GENERAL NOTES:

DO NOT SCALE DRAWINGS. USE WRITTEN DIMENSIONS ONLY. SUBMIT TO DESIGNER ANY DISCREPANCIES FOR CLARIFICATION BEFORE CONSTRUCTION BEGINS.

ALL WORK SHALL BE IN COMPLIANCE WITH NORTH CAROLINA BUILDING CODES, RECOGNIZED INDUSTRY STANDARDS, ALL MANUFACTURER'S RECOMMENDATIONS AND ALL OTHER APPLICABLE CODES.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING AND PAYING FOR ALL NECESSARY PERMITS FROM THE GOVERNMENTAL AGENCIES.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR BUILDING THIS PROJECT IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS UNLESS HE RECEIVES A WRITTEN NOTIFICATION FROM THE DESIGNER TO THE CONTRARY.

THE CONTRACTOR SHALL BE RESPONSIBLE TO PROVIDE AND BUILD THE STRUCTURE IN SUCH A WAY AS TO ACCOMMODATE ANY ANCHORS, SLEEVES, RECESSES, OPENINGS, HANGERS, DEPRESSIONS, ETC NEEDED FOR HIS OR OTHER WORK.

THE CONTRACTOR SHALL BE RESPONSIBLE TO VERIFY SOIL CONDITIONS PRIOR TO CONSTRUCTION AND NOTIFY DESIGNER AND ENGINEER OF ANY DISCREPANCIES IN THE DESIGN PRIOR TO PROCEEDING WITH THE WORK. FAILURE OF THE STRUCTURE DUE TO UNSUITABLE SOIL BEARING CONDITIONS SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR IF HE FAILS TO NOTIFY AS DIRECTED.

IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO MAINTAIN A SAFE JOBSITE AND TO FOLLOW THE STANDARDS AND REGULATIONS FOR CONSTRUCTION ESTABLISHED FOR THE LOCATION OF THE PROJECT. THE OWNER, DESIGNER AND ENGINEER SHALL BE HELD HARMLESS FOR THE ACTS AND FAILURES OF THE CONTRACTOR.

THE LOCATION OF EXISTING UTILITIES, GRADES, TREES AND EXISTING STRUCTURES ARE APPROXIMATE. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTENCE AND LOCATION OF SUCH ITEMS WHETHER SHOWN HEREON OR NOT. ANY DISCREPANCIES SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR AND ANY ADDITIONAL COSTS SHALL BE HIS.

HANDRAILS AND GUARDRAILS SHALL BE INSTALLED PER NORTH CAROLINA STATE BUILDING CODE.





02/26/2024





ADDA is an international non-profit organization devoted to the science of graphic communication and the professional art of design, drafting, graphic arts, technical illustration, digital design and graphic presentation hereby acknowledges,

Joshua S. Lee

has demonstrated the standards set forth by this association and is deerned a Certified Architectural Drafter

11-2025 Valid Through





edition of all applicable Nation, State, and Local Building Codes.

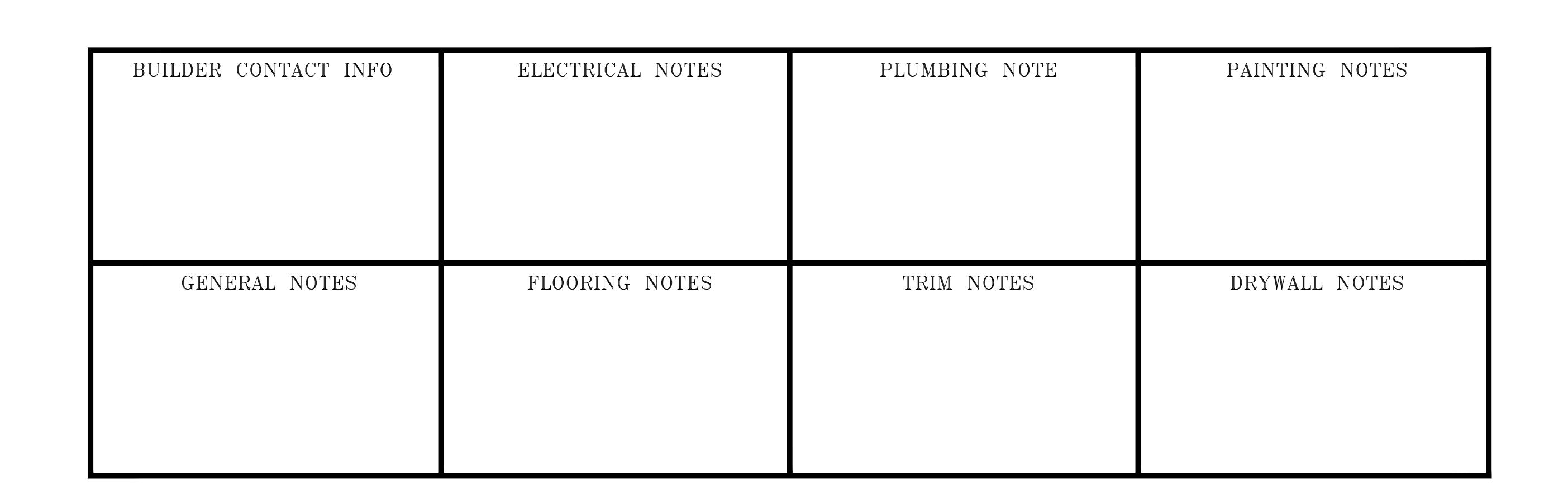
It is the responsibility of the builder to check all dimension and details for overall accu appropriate to the local on site conditions. The draftsman is not an architectural firm a stands no liability for structural or architectural design integrity. Every effort has been ensure all dimensions are correct and governmental regulations have been met. If an er omission does occur it is the sole responsibility of the contractor to correct the error

J Lee Design Dream. Create. Li

C

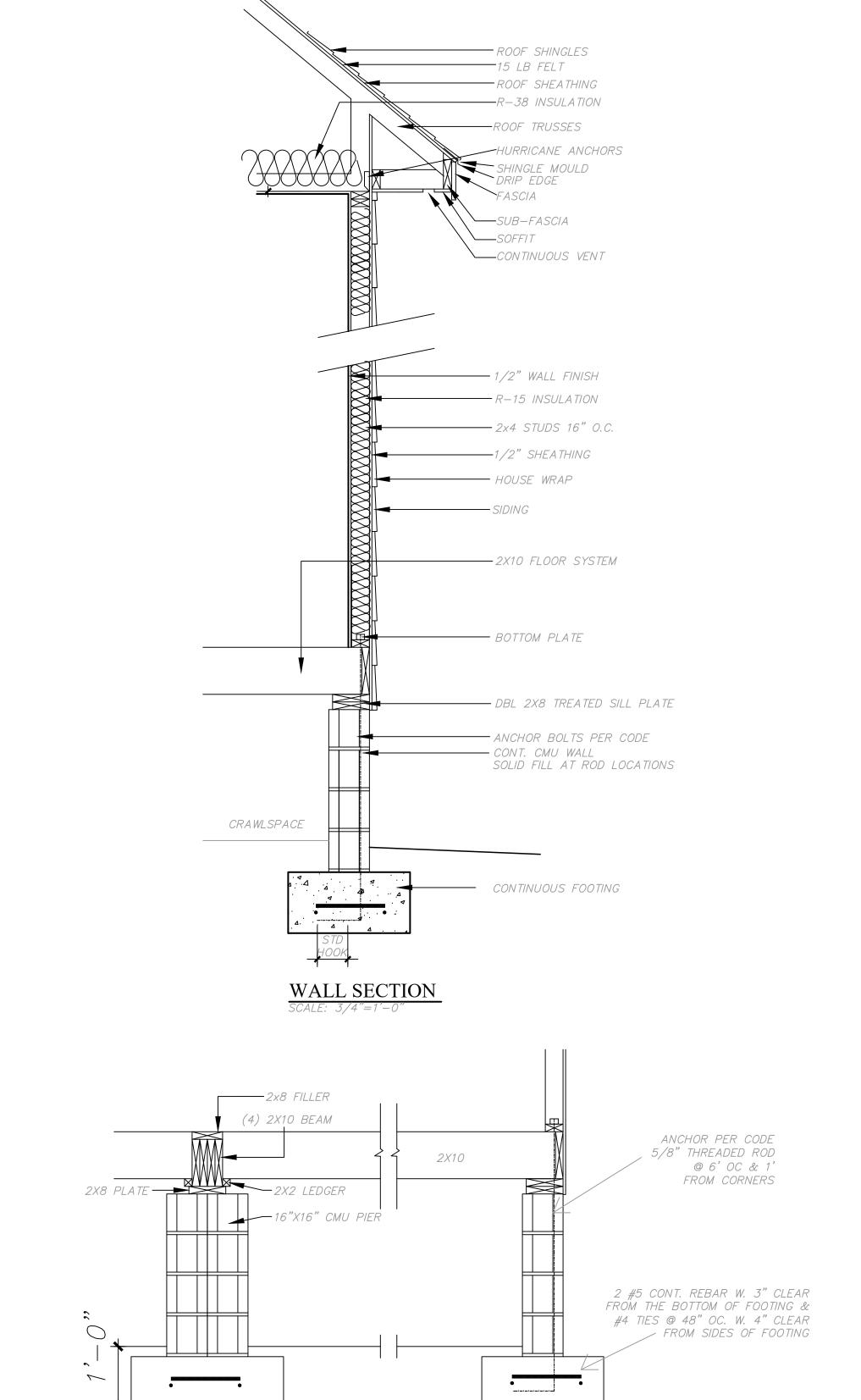
SCALE : 1/4"=1'

JANUARY 14TH, 2024



JANUARY 14TH, 2024

A1



24" WIDE X 10" THICK CONTINUOUS FOOTING

SCALE: 3/4"=1'-0"

FOUNDATION DETAIL

30"X30"X10" PIER FOOTING

FOUNDATION PLAN

1/4" = 1'-0"

BLOCK HEIGHT CAN VARY DEPENDING ON SITE CONDITIONS

NO INTERIOR LOAD POINTS FROM TRUSS SYSTEM

12" X 12" PIERS
ON 24" X 24" CONC. FOOTING

DBL 2X10 BAND 2X12 FACE BAND

10'-0"

32**'**-0"

8'-0"

8'-0"

8'-0"

11'-0"

(4) 2X10 BEAM

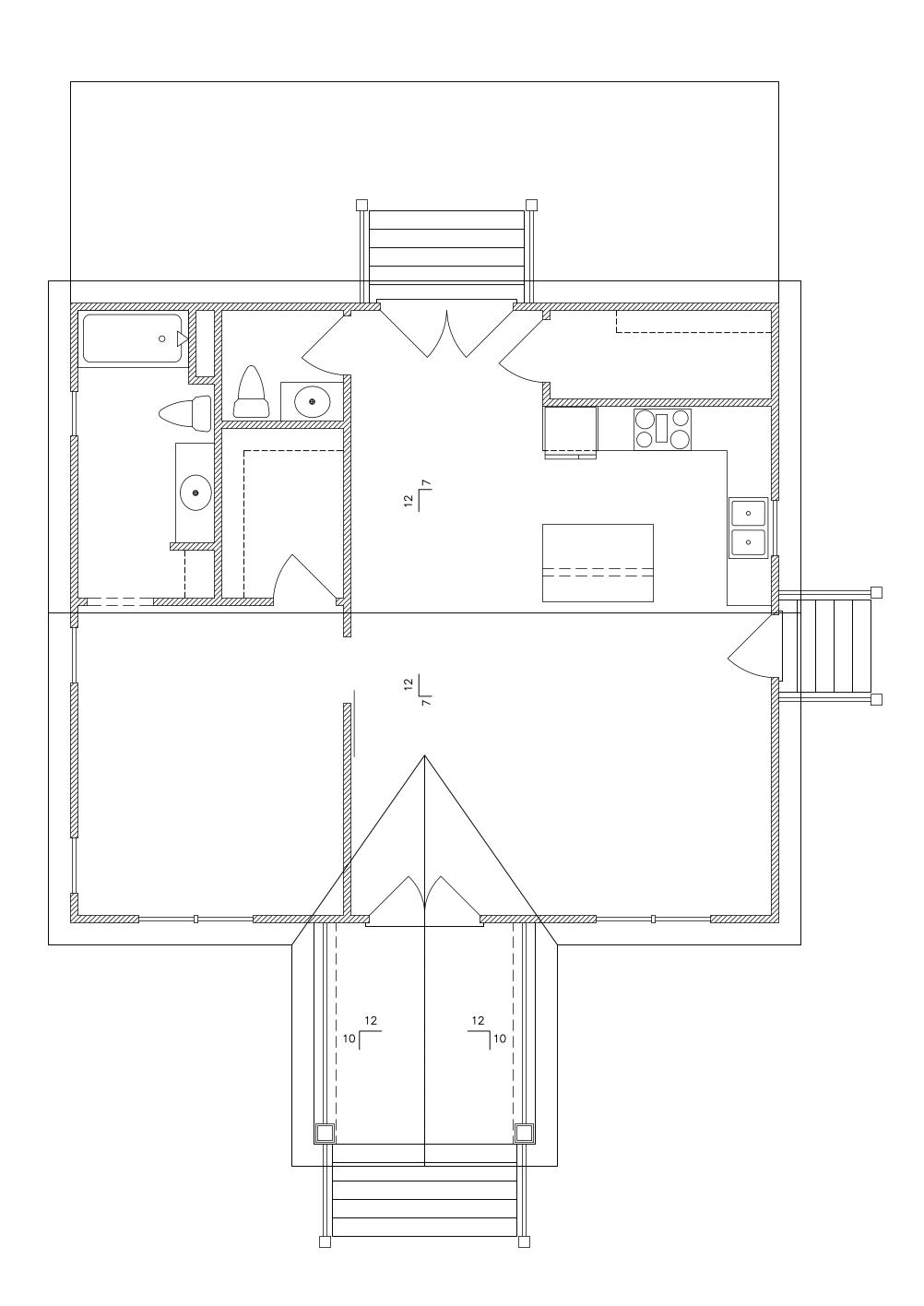
9'-5"

8'-0"

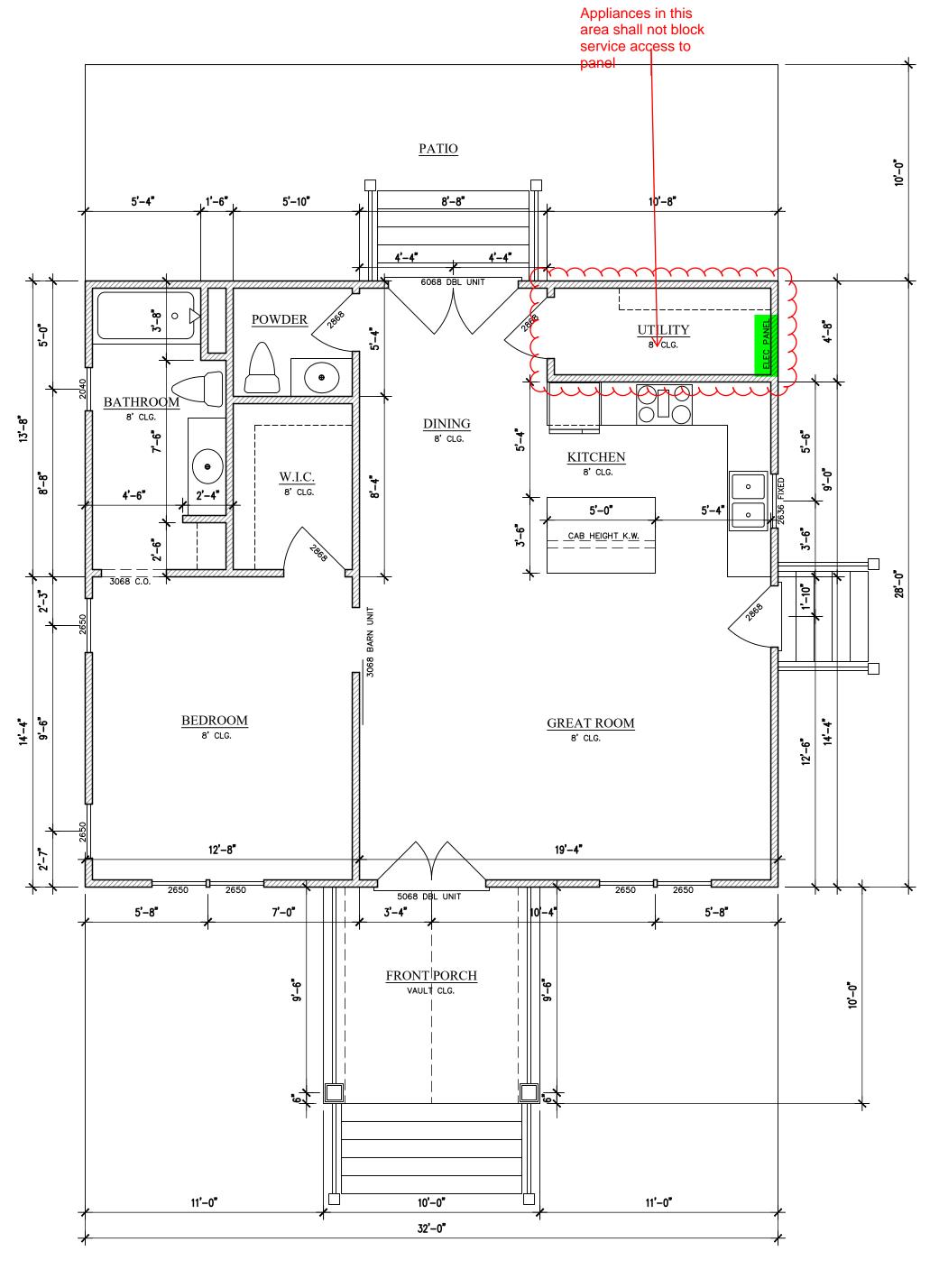
8'-0"

16" X 16" PIERS ON 30" X 30" CONC. FOOTING

11'-0"



ROOF PLAN 1/4" = 1'-0" 12" OVERHANGS U.N.O. RIDGE VENT PER CODE



FLOOR PLAN 1/4" = 1'-0" HEATED LIVING — 896 sq. ft. FRONT PORCH — 100 sq. ft. REAR PATIO — 320 sq. ft. Designs

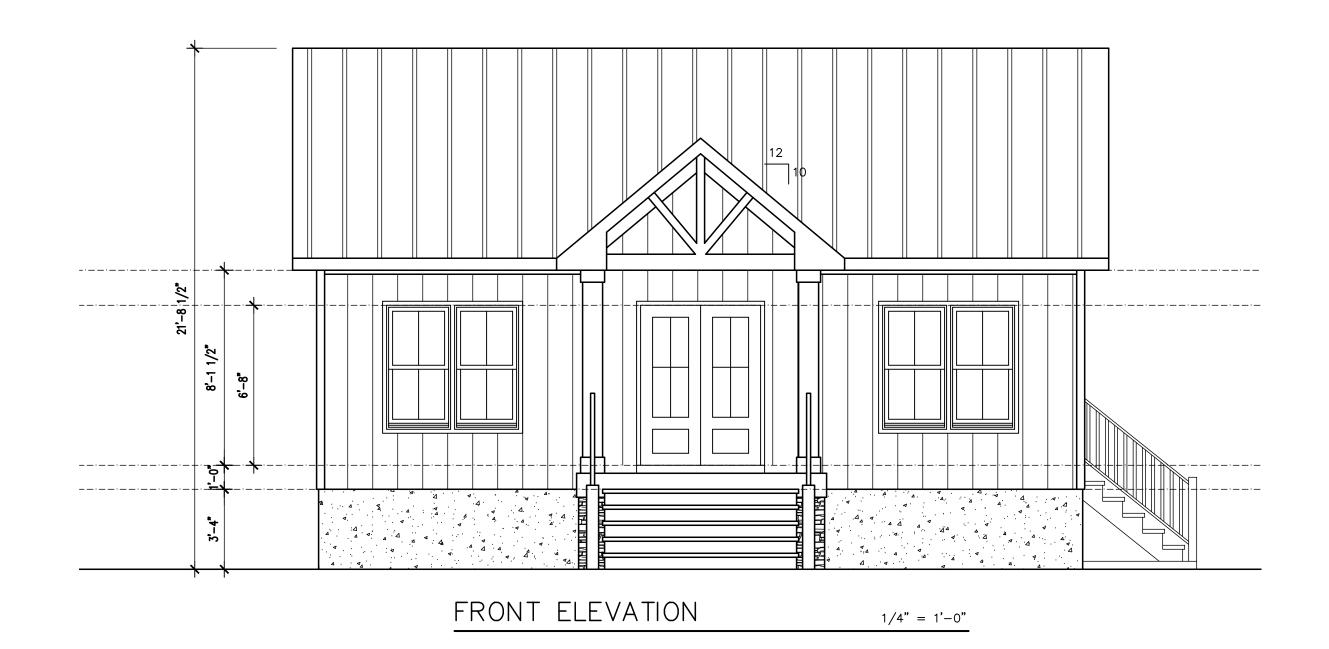
SCALE: 1/4"=1'

JANUARY 14TH, 2024

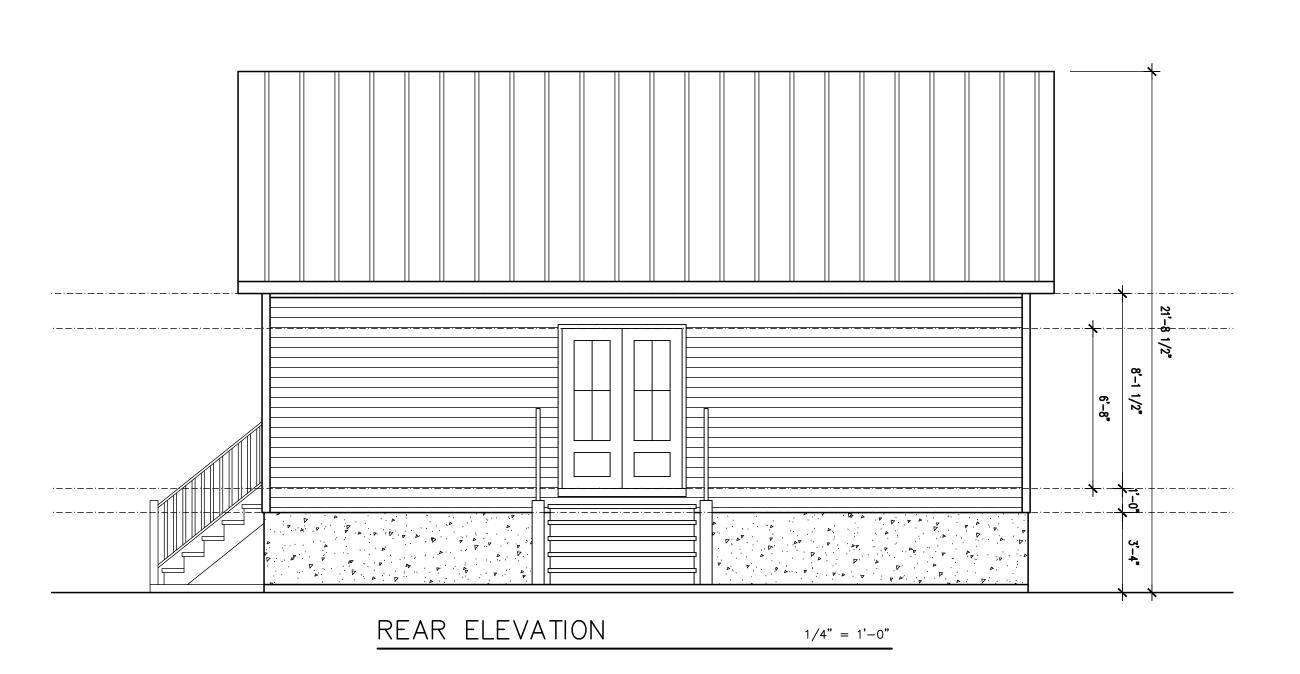
SCALE : 1/4"=1'

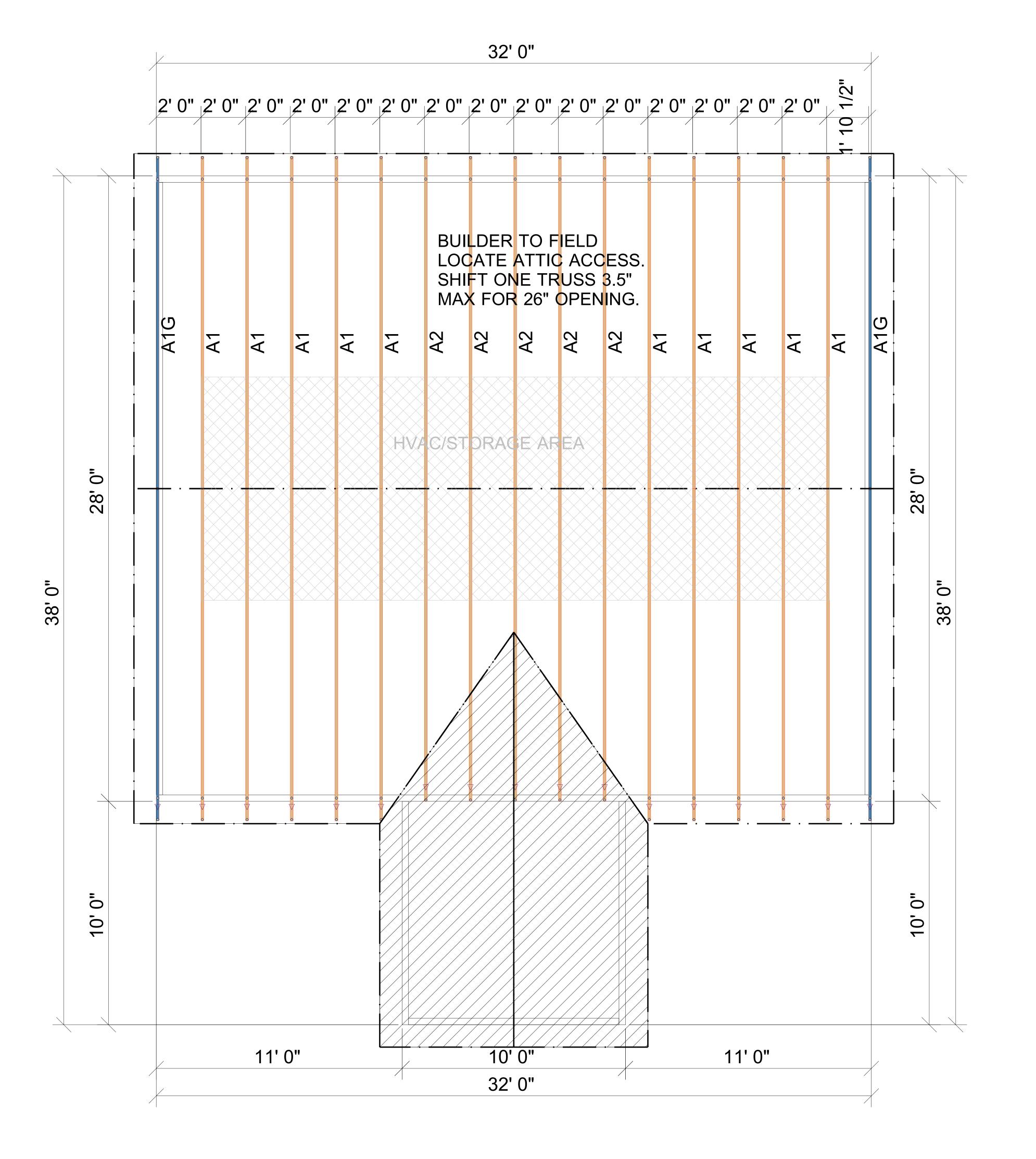
JANUARY 1ST, 2024

RIGHT ELEVATION 1/A" = 1'-0"









RIDGE LINE: 52.57 AREA: **DESIGNER** JBP ARCH DATE -STRUC DATE -JOB #: 24011562



UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Mary-Anne Judd

Run: 8.62 S Sep 22 2022 Print: 8.620 S Sep 22 2022 MiTek Industries, Inc. Tue Jan 23 07:32:26

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Structural wood sheathing directly applied or 2-2-0 oc purlins

Page: 1

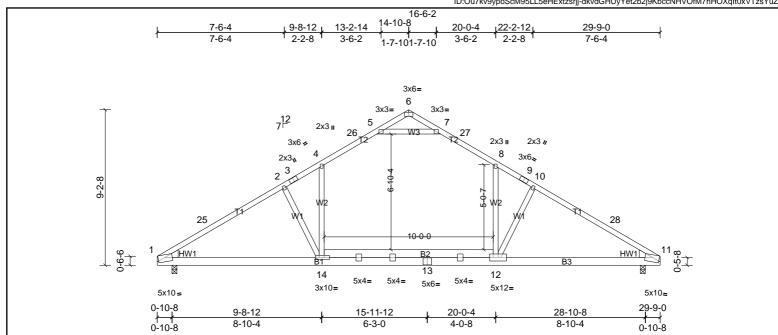


Plate Offsets (X, Y): [1:0-4-13,Edge], [6:0-3-0,Edge], [11:0-4-13,Edge], [14:0-2-12,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.86	Vert(LL)	0.49	14-19	>727	240	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.88	Vert(CT)	-0.72	12-14	>493	180			
BCLL	0.0*	Rep Stress Incr	YES	WB	0.32	Horz(CT)	0.03	11	n/a	n/a			
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MSH		Attic	-0.22	12-14	>569	360	Weight: 184 lb	FT = 20%	

LUMBER **BRACING**

TOP CHORD TOP CHORD 2x4 SP SS *Except* T1:2x4 SP No.2

BOT CHORD **BOT CHORD** 2x6 SP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No.3 WEBS

WEDGE Left: 2x6 SP No.2 Right: 2x6 SP No.2

REACTIONS (lb/size) 1=1241/0-3-8, (min. 0-1-9), 11=1241/0-3-8, (min. 0-1-9)

> Max Horiz 1=305 (LC 7)

Max Uplift 1=-314 (LC 10), 11=-314 (LC 11) 1=1339 (LC 18), 11=1339 (LC 19)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-25=-1750/378, 2-25=-1706/412, 2-3=-1630/429, 3-4=-1610/442, 4-26=-1255/381, 5-26=-1211/407, 5-6=-151/685, 6-7=-151/685, 7-27=-1211/407, 8-27=-1255/381, 8-9=-1610/442, 3-26=-1251/685, 3-26=-1211/407, 3-6=-151/685, 3-26=-1211/407, 3-6=-151/685, 3-26=-1211/407, 3-6=-151/685, 3-26=-1211/407, 3-6=-151/685, 3-26=-1211/407, 3-6=-151/685, 3-26=-1211/407, 3-6=-151/685, 3-26=-1211/407, 3-6=-151/685, 3-6=-1211/407, 3-6=-151/685, 3-6=-1211/407, 3-6=-151/685, 3-6=-1211/407, 3-6=-151/685, 3-6=-1211/407, 3-6=-151/685, 3-6=-1211/407, 3-6=-151/685, 3-6=-1211/407, 3-6=-151/685, 3-6=-1211/407, 3-6=-151/685, 3-6=-1211/407, 3-6=-151/685, 3-6=-1211/407, 3-6=-151/685, 3-6=-1211/407, 3-6=-151/685, 3-6=-1211/407, 3-6=-151/685, 3-6=-1211/407, 3-6=-151/685, 3-6=-1211/407, 3-6=-151/685, 3-6=-1211/407, 3-6=-151/685, 3-6=-1211/407TOP CHORD

9-10=-1630/429, 10-28=-1706/412, 11-28=-1750/378 1-14=-350/1598, 13-14=-160/1291, 12-13=-160/1291, 11-12=-223/1458

BOT CHORD WEBS 10-12=-680/420, 2-14=-680/420, 8-12=-163/758, 4-14=-163/757, 5-7=-2100/602

NOTES

FORCES

- Unbalanced roof live loads have been considered for this design. 1)
- Wind: ASCE 7-10; Vult=155mph (3-second gust) Vasd=123mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) 2) exterior zone and C-C Exterior (2) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 11-10-8, Exterior (2) 11-10-8 to 17-10-8, Interior (1) 17-10-8 to 26-9-0, Exterior (2) 26-9-0 to 29-9-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3)
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between
- the bottom chord and any other members. 5) Ceiling dead load (5.0 psf) on member(s), 4-5, 7-8, 5-7
- Bottom chord live load (20.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 12-14 6)
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 1 and 314 lb uplift at joint 11.
- 8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/
- 9) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute

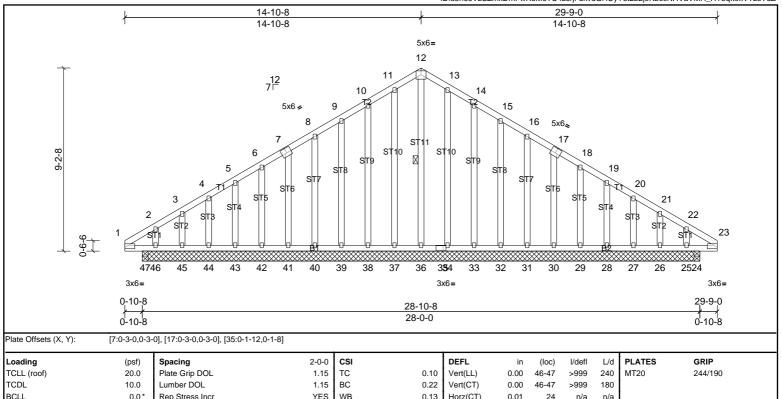




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Run: 8.62 S Sep 22 2022 Print: 8.620 S Sep 22 2022 MiTek Industries, Inc. Tue Jan 23 07:32:26

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LUMBER **BRACING**

TOP CHORD 2x4 SP No.2 TOP CHORD BOT CHORD 2x4 SP No.2 BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing. OTHERS 2x4 SP No.3 WEBS 1 Row at midpt 12-36

Matrix-MSH

REACTIONS All bearings 28-0-0. except 24=0-3-8, 47=0-3-8

10.0

Code

46=305 (LC 7) (lb) - Max Horiz

All uplift 100 (lb) or less at joint(s) 26, 27, 28, 29, 30, 31, 32, 33, 34, 37, 38, 39, 40, 41, 42, 43, 44, 45 except 24=-211 (LC 9), 25=-279 (LC 11), Max Unlift

46=-332 (LC 10), 47=-320 (LC 8) All reactions 250 (lb) or less at joint(s) 26, 27, 28, 29, 30, 31, 32, 33, 34,

IRC2015/TPI2014

Max Grav

37, 38, 39, 40, 41, 42, 43, 44, 45 except 24=292 (LC 6), 25=252 (LC 9),

36=260 (LC 11), 46=331 (LC 8), 47=400 (LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 8-9=-189/258, 9-10=-228/286, 10-11=-272/330, 11-12=-293/356, 12-13=-293/356, 13-14=-272/330, 14-15=-228/277 WEBS

12-36=-260/175

NOTES

BCDI

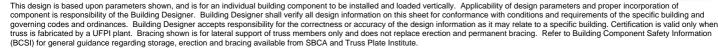
- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=155mph (3-second gust) Vasd=123mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner (3) 0-0-0 to 2-10-8, Exterior (2) 2-10-8 to 11-10-8, Corner (3) 11-10-8 to 17-10-8, Exterior (2) 17-10-8 to 26-9-0, Corner (3) 26-9-0 to 29-9-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only
- 4) All plates are 2x3 MT20 unless otherwise indicated
- 5) Gable studs spaced at 1-4-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 37, 38, 39, 40, 41, 42, 43, 44, 45, 34, 33, 32, 31, 30, 29, 28, 27, 26 except (jt=lb) 46=331, 25=278, 24=211, 47=319.
- 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/ TPI 1.



Weight: 239 lb

Structural wood sheathing directly applied or 10-0-0 oc purlins.

FT = 20%







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Run: 8.62 S Sep 22 2022 Print: 8.620 S Sep 22 2022 MiTek Industries, Inc. Tue Jan 23 07:32:27

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Structural wood sheathing directly applied or 2-2-0 oc purlins

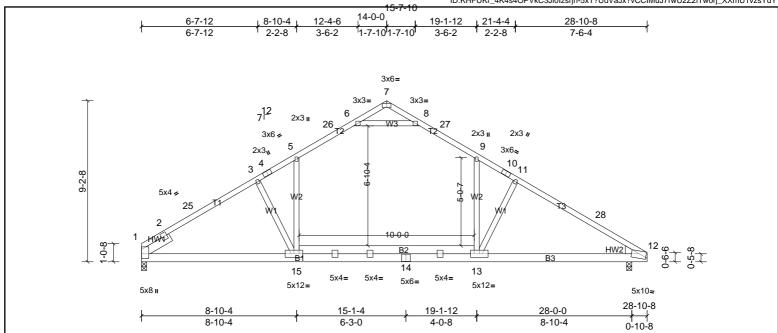


Plate Offsets (X, Y):	[7:0-3-0, Edge],	[12:0-4-13,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.88	Vert(LL)	0.50	15-18	>698	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.88	Vert(CT)	-0.72	13-15	>484	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.32	Horz(CT)	0.06	1	n/a	n/a		
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MSH		Attic	-0.22	13-15	>558	360	Weight: 183 lb	FT = 20%

LUMBER **BRACING**

TOP CHORD TOP CHORD 2x4 SP SS *Except* T1,T3:2x4 SP No.2 BOT CHORD BOT CHORD 2x6 SP No.2

Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No.3 WEBS WEDGE Right: 2x6 SP No.2 **SLIDER** Left 2x6 SP No.2 -- 1-11-0

REACTIONS 1=1170/0-3-8, (min. 0-1-8), 12=1243/0-3-8, (min. 0-1-9) (lb/size)

> 1=-303 (LC 6) Max Horiz

Max Uplift 1=-287 (LC 10), 12=-314 (LC 11) 1=1264 (LC 18), 12=1340 (LC 19)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-817/0, 2-25=-1732/395, 3-25=-1690/417, 3-4=-1635/435, 4-5=-1614/447, 5-26=-1259/383, 6-26=-1215/410, 6-7=-152/690, 7-8=-153/689, 8-27=-1216/411, 9-27=-1260/384

-10=-1614/446, 10-11=-1635/433, 11-28=-1710/415, 12-28=-1755/381 1-15=-348/1599, 14-15=-161/1296, 13-14=-161/1296, 12-13=-224/1461 3-15=-669/413, 11-13=-677/419, 9-13=-162/756, 5-15=-161/762, 6-8=-2111/610

WEBS NOTES

BOT CHORD

- Unbalanced roof live loads have been considered for this design. 1)
- Wind: ASCE 7-10; Vult=155mph (3-second gust) Vasd=123mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) 2) exterior zone and C-C Exterior (2) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 11-0-0, Exterior (2) 11-0-0 to 17-0-0, Interior (1) 17-0-0 to 25-10-8, Exterior (2) 25-10-8 to 28-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3)
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between
- the bottom chord and any other members.
 Ceiling dead load (5.0 psf) on member(s). 5-6, 8-9, 6-8 5)
- Bottom chord live load (20.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 13-15 6)
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 1 and 314 lb uplift at joint 12. 7)
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/ 8)
- 9) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute

