEX OUTLET CELLING MOUNTED LED CELLING MOUNTED LED	
HALF-SHITCHED DUPLEX CUTLET	
□ LIGHT FIXTURE (VP) = VAPOR PROOF □ ENHANCE AND LIGHT FIXTURE (VP) = VAPOR PROOF □ ENHANCE AND LIGHT COMBINATION □ ENHANCE FAVOR IT O EXTERIOR □ ENHANCE FAVOR IT O EXTERIOR □ ENHANCE FAVOR IT O EXTERIOR □ ENHANCE FAVOR IT OF EXTER	
\$ MALL SMITCH BHAUST FAMULISHT COMBINATION (VENT TO EXTERIOR) \$ THREE-MAY SMITCH \$ FOUR-MAY SMITCH TEXT MER SYSTEM TEXT MER SYSTEM	
\$ 1 THREE-WAY SWITCH FLUORESCENT LIGHT FIXTURE \$ 4 FOUR-WAY SWITCH TEVAL WIR SYSTEM	
\$3 THREE-WAY SWITCH FLUORESCENT LIGHT FIXTURE \$4 FOUR-WAY SWITCH TEXT HER SYSTEM	
\$4 FOUR-MAY SMITCH	
TECH HUB SYSTEM	
Sil 125	
PUSHBUTTON SMITCH CEILING FAN (PROVIDE ADEQUATE SUPPORT)	
IIOV SMOKE ALARM W BATTERY BACKUP CEILING FAN WITH INCANDESCENT	
69 IIOV SMOKE ALARM CO2 DETECTOR COMBO LIGHT FIXTURE (PROVIDE ADEQUATE SUPPORT)	
① THERMOSTAT	
PH TELEPHONE	
™ TELEVISION +HB HOSE BIBB	
ELECTRIC METER TUB OUT	
ELECTRIC PANEL	

DESIGN SPECIFICATIONS:

Construction Type: Commerical ☐ Residential ☒

Applicable Building Codes:

• 2018 North Carolina Residential Building Code with All Local Amendments

• ASCE 7-10: Minimum Design Loads for Buildings and Other Structures

9,, -	oucio.			
٦.	Roof	Live Loads		
	1.1.	Conventional 2x	20	PS
	1.2.	Truss	20	PS
		12.1. Attic Truss	60	P
2.	Roof	Dead Loads		
	2.1.	Conventional 2x	10 F	-91
	22.	Trues	20	PS
3.	Snow		15 F	-SF
	3.1.	Importance Factor	IØ	
4.		Live Loads		
	4.1.	Typ. Dwelling	40	Pe
		Sleeping Areas		
	4.3.	Decks	40	P
		Passenger Garage		

5. Floor Dead Loads 5.1. Conventional 2x . 5.2. I-Joist IØ PSF 5.3. Floor Truss ... Ultimate Design Wind Speed (3 sec. gust)
 Exposure
 Importance Factor 130 MPH

631. VX = 632. Vy = 7. Component and Cladding (in PSF)

63 Wind Base Shear

MEAN ROOF HT.	UP TO 30°	30'1"-35'	35'1"-40'	40'1"-45'
ZONE I	16.7,-18.0	17.5,-18.9	18.2,-19.6	18.7,-202
ZONE 2	16.7,-21.0	17.5,-22.1	182,-22.9	8. 7,-23.5
ZONE 3	16.7,-21.0	17.5,-22.1	182,-22.9	18 .7,-23.5
ZONE 4	182,-19.0	192,-20 0	19.9,-20.7	20.4,-21.3
ZONE 5	182,-24.0	19.2,-25.2	19.9,-26.1	20.4,-26.9

Seismi	c	
8.1.	Site Class	₽
8.2.	Design Category	Ç
8.3.	Importance Factor	Ø
	Seismic Use Group	1
	Spectral Response Acceleration	
	8.5.1. Sms = %q	

8.52.5ml = %g 8.6. Seismic Base Shea 861. Vx =

8.6.2.Vy = 8.1. Basic Structural Sustem (check one)

□ Bearing Wall
 □ Building Frame
 □ Moment Frame

□ Dual w/ Special Moment Frame
□ Dual w/ Intermediate R/C or Special Stes ☐ Inverted Pendulum

8.8. Arch/Mech Components Anchored 8.9. Lateral Design Control: Seismic
Assumed Soil Bearing Capacity



STRUCTURAL PLANS PREPARED FOR:

WILMINGTON - RH

PROJECT ADDRESS:

OWNER: DR Horton, Inc. 8001 Arrowridge Blvd. Charlotte, NC 28273

DESIGNER: GMD Design Group 102 Fountain Brook Circle **C**ary, NC 27511

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory & Testing, P.C. before construction begins.

PLAN ABBREVIATIONS:

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	9C	STUD COLUMN
CLR	CLEAR	ಕ್ರ	SINGLE JOIST
DJ	DOUBLE JOIST	5PF	SPRUCE PINE FIR
D9P	DOUBLE STUD POCKET	551	SIMPSON STRONG-TIE
EE	EACH END	SYP	SOUTHERN YELLOW PINE
ΕW	EACH WAY	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
ОС	ON CENTER	TYP	TYPICAL
P9F	POUNDS PER SQUARE FOOT	uno	UNLESS NOTED OTHERWISE
P9I	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory 4 Testing, P.C. (SUMMIT) prior to the Initial design. Therefore, truss and joist directions were assumed based on the information provided by <u>DR Horton</u>, <u>Subsequent plan</u> revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify SUMMIT immediately.

SHEET LIST:

Sheet No.	Des c ription	
CSI	Cover Sheet, Specifications, Revisions	
S1.Øm	Monolithic Slab Foundation	
51.Øs	Stem Wall Foundation	
51.Øc	Crawl Space Foundation	
51.0b	Basement Foundation	
S2.Ø	Basement Plan	
63.Ø	First Floor Plan	
54.0	Second Floor Plan	
55.Ø	Roof Framing Plan	

DR HORTON PROJECT SIGN-OFF:

Mana g er	Signatur e
Operations	
Operations System	
Operations Product Development	

REVISION LIST:

Revision No.	Date	Project No.	Descri p tion
1	5.16.17	1261IR	Revised garage slab note. Revised roof overframing. Verified roof truss layouts provided by 84 Lumber on 3.28.11. Verified floor Joist layouts provided by 84 Lumber on 82.15
2	6.14.17	12611R2	Added stem wall foundation plan
3	4.23.18	17862	Added crawl space foundation plan
4	7.10.18	17862R	Revised per new architectural files dated 6.12.18
5	8.30.18	17862R2	Added dimensions at taped porch columns
6	10.5.18	17862R3	Included stick framing option at extended parch
1	11.30.18	17862R4	Revised NC version only for 2018 NCRC
8	3.1.21	TØØ9I	Added OX-16 Structural Insulated Sheating Option

summit

over framing. Apply building paper over the sheathing as required by the state Building Code.

Wood floor sheathing shall be APA rated sheathing exposure I or 2. Attach sheathing to its supporting framing with (I)-8d CC ringshark nail at 6"or a to panel edges and at 12"or in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing, Sheathing shall have a span rating consistent with the framing spacing. We suitable edge support by use of 14G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the

Sheathing shall have a I/B" gap at panel ends and edges as recommended in accordance with the APA.

Apply building paper over the sheathing as required by the

Wood wall sheathing shall comply with the requirements of local

information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.

Roof sheathing shall be APA rated sheathing exposure 1 or 2.

Roof sheathing shall be continuous over two supports and attached to this supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing, like

suitable edge support by use of plywood ellips or lumber blocking unless otherwise noted. Panel end Joints shall occur over framing. Apply building paper over the sheathing as

Roof sheathing shall be continuous over two supports and

building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more

TRUCTURAL FIBERBOARD PANELS:

state Building Code.

- Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.
 All structurally required fiberboard sheathing shall bear the
- mark of the AFA.

 Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more
- Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

GENERAL STRUCTURAL NOTES:

- The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise alter, or delete any structural aspects of these construction documents without written permission of SUPMIT Engineering, Laboratory 4 Testing, P.C. (SUPMIT) or the SER. For the purposes of these construction documents the SER and SUPMIT
- shall be considered the same entity.

 The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
- The SER is not responsible for construction sequences, methods or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur.

 Any structural elements or details not fully developed on the
- construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of
- the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.

 Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions fo of the SEK. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically
- noted on the structural drawings.

 This structure and all construction shall conform to all
- applicable sections of the international residential code. This structure and all construction shall conform to all applicable sections of local building codes. All structural assembles are to meet or exceed to requirements
- of the current local building code.

FOUNDATIONS:

The structural engineer has not performed a subsurface responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be

- The bottom of all footings shall extend below the frost line for The bottom of all footings scale extend below to find the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12' below grade. Any fill shall be placed under the direction or recommendation
- of a licensed professional engineer.

 The resulting soil shall be compacted to a minimum of 95% maximum dry clereity.

 Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur
- within 24 hours of excavation. No concrete shall be placed against any subgrade containing

- STRUCTURAL STEEL:

 1. Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design" latest editions.
- Structural steel shall receive one coat of shop applied

water, ice, frost, or loose materia

- rust-inhibitive paint.
 All steel shall have a minimum yield stress (F_n) of 36 ksi unless
- when the state of for shop and field welding shall be class ETOXX. All welding shall be performed by a certified welder per the above

- NUMBELE:
 Concrete shall have a normal weight aggregate and a minimum compressive strength (Fe) at 28 days of 3000 psi, unless otherwise noted on the plan.
 Concrete shall be proportioned, mixed, and placed in
- accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings"
- Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to 42% of target values as follows: 3.1. Footings: 5% 3.2. Exterior **S**labs: 5%

No admixtures shall be added to any structural concrete without written permission of the SER.

- Concrete slabs-on-grade shall be constructed in accordance with ACI 302.1R-96: "Guide for Concrete Slab and Slab Construction".
- The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported
- cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions. Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 10°-0° DC, and in exterior slabs-on-grade at a maximum of 10°-0° unless otherwise noted. Control or saw cut joints shall be produced using conventional
- process within 4 to 12 hours after the slab has been finished
- process within 4 to 12 nours after the shap has been finished Reinforcing steel may not extend through a control Joint.

 Reinforcing steel may extend through a saw cut Joint.

 All welded wire fabric (www.) for concrete slabs-on-grade shall be placed at mild-depth of slab. The WWF, shall be securely supported during the concrete pour.

- CONCRETE RENFORCEMENT:

 1. Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and themal expansion/contraction, lowered water migration, an increase in impact capacity, increased
- abrasion resistance, and residual strength.
 Fibernesh reinforcing to be 120% virgin polypropylene fibers
- Filbermesh reinforcing to be 100% virgin polypropylene liners containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement. Application of filbermesh per cubic yard of concrete shall equal a minimum of 0% by volume (15 pounds per cubic yard) Filbermesh shall comply with ASTM CIII6, any local building code requirements, and shall meet or exceed the current industry
- Steel reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- ASIM A615, grade 60.

 Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures*
 Horizontal footing and wall reinforcement shall be continuous
 and shall have 90° bends, or comer bars with the same
 size/spacing as the horizontal reinforcement with a class B
- Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.

- 9. Where reinfarcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters
- into the footing. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.
- WOOD PRAINING:

 1. Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS), Unless otherwise noted, all wood framing members are designed to be Southern-Yellow-Pine (SYP) 2 or Southm-Spruce Pine (SYF) 2. LVL or PSL engineered wood shall have the following minimum
 - design values: 2.1. E = 1,900,000 psi 22.Fb = 2600 psi 23.Fv = 285 psi 2.4.Fc = 100 psi
- Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance with AWPA standard C-2. wirth AMPA Blandard C-2
 Mails shall be common wire nails unless otherwise noted.
 Lag screws shall conform to ANSI/ASME standard B1821-1981.
 Lead holes for lag screws shall be in accordance with NDS
- specifications All beams shall have full bearing on supporting framing members
- unless otherwise noted.

 Exterior and load bearing stud walls are to be 2x4 SYP 2 a 16" OC unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header.
- King studs shall be continuous. Individual studs forming a column shall be attached with one lod nail 9 6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfer. Multi-ply beams shall have each ply attached with (3) 10d nails @
- Four and five ply beams shall be boilted together with (2) rows of 1/2" diameter through boilts staggered @ 16" O.C. unless

WOOD TRUSSES:

- The wood trues manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for
- the wood trusses.
 The wood trusses shall be designed for all required loadings. as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to
- the trusses. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- The truss manufacturer shall provide adequate bracing nformation in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings.

 Also, the shop drawings shall show the required attachments for the trusses.
- Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be not the trusses shall be per the manufacturer.

EXTERIOR WOOD FRAMED DECKS:

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

- UCOD STRUCTURAL PANELS.

 I. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA
- standards.
 All structurally required wood sheathing shall bear the mark of



SCALE 2564 147-1-67 DOMEN BY JOSE CHECKED BY: BCP

CREATE PARTY DATE REFER TO COVER SHEET FOR A CONFLETE LIST OF REVISION

FOUNDATION NOTES:

- FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AMENDMENTS
- AMENDMENTS.

 STRUCTURAL CONCRETE TO BE F. = 3000 PSI, PREPARED AND PLACED IN ACCORDANCE WITH ACI STANDARD 318.

 FOOTINGS TO BE PLACED ON UNDISTURBED EARTH, BEARING A MINIMUM OF 12" BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE CODE ENFORCEMENT OFFICIAL.

 FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF
- 2000 PSF. CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFYING THE SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION.
- 5. FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE ELEMENTS. PROVIDE 2" MINIMUM FOOTING PROJECTION FROM THE FACE OF
- 6. MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS SPECIFIED IN SECTION R404.1 OF THE 2018 NORTH CAROLINA RESIDENTIAL
- BUILDING CODE.
 PILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL.
- PROVIDE FOUNDATION WATERPROOFING, AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS.
 PROVIDED PERMETER INSULATION FOR ALL FOUNDATIONS PER 2018 NORTH
- CAROLINA RESIDENTIAL BUILDING CODE.

 10. CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK
- VENEERS. CRAWL SPACE TO BE GRADED LEVEL, AND CLEARED OF ALL DEBRIS.
- CRAIL SPALE TO BE GRAVED LEVEL, AND CLEARED OF ALL DEBRIS.

 FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH

 CAROLINA RESIDENTIAL CODE SECTION RASJAG. MINIMUM [2] DIA BOLTS

 SPACED AT 6'-0" ON CENTER WITH A "I" MINIMUM EMBEDMENT INTO MASONRY. OR CONCRETE, ANCHOR BOLTS SHALL BE IN THE BND OF EACH PLATE SECTION MINIMUM (2) ANCHOR BOLTS FER PLATE SECTION ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.
- - DJ = DOUBLE JOIST SJ = SINGLE JOIST GT = GIRDER TRUSS SC = STUD COLUMN FT = FLOOR TRUSS DR = DOUBLE RAFTER EE = EACH END TR = TRIPLE RAFTER TJ = TRIPLE JOIST CL = CENTER LINE OC = ON CENTER PL = POINT LOAD
- 10. ALL PIERS TO BE 16 "x16" MASONRY AND ALL PILASTERS TO BE 8 "x16"
- MASONRY, TYPICAL (UNO)

 WALL FOOTINGS TO BE CONTINUOUS CONCRETE, SIZES PER STRUCTURAL PLAN.
- 12. A FOUNDATION EXCAYATION OBSERVATION SHOULD BE CONDUCTED BY A
 PROFESSIONAL GEOTECHNICAL ENGINEER OR HIS QUALIFIED
 REPRESENTATIVE. IF ISOLATED AREAS OF YIELDING MATERIALS AND/OR POTENTIALLY EXPANSIVE SOILS ARE OBSERVED IN THE FOOTING EXCAVATIONS AT THE TIME OF CONSTRUCTION, SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. MUST BE PROVIDED THE OPPORTUNITY TO REVIEW THE FOOTING DESIGN PRIOR TO CONCRETE PLACEMENT.
- ALL FOOTINGS & SLABS ARE TO BEAR ON UNDISTURBED SOIL OR 95% COMPACTED FILL, VERIFIED BY ENGINEER OR CODE OFFICIAL.

REFER TO BRACED WALL PLAN FOR PANEL LOCATIONS AND ANY REQUIRED HOLDOWNS, ADDITIONAL INFORMATION PER SECTION R602.108 AND FIGURES R602.1065, R602.10.1 R602.10.8(1) AND R602.10.8(2) OF THE 2015 IRC

NOTE: ALL EXTERIOR FOUNDATION DIMENSIONS ARE TO FRAMING AND NOT BRICK VENEER UNO

NOTE: A 4" CRUSHED STONE BASE COURSE IS NOT REQUIRED WHEN SLAB IS INSTALLED ON
WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS
CLASSIFIED AS GROUP I PER TABLE R405.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>DR HORTON</u>

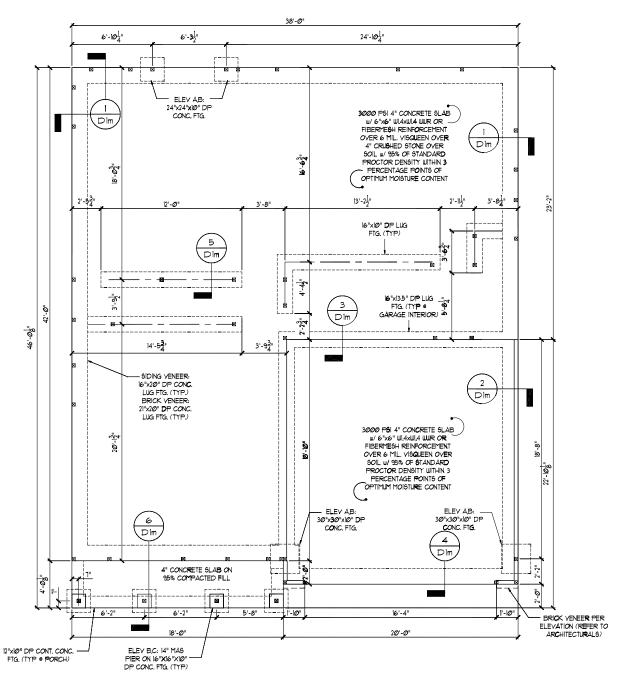
COMPLETED/REVISED ON 02/28/2020, IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY 4 TESTING, P.C. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES. PROCEDURES OR SAFETY PRECAUTIONS ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

MONOLITHIC SLAB FOUNDATION PLAN



MONOLITHIC SLAB FOUNDATION - ALL ELEVATIONS





Foundation Slab PROJECT: Winington - RH Monolithic



SCALE 2564 MAT-87 COMMINISTRACES

REFER TO COVER SHEET ROP A

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	REQUIRED	BRACED W	ALL PANEL CONNEC	CTI O NS
			REQUIRED CONNECTION	
METHOD	MATERIAL	MIN. THICKNES	PANEL EDGES	INTERMEDIATE SUPPORTS
C \$ -W5P	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS 6 6 O.C.	6d COMMON NAILS # 12" O.C.
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS** ⊕ 7" O.C.	5d COOLER NAILS** # 7" O.C.
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS 9 6" O.C.	6d COMMON NAILS 12" O.C.
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.6.4	PER FIGURE R6/02.10/6.4
		"OR EQUIVALEN	T PER TABLE RT@2.35	

GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING
- CODE WITH ALL LOCAL AMENOMENTS.

 CONTRACTOR SHALL VERRY ALL DIMENSIONS. CONTRACTOR SHALL COMPLY WITH THE CONTRINTS OF THE DRAWNE FOR THIS SPECIFIC PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.

- RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN

 3. CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO RESIST ALL PORCES ENCONTRRED DURING ERECTION.

 4. PROPERTIES USED IN THE DESIGN ARE AS FOLLOUS:

 MICROLLAM (LVL), F., 2000 PSI, F., 205 PSI, E = 125 M0° PSI

 PARALLAM (PSL): F., 2000 PSI, F., 229 PSI, E = 125 M0° PSI

 5. ALL LUCOD MEMBERS SHALL BE 12 STPM SFF UNLESS NOTED ON PLAN, ALL STUD COLUMN AND JOISTS SHALL BE 12 STPM SFF (UND.).

 6. ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2X4 12 STPM SFF STUD COLUMN AT

- ALL BEAM'S SHALL BE SUPPORTED UITH A 127 724 * "2 STP/"2 SPF STUD COLUMN AT EACH END UNLESS NOTED OTHERWISE.

 ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM A615 AND SHALL HAVE A MINIMUM COVER OF 3".

 FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION RAFGLIS, MINIMUM 10" DIA BOLTS SPACED AT 6"-0" ON CENTER UITH A 1" MINIMUM PRIBEDIBMY INTO MASORY OR CONCRETE. ANCHOR BOLTS SHALL BE 12" FROM THE END OF EACH PLATE. SECTION, MINIMUM (2) ANCHOR BOLTS PER PLATE SECTION, ANCHOR BOLTS SHALL
- BE LOCATED IN THE CENTER THIRD OF THE PLATE.

 CONTRACTOR TO PROVIDED LOCKOUTS WHEN CEILING JOISTS SPAN
 PERPENDICULAR TO RAFTERS.
- ID. FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH IV? DIA THRU BOLTS SPACED AT 24" OC. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL I/D3", MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMIM 6" FROM EACH END OF THE BEAM.
- ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2'x4 STP "2'SPF" 2',
 DROPPED, FOR NON-LOAD BEARING HEADERS EXCEEDING 8'-0" IN WIDTH
 AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2'x4 SYP 12/SPF 12, DROPPED. (UNLESS NOTED OTHERWISE)

DJ = DOUBLE JOIST SJ = SINGLE JOIST FT = FLOOR TRUSS GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END DR = DOUBLE RAFTER TR = TRIPLE RAFTER TJ = TRIPLE JOIST OC = ON CENTER CL = CENTER LINE PI - POINT LOAD

DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL

DIST & REAM SIZES SHOUN ARE MINIMUMS BUILDER MAY

INSTALL ANY REQUIRED HOLDOWNS PER SECTION R602.108 AND FIGURES R602.1065, R602.10.1, R602.108(1) AND R602.108(2) OF THE 2015 IRC

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>DR HORTON</u>

COMPLETED/REVISED ON <u>02/28/020</u>, IT IS THE RESPONSIBILITY OF
THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY 4 TESTING, P.C. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

NOTE: MEMBER NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION

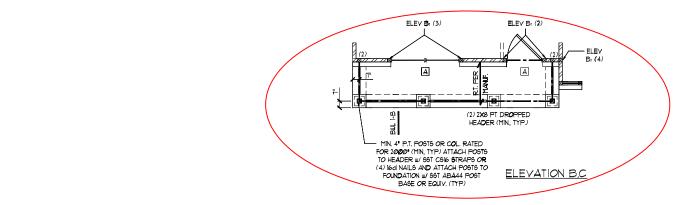
NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS. GRANITE COUNTERTOPS AND/OR ISLANDS

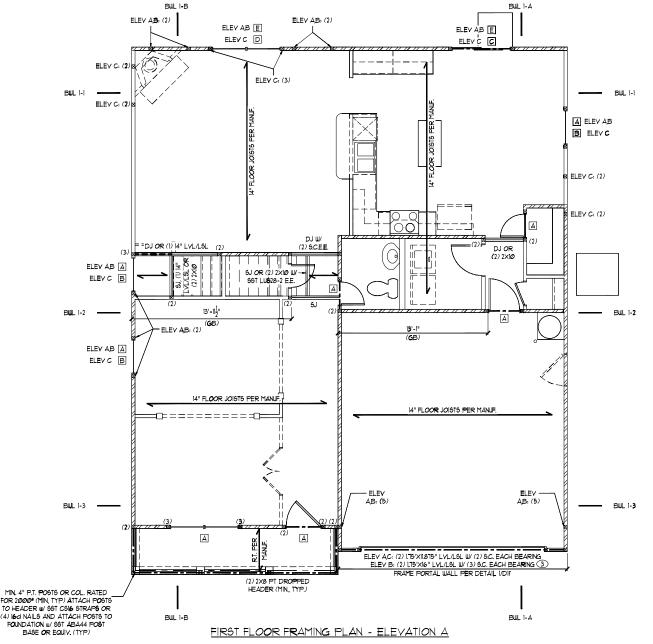
STRUCTURAL MEMBERS ONLY

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STRUCTURAL ANALYSIS BASED ON 2018 NORC

FIRST FLOOR FRAMING PLAN





FIRST F	LOOR BRAC	ING (FT)
C O NTI	NUOUS SHEATHING M	ETHO D
	REQUIRED	PROVIDED
BWL 1-1	4.8	26.5
BWL 1-2	4.8	13.5
BWL 1-3	43	13.1
BWL 1-A	11.5	41.0
21 III	115	27.00



HEADER SIZES SHOWN ON PLANS ARE MINIMUMS. GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE SC NOTED ON PLAN OVERRIDE SC LISTED ABOVE

LINTEL SCHEDULE			
TAG	SIZE	OPPENING SIZE	
Θ	L3x3xI/4"	LESS THAN 6'-0"	
2	L 5 x3x1/4"	6'-0" TO 10'-0"	
3	L5x3-1/2"x5/16"	GREATER THAN 10'-0"	
4	L5x3-1/2"x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS	

SECURE LINTEL TO HEADER w/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED . 16" O.C. (TYP FOR 3).

ALL HEADERS WHERE BRICK IS USED, TO BE: () (UNO)

WALL STUD SCHEDULE

16T & 2ND FLOOR LOAD BEARING STUDS: 2x4 STUDS @ 16" O.C. OR 2x6 STUDS ● 24" O.C. IST FLOOR LOAD BEARING STUDS W/WALK-UP ATTIC: 2x4 STUDS @ 12" OC. OR 2x6 STUDS @ 16" OC. 2x4 STUDS @ 12" OC. OR 2x6 STUDS @ 16" OC. 2x4 STUDS @ 12" OC. OR 2x6 STUDS @ 16" OC. NON-LOAD BEARING STUDS (ALL FLOORS): 2x4 STUDS * 24" O.C. TWO STORY WALLS: 2x4 STUDS ● 16" O.C. BALLOON

FRAMED W/ CROSS BRACING @ 6'-0" O.C. VERTICALLY

KING STUD REQUIREMENTS				
OPENING WIDTH	KING\$ (EACH END)			
LESS THAN 3'-0"	(1)			
3'-Ø TO 4'-Ø"	(2)			
4'-0" TO 8'-0"	(3)			
8'-0" TO 12'-0"	(5)			
12'-0" TO 16'-0"	(6)			
KING STUD REQUIREMENTS ABOVE DO NO APPLY TO PORTAL FRAMED OPENINGS				

BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2015 INTERNATIONAL RESIDENTIAL CODE AS ALLOWED PER SECTION R60210 OF THE 2018 NC RESIDENTIAL CODE.
- WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE WIND SPEEDS UP TO 130 MPH.
 REFER TO ARCHITECTURAL PLAN FOR DOORWINDOW OPENING
- 3. BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH IRC TABLE R602/04.

 4. ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND
- SHALL NOT EXCEED IN FEET FOR ISOLATED PANEL METHOD AND IZ FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATION. MINIMUM PANEL LENGTH SHALL BE PER TABLE R602,005.
- FINITION FANEL LENGTH SHALL BE FYER TABLE 1862/105.

 THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 121" GYPEND MODARD (MO).

 FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFILL AREAS BETWEEN BRACED WALL PANELS, ARDOY AND BELOW WALL OFFINICS, AND ON GABLE END WALLS.
- 8. FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.

 9. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 10 FEET OF
- EACH END OF A BRACED WALL LINE.
- II. THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 20 FEET.

 II. MASONRY OR CONCRETE STEM WALLS WITH A LENGTH OF 48" OR
- LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESGNED IN ACCORDANCE WITH FIGURE REQUIPS OF THE 2015 IRC.

 12. BRACED WALL PANEL CONNECTIONS TO FLOOR SCELLING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION REQUIPS.
- BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.82 AND
- FIGURES R602.10.8(1)4(2)4(3). 14. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE
- DESIGNED IN ACCORDANCE WITH SECTION R602.10.11 PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE
- R602.106.4 (UNO) 16 ON SCHEMATIC SHADED WALLS INDICATE BRACED WALL PANELS

GB = GYPSUM BOARD WSP = WOOD STRUCTURAL PANEL summit



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S3.Ø

REQUIRED BRACED WALL PANEL CONNECTIONS					
			REQUIRED CONNECTION		
METHOD	MATERIAL	MIN. THICKNES6	# PANEL EDGES	# INTERMEDIATE SUPPORTS	
C 5-W5P	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS	6d COMMON NAILS 9 12" O.C.	
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS **	5d COOLER NAIL 6 a T" O.C.	
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS	6d COMMON NAILS © 12" O.C.	
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R 602.10.6.4	PER FIGURE R602.106.4	
"OR EQUIVALENT PER TABLE RT0235					

GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING
- CODE WITH ALL LOCAL AMENDMENTS.

 CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.

 3. CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED

- CONTRACTOR IS RESPONSIBLE FOR PROVINGE TENTIFICARY BRACING REGULTED TO RESIST ALL PORCES ENCOUNTERED DURING ERECTION.

 PROPERTIES USED IN THE DESIGN ARE AS FOLLOUS:

 MICROLLAM (I.V.L.): F₆ = 2600 PS); F₇ = 285 PS), E = 19x10° PS|

 PARALLAM (PSL.): F₇ = 29200 PS); F₇ = 290 PS], E = 125x10° PS|

 ALL BUOOD MEMBERS SHALL BE "\$ 51PPV SPF UNLESS NOTED ON PLAN. ALL STUD COLUMNS AND JOISTS SHALL BE "\$ 19PPV SPF UNLESS NOTED ON PLAN. ALL STUD COLUMNS SHALL BE SUPPORTED WITH A (2) 2x4 °2 SYPPV SPF STUD COLUMN AT EACH END UNLESS NOTED OTHERWISE.
- ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM A615 AND SHALL HAVE A MINIMUM COVER OF 3".
- 8. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION RADIALE MINIMIM 12" DIA BOLTS SPACED AT 6"-0" ON CENTER WITH A T" MINIMIM EMBEDMENT INTO MASONRY OR CONCRETE, ANCHOR BOLTS SHALL BE 12" FROM THE END OF EACH PLATE SECTION, MINIMUM (2) ANCHOR BOLTS PER PLATE SECTION, ANCHOR BOLTS SHALL DELICATED IN THE CENTER THIRD OF THE PLATE.

 CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN
 PERPENDICULAR TO RAFTERS.
- 10. FLITCH BEAMS, 4-PLY LYLS AND 3-PLY SIDE LOADED LYLS SHALL BE BOLTED TOSETHER WITH 1/2" DIA. THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/D3f. MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
- ALL NON-LOAD BEARING HEADERS SHALL BE (1)FLAT 2x4 SYP "2,6FF" 2, DROPPED, FOR NON-LOAD BEARING HEADERS EXCEEDING 8'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL, ABOVE, SHALL BE (2) FLAT 2x4 SYP 1/SPF 12, DROPPED. (UNLESS NOTED OTHERWISE)
- ABBREVIATIONS:
 - DJ = DOUBLE JOIST SJ = SINGLE JOIST FT = FLOOR TRUSS GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END DR = DOUBLE RAFTER TR = TRIPLE RAFTER TJ = TRIPLE JOIST OC = ON CENTER CL = CENTER LINE PL = POINT LOAD

NOTE:

DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE, PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

JOIST 4 BEAM SIZES SHOWN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

INSTALL ANY REQUIRED HOLDOWNS PER SECTION R602.10.8 AND FIGURES R602.106.5, R602.10.1, 18602.10.8(1) AND R602.10.8(2) OF THE 2015 IRC

NOTE: MEMBER NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER
PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED
TO PREVENT MOISTURE INTRUSION.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DR HORTON COMPLETED/REVISED ON 2012/2012/11 IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. IF ANY CHANCES ARE MADE TO THE ARCHITECTURAL BY MARE POOR TO CONSTRUCTION. PLANS PRIOR TO CONSTRUCTION, SUMMIT ENGINEERING. LABORATORY & TESTING P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

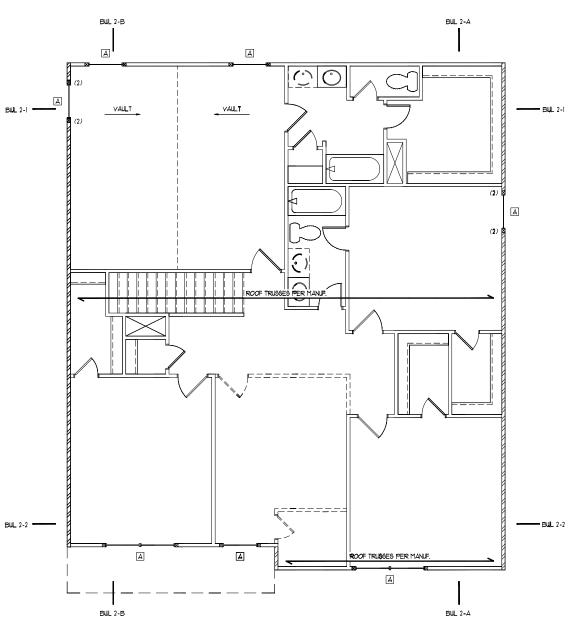
STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS.
ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO
BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR FRAMING PLAN

SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x1"1"



	ROOF TRUSSES FER MANUF.
BWL 2-2	BUL 2-2
 	BUL 2-B
	SECOND FLOOR FRAMING PLAN - ELEVATION C

SECOND FLOOR BRACING (FT)					
CONTINUOUS SHEATHING METHOD					
	REQUIRED PROVIDED				
BWL 2-1	6.8	3Ø.1			
BWL 2-2	6.8	21.1			
BWL 2-A	5.9	41.0			
BWL 2-B	5.9	37.1			

HEADER SCHEDULE				
TAG SIZE		JACKS (EACH END)		
А	(2) 2x6	(1)		
В	(2) 2x8	(2)		
С	(2) 2xlØ	(2)		
D	(2) 2x12	(2)		
E	(2) 9-1/4" LSL/LVL	(3)		
F	(3) 2x6	(1)		
G	(3) 2x8	(2)		
Н	(3) 2xlØ	(2)		
	(3) 2×12	(2)		

HEADER SITES SHOUN ON PLANS ARE MINIMUMS GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION.
ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE. SC NOTED ON PLAN OVERRIDE SC LISTED ABOVE.

	LINTEL SCHEDULE				
TAG		SIZE	OPENING SIZE		
		L3x3x1/4"	LESS THAN 6'-0"		
	@	L5x3x1/4"	6'-0" TO 10'-0"		
	3	L 5 x3-1/2"x5/16"	GREATER THAN 10'-0"		
	4	L 5 x3-1/2"x5/16" ROLLED OR EQUIV.	ALL AR C HED OPENINGS		

SECURE LINTEL TO HEADER W/ (2) 1/2" DIAMETER LAG SCREUS STAGGERED & 16" O.C. (TYP FOR 3)

ALL HEADERS WHERE BRICK IS USED, TO BE: (UNO)

WALL STUD SCHEDULE

1ST & 2ND FLOOR LOAD BEARING STUDS: 2x4 STUDS @ 16" O.C. OR 2x6 STUDS @ 24" O.C. ST FLOOR LOAD BEARING STUDS W/WALK-UP ATTIC: 2x4 STUDS = 12* OC. OR 2x6 STUDS = 16* OC. BASETIENT LOAD BEARING STUDS: 2x4 STUDS = 12* OC. OR 2x6 STUDS = 16* OC. NON-LOAD BEARING STUDS (ALL FLOORS): 2x4 STUDS # 24" O.C. TWO \$TORY WALLS: 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED W/ CROSS BRACING & 6'-0" O.C. VERTICALLY

OPENING WIDTH	KINGS (EACH END.
LE \$ 6 THAN 3'-Ø"	(1)
3'-Ø TO 4'- Ø "	(2)
4'-0" TO 8'-0"	(3)
8'-0" TO 12'-0"	(5)
12'-0" TO 16'-0"	(6)

BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2015 INTERNATIONAL RESIDENTIAL CODE AS ALLOWED PER SECTION R602/20 OF THE 2018 NC RESIDENTIAL CODE.
 WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE WIND
- SPEEDS UP TO 130 MPH.
- REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH IRC TABLE R602 IO.4.
- ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED 10 FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.

 MINIMUM PANEL LENGTH SHALL BE PER TABLE R6/02/10/5.
 THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM
- NIENCK WALLS SHALL DE SHEATHED SONTHINGS SHALL BE SHALL BE SHEATHED ON ALL SHEATHANG METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFILL AREAS BETWEEN BRACED WALL PAINELS, ABOVE AND BELOW WALL
- OPENINGS, AND ON GABLE END WALLS. FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND
- THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS
- A BRACED WALL PANEL SHALL BE LOCATED WITHIN 10 FEET OF
 EACH END OF A BRACED WALL LINE.

 THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS
 EACH WAS EXCEPT. 20 EEET
- SHALL NOT EXCEED 20 FEET MASONRY OR CONCRETE STEM WALLS WITH A LENGTH OF 48" OR LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602 10.9 OF THE 2015 IRC.
- BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R6021/03
 BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE
- CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.82 AND
- FIGURES R602:108(1)4(2)4(3).
 CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE
- DESIGNED IN ACCORDANCE WITH SECTION R602.10.11 PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE
- R602,0664 (UNO)
 ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.
- ABBREVIATIONS:

GB = GYPSUM BOARD | WSP = WOOD STRUCTURAL PANEL C5-XXX = CONT. SHEATHED | ENG = ENGINEERED SOLUTION | FF = PORTAL FRAME | FF-ENG = ENG. PORTAL FRAME





aming 芷 ö 正 PROJECT: Winington First F



SCALE 2564 MATER COMMUNICATION CHECKED BY: BCP PROJECT OF DATE

1041 SYMPOSIS

REFER TO COVER SHEET FOR A CONFLETE LIST OF FREVERIOR

S4.1

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DR HORION COMPLETED/REVISED ON 02/08/02/0. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT BY SINKERING, LABORATORY 4 TESTING, PC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY 4 TESTING, PC. CANNOT GLARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

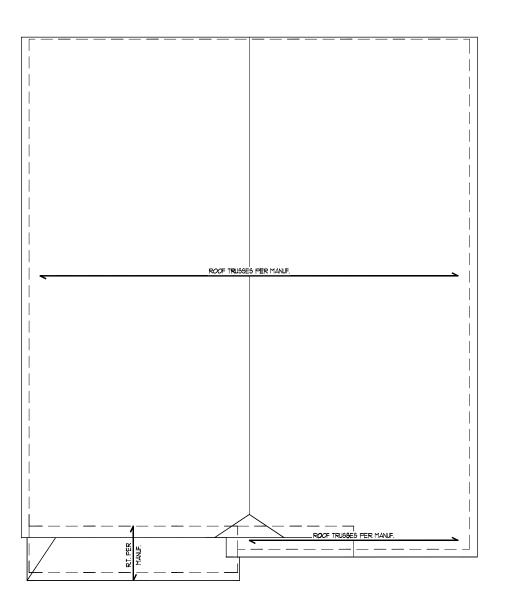
NOTE: 19T PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNO)

NOTE: ROOF TRUSSES SHALL BE SPACE TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

STRUCTURAL MEMBERS ONLY
ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL
COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT
INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES,
SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS.
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SUMMIT ENGINEERING, LABORATORY & TESTING, P.C.
FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

ROOF FRAMING PLAN
9CALE: 1/4"=1"-6" ON 22"×34" OR 1/8"=1"-6" ON 11"×17"



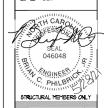
ROOF FRAMING PLAN - ELEVATION C





DR Horton, Inc. 8001 Arroundge Blvd. Charlotte, NC 28213

Project. Wingen - Fat First Floor Framing Plan



DRAINS
DATE SCARN

CALL 2004 MANAGE

FRANCI A 200-666 MINISTRACE

CHECKED BY JCCP

CHECKED BY JCCP

PROJECT * DATE OF STATE OF STA

REFER TO COVER SHEET FOR A CONFLETE LIST OF PRIVISIONS

5.2

Applicable Building Codes:

• 2018 North Carolina Residential Building Code with All Local Amendments ASCE 1-10: Minimum Design Loads for Buildings and Other Structures

sign L	oads:		
1.	Roof	Live Loads	
	1.1.	Conventional 2x	20 PSF
	1.2.	Truss	20 PSF
		1.2.1. Attic Truss	60 PSF
2.	Roof	Dead Loads	
	2.1.	Conventional 2x	10 PSF
		Truss	
3.	Snow		15 PSF
	3.1.	Importance Factor	1.0
4.	Floor	Live Loads	
	4.1.	Typ. Dwelling	40 PSF
		Sleeping Areas	
	4.3.	Decks	40 PSF
		Passenger Garage	

5. Floor Dead Loads 5.1. Conventional 2x 5.2. I-Joist 15 PSF . 15 PSF 5.3. Floor Truss 6. Ultimate Wind Speed (3 sec. qust). PER PLAN 6.1. Exposure 6.2. Importance Factor.

6.3.2.Vy = 7. Component and Cladding (in PSF)

6.3. Wind Base Shear

6.3.l. Vx =

MEAN ROOF HT.	UP TO 30'	3Ø'1"-35'	35'1"-40'	40'1"-45'
ZONE 1	16.7,-18.0	17.5,-18.9	18.2,-19.6	18.7,-20.2
ZONE 2	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 3	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 4	18.2,-19.0	19.2,-20.0	19.9,-20.7	20.4,-21.3
ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.1	20.4,-26.9

Seismi	ic
8.1.	Site Class
8.2.	Design CategoryC
	Importance Factor
8.4.	Seismic Use Group1
8.5.	Spectral Response Acceleration

8.5.1. Sms = %a 8.5.2. Sml = %q 8.6. Seismic Base Shear 8.6.1. Vx =

8.7. Basic Structural System (check one)

 Bearing Wall
 ■ ☐ Building Frame ☐ Moment Frame

☐ Dual w/ Special Moment Frame ☐ Dual w/ Intermediate R/C or Special Steel □ Inverted Pendulum

8.8. Arch/Mech Components Anchored ...

8.9. Lateral Design Control: Seismic 🗆 Wind 🖂 9. Assumed Soil Bearing Capacity



STRUCTURAL PLANS PREPARED FOR:

STANDARD DETAILS

PROJECT ADDRESS:

DR Horton Carolinas Division 8001 Arrowridge Blvd Charlotte, NC 28273

ARCHITECT/DESIGNER: GMD Design Group 1845 Satellite Blvd. Duluth, GA 30097

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory & Testing, P.C. before construction begins.

PLAN ABBREVIATIONS:

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	SC	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
DSP	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
EE	EACH END	SYP	SOUTHERN YELLOW PINE
ΕW	EACH WAY	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
<i>0</i> C	ON CENTER	TYP	TYPICAL
PSF	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
PSI	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by <u>DR Horton</u>, <u>Inc.</u> Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify SUMMIT immediately.

SHEET LIST:

Sheet No.	Description
CS1	Cover Sheet, Specifications, Revisions
Dim	Monolithic Slab Foundation Details
Dis	Stem Wall Foundation Details
Dlc	Crawl Space Foundation Details
Dlb	Basement Foundation Details
Dlf	Framing Details

DR HORTON PROJECT SIGN-OFF:

Manager	Signature
Operations	
Operations System	
Operations Product Development	

REVISION LIST:

Revision No.	Date	Project No.	Description
1	5.11.17		Added box bay detail (2/D2f). Added deck options with basement. Revised deck options with stem wall and crawl space foundations
2	T.12.1T		Revised stem wall insulation note.
3	2.15.18		Revised garage door detail, NC only
4	2.28.18		Added high-wind foundation details
5	12.19.18		Revised per 2018 NCRC
6	2.19.19		Revised per Mecklenburg County Comments
٦	3.1.19		Revised stem wall deck attachment and roof sheathing on wall sections.
8	3.6.19		Corrected dimensions at perimeter footings
9	3.2.20		Added tall turndown detail
10	3.18.20		Added balloon framing detail
11	10.20.20		Added alternate two-pour detail for slab and added note for crawl girder above grade
12	3.1.21		Added OX-15 Standard Details
13	5.18.21		Updated OX-IS Standard Details
14	Ø2.14.23		Added 4/D2m - Tall Slab Detail w/ Siding

- GENERAL STRUCTURAL NOTES: The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT
- shall be considered the same entity. 2. The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
- 3. The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur
- 4. Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.
- 5. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.
- 6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- 7. This structure and all construction shall conform to all applicable sections of the international residential code.
- 8. This structure and all construction shall conform to all applicable sections of local building codes. 9. All structural assemblies are to meet or exceed to requirements

FOUNDATIONS:

The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.

of the current local building code.

- 2. The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
- 3. Any fill shall be placed under the direction or recommendation of a licensed professional engineer. 4. The resulting soil shall be compacted to a minimum of 95%
- maximum dry density. 5. Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
- 6. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.

STRUCTURAL STEEL:

- Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design" latest editions.
- Structural steel shall receive one coat of shop applied rust-inhibitive paint.
- 3. All steel shall have a minimum yield stress (F_{ij}) of 36 ksi unless otherwise noted.
- 4. Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D.I. Electrodes for shop and field welding shall be class ETØXX. All welding shall be performed by a certified welder per the above standards.

- Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- 2. Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- 3. Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows:
 - 3.1. Footings: 5%
- 3.2.Exterior Slabs: 5% 4. No admixtures shall be added to any structural concrete without written permission of the SER.

- 5. Concrete slabs-on-grade shall be constructed in accordance with ACI 302.IR-96: "Guide for Concrete Slab and Slab Construction".
- 6. The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported
- conditions not in accordance with the above assumptions. 7. Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted.
- 8. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished 9. Reinforcing steel may not extend through a control joint.
- Reinforcing steel may extend through a saw cut joint. 10. All welded wire fabric (W.W.F.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The W.W.F. shall be securely supported during the concrete pour.

CONCRETE REINFORCEMENT:

- Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- 2. Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- 3. Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard) 4. Fibermesh shall comply with ASTM CIII6, any local building code requirements, and shall meet or exceed the current industry
- 5. Steel reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- 6. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures" Horizontal footing and wall reinforcement shall be continuous

and shall have 90° bends, or corner bars with the same

masonry shall be a minimum of 48 bar diameters.

size/spacing as the horizontal reinforcement with a class B tension splice. 8. Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in

- 9. Where reinforcing dowels are required , they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters
- 10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.
- WOOD FRAMING: 1. Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Spruce-Yellow-Pine (SYP) #2.
- 2. LVL or PSL engineered wood shall have the following minimum design values:
 - 2.1. E = 1,9*00,000* psi $2.2.\,F_{\rm b} = 2600\,$ psi $2.3.F_{V} = 285 \text{ psi}$
- 2.4.Fc = 700 psi 3. Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance with AWPA standard C-2
- 4. Nails shall be common wire nails unless otherwise noted. 5. Lag screws shall conform to ANSI/ASME standard B18.2.1-1981. Lead holes for lag screws shall be in accordance with NDS
- specifications. 6. All beams shall have full bearing on supporting framing members
- 7. Exterior and load bearing stud walls are to be 2x4 SYP #2 @ 16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- 8. Individual studs forming a column shall be attached with one 10d nail @ 6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfer.

9. Multi-ply beams shall have each ply attached with (3) 10d nails a

10. Flitch beams, 4-ply beams and 3-ply side loaded beams shall be bolted together with (2) rows of 1/2" diameter through bolts staggered @ 16" O.C. unless noted otherwise. Min. edge distance shall be 2" and (2) bolts shall be located a min. 6" from each end of the beam.

WOOD TRUSSES:

- l. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- 2. The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-05), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to
- 3. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- 4. The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
- 5. Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

EXTERIOR WOOD FRAMED DECKS:

 Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

WOOD STRUCTURAL PANELS:

- Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA
- 2. All structurally required wood sheathing shall bear the mark of the APA.

- 3. Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Wood floor sheathing shall be APA rated sheathing exposure I or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T&G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- 6. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

STRUCTURAL FIBERBOARD PANELS:

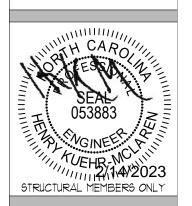
Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards. 2. All structurally required fiberboard sheathing shall bear the

- mark of the AFA. 3. Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more
- 4. Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

SUMMIT RALEIGH, NC 27603 OFFICE: 919.380.9991

120 PENMARC DR., SUITE 108 FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM





DATE: 02/14/2023 SCALE: 22x34 |/4"=1'-0" ||x|T |/8"=1'-0" PROJECT *: 528-06R DRAWN BY: JCEF CHECKED BY: BCP

PRIGINAL INFORMATION

PROJECT *

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

UNDISTURBED SOIL

6A COVERED PATIO DETAIL

STANDARD - BRICK

CHARTS

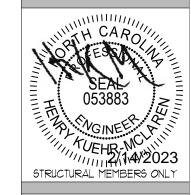
STANDARD - SIDING

6 PATIO SLAB DETAIL

SUMMIT 120 PENMARC DR., SUITE 108 RALEIGH, NC 27603 OFFICE: 919.380.9991 FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM

& Testing, Inc.

CLIENT: DR Horton Carolina Divis 8001 Arrowridge Blvd. Charlotte, NC 28273



DRAWING DATE: Ø2/14/2Ø23 PROJECT *: 528-06R DRAWN BY: JCEF CHECKED BY: BCP

ORIGINAL INFORMATION

4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR

5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL

AMENDMENTS AND REQUIREMENTS NOT SHOWN

CONNECTIONS

BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND

6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE

ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

Dm

PER PLAN CONTINUOUS

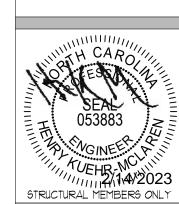
4 TALL SLAB DETAIL W/ SIDING

1. REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET

- FOR ADDITIONAL INFORMATION. 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE. 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.
- 4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND CONNECTIONS
- 5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN
- 6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC

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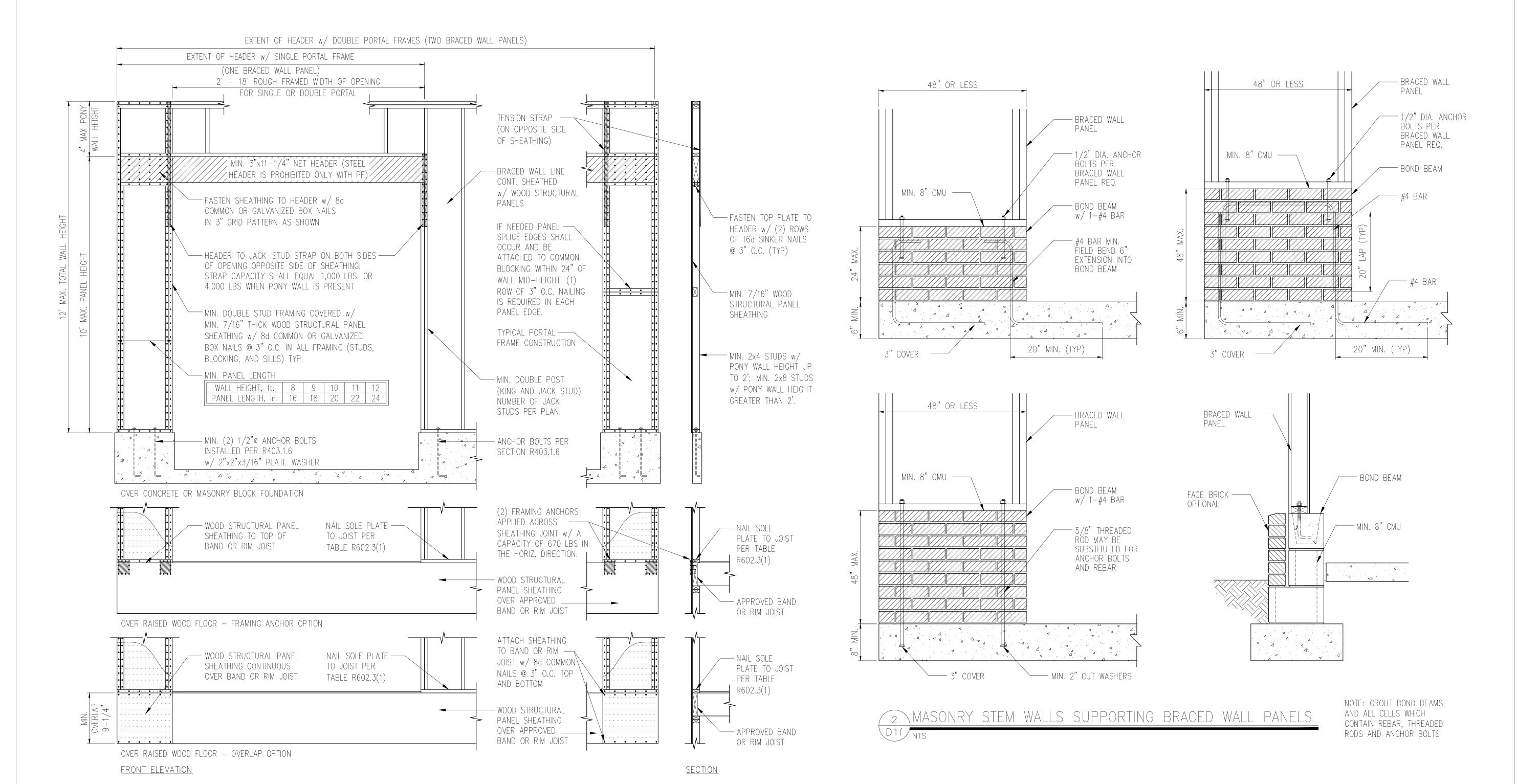


DRAWING DATE: Ø2/14/2Ø23 PROJECT *: 528-06R DRAWN BY: JCEF CHECKED BY: BCP

> ORIGINAL INFORMATION PROJECT • DATE 1/31/2017

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

D2m







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& Testing, Inc. No. F-1454

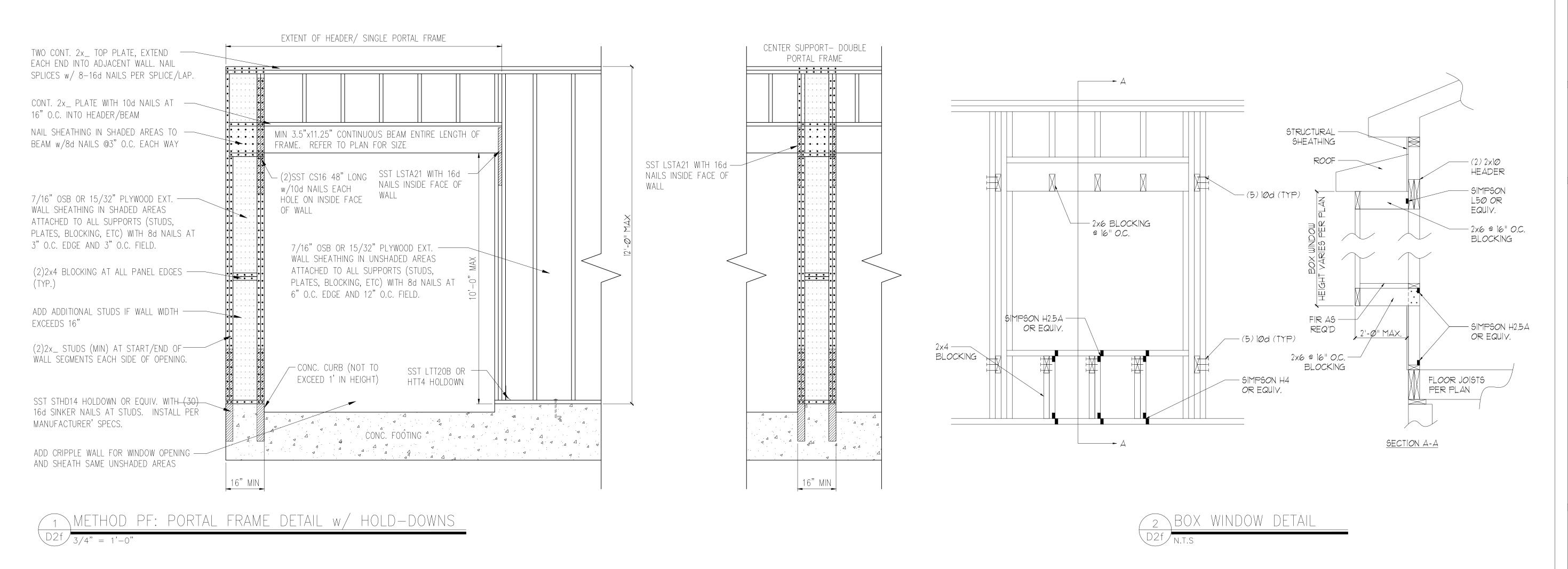


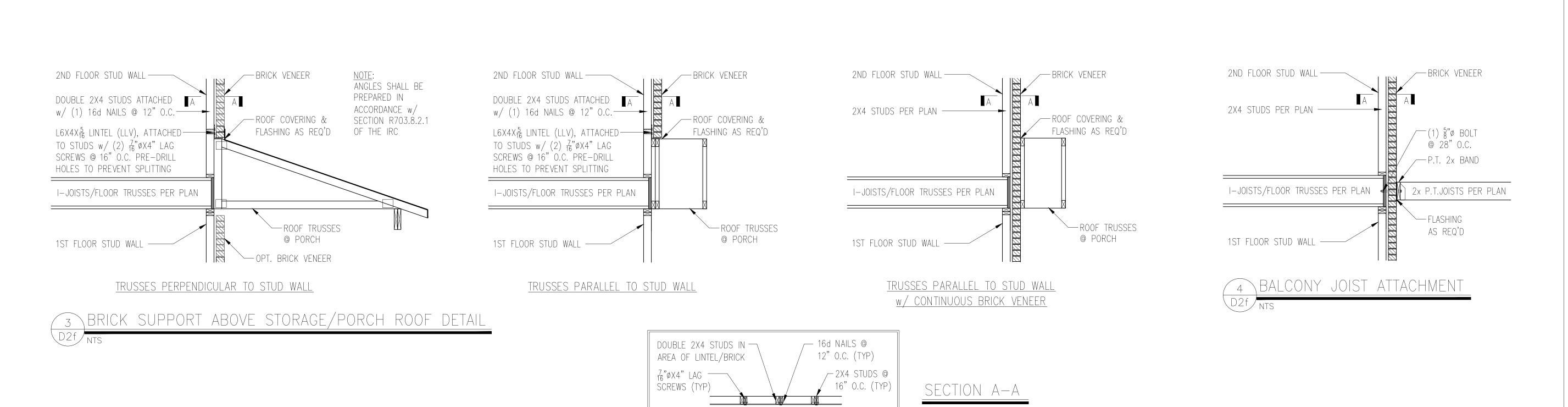
DRAWING DATE: Ø2/14/2Ø23 SCALE: 22x34 1/4"=1'-0" ||x|7 ||/8"=1'-0" PROJECT *: 528-06R DRAWN BY: JCEF CHECKED BY: BCP

ORIGINAL INFORMATION

PROJECT • DATE 1/31/2017

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

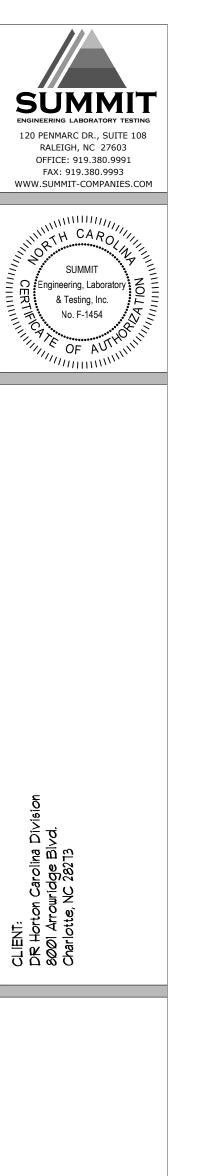




 $L6X4X_{\overline{16}}^{5}$ LINTEL (LLV), ATTACHED

_ TO STUDS w/ (2) $\frac{7}{16}$ "øx4" LAG SCREWS @ 16" O.C. PRE-DRILL

HOLES TO PREVENT SPLITTING



ille (0x-15) □@ţâ∐s

 \bigcirc

STRUCTURAL MEMBERS ONLY

9CALE: 22x34 |/4"=1'-@" ||x|1 |/6"=1'-@" |PROJECT *: 528-06R

PROJECT • DATE 1/31/2017

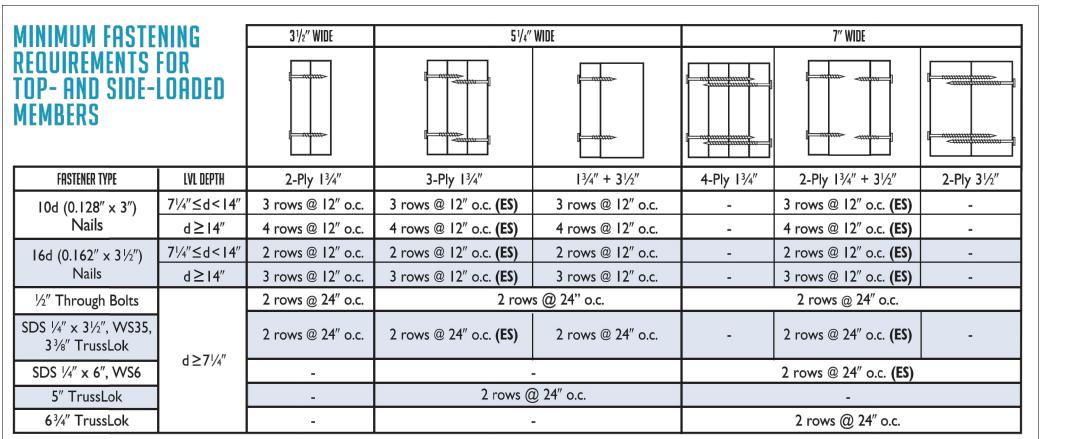
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

DRAWING

DATE: Ø2/14/2Ø23

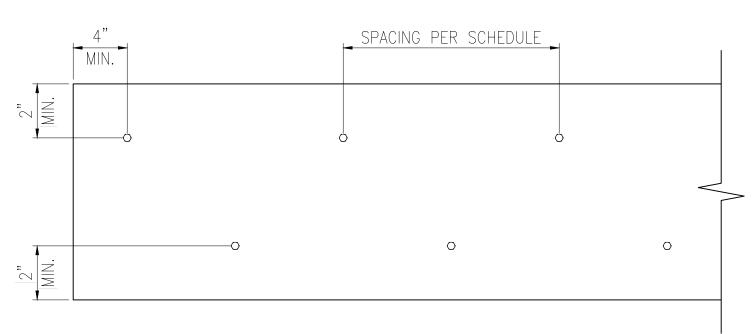
DRAWN BY: JCEF
CHECKED BY: BCP

ORIGINAL INFORMATION



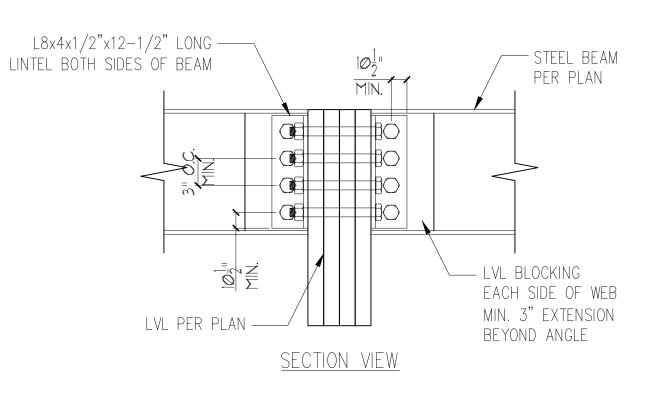
NOTES:

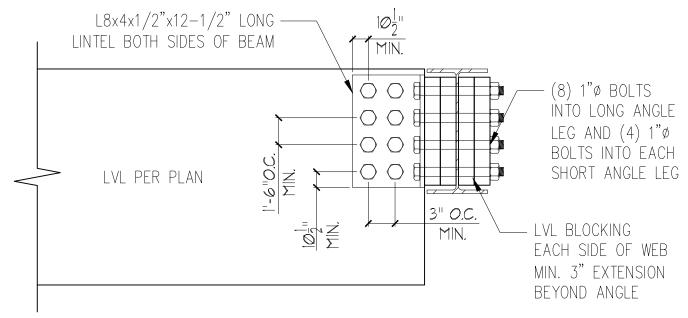
- 1. All fasteners must meet the minimum requirements in the table above. Side-loaded multiple-ply members must meet the minimum fastening and side-loading capacity requirements given on page 48.
- 2. Minimum fastening requirements for depths less than $7\frac{1}{4}$ require special consideration. Please contact your technical representative.
- 3. Three general rules for staggering or offsetting for a certain fastener schedule:
- (I) if staggering or offsetting is not referenced, then none is required; (2) if staggering is referenced, then fasteners installed in adjacent rows on the front side are to be staggered up to one-half the o.c. spacing, but maintaining the fastener
- clearances above; and (3) if "ES" is referenced, then the fastener schedule must be repeated on each side, with the fasteners on the back side offset up to one-half the o.c. spacing of the front side (whether or not it is staggered).



ELEVATION VIEW

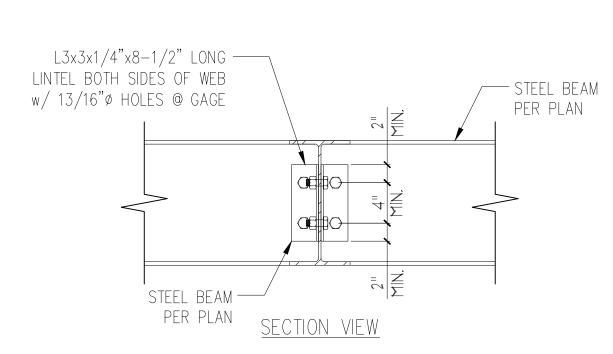
MULTI-PLY BEAM CONNECTION DETAIL

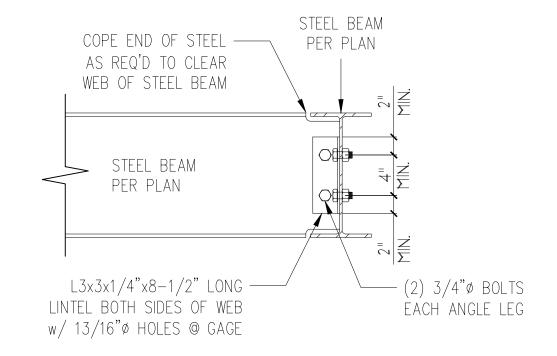




ELEVATION VIEW

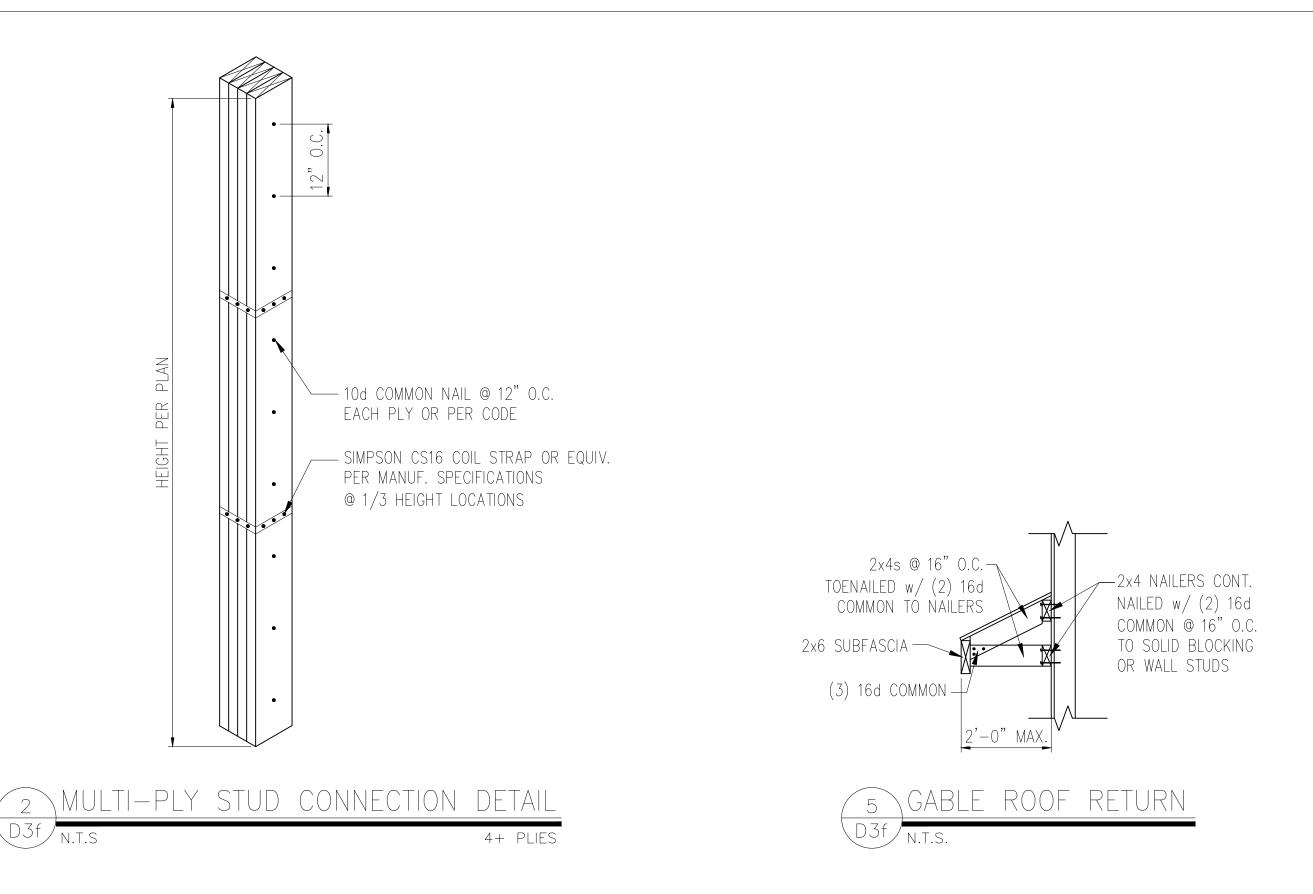


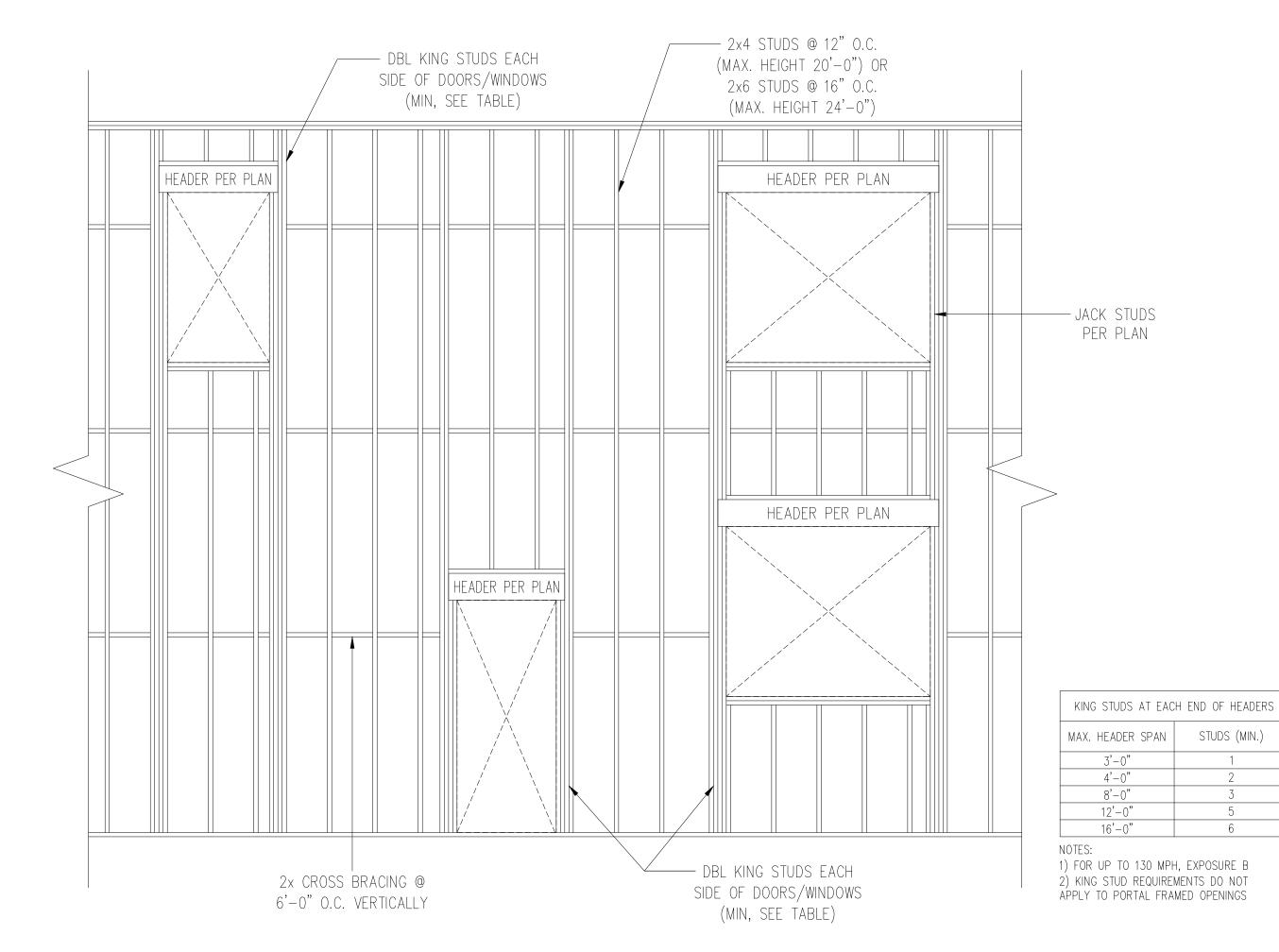




ELEVATION VIEW

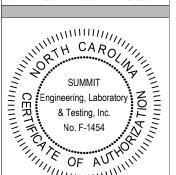


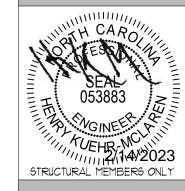




TYP. BALLOON FRAMING DETAIL



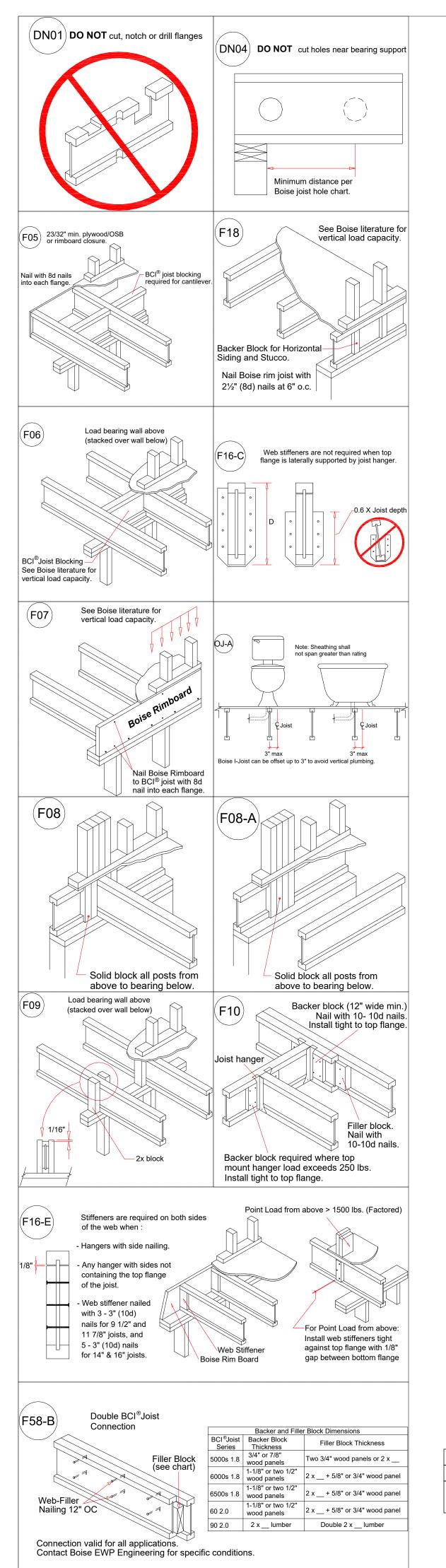




DATE: Ø2/14/2Ø23 SCALE: 22x34 |/4"=1'-0" ||x|1 |/8"=1'-0" PROJECT *: 528-06R DRAWN BY: JCEF CHECKED BY: BCP

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

D3f





		Elevation (2	
		Products		
PlotID	Length	Product	Plies	Net Qty
1	24' 0"	14" BCI® 4500s-1.8	1	1
2	24' 0"	14" BCI® 4500s-1.8	1	8
3	20' 0"	14" BCI® 4500s-1.8	1	1
4	20' 0"	14" BCI® 4500s-1.8	2	4
5	19' 0"	14" BCI® 4500s-1.8	1	3
6	19' 0"	14" BCI® 4500s-1.8	1	1
7	18' 0"	14" BCI® 4500s-1.8	1	2
8	18' 0"	14" BCI® 4500s-1.8	1	3
9	18' 0"	14" BCI® 4500s-1.8	1	3
10	17' 0"	14" BCI® 4500s-1.8	1	2
11	15' 0"	14" BCI® 4500s-1.8	1	1
12	7' 0"	14" BCI® 4500s-1.8	1	2
13	4' 0"	14" BCI® 4500s-1.8	1	2
13	4' 0"	14" BCI® 4500s-1.8	1	1
14	3' 0"	14" BCI® 4500s-1.8	1	1
15	38' 0"	14" BCI® 5000s-1.8	1	9
16	19' 0"	14" BCI® 5000s-1.8	1	1
17	5' 0"	14" BCI® 4500s-1.8	2	2
18	4' 0"	14" BCI® 4500s-1.8	2	2
GDH	20' 0"	1-3/4" x 11-7/8" VERSA-LAM® LVL 2.1E 3100 SP	2	2
19	8' 0"	1-3/4" x 14" VERSA-LAM® LVL 2.1E 3100 SP	1	1
20	4' 0"	1-3/4" x 14" VERSA-LAM® LVL 2.1E 3100 SP	1	1
Rm-1	12' 0"	1" x 14" BC RIM BOARD OSB	1	8
Bk1	2' 0"	14" BCI® 4500s-1.8	1	26

Connector Summary						
	PlotID	Qty	Manuf	Product		
	H1	1	Simpson	HU11		
	H2	13	Simpson	IUS1.81/1		



ied Members!! ***
Iminated veneer lumber, g

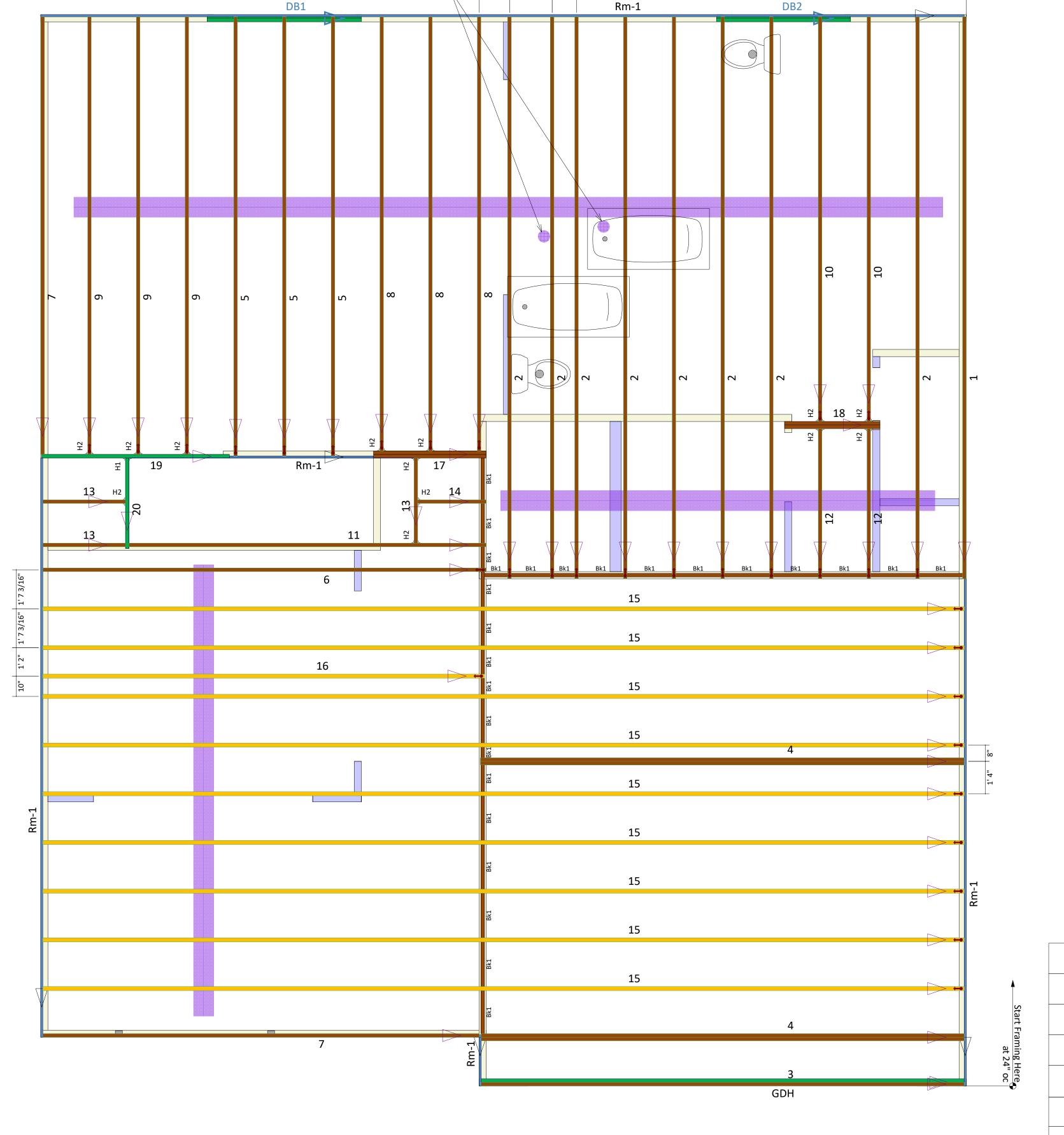
to Setting the Carried Name of the design of the member or manner

F be Installed PRIOR ts --- Cuts, notches and he specifically considered in the

*** ANY Concealed Flange Hangers nternational Residential Code - R502.8.2 Engineered Wood I members or I-joists are not permitted unless such penetratic

"NO CUT END"

Start Framing Here



Opt Owner's Bath

Builder or framer should review the material placement layout prior to beginning construction of floor system. This layout DOES NOT supercede the plan set.

Plan Information

Lot Number: 81 Woodgrove Model: Wilmington C **Builder: DR Horton Express**

Boise BC FRAMER II / SAPPHIRE Structure

Plan Date: 02/28/2020

Structural Date: 05/28/2021

Not To Scale By: CK Sheet: 2F

Current Date: 06/29/2023

