# Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 49220 JOB: 24-0289-R01

JOB NAME: LOT 62 PROVIDENCE CREEK

Wind Code: ASCE7-16

Wind Speed: Vult= 120mph

**Exposure Category: B** 

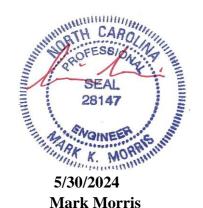
Mean Roof Height (feet): 35

These truss designs comply with IRC 2015 as well as IRC 2018.

20 Truss Design(s)

## Trusses:

P01, R01, R02, R02A, R03, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, V01, V02, V03, V04, V05

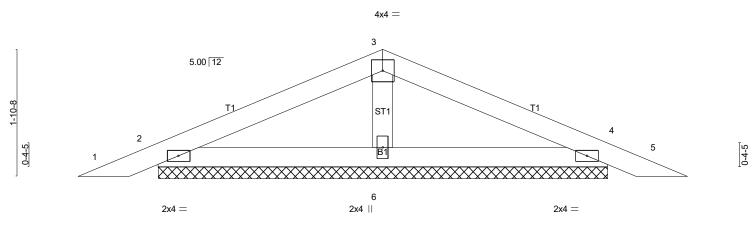


### Warning !—Verify design parameters and read notes before use.

This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to

Job LOT 62 PROVIDENCE CREEK | 29 WINDSWEPT WAY FUQUAY-VARINA, NC Truss Truss Type 24-0289-R01 P01 Piggyback 21 # 49220 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 30 21:34:45 2024 Page 1 ID:av29u\_vm2cwLtXF0Wc5ybwyV6X0-a1vX9YPTDoJEobhztEsc0Gy85YBKtYJI\_r7E0gzBC98 4-6-0 9-0-0 4-6-0 4-6-0

Scale = 1:17.0



9-0-0 9-0-0									
TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.26 BC 0.19 WB 0.04 Matrix-P	Vert(CT) 0.	in (loc) 0.01 5 0.01 5 0.00 4	l/defl L/d n/r 180 n/r 80 n/a n/a	PLATES GRIP MT20 244/190 Weight: 26 lb FT = 20%			

LUMBER-

0-1-10

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 OTHERS 2x4 SP No.3

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS.** (lb/size) 2=184/6-7-10 (min. 0-1-8), 4=184/6-7-10 (min. 0-1-8), 6=258/6-7-10 (min. 0-1-8)

Max Horz 2=26(LC 14)

Max Uplift2=-48(LC 14), 4=-52(LC 15), 6=-4(LC 14) Max Grav 2=256(LC 21), 4=256(LC 22), 6=258(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard

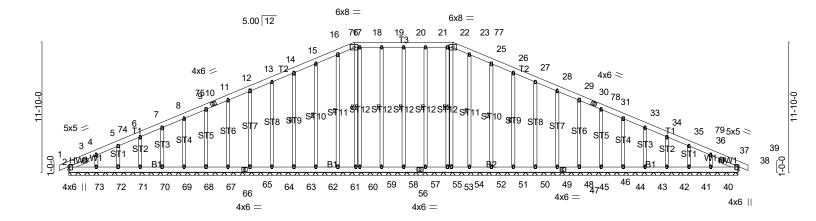


5/30/2024

Warning !--Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job Truss Truss Type LOT 62 PROVIDENCE CREEK | 29 WINDSWEPT WAY FUQUAY-VARINA, N 24-0289-R01 R01 GABLE # 49220 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 30 21:34:47 2024 Page 1 ID:av29u\_vm2cwLtXF0Wc5ybwyV6X0-WQ1laERjlPay1vrM?fu45h1YrMvDLPXaR8cL4YzBC96 35-0-0 -0<sub>7</sub>10-8 0-10-8 26-0-0 61-0-0 26-0-0 9-0-0 26-0-0 0-10-8

Scale = 1:105.0



		61-0	-0			——————————————————————————————————————
TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.05 BC 0.03 WB 0.25 Matrix-SH	Vert(CT) -	in (loc) 0.00 38 0.00 38 0.01 38	l/defl L/d n/r 180 n/r 80 n/a n/a	PLATES GRIP MT20 244/190 Weight: 624 lb FT = 20%

61-0-0

LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WFBS 2x4 SP No.3 OTHERS 2x4 SP No 3

Left 2x4 SP No.3 -° 1-11-0, Right 2x4 SP No.3 -° 1-11-0 SLIDER

BRACING-

TOP CHORD **BOT CHORD** WFBS

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing 1 Row at midpt

20-57, 19-58, 18-59, 16-61, 15-62, 14-63, 17-60, 21-55, 22-54, 24-52, 25-51, 26-50, 23-53

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS. All bearings 61-0-0.

(lb) - Max Horz 2=-167(LC 15)

Max Uplift All uplift 100 lb or less at joint(s) 2, 57, 58, 61, 62, 63, 64, 65, 67, 68, 69, 70, 71, 72, 55, 52, 51, 50, 49, 48, 46, 45, 44, 43, 42, 41, 40 except

Max Grav All reactions 250 lb or less at joint(s) 2, 38, 59, 69, 70, 71, 72, 60, 54, 44, 43, 42, 41, 53, 73, 40 except 57=291(LC 44), 58=292(LC 44), 61=254(LC 45),

62=294(LC 45), 63=287(LC 45), 64=286(LC 45), 65=287(LC 45), 67=289(LC 45), 68=266(LC 45), 55=292(LC 44), 52=254(LC 45), 51=294(LC 45), 50=287(LC 45),

49=286(LC 45), 48=287(LC 45), 46=289(LC 45), 45=266(LC 45)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

14-15=-111/263, 15-16=-127/299, 16-76=-135/317, 17-76=-130/320, 17-18=-130/312, 18-19=-130/312, 19-20=-130/312, 20-21=-130/312, 21-22=-130/312, 22-23=-130/312, 21-22=-130/312, 22-23=-130/312, 21-22=-130/312

23-77=-130/320, 24-77=-135/317, 24-25=-127/299, 25-26=-111/263

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 5-2-11, Exterior(2N) 5-2-11 to 19-10-13, Corner(3R) 19-10-13 to 41-1-3, Exterior(2N) 41-1-3 to 55-9-5, Corner(3E) 55-9-5 to 61-10-8 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Roug Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This trus has been designed for greater of min root live load of 12.0 psi of 2.00 minor mon-concurrent with other live loads.

7) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling, was a subject to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), with a qualified registered design jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.

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8) Provide adequate dramage to prevent water ponding.
Continuing by person parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Ī	Job	Truss	Truss Type	Qty	Ply	LOT 62 PROVIDENCE CREEK   29 WINDSWEPT WAY FUQUAY-VARINA, NC
	24-0289-R01	R01	GABLE	1	1	Job Reference (optional) # 49220
						8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 30 21:34:48 2024 Page 2 c5ybwyV6X0-?cbgnaSMWjipf2PYZMQJevajbmES4snkgoMuc?zBC95

#### NOTES-

9) All plates are 2x4 MT20 unless otherwise indicated.

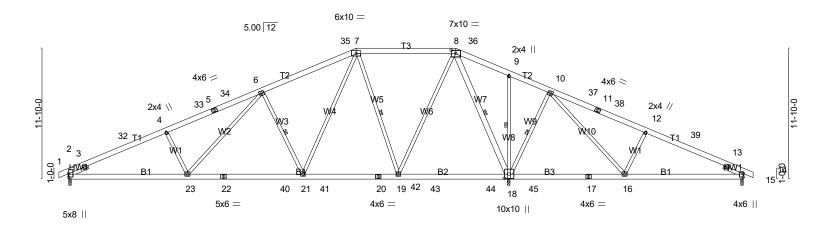
- 10) Gable requires continuous bottom chord bearing.
- 11) Gable studs spaced at 2-0-0 oc.
- 12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  13) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 57, 58, 61, 62, 63, 64, 65, 67, 68, 69, 70, 71, 72, 55, 52, 51, 50, 49, 48, 46, 45, 44, 43, 42, 41, 40 except (jt=lb) 73=105.

LOAD CASE(S) Standard



Job	lob Truss			Truss Type C		Qty	Ply	LOT 6	2 PROVIDENCE CREEK   29	WINDSWEPT WAY	FUQUAY-VARINA, NC
24-0289-R01 R02		Piggyback Base	;	3	1	Job R	deference (optional)	#	49220		
`									s Feb 12 2021 MiTek Industri V6X0-To92?wT_H1qgH0		
-0 <sub>1</sub> 10-8	8-10-5		17-5-3	26-0-0	35-0-0	39-9-	12   43	3-6-13	52-1-11	61-0-0	61 <sub>-</sub> 10-8
0-10-8	8-10-5	- 1	8-6-13	8-6-13	9-0-0	4-9-	12 3	3-9-1	8-6-13	8-10-5	0-10-8

Scale = 1:104.1



	0-8-14	10-5-6	+ <del>29-9-12</del> 8-7-8	10-0-0	10-5-6	10-8-14
Plate Offsets (X,Y)						
LOADING (psf)   TCLL (roof)   20.0   Snow (Pf)   20.0   TCDL   10.0   BCLL   0.0   BCDL   10.0	Pla Lui * Re	ACING- 2-0-0 ate Grip DOL 1.15 mber DOL 1.15 p Stress Incr YES de IRC2021/TPI2014	CSI. TC 0.84 BC 0.83 WB 0.93 Matrix-MSH	\ /	21-23 >999 240 21-23 >999 180	PLATES         GRIP           MT20         244/190           Weight: 466 lb         FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 2x4 SP No.3 \*Except\* WFBS

W7: 2x6 SP DSS SLIDER Left 2x4 SP No.3 -° 1-11-0, Right 2x4 SP No.3 -° 1-11-0 BRACING-

TOP CHORD BOT CHORD WFBS

Structural wood sheathing directly applied or 2-10-2 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing

6-21, 7-19, 8-18, 10-18, 9-18 1 Row at midpt

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 2=1412/0-3-8 (min. 0-1-14), 14=461/0-3-8 (min. 0-1-8), 18=3112/0-3-8 (min. 0-2-13)

Max Horz 2=-167(LC 15)

Max Uplift2=-222(LC 14), 14=-145(LC 15), 18=-232(LC 11) Max Grav 2=1577(LC 39), 14=583(LC 55), 18=4124(LC 45)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-975/1. 3-32=-2865/374. 4-32=-2774/390. 4-33=-2709/391. 5-33=-2628/394.

5-34=-2613/395, 6-34=-2510/412, 6-35=-1782/341, 7-35=-1533/343, 7-8=-646/242, 8-36=0/1500, 9-36=0/1429, 9-10=-26/1462, 10-37=-256/493, 11-37=-280/374,

11-38=-286/363, 12-38=-388/332, 12-39=-432/315, 13-39=-582/270, 13-14=-402/2 2-23=-433/2561, 22-23=-250/1951, 22-40=-250/1951, 21-40=-250/1951, 21-41=-26/1001,

20-41=-26/1001, 20-42=-26/1001, 19-42=-26/1001, 19-43=-256/220, 43-44=-256/220, 18-44=-256/220, 18-45=-905/158, 17-45=-905/158, 16-17=-905/158, 14-16=-249/483

**WEBS** 4-23=-421/229, 6-23=-131/727, 6-21=-1190/324, 7-21=-217/1520, 7-19=-1292/235,

8-19=-138/1835, 8-18=-2747/243, 10-18=-1061/267, 10-16=-158/1081, 12-16=-599/251,

9-18=-347/97

## NOTES-

**BOT CHORD** 

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof line.

non-concurrent with other live loads.

non-concurrent with other live loads.

WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general managing and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCST), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design and inspection of the temporary installation restraint/bracing and the permanent individual truss member 6) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.

7) Provide adequate drainage to prevent water ponding.

8) All plates are 5x5 MT20 unless otherwise indicated.

5/30/2024

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971 instructs has been designed for a 10.0 psi bottom choicing load nonconcurrent with any other loads.

Containing on pergy design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 62 PROVIDENCE CREEK   29 WINDSWEF	PT WAY FUQUAY-VARINA, NC
24-0289-R01	R02	Piggyback Base	3	1	Job Reference (optional)	# 49220

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 30 21:34:50 2024 Page 2 ID:av29u\_vm2cwLtXF0Wc5ybwyV6X0-x\_iQCGTc2KyXuMZxgnSnjKfsmZkVYba186r?htzBC93

#### NOTES-

- 10) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Bearing at joint(s) 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

12) Provide metal plate or equivalent at bearing(s) 18 to support reaction shown.

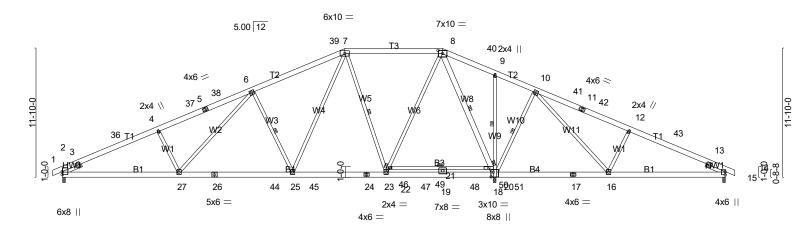
13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=222, 14=145, 18=232.

LOAD CASE(S) Standard



Job		Truss		Truss Type		Qty	Ply	LOT 62 PROVIDENCE CREEK	(   29 WINDSWEPT WAY	FUQUAY-VARINA, NC
24-0289-R01	-0289-R01 R02A			Piggyback Base 6		6	1 Job Reference (optional)		#	49220
								8.430 s Feb 12 2021 MiTek Ind Wc5ybwyV6X0-x_iQCGTc2		
-0 <sub>1</sub> 10-8	8-10-5	1	17-5-3	26-0-0	35-0-0	39-9-12	2   43-6	-13 52-1-11	61-0-0	61 <sub>г</sub> 1Q-8
0-10-8	8-10-5	- 1	8-6-13	8-6-13	9-0-0	4-9-12	3-9	-1 8-6-13	8-10-5	0-10-8

Scale = 1:106.0



<u> </u>	10-8-14	10-5-6	8-7-8	5-2-4	4-9-12	10-5-6	10-8-14	4
Plate Offsets (X,	Y) [8:0-5-0,0-3-7],		0.7.0	024	4012	1000	10 0 14	
Snow (Pf) 2 TCDL BCLL	20.0 Pla 20.0 Lur 10.0 Re	ACING- 2-0-0  Ite Grip DOL 1.15  mber DOL 1.15  p Stress Incr YES  de IRC2021/TPI2014	CSI. TC 0.85 BC 0.87 WB 0.93 Matrix-MSH	Ve Ve	FL. in (loc) rt(LL) -0.33 25-27 rt(CT) -0.50 25-27 rz(CT) 0.06 18	>957 180	MT20	<b>GRIP</b> 244/190 FT = 20%

LUMBER-BRACING-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 \*Except\*

B3: 2x4 SP No.2, B2: 2x6 SP No.1

**WEBS** 2x4 SP No.3 \*Except\*

W8: 2x6 SP DSS

SLIDER Left 2x4 SP No.3 -° 1-11-0, Right 2x4 SP No.3 -° 1-11-0 TOP CHORD

WFBS

BOT CHORD

Structural wood sheathing directly applied or 2-7-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing. Except:

6-0-0 oc bracing: 20-22

6-25, 7-23, 8-20, 10-18, 9-18 1 Row at midpt

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

REACTIONS. (lb/size) 2=1429/0-3-8 (min. 0-1-14), 14=448/0-3-8 (min. 0-1-8), 18=3298/0-3-8 (min. 0-3-2)

Max Horz 2=-167(LC 15)

Max Uplift2=-212(LC 14), 14=-148(LC 15), 18=-144(LC 11)

Max Grav 2=1593(LC 39), 14=572(LC 55), 18=4541(LC 45)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

2-3=-984/0, 3-36=-2959/350, 4-36=-2867/366, 4-37=-2803/367, 5-37=-2723/371,

5-38=-2707/372, 6-38=-2604/388, 6-39=-1877/316, 7-39=-1631/318, 7-8=-774/211, 8-40=0/1545, 9-40=0/1475, 9-10=-21/1508, 10-41=-228/556, 11-41=-252/438,

11-42=-258/426, 12-42=-360/394, 12-43=-404/377, 13-43=-554/330, 13-14=-402/0

**BOT CHORD**  $2 - 27 = -412/2646, \ 26 - 27 = -227/2039, \ 26 - 44 = -227/2039, \ 25 - 44 = -227/2039, \ 25 - 45 = -1/1105, \ 26 - 27 = -227/2039, \ 26 - 27 = -227/2039, \ 26 - 27 = -227/2039, \ 27 - 27 = -27/2$ 24-45=-1/1105, 24-46=-1/1105, 23-46=-1/1105, 23-47=-24/423, 19-47=-24/423,

19-48=-24/423, 18-48=-24/423, 22-49=-326/9, 21-49=-326/9, 21-50=-326/9, 20-50=-326/9,

18-51=-953/153, 17-51=-953/153, 16-17=-953/153, 14-16=-304/457

WEBS 4-27=-418/230, 6-27=-131/726, 6-25=-1190/323, 7-25=-224/1483, 7-23=-1264/251,

22-23=-117/1831, 8-22=-96/2043, 8-20=-2957/200, 18-20=-3174/181, 10-18=-1056/270,

10-16=-162/1061, 12-16=-602/250, 19-21=-323/0, 9-18=-348/96

## NOTES-

Unbalanced roof live loads have been considered for this design.

(envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 5-2-11, Interior(1) 5-2-11 to 17-4-8, Exterior(2R) 17-4-8 to 43-6-6, Interior(1) 43-6-6 to 55-9-5, Exterior(2E) 55-9-5 to 61-10-8 zone; cantilever left and right exposed; end vertical left and right exposed C-C for members and forces & MWFRS for reactions shown: Lumber DOL =1.60-71-1 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph, TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This trus has been designed for greater of min root live load of 12.0 psi of 2.00 minor mon-concurrent with other live loads.

6) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling, was a suited as a see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), and the suited see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), and the suited see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), and the suited see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), and the suited seed of t jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.

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7) Provide adequate dramage to prevent water ponding.
Continuing by pegip 2 lesign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 62 PROVIDENCE CREEK   29 WINDSWEPT	WAY FUQUAY-VARINA, N
24-0289-R01	R02A	Piggyback Base	6	1	Job Reference (optional)	# 49220

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#### NOTES-

- 8) All plates are 5x5 MT20 unless otherwise indicated.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Bearing at joint(s) 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 12) Provide metal plate or equivalent at bearing(s) 18 to support reaction shown.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=212, 14=148, 18=144.

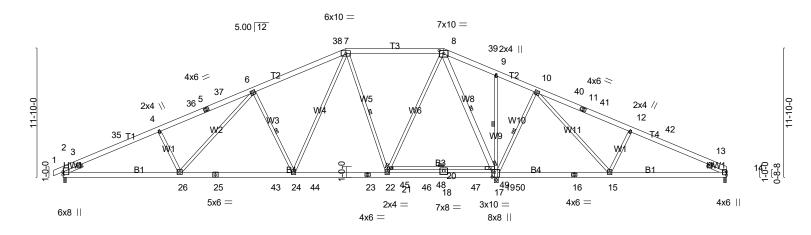
LOAD CASE(S) Standard



Warning!—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job		Truss		Truss Type		Qty	Ply	LOT 62 PROVIDENCE CREE	K   29 WINDSWEPT WAY	FUQUAY-VARINA, NC
24-0289-R01		R03		PIGGYBACK BASE		1	1	Job Reference (optional)	# .	49220
								: 8.430 s Feb 12 2021 MiTek In V6X0-PBGpQcUEpe4OWV		
-0 <sub>t</sub> 10-8	8-10-5	1	17-5-3	26-0-0	35-0-0	39-9-1	2   43-6	5-13 <sub>1</sub> 52-1-11	61-0-0	1
0-10-8	8-10-5	ı	8-6-13	8-6-13	9-0-0	4-9-12	2 3-9	9-1 8-6-13	8-10-5	

Scale = 1:105.9



	10-8-14		10-5-6	8-7-8	+	5-2-4 4-9-	·12	10-5-6	-	10-8-14	$\dashv$
Plate Offs	ets (X,Y) [8:0-5	-0,0-3-7], [17:0-5-4	,0-2-8]								
LOADING TCLL (roo Snow (Pf) TCDL BCLL BCDL	f) 20.0	SPACING- Plate Grip I Lumber DO Rep Stress Code IRC20	1.15	ВС	0.85 0.87 0.93 -MSH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.33 24-26 -0.50 24-26 0.06 17	I/defl >999 >956 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 478 lb	<b>GRIP</b> 244/190 FT = 20%

29-9-12

35-0-0

LUMBER-BRACING-

21-2-4

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 \*Except\*

B3: 2x4 SP No.2, B2: 2x6 SP No.1

**WEBS** 2x4 SP No.3 \*Except\*

W8: 2x6 SP DSS

10-8-14

SLIDER Left 2x4 SP No.3 -° 1-11-0, Right 2x4 SP No.3 -° 1-11-0

WFBS

TOP CHORD BOT CHORD

39-9-12

Structural wood sheathing directly applied or 2-7-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing. Except:

6-0-0 oc bracing: 19-21

50-3-2

6-24, 7-22, 8-19, 10-17, 9-17 1 Row at midpt

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

61-0-0

REACTIONS. (lb/size) 2=1430/0-3-8 (min. 0-1-14), 14=397/0-3-8 (min. 0-1-8), 17=3295/0-3-8 (min. 0-3-2)

Max Horz 2=172(LC 14)

Max Uplift2=-211(LC 14), 14=-129(LC 15), 17=-147(LC 11) Max Grav 2=1594(LC 39), 14=520(LC 55), 17=4539(LC 45)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-985/0, 3-35=-2961/349, 4-35=-2869/365, 4-36=-2806/366, 5-36=-2725/369,

5-37=-2709/370, 6-37=-2606/387, 6-38=-1879/314, 7-38=-1629/316, 7-8=-773/208, 8-39=0/1540, 9-39=0/1470, 9-10=-24/1504, 10-40=-237/547, 11-40=-261/430,

11-41=-267/418, 12-41=-369/391, 12-42=-413/374, 13-42=-563/326, 13-14=-411/0

**BOT CHORD** 2-26=-416/2648, 25-26=-231/2042, 25-43=-231/2042, 24-43=-231/2042, 24-44=-5/1102,

23-44=-5/1102, 23-45=-5/1102, 22-45=-5/1102, 22-46=-28/418, 18-46=-28/418,

18-47=-28/418, 17-47=-28/418, 21-48=-326/9, 20-48=-326/9, 20-49=-326/9, 19-49=-326/9,

17-50=-948/143, 16-50=-948/143, 15-16=-948/143, 14-15=-301/466 4-26=-418/230, 6-26=-132/726, 6-24=-1190/323, 7-24=-224/1483, 7-22=-1262/252,

21-22=-118/1830, 8-21=-97/2041, 8-19=-2954/203, 17-19=-3171/184, 10-17=-1057/270,

10-15=-163/1064, 12-15=-604/251, 18-20=-323/0, 9-17=-348/97

## NOTES-

**WEBS** 

Unbalanced roof live loads have been considered for this design.

(envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 5-2-11, Interior(1) 5-2-11 to 17-4-8, Exterior(2R) 17-4-8 to 43-6-6, Interior(1) 43-6-6 to 54-10-13, Exterior(2E) 54-10-13 to 61-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C formembers and forces & MWFRS for reactions shown: Lumber DOL =1.60-114 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph, TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This trus has been designed for greater of min root live load of 12.0 psi of 2.00 minor mon-concurrent with other live loads.

6) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling, was a suidence see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), which is a qualified registered design. jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.

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7) Provide adequate dramage to prevent water ponding.
Continuing by pegip 2 lesign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 62 PROVIDENCE CREEK   29 WINDSWEPT V	VAY FUQUAY-VARINA, NC
24-0289-R01	R03	PIGGYBACK BASE	1	1	Job Reference (optional)	# 49220

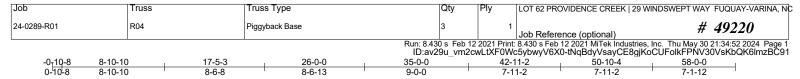
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 30 21:34:51 2024 Page 2 ID:av29u\_vm2cwLtXF0Wc5ybwyV6X0-PBGpQcUEpe4OWW87EVz0FXB1Qz33H2mAMmaYDKzBC92

#### NOTES-

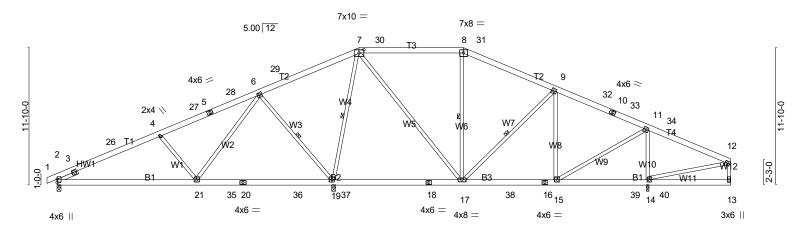
- 8) All plates are 5x5 MT20 unless otherwise indicated.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Bearing at joint(s) 17 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 12) Provide metal plate or equivalent at bearing(s) 17 to support reaction shown.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=211, 14=129, 17=147.

LOAD CASE(S) Standard





Scale = 1:99.2



	12-1		23-9-12	33-0-	.0	42-11-2	1	50-10-4		1
	12-0	0-0	11-9-12	11-2-	4 '	7-11-2	'	7-11-2	7-1-12	<u>'</u>
Plate Offs	ets (X,Y) [7:0-5	-0,0-3-7]								
LOADING TCLL (roc Snow (Pf) TCDL BCLL BCDL	f) 20.0		15 TC 15 BC ES WB	0.64 0.50 0.94 x-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.17 19-21 -0.22 19-21 0.02 13	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 435 lb	<b>GRIP</b> 244/190 FT = 20%

25 N N

LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 \*Except\*

B2: 2x6 SP DSS WFBS 2x4 SP No.3 \*Except\*

W4: 2x4 SP No.2 SLIDER Left 2x4 SP No.3 -° 1-11-0 BRACING-

TOP CHORD **BOT CHORD** WFBS

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

50 N N

6-19, 7-19, 8-17, 9-17 1 Row at midpt

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 0-3-8 except (jt=length) 13=Mechanical.

(lb) - Max Horz 2=172(LC 18)

12 0 0

Max Uplift All uplift 100 lb or less at joint(s) 13 except 2=-126(LC 14), 19=-276(LC 14), 14=-217(LC 15) Max Grav All reactions 250 lb or less at joint(s) 13 except 2=882(LC 39), 19=3049(LC 45), 14=1599(LC 39)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-707/0, 3-26=-1232/167, 4-26=-1107/183, 4-27=-923/136, 5-27=-813/148,

5-28=-805/149, 6-28=-697/166, 6-29=-24/436, 7-29=-22/714, 7-30=-611/247, 8-30=-611/247, 8-31=-574/227, 9-31=-756/225, 9-32=-800/185, 10-32=-881/169,

22 0 12

10-33=-905/166, 11-33=-975/157

2-21=-246/1075, 21-35=-38/311, 20-35=-38/311, 20-36=-38/311, 19-36=-38/311,

17-38=-17/813, 16-38=-17/813, 15-16=-17/813

**WEBS** 4-21=-527/246, 6-21=-97/840, 6-19=-1309/325, 7-19=-1600/239, 7-17=-98/1109, 8-17=-401/111, 9-17=-483/154, 9-15=-350/116, 11-15=-49/993, 11-14=-1397/286

## NOTES-

**BOT CHORD** 

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 4-11-2, Interior(1) 4-11-2 to 17-9-9, Exterior(2R) 17-9-9 to 42-11-2, Interior(1) (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 4-11-2, Interior(1) 4-11-2 to 17-9-9, Exterior(2R) 17-9-9 to 42-11-2, Interior(1) 42-11-2 to 52-0-10, Exterior(2E) 52-0-10 to 57-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

7) All plates are 5x5 MT20 unless otherwise indicated.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

10) Refer to girder(s) for truss to truss connections.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13 except (jt=lb)

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13 except (jt=lb) 2=126, 19=276, 14=217,

5/30/2024

Continuing by period assign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 62 PROVIDENCE CREEK   29 WINDSWEPT	WAY FUQUAY-VARINA, NC
24-0289-R01	R04	Piggyback Base	3		Job Reference (optional)	# 49220

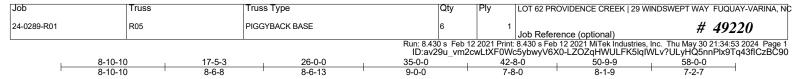
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 30 21:34:52 2024 Page 2 ID:av29u\_vm2cwLtXF0Wc5ybwyV6X0-tNqBdyVsayCE8gjKoCUFolkFPNV30VsKbQK6lmzBC91

NOTES-12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

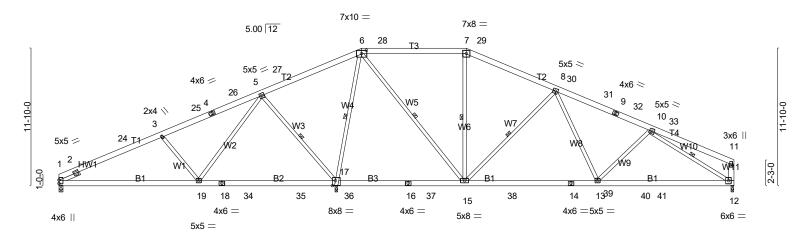
LOAD CASE(S) Standard



5/30/2024



Scale = 1:99.0



	12-0	0-0	11-9-12		11-2-4	11-3-4		11-8-12			
Plate Offs	Plate Offsets (X,Y) [6:0-5-0,0-3-7], [17:0-3-0,0-5-4]										
LOADING TCLL (roo Snow (Pf) TCDL BCLL BCDL		SPACING- Plate Grip D Lumber DOI Rep Stress Code IRC20	1.15	CSI. TC 0.65 BC 0.75 WB 1.00 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) I/defl -0.28 17-19 >999 -0.37 17-19 >769 0.04 12 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 427 lb	<b>GRIP</b> 244/190 FT = 20%		

35-0-0

LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2

2x4 SP No.3 \*Except\* WFBS W4: 2x4 SP SS, W11: 2x6 SP No.2

SLIDER

Left 2x4 SP No.3 -° 1-11-0

BRACING-

TOP CHORD **BOT CHORD** WFBS

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

58-0-0

5-17, 6-17, 6-15, 7-15, 8-15, 10-12 1 Row at midpt MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer Installation guide.

46-3-4

(lb/size) 1=674/0-3-8 (min. 0-1-8), 17=2787/0-3-8 (min. 0-1-8), 12=1160/0-3-8 (min. 0-1-9) REACTIONS.

23-9-12

Max Horz 1=161(LC 18)

12-0-0

Max Uplift1=-110(LC 14), 17=-272(LC 14), 12=-188(LC 15) Max Grav 1=743(LC 40), 17=3632(LC 44), 12=1332(LC 38)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-639/0, 2-24=-1036/167, 3-24=-936/184, 3-25=-738/121, 4-25=-644/134,

4-26=-635/135, 5-26=-543/151, 5-27=-19/790, 6-27=-17/1058, 6-28=-735/260, 7-28=-735/260. 7-29=-721/243. 8-29=-873/242. 8-30=-1555/279. 30-31=-1659/277

9-31=-1674/263, 9-32=-1697/260, 10-32=-1767/251, 11-33=-308/74, 11-12=-288/99

1-19=-247/896, 17-36=-453/172, 16-36=-453/172, 16-37=-453/172, 15-37=-453/172

15-38=-108/1349, 14-38=-108/1349, 14-39=-108/1349, 13-39=-108/1349, 13-40=-234/1579,

40-41=-234/1579, 12-41=-234/1579

WEBS 3-19=-546/248, 5-19=-91/914, 5-17=-1329/321, 6-17=-2201/292, 6-15=-160/1725,

7-15=-338/105, 8-15=-1122/261, 8-13=-4/510, 10-12=-1674/266

### NOTES-

BOT CHORD

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 5-9-10, Interior(1) 5-9-10 to 17-9-9, Exterior(2R) 17-9-9 to 43-2-7, Interior(1) 43-2-7 to 51-11-10, Exterior(2E) 51-11-10 to 57-9-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) Provide adequate drainage to prevent water ponding.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
8) Bearing at joint(s) 17 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=110, 17=272, 12=188.
10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 17. (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 5-9-10, Interior(1) 5-9-10 to 17-9-9, Exterior(2R) 17-9-9 to 43-2-7, Interior(1)

10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 17.

11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum

5/30/2024

stock be applied directly to the bottom chord.

g on page Zesign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

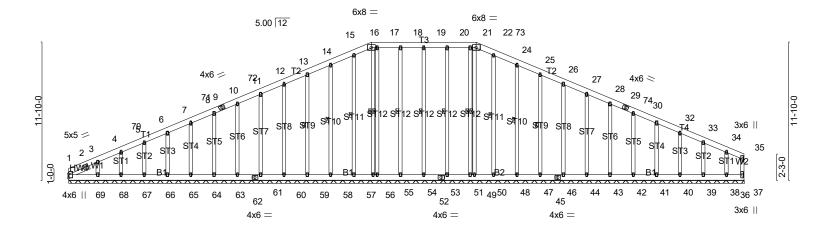
Job		Truss	Truss Type	Qty	Ply	LOT 62 PROVIDENCE CREEK   29 WINDSWEPT	WAY	FUQUAY-VARINA, NC		
24-02	289-R01	R05	PIGGYBACK BASE	6	1	Job Reference (optional)	#	49220		
	Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Índustries, Inc. Thu May 30 21:34:53 2024 Page ID:av29u_vm2cwLtXF0Wc5ybwyV6X0-LZOZqHWULFK5IqIWLv?ULyHQ5nnPlx9Tq43flCzBC9l									

LOAD CASE(S) Standard



Job Truss Truss Type LOT 62 PROVIDENCE CREEK | 29 WINDSWEPT WAY FUQUAY-VARINA, N 24-0289-R01 R06 GABLE # 49220 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 30 21:34:55 2024 Page ID:av29u\_vm2cwLtXF0Wc5ybwyV6X0-HyWJFzXIstap?7SuTK2yQNMv8ad6D1WmHOYmM5zBC9 26-0-0 58-0-0 26-0-0 9-0-0 23-0-0

Scale = 1:98.9



58-0-0										
COADING (psf)   TCLL (roof)   20.0   Snow (Pf)   20.0   TCDL   10.0   BCLL   0.0 *   BCDL   10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.10 BC 0.03 WB 0.25 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 36	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES GRIP MT20 244/190 Weight: 603 lb FT = 20%				

58-0-0

LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WFBS 2x4 SP No.3

OTHERS 2x4 SP No 3

Left 2x4 SP No.3 -° 1-11-0 SLIDER

BRACING-

TOP CHORD

**BOT CHORD** WFBS

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

19-53, 18-54, 17-55, 15-57, 14-58, 13-59, 1 Row at midpt 16-56, 20-51, 21-50, 23-48, 24-47, 25-46, 22-49

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 58-0-0.

(lb) - Max Horz 1=174(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 1, 53, 54, 57, 58, 59, 60, 61, 63, 64, 65, 66, 67, 68, 51, 48, 47, 46, 44, 43, 42, 41, 40, 39, 38, 37 except 69=-103(LC

Max Grav All reactions 250 lb or less at joint(s) 1, 55, 65, 66, 67, 68, 56, 50, 40,

39, 38, 37, 49, 36, 69 except 53=291(LC 43), 54=292(LC 43), 57=255(LC 44), 58=294(LC 44), 59=287(LC 44), 60=286(LC 44), 61=287(LC 44), 63=289(LC 44), 64=266(LC 44), 51=292(LC 43), 48=255(LC 44), 47=294(LC 44), 46=287(LC 44),

44=286(LC 44), 43=287(LC 44), 42=289(LC 44), 41=266(LC 44)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 11-72=-95/250, 11-12=-114/285, 12-13=-128/319, 13-14=-143/354, 14-15=-159/390,

15-16=-166/408, 16-17=-160/396, 17-18=-159/395, 18-19=-159/395, 19-20=-159/395,

20-21=-159/395, 21-22=-160/396, 22-73=-161/408, 23-73=-166/406, 23-24=-159/390,

24-25=-143/354, 25-26=-128/319, 26-27=-114/285, 27-28=-99/250

## NOTES-

Unbalanced roof live loads have been considered for this design.

OTESUnbalanced roof live loads have been considered for this design.
Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) 0-0-0 to 5-9-10, Exterior(2N) 5-9-10 to 20-2-6, Corner(3R) 20-2-6 to 40-6-0, Exterior(2N) 40-6-0 to 52-0-10, Corner(3E) 52-0-10 to 57-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry
Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15); Is=1.0; Reugh
Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
Unbalanced snow loads have been considered for this design.
Provide adequate drainage to prevent water ponding.
All plates are 2x4 MT20 unless otherwise indicated.
Gable requires continuous bottom chord bearing.
Gable studs spaced at 2-0-0 oc. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph, TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

5) Unbalanced snow loads have been considered for this design.

6) Provide adequate drainage to prevent water ponding.

7) All plates are 2x4 MT20 unless otherwise indicated.

8) Gable requires continuous bottom chord bearing.

9) Gable studs spaced at 2-0-0 oc.

10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5/30/2024

Continuing by period assign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 62 PROVIDENCE CREEK   29 WINDSWEPT V	VAY FUQUAY-VARINA, NC
24-0289-R01	R06	GABLE	1		Job Reference (optional)	# 49220

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#### NOTES-

- 11) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 53, 54, 57, 58, 59, 60, 61, 63, 64, 65, 66, 67, 68, 51, 48, 47, 46, 44, 43, 42, 41, 40, 39, 38, 37 except (jt=lb) 69=103.

LOAD CASE(S) Standard



Job Truss Truss Type Qtv LOT 62 PROVIDENCE CREEK | 29 WINDSWEPT WAY FUQUAY-VARINA, N 24-0289-R01 R07 Common Structural Gable # 49220 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 30 21:34:57 2024 Page 1 ID:av29u\_vm2cwLtXF0Wc5ybwyV6X0-ELd4gfZ?OUqXERcHal4QVoRCNOGvhul3li1tQzzBC8y 19-5-0 20-11-0 -1-6-0 9-8-8 13-8-8 17-8-8 1-6-0 1-8-8 4-0-0 4-0-0 4-0-0 4-0-0 1-8-8 1-6-0 Scale = 1:44.6 4x4 = 5 7.00 12 3x6 / 3x6 < 6 29 28 stтb 30 27 3x6 < 3x6 < 3 3x6 / 3x6 < M/6 Bî⊳ **⊿**81 ∭ 16 ₩ 10 31 32 33 34 14 13 12 11 3x6 || 3x6 = 3x4 =5x8 = 3x4 3x6 3x6 || 19-5-0 4-0-0 4-0-0 1-8-8

Plate Offsets (X,Y)-- [13:0-4-0,0-3-0]

LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDI 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	<b>CSI.</b> TC 0.31 BC 0.20	DEFL.         in (loc)         l/defl         L/d           Vert(LL)         0.03 12-13         >999         240           Vert(CT)         -0.04 12-13         >999         180
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2021/TPI2014	WB 0.42 Matrix-AS	Vert(CT) -0.04 12-13 >999 180 Horz(CT) 0.01 10 n/a n/a

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

**PLATES** 

Weight: 156 lb

MT20

GRIP

244/190

FT = 20%

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No 3 WFBS 2x4 SP No 3 OTHERS

**REACTIONS.** (lb/size) 16=864/0-3-0 (min. 0-1-8), 10=864/0-3-0 (min. 0-1-8)

Max Horz 16=-186(LC 12)

Max Uplift16=-118(LC 14), 10=-118(LC 15) Max Grav 16=906(LC 21), 10=906(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-620/460, 3-27=-892/614, 27-28=-842/615, 4-28=-777/626, 4-5=-721/558,

5-6=-721/558, 6-29=-777/626, 29-30=-842/615, 7-30=-892/614, 7-8=-620/460,

2-16=-894/590, 8-10=-894/590

15-31=-293/509, 31-32=-293/509, 14-32=-293/509, 13-14=-397/728, 12-13=-397/728, **BOT CHORD** 

12-33=-293/509, 33-34=-293/509, 11-34=-293/509

**WEBS** 5-13=-464/395, 6-13=-295/202, 7-11=-380/166, 4-13=-295/202, 3-15=-380/166,

2-15=-430/682, 8-11=-430/682

### NOTES-

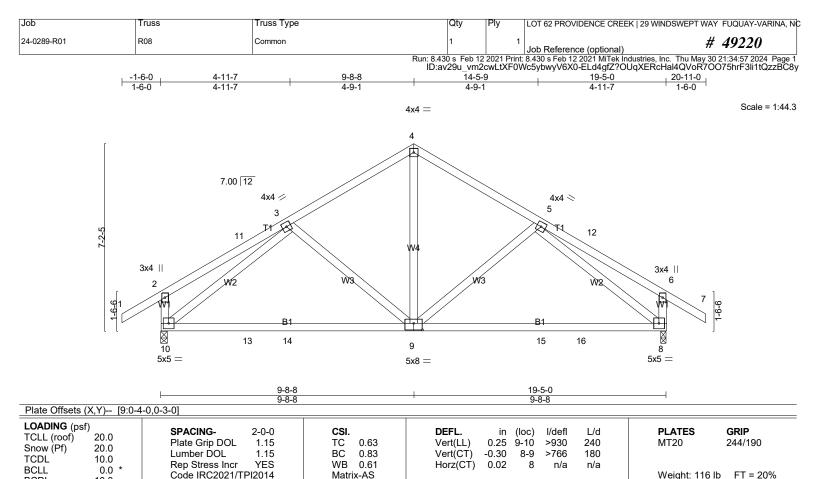
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 3-3-10, Interior(1) 3-3-10 to 4-10-14, Exterior(2R) 4-10-14 to 14-6-2, Interior(1) 14-6-2 to 16-1-6, Exterior(2E) 16-1-6 to 20-11-0 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

- 12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

5/30/2024

LONG PRINCIPLE SET STATES OF STATES vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



**BCDL** LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No 3 WFBS

10.0

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

Weight: 116 lb

FT = 20%

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

**REACTIONS.** (lb/size) 10=864/0-3-0 (min. 0-1-8), 8=864/0-3-0 (min. 0-1-8)

Max Horz 10=-186(LC 12)

Max Uplift10=-118(LC 14), 8=-118(LC 15) Max Grav 10=909(LC 21), 8=909(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-11=-223/261, 3-11=-181/282, 3-4=-773/602, 4-5=-773/602, 5-12=-181/282,

6-12=-223/261, 2-10=-308/237, 6-8=-308/237

**BOT CHORD** 10-13=-357/720, 13-14=-357/720, 9-14=-357/720, 9-15=-357/720, 15-16=-357/720,

8-16=-357/720

**WEBS** 4-9=-482/427, 3-10=-820/322, 5-8=-820/322

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph, TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 3-3-10, Interior(1) 3-3-10 to 4-10-1, Exterior(2R) 4-10-1 to 14-6-15, Interior(1) 14-6-15 to 16-1-6, Exterior(2E) 16-1-6 to 20-11-0 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-AS

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Unbalanced snow loads have been considered for this design.

  This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

  This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

  This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

  This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide with the between the bottom chord and any other members.

  Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=118

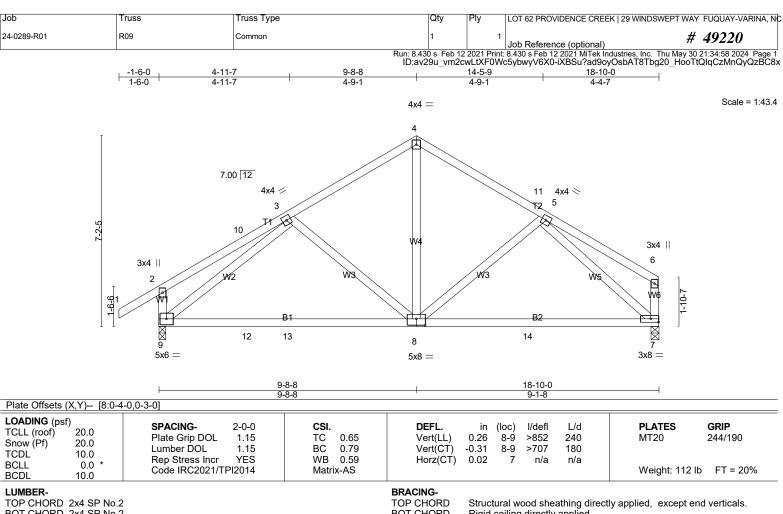
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  This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

  OAD CASE(S) Standard
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=18
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum

LOAD CASE(S) Standard

5/30/2024



TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No 3 WFBS

BOT CHORD

Rigid ceiling directly applied.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

**REACTIONS.** (lb/size) 9=845/0-3-0 (min. 0-1-8), 7=737/0-3-8 (min. 0-1-8)

Max Horz 9=183(LC 13)

Max Uplift9=-116(LC 14), 7=-81(LC 15) Max Grav 9=892(LC 21), 7=784(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-10=-228/268, 3-10=-186/289, 3-4=-745/572, 4-11=-615/572, 5-11=-739/550,

2-9=-309/242

**BOT CHORD** 9-12=-378/700, 12-13=-378/700, 8-13=-378/700, 8-14=-344/651, 7-14=-344/651

4-8=-452/398, 3-9=-791/287, 5-7=-805/372

## NOTES-

WFBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph, TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 3-3-10, Interior(1) 3-3-10 to 4-10-1, Exterior(2R) 4-10-1 to 13-10-10, Exterior(2É) 13-10-10 to 18-8-4 zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7 except (jt=lb) 9-\$16. 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum

LOAD CASE(S) Standard

This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

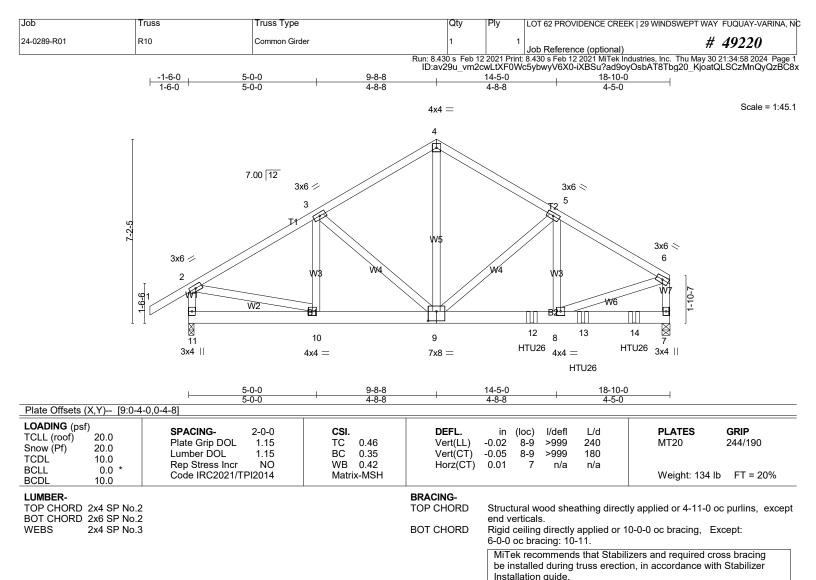
This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7 except (jt=lb) 9=16.

This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

OAD CASE(S) Standard

5/30/2024



REACTIONS. (lb/size) 11=945/0-3-0 (min. 0-1-8), 7=1207/0-3-8 (min. 0-1-8)

Max Horz 11=180(LC 9)

Max Uplift11=-153(LC 12), 7=-251(LC 13) Max Grav 11=987(LC 19), 7=1249(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1056/156, 3-4=-887/194, 4-5=-882/196, 5-6=-1238/239, 2-11=-931/171, TOP CHORD

6-7=-1047/210

**BOT CHORD** 9-10=-213/854, 9-12=-179/1027, 8-12=-179/1027

3-9=-258/134, 4-9=-111/513, 5-9=-492/207, 2-10=-79/832, 6-8=-155/1025 WEBS

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide with fit between the bottom chord and any other mambers. between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=353
- 9) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 13-5-4 from the left end to 17-5-4 to connect truss(es) R04 (1 ply 2x6 SP) to back face of bottom chord.

10) Fill all nail holes where hanger is in contact with lumber.

11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

SEAL 28147

NOINEER MORRESULLING

(30/20 Continuing by period assign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 62 PROVIDENCE CREEK   29 WINDSWEPT	WAY FUQUAY-VARINA, N
24-0289-R01	R10	Common Girder	1	1	Job Reference (optional)	# 49220

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LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-4=-60, 4-6=-60, 7-11=-20

Concentrated Loads (lb)

Vert: 12=-190(B) 13=-190(B) 14=-190(B)



5/30/2024

Job Truss Type Truss LOT 62 PROVIDENCE CREEK | 29 WINDSWEPT WAY FUQUAY-VARINA, NO 24-0289-R01 R11 Common Supported Gable # 49220 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MTek Industries, Inc. Thu May 30 21:34:59 2024 Page 1 ID:av29u\_vm2cwLtXF0Wc5ybwyV6X0-Ajlq5LaFw54FTllgiA6vaDXasC?l9uiLC0WzUszBC8w 12-10-8 6-0-0 12-0-0 -0-10-8 0-10-8 6-0-0 6-0-0 0-10-8

4x4 = 5 4.00 12 6 ST3 3x4 || 3x4 || 8 ST2 2 18 17 Ñ 16 15 14 13 12 11 10

<u> </u>		—						
TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.12 BC 0.05 WB 0.04 Matrix-R	DEFL. i Vert(LL) -0.0 Vert(CT) -0.0 Horz(CT) 0.0	0 9	l/defl n/r n/r n/a	L/d 180 80 n/a	PLATES MT20 Weight: 52 lb	<b>GRIP</b> 244/190 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WFBS 2x4 SP No.3 2x4 SP No 3 **OTHERS** 

3x4 ||

BRACING-

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals

Scale = 1:21.6

3x4 ||

Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS. All bearings 12-0-0.

(lb) - Max Horz 16=15(LC 18)

Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 15, 12, 11 Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 4-0-0, Corner(3R) 4-0-0 to 8-0-0, Corner(3E) 8-0-0 to 12-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

10) Gable studs spaced at 2-0-0 oc.

- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide 🔊 🛚 fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15,

LOAD CASE(S) Standard

SEAL 28147

MONEE MORRISHING

7/202

Warning !--Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job Truss Truss Type LOT 62 PROVIDENCE CREEK | 29 WINDSWEPT WAY FUQUAY-VARINA, N 24-0289-R01 R12 Common # 49220 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MITek Industries, Inc. Thu May 30 21:34:59 2024 Page 1 ID:av29u\_vm2cwLtXF0Wc5ybwyV6X0-Ajlq5LaFw54FTllgiA6vaDXVFCwP9t1LC0WzUszBC8w 12-10-8 6-0-0 12-0-0 -0-10-8 0-10-8 6-0-0 6-0-0 0-10-8 Scale = 1:21.6 4x4 =4.00 12 18 4x4 = 5 4x4 < 3 W1 6 HW1 HW1 B٦ 19 20 8 2x4 || 3x8 || 3x8 || 12-0-0 6-0-0 6-0-0 Plate Offsets (X,Y)-- [2:Edge,0-0-0], [6:Edge,0-0-0] LOADING (psf) DEFL. **PLATES** GRIP SPACING-2-0-0 CSI. in (loc) I/defl I/d TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.41 Vert(LL) 0.05 8-15 >999 240 MT20 244/190 Snow (Pf) 20.0 Lumber DOL 1.15 вс 0.33 Vert(CT) -0.06 8-15 >999 180 TCDL 10.0 WB 0.08 Rep Stress Incr YES Horz(CT) 0.02 6 n/a n/a

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

**BCLL** 

**BCDL** 

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No 3 WFBS

0.0

10.0

Left 2x6 SP No.2 -° 1-11-0, Right 2x6 SP No.2 -° 1-11-0 SLIDER

**REACTIONS.** (lb/size) 2=532/0-3-8 (min. 0-1-8), 6=533/0-3-8 (min. 0-1-8)

Max Horz 2=-34(LC 19)

Max Uplift2=-197(LC 10), 6=-197(LC 11) Max Grav 2=614(LC 21), 6=614(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code IRC2021/TPI2014

TOP CHORD 2-3=-187/278, 3-17=-648/918, 4-17=-620/925, 4-18=-620/925, 5-18=-648/918,

5-6=-187/278

BOT CHORD 2-19=-783/593, 8-19=-783/593, 8-20=-783/593, 6-20=-783/593

4-8=-359/219 WEBS

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Exterior(2R) 3-11-2 to 8-0-14, Exterior(2E) 8-0-14 to 12-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-AS

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=19

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum

LOAD CASE(S) Standard

This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=197, 6=197.

This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

OAD CASE(S) Standard

Weight: 52 lb

Structural wood sheathing directly applied.

MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer

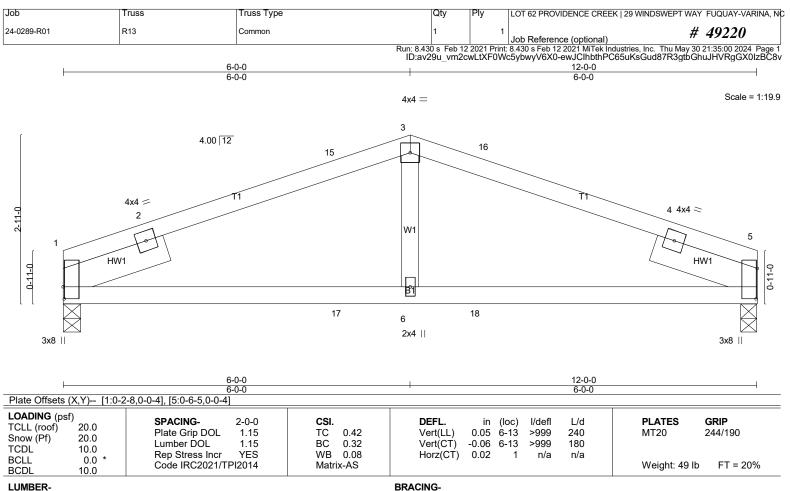
Rigid ceiling directly applied.

Installation guide

FT = 20%

5/30/2024

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TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer

Rigid ceiling directly applied.

Installation guide

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WFBS

Left 2x6 SP No.2 -° 1-11-0, Right 2x6 SP No.2 -° 1-11-0 SLIDER

**REACTIONS.** (lb/size) 1=480/0-3-8 (min. 0-1-8), 5=480/0-3-8 (min. 0-1-8)

Max Horz 1=-30(LC 15)

Max Uplift1=-165(LC 10), 5=-165(LC 11) Max Grav 1=561(LC 20), 5=561(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-200/301, 2-15=-657/901, 3-15=-618/908, 3-16=-618/908, 4-16=-657/901,

4-5=-200/301

BOT CHORD 1-17=-788/604, 6-17=-788/604, 6-18=-788/604, 5-18=-788/604

3-6=-353/220 WEBS

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

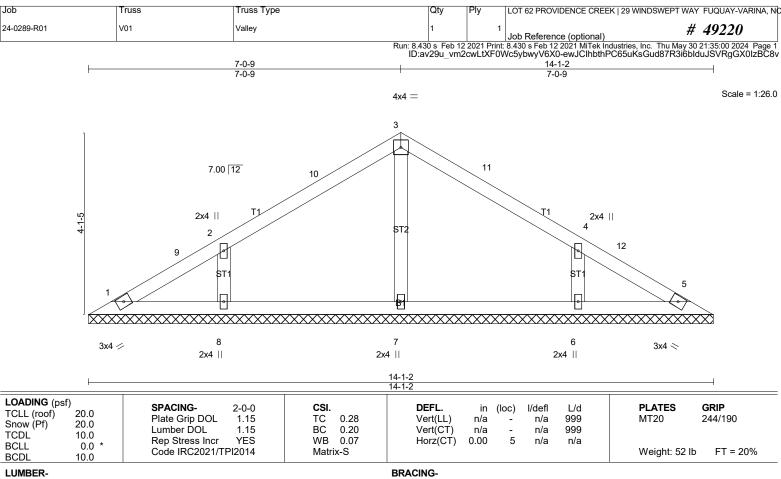
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 4-9-10, Exterior(2R) 4-9-10 to 7-2-6, Exterior(2E) 7-2-6 to 12-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B: Partially Exp.: Ce=1.0: Cs=1.00: Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=165 5=165.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard IM AND MARKET OF THE PARTY OF T

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TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 OTHERS 2x4 SP No.3 TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS.** All bearings 14-1-2.

(lb) - Max Horz 1=84(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-108(LC 14), 6=-108(LC 15)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=290(LC 21), 8=429(LC 20), 6=429(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-8=-359/143, 4-6=-359/143

## NOTES-

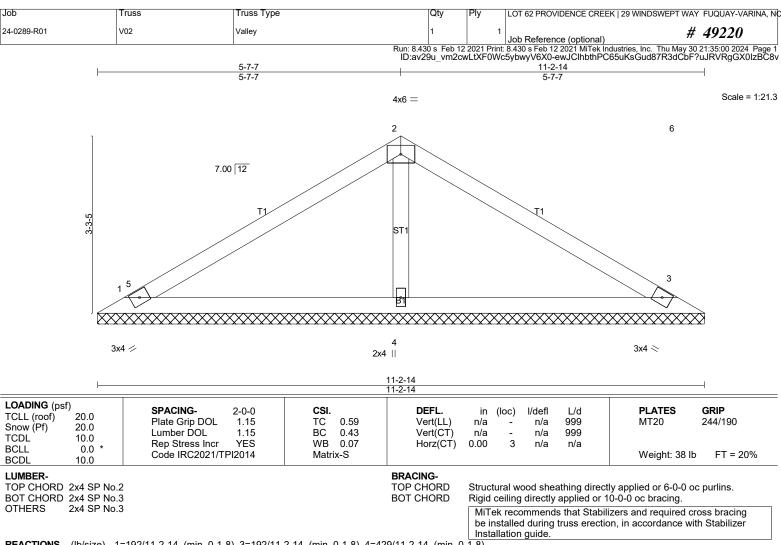
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-6-8 to 5-4-1, Exterior(2R) 5-4-1 to 8-9-1, Exterior(2E) 8-9-1 to 13-6-11 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=108, 6=108.

LOAD CASE(S) Standard



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REACTIONS. (lb/size) 1=192/11-2-14 (min. 0-1-8), 3=192/11-2-14 (min. 0-1-8), 4=429/11-2-14 (min. 0-1-8)

Max Horz 1=-65(LC 10)

Max Uplift1=-37(LC 14), 3=-46(LC 15), 4=-23(LC 14) Max Grav 1=272(LC 20), 3=272(LC 21), 4=449(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-4=-299/105

## NOTES-

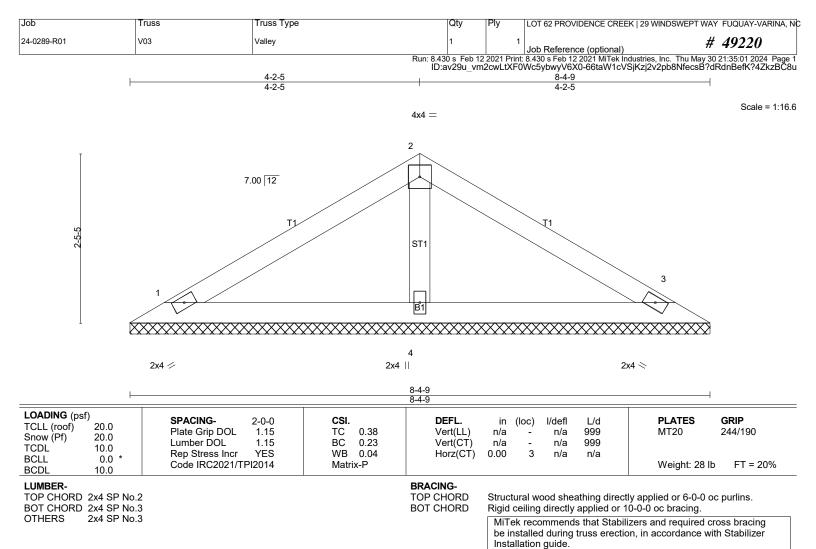
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard



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REACTIONS. (lb/size) 1=152/8-4-9 (min. 0-1-8), 3=152/8-4-9 (min. 0-1-8), 4=280/8-4-9 (min. 0-1-8)

Max Horz 1=47(LC 13)

Max Uplift1=-33(LC 14), 3=-40(LC 15), 4=-2(LC 14) Max Grav 1=209(LC 20), 3=209(LC 21), 4=284(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES

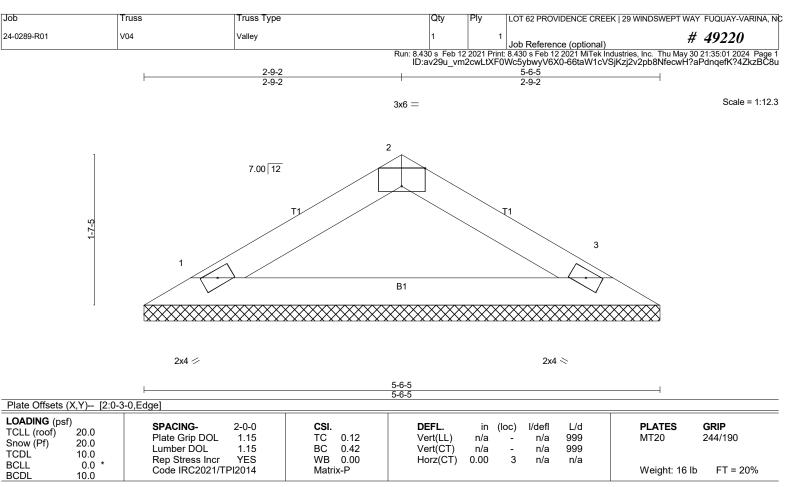
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard



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LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 **BRACING-**

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-6-5 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS.** (lb/size) 1=178/5-6-5 (min. 0-1-8), 3=178/5-6-5 (min. 0-1-8)

Max Horz 1=29(LC 13)

Max Uplift1=-21(LC 14), 3=-21(LC 15) Max Grav 1=203(LC 20), 3=203(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



5/30/2024

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Job Truss Truss Type LOT 62 PROVIDENCE CREEK | 29 WINDSWEPT WAY FUQUAY-VARINA, N 24-0289-R01 V05 Valley # 49220 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 30 21:35:01 2024 Page 1 ID:av29u\_vm2cwLtXF0Wc5ybwyV6X0-66taW1cVSjKzj2v2pb8NfecwC?fWdnlefK?4ZkzBČ8u 6-6-0 3-3-0 3-3-0 Scale = 1:11.7 4x4 = 4.00 12 2 W1 3 В1 4 2x4 = 2x4 || 2x4 > 6-6-0 3-3-0 LOADING (psf) SPACING-GRIP CSI. DEFL. **PLATES** 2-0-0 in (loc) I/defl L/d TCLL (roof) 20.0 244/190 Plate Grip DOL 1.15 TC 0.13 Vert(LL) n/a n/a 999 MT20 Snow (Pf) 20.0 вс

LUMBER-

TCDL

**BCLL** 

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WFBS 2x4 SP No.3

10.0

10.0

0.0

BRACING-

Vert(CT)

Horz(CT)

n/a

0.00

n/a

n/a

3

999

n/a

0.09

WB 0.03

Matrix-P

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

Weight: 18 lb

FT = 20%

REACTIONS. (lb/size) 1=91/6-6-0 (min. 0-1-8), 3=91/6-6-0 (min. 0-1-8), 4=195/6-6-0 (min. 0-1-8)

1.15

YES

Max Horz 1=12(LC 14)

Max Uplift1=-21(LC 10), 3=-22(LC 11), 4=-11(LC 10) Max Grav 1=113(LC 20), 3=113(LC 21), 4=195(LC 1)

Lumber DOL

Rep Stress Incr

Code IRC2021/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard



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