

Building Code:

Risk Category:

Service Condition:

LL Deflection Limit:

TL Deflection Limit:

Top: 1'- 10 1/2"

Lateral Restraint Requirements:

following maximum unbraced length:

Bearing Stress of Support Material:

• 1323 psi Wall @ 0'- 2" • 1323 psi Wall @ 5'- 4"

Design Methodology:

Customer: Street 1: City:

DESIGN INFORMATION

ASD

Dry

Both ends of the member and the outer supports

must be laterally restrained. Top and bottom edges

of the member must be fully restrained or have the

Bottom: 5'- 6"

IRC 2018

Residential

L/360, 0.75" (absolute)

L/240, 1.00" (absolute)

Customer Ph.

Job Name: Baker 2024-SAN-008

Level: 1st Floor Label: 1DBM1 - i15 Type **Beam**

2 Ply Member 1 3/4" x 9 1/4" 2.0E Microllam® LVL

Report Version: 2021.03.26

Status: Design **Passed**

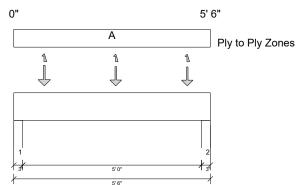
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776/-3 lb

175 lb/ -502 lb

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.7.2.270.Update10.13



ANALYSIS RESULTS Design Criteria Load Combination LDF Limit Result Location Design Passed - 17% Max Pos. Moment: D + Lr 2183 lb ft 12824 lb ft 2'- 10 1/4" 1.15 II (General Construction) Max Shear 7074 lb 1'- 1/4" D + Ir1 15 1469 lb Passed - 21% Passed - L/999 Live Load (LL) Pos. Defl.: 2'- 8 7/8" Lr 0.016" L/360 Total Load (TL) Pos. Defl.: 2'- 8 15/16" D + Lr 0.037" L/240 Passed - L/999

SUPPORT AND REACTION INFORMATION Input Controlling Load Downward Uplift Resistance Resistance Bearing LDF Result Combination Reaction Reaction of Member of Support Length 3" D + Ir1746 lb 7875 lb 13891 lb Passed - 22% 1 15 3" D + Lr 1785 lb 7875 lb 13891 lb Passed - 23% 2 1.15 **LOADING** Type Start Loc End Loc Source Face Dead (D) Live (L) Snow (S) Roof Live (Lr) Wind (W) Self

Weight	0'	5'- 6"	Self Weight	Тор	9 lb/ft	-	-	-	-
Point	0'- 10 1/4"	0'- 10 1/4"	A04(c01)	Тор	666 lb	-	-	569/-1 lb	108/-262 lb
Point	2'- 10 1/4"	2'- 10 1/4"	A04(c02)	Тор	633 lb	-	-	506/-1 lb	95/-224 lb
Point	4'- 10 1/4"	4'- 10 1/4"	A04(c03)	Тор	623 lb	-	-	482/-3 lb	91/-215 lb
UNFACTORED REACTIONS									
ID	Start Loc	End Loc	Source		Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
1	0'	0'- 3"	E13(i13)		965 lb	-	-	781/-1 lb	175 lb/ -502 lb

1009 lb

DESIGN NOTES

5'- 3"

The dead loads used in the design of this member were applied to the structure as projected dead loads.

E12(i5)

- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- Beam Stability Factor used in the calculation for Allowable Max Pos Moment (CL) = 0.99

PLY TO PLY CONNECTION

Zone A: Factored load = 0 plf. Use 12d (0.131"x3.25") nails. LDF = 1.00. Qty = 12. Row = 2, Spacing = 12" 12d (0.131"x3.25") nails properties: D = 0.131", L = 3.25". Fastener capacity = 96 lbs. X1 = 2", Y1 = 0.75", Y2 = 1.5" Install fasteners from one face.

X1 = Minimum end distance, X2 = Minimum edge distance, Y2 = Minimum row spacing.



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Passed

PLY TO PLY CONNECTION

FASTENER INSTALLATION – 2 ROWS (FROM ONE FACE)

