CALI -A, B, F, M, N, P

PORCH

COVERED PORCH

PLAN ID: 1764/1765 - LEFT HAND - NORTH CAROLINA

DATE: **REVISION:**

10/10/2017 INITIAL RELEASE OF PLANS

REVISED PLATE HEIGHT TO 9'-1" FROM 8'-1" 10/20/2017

REVISED ELEVATIONS TO OMIT SOFFIT AT FRONT PORCH

CHANGED ALL ELEVATIONS CLIENT REVISIONS

FLECTRICAL REVISIONS 02/07/2018 REVISED PLAN'S 03/16/2018

CLIENT REVISIONS 08/24/2018

MADE COVERED PATIO STANDARD

REVISED WINDOW AT OPTIONAL MASTER BATH TO BE STANDARD

11/14/2018 CLIENT REVISIONS

REVISED CODE REFERENCES 01/09/2019 12/12/2019 ADDED MASONRY CALCULATIONS

PLATE HEIGHT REVISIONS 03/08/2021

SHEET INDEX:

ARCHITECTURALS - COVERSHEET ARCHITECTURALS - QUICK VIEW

ARCHITECTURALS - QUICK VIEW

ARCHITECTURALS - ELEVATIONS A

ARCHITECTURALS - ELEVATIONS B ARCHITECTURALS - ELEVATIONS F

ARCHITECTURALS - ELEVATIONS M

ARCHITECTURALS - ELEVATIONS N

ARCHITECTURALS - ELEVATIONS P ARCHITECTURALS - FLOOR PLANS A

ARCHITECTURALS - FLOOR PLANS B

ARCHITECTURALS - FLOOR PLANS F ARCHITECTURALS - FLOOR PLANS M

ARCHITECTURALS - FLOOR PLANS N

ARCHITECTURALS - FLOOR PLANS F **ELECTRICAL - FLOOR PLANS**

18 SF

88 SF

MODEL 'CALL' SQUARE FOOTAGES

ELEV 'N' list FLOOR 1764 SF TOTAL LIVING 1764 SF GARAGE 425 SF

COVERSHEET

PLAN REV DATE

CS

McKay Place Lot 37 195 Hawksmoore Lane Lillington, NC 27546



QUICK VIEW 'CALI' - 1765

03.08.21

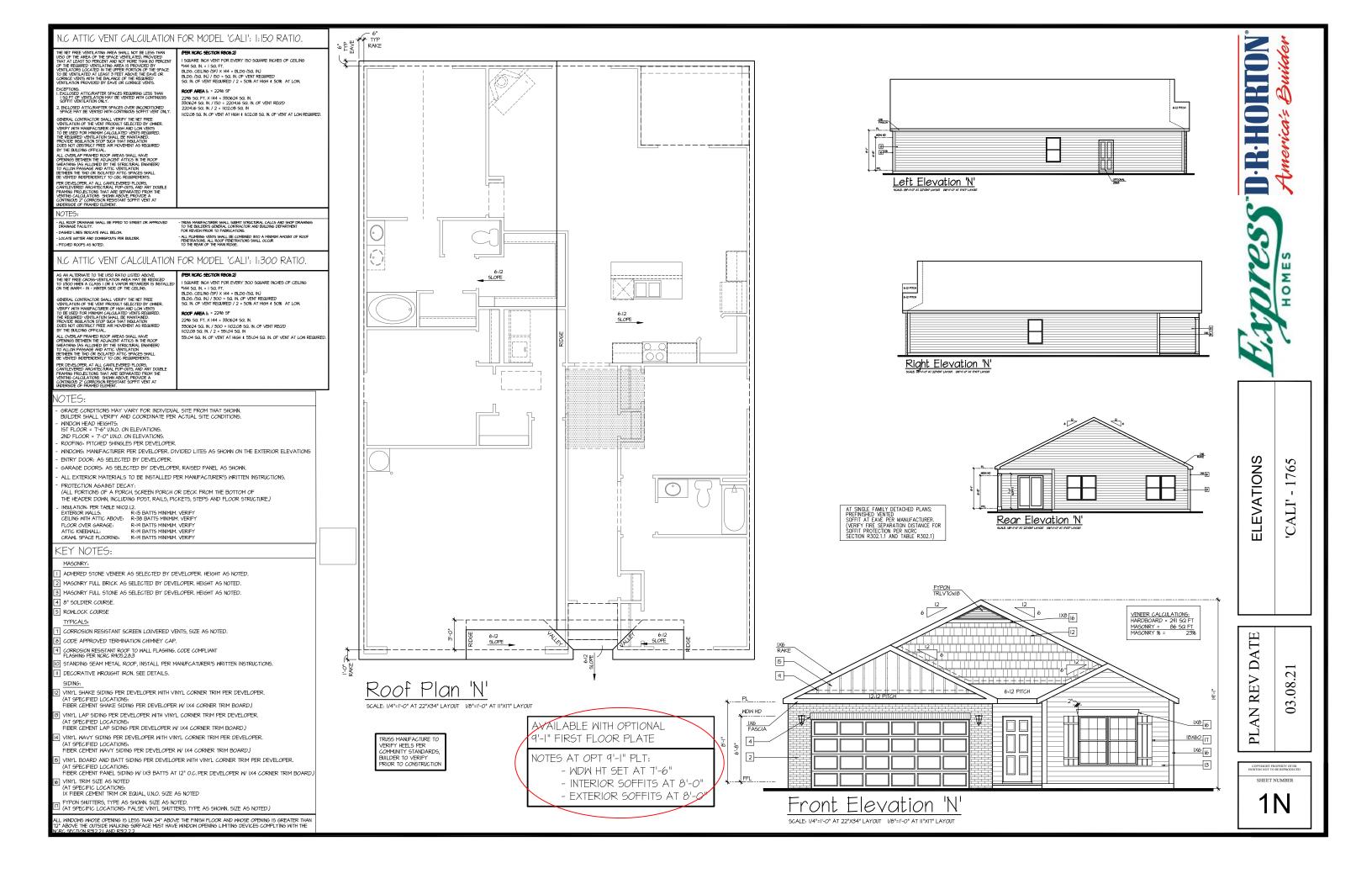
PLAN REV DATE

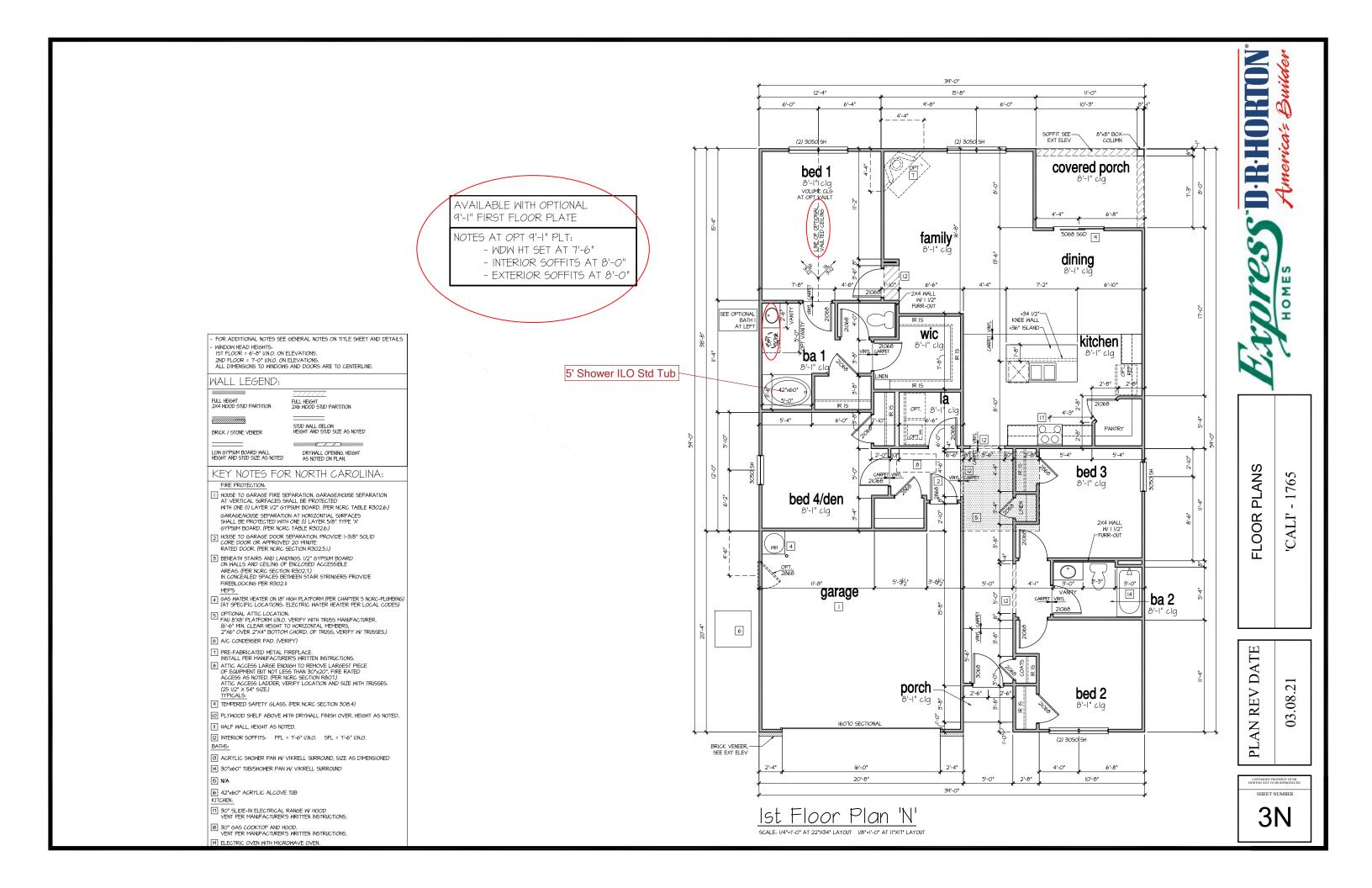
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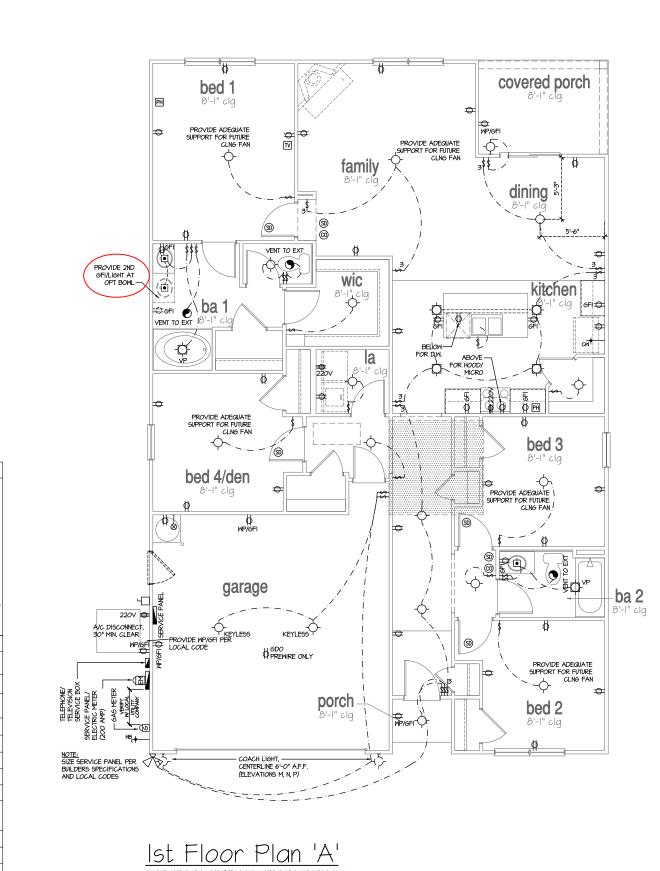












America's

HOMES

FLOOR PLANS

PLAN REV DATE

'CALI'

.21

03.08.

SHEET NUMBER



- PROVIDE GROUNDING ELECTRICAL ROD PER LOCAL CODES. PROVIDE AND INSTALL ARC FAULT CIRCUIT-INTERRUPTERS (AFCI) AS REQUIRED BY NATIONAL ELECTRICAL CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. ALL EXHAUST FANS SHALL HAVE BACKDRAFT DAMPERS.
- FAN/LIGHTS IN WET/DAMP LOCATIONS SHALL BE LABLED "SUITABLE FOR WET OR DAMP LOCATIONS." ELECTRICAL SYSTEMS ARE SHOWN FOR INTENT ONLY. THESE SYSTEMS SHALL BE ENGINEERED BY OTHERS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER INSTALLATION AND PLACEMENT.
- PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES
- PROVIDE AND INSTALL GROUND FAULT CIRCUIT-INTERRUPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRICAL CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
- ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS.
- HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS.
- ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FURNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUMP PITS, DRAIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS.
- PROVIDE POWER, LIGHT AND SMITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S WRITTEN INSTRUCTIONS.

I EGEND.

| LEGI | END: | | |
|----------------|---|--------------------------|---------------------------------|
| Ф | DUPLEX OUTLET | -ф- | FLUSH-MOUNT LED CEILING FIXTURE |
| ØwP/6FI | WEATHERPROOF GFI DUPLEX OUTLET | Ψ | TEST TOTAL PED OFFER TIME |
| ∯ 6FI | GROUND-FAULT GIRCUIT-INTERRUPTER DUPLEX OUTLET | -ф- | HANGING FIXTURE |
| Ф | HALF-SWITCHED DUPLEX OUTLET | Ϋ́D | FLUSH-MOUNT LED CEILING FIXTURE |
| \$ 220∨ | 220 VOLT OUTLET | CFP ` | (PROVIDE CEILING FAN SUPPORT) |
| 0 | REINFORCED JUNCTION BOX | -\$ | 2-LIGHT VANITY FIXTURE |
| \$ | WALL SWITCH | -\$ | 3-LIGHT VANITY FIXTURE |
| \$3 | THREE-WAY SWITCH | - | |
| \$4 | FOUR-WAY SWITCH | -@ | 4-LIGHT VANITY FIXTURE |
| CH | CHIMES | \(\rightarrow \) | WALL MOUNT FIXTURE |
| 9 | PUSHBUTTON SWITCH | • | EXHAUST FAN (VENT TO EXTERIOR) |
| 99 | IIOV SMOKE DETECTOR W BATTERY BACKUP | V | CEILING FAN |
| 00 | CO2 DETECTOR | | (PROVIDE ADEQUATE SUPPORT) |
| • | THERMOSTAT | ∞ | GAS SUPPLY WITH VALVE |
| PH | TELEPHONE | | |
| īV | TELEVISION | —+ _{HB} | HOSE BIBB |
| | ELECTRIC METER | -+ _{CM} | I/4" WATER STUB OUT |
| | ELECTRIC PANEL | Ж | |
| <u></u> | DISCONNECT SWITCH | l 7 | WALL SCONCE |

DESIGN SPECIFICATIONS: Construction Type: Commerical □ Residential ⊠ Conventional 2x Truss 20 PSF 121 Attic Truss 60 PSF A Universal Engineering Sciences Company STRUCTURAL PLANS PREPARED FOR: CALI 4.1. Typ. Dwelling 4.2. Sleeping Areas . 40 PSE 43 Decks 40 PSF PROJECT ADDRESS: 50 PSF . 10 PSF 5.1. Conventional 2x 130 MPH DESIGNER:

DR Horton, Inc. 8001 Arrowridge Blvd. Charlotte, NC 28273

GMD Design Group 102 Fountain Brook Circle Suite C Cary, NC 27511

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory 4 Testing, INC. before construction begins.

PLAN ABBREVIATIONS

| AB | ANCHOR BOLT | PT | PRESSURE TREATED |
|-----|------------------------|-----|------------------------|
| AFF | ABOVE FINISHED FLOOR | R9 | ROOF SUPPORT |
| CJ | CEILING JOIST | 5C | STUD COLUMN |
| CLR | CLEAR | SJ | SINGLE JOIST |
| DJ | DOUBLE JOIST | SPF | SPRUCE PINE FIR |
| DSP | DOUBLE STUD POCKET | SST | SIMPSON STRONG-TIE |
| EE | EACH END | SYP | SOUTHERN YELLOW PINE |
| ΕW | EACH WAY | TJ | TRIPLE JOIST |
| NTS | NOT TO SCALE | TSP | TRIPLE STUD POCKET |
| oc | ON CENTER | TYP | TYPICAL |
| P9F | POUNDS PER SQUARE FOOT | UNO | UNLESS NOTED OTHERWISE |
| P6I | POUNDS PER SQUARE INCH | WWF | WELDED WIRE FABRIC |
| | | | |

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory 4 Testing, INC. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed prior to the initial design, inerelore, truss and joist directions were assumed based on the information provided by <u>DR Horton</u>, <u>Dussequent</u> plan revisions based on roof truss and floor, joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify <u>BUMINIT</u> immediately.

SHEET LIST:

REVISION LIST:

Date

9.7.18

12.14.18

3 |2 |9

3 16 21

5.3.21

5.1.19

11.20.18 195.83R

3.8.21 TØØ54

9/26/22 10054 8/16/23 TØØ54

Project No

19583

19583R2

21790

2170/20

T0054

TØØ54

9 6/30/2021 T0054 updated 0X-IS bracing Table for Framing and

Revision

| Sheet No. | Description | |
|-------------------------|--|--|
| CSI | Cover Sheet, Specifications, Revisions | |
| SI.Øm | Monolithic Slab Foundation | |
| S1.Øs | Stem Wall Foundation | |
| 51.0c | Crawl Space Foundation | |
| S1.Ø6 | Basement Foundation | |
| 52.0 | Basement Plan | |
| 53.Ø | First Floor Plan | |
| 54.Ø | Second Floor Plan | |
| \$5.0 Roof Framing Plan | | |

Description

Revised per new architecturals

Revised NC version only for 2018 NCRC

Covered porch standard on all elevations

Updated TN version only to 2018 IRC

Added elevation L

Updated Garage Foundation Wal

Added OX-16 Bracing Plan

Added SPF note option

date
Added missing dimension in monolithic slab

Added Note (or Wrapped) to Front and Rea

DR HORTON PROJECT SIGN-OFF

| Manager | Signature |
|--------------------------------------|-----------|
| Operations | |
| Operations System | |
| Operations Product Development | |

SUMMIT





RAWNG DATE: 10/103/2023

9CALE: 22x34 |/4"∗1"-Ø" |bx∏ |/8"∗1"-Ø" PROJECT 4 528.TØØ54 DRAWN BY: EO CHECKED BY: M6B

REFER TO COVER SHEET FOR A

Applicable Building Codes:

• 2018 North Carolina Residential Building Code with All Local Amendments ASCE 7-10: Minimum Design Loads for Buildings and Other Structures Roof Live Loads 2. Roof Dead Loads 2.l. Conventional 2x ...
22. Truss 3.1. Importance Factor 4. Floor Live Loads 4.4. Passenger Garage Floor Dead Loads 6. Ultimate Design Wind Speed (3 sec. gust) 6.1. Exposure _____ 62. Importance Factor 63. Wind Base Shear 6.3.1. VX = 6.32. VY = 7. Component and Cladding (in PSF) MEAN ROOF UP TO 30' 30'!"-35' 35'!"-40' 40'!"-45' ZONE 1 16.7,-18.0 17.5,-18.9 182,-19.6 18.7,-2.02 ZONE 2 | 6-1,-210 | 175,-221 | 82,-22.9 | 8-1,-235 | ZONE 3 | 6-1,-210 | 175,-221 | 182,-22.9 | 181,-235 | ZONE 4 | 182,-19.0 | 192,-20.0 | 19,9,-20.1 | 20.4,-21.3 ZONE 5 182,-24,0 19.2,-25.2 19.9,-26.1 20.4,-26.9 8.4. Seismic Use Group . 85. Spectral Response Acceleration 8.5.1. Sms = %g 8.5.2. Sm1 = %g 8.6. Seismic Base Shear 8.6.2.Vy = 87. Basic Structural Sustem (check one) □ Dual w/ Special Moment Frame □ Dual w/ Intermediate R/C or Special Steel ☐ Inverted Pendulum Wind 🖂

GENERAL STRUCTURAL NOTES:

1. The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise alter, or delete any structural aspects of these construction documents without unitien permission of SUPMIT Engineering, Laboratory & Testing, INC. (SUPMIT) or the SER. For the purposes of these construction documents the SER and SUPMIT shall be considered the same entity. The structure is only stable in its completed form. The contractor

- shall provide all required temporary bracing during construction
- to stabilize the structure.

 The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents should any non-conformities occur.
- Anu structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions
- is not the responsibility of the SER or SWMMIT.

 Verification of assumed field conditions is not the responsibilit of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.

 The SER is not responsible for any secondary structural elements
- or non-structural elements, except for the elements specifically noted on the structural drawings. This structure and all construction shall conform to all
- applicable sections of the international residential code.
 This structure and all construction shall conform to all
- applicable sections of local building codes.
- All structural assemblies are to meet or exceed to requirements of the current local building code.

FOUNDATIONS:

I. The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.

- The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade Any fill shall be placed under the direction or recommendation of a licensed professional engineer.
- The resulting soil shall be compacted to a minimum of 95%
- maximum dry density.

 Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation
- No concrete shall be placed against any subgrade containing water, ice, frost, or loose material

STRUCTURAL STEEL:

Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design" latest editions.

- Structural steel shall receive one coat of shop applied rust-inhibitive paint.
- All steel shall have a minimum yield stress (F,,) of 36 ksi unless otherwise noted.

 Welding shall conform to the latest edition of the American
- Welding Societu's Structural Welding Code AWS D.I. Electrodes for shop and field welding shall be class ETOXX. All welding shall be performed by a certified welder per the above standards.

- NCNCIE:
 Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- Concrete shall be proportioned mixed and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings
- Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to 42% of target values as follows:
 3.l. Footings: 5%
 3.2. Exterior Glabs: 5%
- No admixtures shall be added to any structural concrete without written permission of the SER.

- Concrete slabs-on-grade shall be constructed in accordance with ACI 302.IR-96: "Guide for Concrete Slab and Slab
- The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from urreported conditions not in accordance with the above assumptions.
- Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted
- Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished nforcing steel may not extend through a control joint.
- Reinforcing steel may extend through a saw cut joint. 10. All welded wire fabric (WWF.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The WWF. shall be securely supported during the concrete pour.

CONCRETE REINFORCEMENT:

Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.

- Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcemen
- minimization of filormesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard)
 Fibermesh shall comply with ASTM CIII6, any local building code equirements, and shall meet or exceed the current industry
- Steel reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
 Detailing, fabrication, and placement of reinforcing steel shall
- be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"

 Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same
- size/spacing as the horizontal reinforcement with a class B Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.

- Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters
- into the footing. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted
- WOOD FRAMING:

 I. Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Southern-Yellow-Pine (SYP) 2 or Southrn-Spruce Pine (SPF) 2.
- LVL or PSL engineered wood shall have the following minimum
- design values: 2.1. E = 1,900,000 psi 22 Fb = 2600 psi
- 2.4.Fc = 700 psi
- Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance with AUPA standard C-2
- Nalls shall be common wire nalls unless otherwise noted.

 Lag screws shall conform to ANSI/ASME standard B182.1-1981 Lead holes for lag screws shall be in accordance with NDS
- specifications.

 All beams shall have full bearing on supporting framing members unless otherwise noted.
- Exterior and load bearing stud walls are to be 2x4 SYP *2 = 16 * O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimu of one king stud shall be placed at each end of the header. King studs shall be continuous.
- Ring studs shall be continuous. Individual studs forming a column shall be attached with one 10d nail 6 6" OC. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfer. Multi-ply beams shall have each ply attached with (3) lØd nails (
- Four and five ply beams shall be bolted together with (2) rows of 1/2" diameter through bolts staggered @ 16" O.C. unless noted otherwise.

JOOD TRUSSES:

- The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to
- The trusses shall be designed fabricated and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Blood Trusses
- The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments fo
- the trusses.

 Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

EXTERIOR WOOD FRAMED DECKS:

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

WOOD STRUCTURAL PANELS:

- Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide
 "Residential and Commercial," and all other applicable APA standards.
- $\ensuremath{\mathsf{All}}$ structurally required wood sheathing shall bear the mark of

- Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.

 Roof sheathing shall be APA rated sheathing exposure 1 or 2.
- Roof sheathing shall be an A tated steading expected in 2.
 Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as
- required by the state Building Code.

 Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing, like suitable edge support by use of T4G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
 Sheathing shall have a 1/8" gap at panel ends and edges as
- recommended in accordance with the APA.

STRUCTURAL FIBERBOARD PANELS:

- Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.

 All structurally required fiberboard sheathing shall bear the mark of the AFA.
- mark or the AFA.

 Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more
- Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

- FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL
- STRUCTURAL CONCRETE TO BE Fc = 3000 PSI, PREPARED AND PLACED IN
- ACCORDANCE WITH ACI STANDARD 318.
 FOOTINGS TO BE PLACED ON UNDISTURBED EARTH, BEARING A MINIMUM OF 12" BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE CODE ENFORCEMENT OFFICIAL.
 FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF
- 2000 FS. CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFYING THE SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION. FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE
- ELEMENTS. PROVIDE 2" MINIMUM FOOTING PROJECTION FROM THE FACE OF MASONRY.

 MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE 46
- SPECIFIED IN SECTION R404.1 OF THE 2018 NORTH CAROLINA RESIDENTIAL
- SPECIFIED IN SECTION RADALOF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE.

 PILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL.

 PROVIDE FOUNDATION WATERPROOFING, AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS.

 PROVIDED PERIMETER INSULATION FOR ALL FOUNDATIONS PER 2018 NORTH
- CAROLINA RESIDENTIAL BUILDING CODE.
- CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK VENEERS.
 CRAWL SPACE TO BE GRADED LEVEL, AND CLEARED OF ALL DEBRIS.
- CRAIL SPACE OF BE GRADE LEVEL, AND LEARED OF ALL DEBRIS. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2016 NORTH CAROLINA RESIDENTIAL CODE SECTION R403.16, MINIMUM 1/2" DIA BOLTS SPACED AT 6'-0" ON CENTER WITH A "I" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE, ANCHOR BOLTS SHALL BE 12" FROM THE END OF EACH PLATE SECTION MINIMUM (2) ANCHOR BOLTS FER PLATE SECTION, ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.
- 9. ABBREVIATIONS:
- DJ = DOUBLE JOIST SJ = SINGLE JOIST FT = FLOOR TRUSS GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END DR = DOUBLE RAFTER
 TR = TRIPLE RAFTER TJ = TRIPLE JOIST OC = ON CENTER
- 10. ALL PIERS TO BE 16 "X16" MASONRY AND ALL PILASTERS TO BE 8 "X16"
- MASONRY, TYPICAL. (UNO)
 WALL FOOTINGS TO BE CONTINUOUS CONCRETE, SIZES PER STRUCTURAL PLAN.
- A FOUNDATION EXCAVATION OBSERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER, OR HIS QUALIFIED REPRESENTATIVE. IF ISOLATED AREAS OF YIELDING MATERIALS AND/OR POTENTIALLY EXPANSIVE SOILS ARE OBSERVED IN THE FOOTING EXCAVATIONS AT THE TIME OF CONSTRUCTION, SUMMIT ENGINEERING, LABORATORY 4 TESTING, INC. MUST BE PROVIDED THE OPPORTUNITY TO REVIEW THE FOOTING DESIGN PRIOR TO CONCRETE PLACEMENT.
- ALL FOOTINGS & SLABS ARE TO BEAR ON UNDISTURBED SOIL OR 95% COMPACTED FILL, VERIFIED BY ENGINEER OR CODE OFFICIAL.

REFER TO BRACED WALL PLAN FOR PANEL LOCATIONS AND ANY REQUIRED HOLDOWNS, ADDITIONAL INFORMATION PER SECTION R602:10.8 AND FIGURES R602:10.6.5, R602:10.11 R602,10.8(1) AND R602,10.8(2) OF THE 2015 IRC

NOTE: ALL EXTERIOR FOUNDATION DIMENSIONS ARE TO FRAMING AND NOT BRICK VENEER, UNO

NOTE: A 4" CRUSHED STONE BASE COURSE IS NOT REQUIRED WHEN SLAB IS INSTALLED ON WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS CLASSIFIED AS GROUP LIPER TABLE R4051

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DR HORTON COMPLETED/REVISED ON 3/8/2021, IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY & TESTING, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

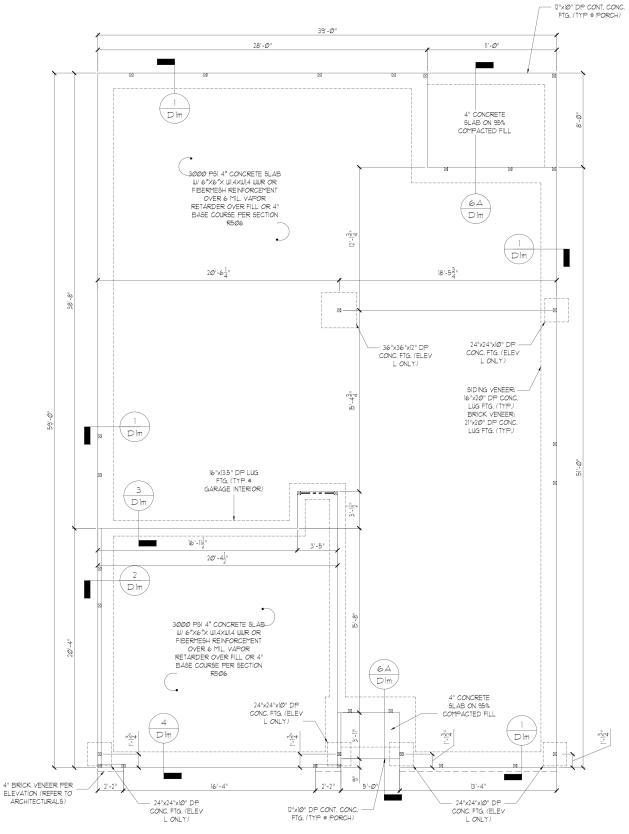
STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, INC. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

MONOLITHIC SLAB FOUNDATION PLAN

SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"



ALL ELEVATIONS





5 ithio



DRAUNG DATE: 10/03/2023 8CALE: 22x34 1/4"+1"-0" lk:|1 1/8"+1"-0" PROJECT 9 528,70054 CHECKED BY: MSB

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS



| REQUIRED BRACED WALL PANEL CONNECTIONS | | | | |
|--|--------------------------|----------------|--------------------------------|--------------------------------|
| | | MIN. THICKNESS | REQUIRED CONNECTION | |
| METHOD | MATERIAL | | @ PANEL EDGES | @ INTERMEDIATE SUPPORTS |
| C5-W5P | WOOD STRUCTURAL PANEL | 3/8" | 6d COMMON NAILS @ 6" O.C. | 6d COMMON NAILS © 12" O.C. |
| GB | GYPSUM BOARD | 1/2" | 5d COOLER NAILS** © 7" O.C. | 5d COOLER NAILS** @ 7" O.C. |
| WSP | WOOD STRUCTURAL PANEL | 3/8" | 6d COMMON NAILS @ 6" O.C. | 6d COMMON NAILS @ 12" O.C. |
| PF | WOOD STRUCTURAL PANEL | 7/16" | PER FIGURE R602.10.6.4 | PER FIGURE R602.10.6.4 |
| "OR EQUIVALENT PER TABLE R1023.5 | | | | |

GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING
- CONTRACTOR SHALL COMENT TO 200 NORTH CARCLINA RESIDENTIAL BUILDING COPE WITH ALL LOCAL AMENDMENTS. CONTRACTOR SHALL VERIEY ALL DIMENSIONS, CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.
 CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED

- CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION.
 PROPERTIES USED IN THE DESIGN ARE AS POLLOUS:
 MICROLLAM (LVL), F_B = 2600 PSI, Fv = 285 PSI, E = 1.9x10° PSI
 PARALLAM (PSL), F_B = 3900 PSI, Fv = 390 PSI, E = 1.9x10° PSI
 ALL WOOD MEMBERS SHALL BE "\$ 5YP", SPF (MLESS NOTED ON PLAN, ALL STUD COLUMN AND JOINTS SHALL BE "\$ 5YP", SPF (MLS)
 ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 "\$ 5YP.72 SPF (SUD COLUMN AT EXCLUSION) IN SEA WITHOUT ONLY SUPPORTED WITH A (2) 2x4 "\$ 5YP.72 SPF (SUD COLUMN AT EXCLUSION) IN SEA WITHOUT ONLY SUPPORTED WITH A (2) 2x4 "\$ 5YP.72 SPF (MLS).
- EACH END UNLESS NOTED OTHERWISE.

 ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM A615

 AND SHALL HAVE A MINIMUM COVER OF 3".

 FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH FOUNDATION AND CONTROLLE SECTION RADIALS HIS MINIMUM IN!" DIA BOLITÓ SPACED AT 6'-0" ON CENTER WITH A 1" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE. ANCHOR BOLITÓ SHALL BE 12" FROM THE END OF EACH PLATE
- SECTION, MINIMUM (2) ANCHOR BOLTS PER PLATE SECTION, ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE. CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- PERFENDICULAR TO RAFIERS. FLITCH BEAMS, 4-PLY LVIS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER UITH 1/2" DIA THAI BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/D31. MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
- ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2'x4 STP "2'SFF" "2', DROPPED, FOR NON-LOAD BEARING HEADERS EXCEEDING 8'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2'x4 SYP 12/SPF 12, DROPPED. (UNLESS NOTED OTHERWISE)
- ABBREVIATIONS:

| DJ = DOUBLE JOIST | SJ = SINGLE JOIST |
|-------------------|--------------------|
| GT = GIRDER TRUSS | FT = FLOOR TRUSS |
| SC = STUD COLUMN | DR = DOUBLE RAFTE |
| EE = EACH END | TR = TRIPLE RAFTER |
| TJ = TRIPLE JOIST | OC = ON CENTER |
| CL = CENTER LINE | PL = POINT LOAD |
| | |

NOTE:

DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE, PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

JOIST & BEAM SITES SHOUN ARE MINIMUMS BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

INSTALL ANY REQUIRED HOLDOWNS PER SECTION R602.10.8 AND FIGURES R602.10.6.5, R602.10.1, R602.10.8(1) AND R602.10.8(2) OF THE 2015 IRC

NOTE: MEMBER NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DR HORTON COMPLETED/REVISED ON 3/8/2021, IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY 4 TESTING, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE

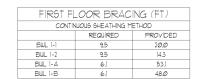
STRUCTURAL MEMBERS ONLY

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR FRAMING PLAN

9CALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"



FIRST FLOOR BRACING (

BWL 1-1

BWL 1-2

BWL 1-A

CONTINUOUS SHEATHING METHOD

9,5

REQUIRED

| | BASE OR EQUIV. (TYP) [A] [A] [A] |
|--------------|---|
| | C) 2x/Ø PT OR WAPPED WAPPED LEVATION MNP: ROOF TRUSSES PER MANUF. ON THE STATE OF TRUSSES PER MANUF. |
| | |
| | |
| (2) A (2) | |
| | |
| 5 | ROOF TRUSSES PER MANUF. (2) 2X8 PT OR (2) 2X12 CONT HEADER W (2) 5C, EACH BEARING DROPFED HEADER (2) 2X12 CONT HEADER W (2) 5C, EACH BEARING DROPFED HEADER |
| , | FRAME PORTAL WALL PER DETAIL I/DIF (MIN, TYP.) ELEVATION A.M BUL I-A BUL I-B |
| (2) | GIRDER TRUSS PER MANUF. (CONT.) (2) ROOF TRUSSES PER MANUF. (2) (2) (2) (2) (2) (2) (2) (2 |

ELEVATION BENE

BWL 1-B

FRAME PORTAL WALL PER DETAIL I/DIF

BWL 1-A

ELEV MN.P ONLY: MIN. 4" P.T. POSTS

ATTACH POSTS TO HEADER W/ 95T CSI6 STRAPS AND ATTACH POSTS TO FOUNDATION W/ 95T ABA44 POST

OR COL. RATED FOR 2000* (MIN. TYP

BUL I-B

BUL 1-A

BWL 1-1 --

BWL 1-2 ---

| HEADER SCHEDULE | | | |
|-----------------|--------------------|------------------|--|
| TAG | SIZE | JACKS (EACH END) | |
| А | (2) 2x6 | (1) | |
| В | (2) 2x8 | (2) | |
| С | (2) 2xlØ | (2) | |
| D | (2) 2xl2 | (2) | |
| E | (2) 9-1/4" LSL/LVL | (3) | |
| F | (3) 2x6 | (1) | |
| G | (3) 2x8 | (2) | |
| Н | (3) 2xlØ | (2) | |
| 1 | (3) 2v12 | (2) | |

HEADER SIZES SHOUN ON PLANS ARE MINIMUMS GREATER HEADER SIZES SHOWN ON FLAMS ARE THIN IN IN GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION. ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE. SC NOTED ON PLAN OVERRIDE SC LISTED ABOVE

| LINTEL SCHEDULE | | | |
|-----------------|-------------------------------------|------------------------|--|
| TAG | SIZE | OPENING SIZE | |
| 1 | L3x3x1/4" | LESS THAN 6'-0" | |
| 2 | L5x3x1/4" | 6'-0" TO 10'-0" | |
| 3 | L5x3-1/2"x5/16" | GREATER THAN 10'-0" | |
| 4 | L5x3-1/2"x5/16" ROLLED OR EQUIV. | ALL ARCHED OPENINGS | |

SECURE LINTEL TO HEADER W/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED & 16" O.C. (TYP FOR 3)

ALL HEADERS WHERE BRICK IS USED, TO BE: (UNO)

WALL STUD SCHEDULE

| 151 # 2ND FLOOR LOAD BEARING STUDS: 2x4 STUDS @ 16" OC. OR 2x6 STUDS @ 24" OC. | 151 FLOOR LOAD BEARING STUDS @ 16" OC. 2x4 STUDS @ 12" OC. OR 2x6 STUDS @ 16" OC. 2x4 STUDS @ 12" OC. OR 2x6 STUDS @ 16" OC. 2x4 STUDS @ 12" OC. OR 2x6 STUDS @ 16" OC. NON-LOAD BEARING STUDS (ALL FLOORS) 2x4 STUDS @ 24" O.C. TWO STORY WALLS: 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED W/ CROSS BRACING @ 6'-0" O.C. VERTICALLY

| KING STUD R | EQUIREMENTS |
|------------------|------------------|
| OPENING WIDTH | KINGS (EACH END) |
| LESS THAN 3'-0" | (1) |
| 3'-Ø TO 4'-Ø" | (2) |
| 4'-0" TO 8'-0" | (3) |
| 8'-0" TO 12'-0" | (5) |
| 12'-0" TO 16'-0" | (6) |
| | |

APPLY TO PORTAL FRAMED OPENINGS

BRACED WALL NOTES:

- 1) WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2015 INTERNATIONAL RESIDENTIAL CODE AS ALLOWED PER SECTION RE02:10 OF THE 2018 NO RESIDENTIAL CODE. UALLS ARE DESIGNED FOR SEIGMIC ZONES A-C AND ULTIMATE WIND SPEEDS UP TO 130 MPH.
- REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING
- 3. BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN
- ACCORDANCE WITH IRC TABLE R602.104.
 4. ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEDE OF RET FOR ISOLATED PANEL METHOD AND IZ FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
 MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.10.5.
- THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD (UNO).
- 1/27 3 FROM BOARD (MO).
 FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFILL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS.
- FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- A BRACED WALL PANEL SHALL BE LOCATED WITHIN 10 FEET OF EACH END OF A BRACED WALL LINE.

 10. THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS
- SHALL NOT EXCEED 20 FEET.
- SHALL NOT EXCEED 20 FEET.

 MASONRY OR CONCRETE STEM WALLS WITH A LENGTH OF 48" OR
 LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN
 ACCORDANCE WITH FIGURE R602 10.9 OF THE 2015 IRC. 12. BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE
- CONSTRUCTED IN ACCORDANCE WITH SECTION R602/03 BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602/03/2 AND
- FIGURES R602.10.8(1)4(2)4(3).
- HOUSES FACILIZATION (2415).

 4. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION RE02.10.11

 15. PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R6021064 (UNO)
- ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS. ABBREVIATIONS:

CS-XXX = CONT. SHEATHED ENG = ENGINEERED SOLUTION
PF = PORTAL FRAME PF-ENG = ENG. PORTAL FRAME





CLIENT: DR Horton, Inc. 8001 Arrowridge Blvv Charlotte, NC 28213

QŢ 00 PROJECT: Cali - LH First



DRAWNG DATE: 10/103/2023 8CALE: 22x34 1/4"+1"-0" lk:|1 1/8"+1"-0" PROJECT 9 528,70054

CHECKED BY: MSB

DATE 12/13/11

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL, PLANS PROVIDED BY DR HORTON COMPLETED/REVISED ON 393/2021 IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENSINEERING, LABORATORY 4 TESTING, NC, IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENSINEERING, LABORATORY 4 TESTING, NC, CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

NOTE: 15T PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNO)

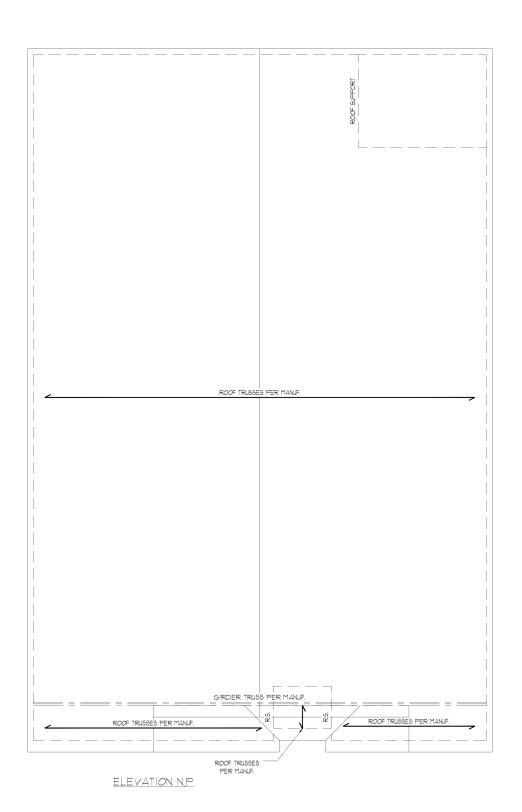
NOTE: ROOF TRUSSES SHALL BE SPACE TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

ROOF FRAMING PLAN 9CALE: 1/4"=1'-Ø" ON 22"x34" OR 1/8"=1'-Ø" ON 11"x17"







Framing PROJECT: Call - LH



DRAUNG DATE: 10/03/2023

8CALE: 22x34 1/4"+1"-@" lb:\(\tau\) 1/8"+1"-@" PROJECT 9 528,10054 CHECKED BY: MSB

ORIGINAL INFORMATION
PROJECT DATE
16381 12/3/11

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS