COLUMBIA -A, B, C

PLAN ID: 3142 - RIGHT HAND - NORTH CAROLINA

DATE:	REVISION:
09/25/2017	INITIAL RELEASE OF PLANS
10/20/2017	REVISED ROOF PITCH AT FRONT GABLE AT ELEVATION 'A'
11/01/2017	RENAMED MASTER BEDROOM AND BATH TO OWNER'S BEDROOM AND BATH
02/07/2018	ELECTRICAL REVISIONS
06/11/2018	CLIENT REVISIONS
11/14/2018	CLIENT REVISIONS
07/23/2019	CLIENT REVISIONS
02/28/2020	CLIENT REVISIONS
06/06/2023	CLIENT REVISIONS



CS ARCHITECTURALS - COVERSHEET

0 ARCHITECTURALS - QUICK VIEW

1A ARCHITECTURALS - ELEVATIONS A

1B ARCHITECTURALS - ELEVATIONS B

1C ARCHITECTURALS - ELEVATIONS C

3A ARCHITECTURALS - FLOOR PLANS A

3B ARCHITECTURALS - FLOOR PLANS B

3C ARCHITECTURALS - FLOOR PLANS C

4 ELECTRICAL - FLOOR PLANS C

REVIEWERS STAMP LOCATION



MODEL 'COLUMBIA' SQUARE FOOTAGES							
AREA	ELEV 'A'						
lst FLOOR	1350 SF						
2nd FLOOR	1758 SF						
TOTAL LIVING	3108 SF						
GARAGE	441 SF						
PORCH	93 SF	`.					

McKay Place Lot 22 84 Finsbury Court Lillington, NC 27546

COVERSHEET 'COLUMBIA'

PLAN REV DATE

COPYREINT PROPERTY OF DE HORTON NOT TO BE REPRODUCE SHEET NUMBER







QUICK VIEW

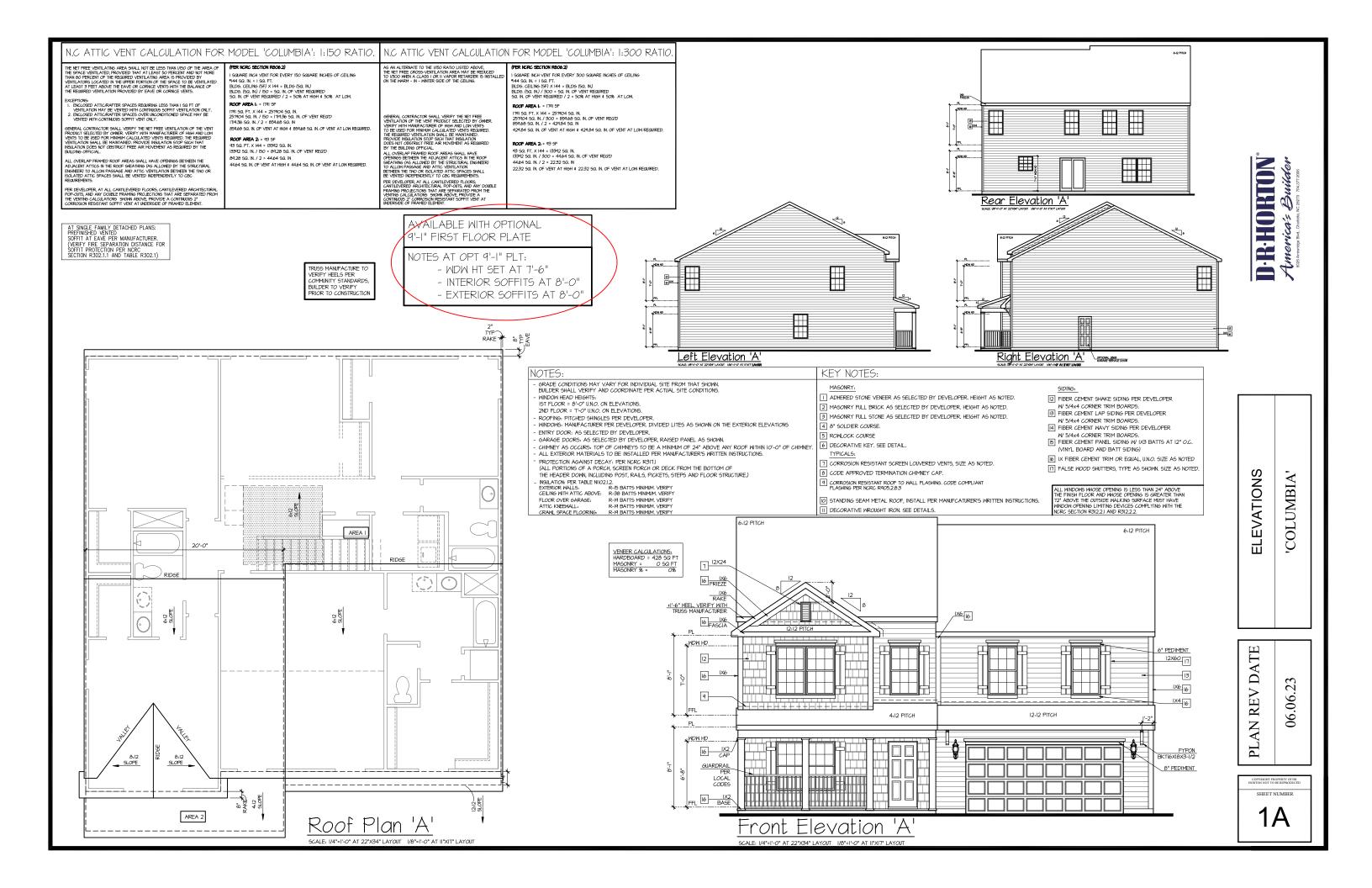
'COLUMBIA'

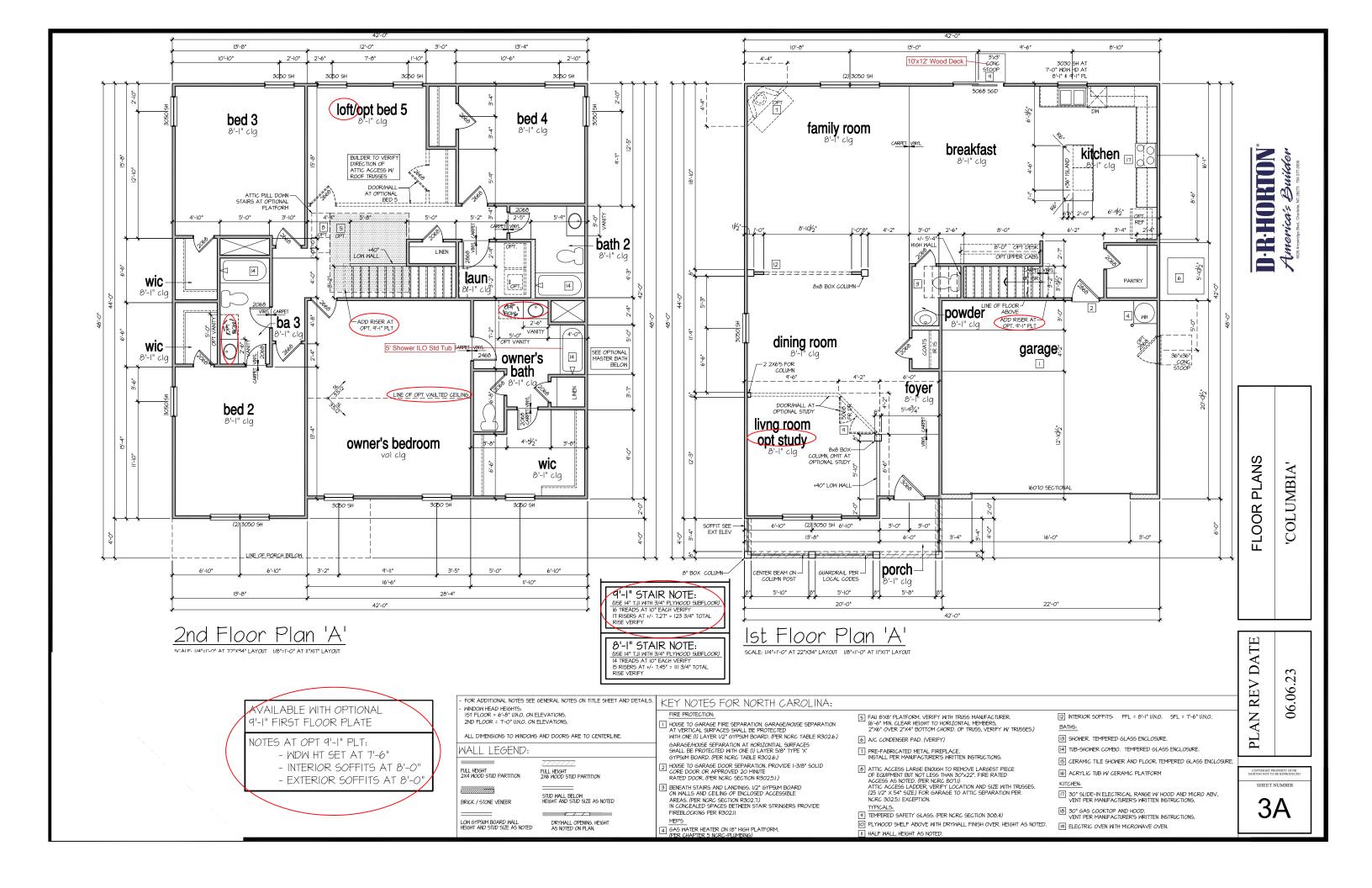
B-R-HORTON America's Builder

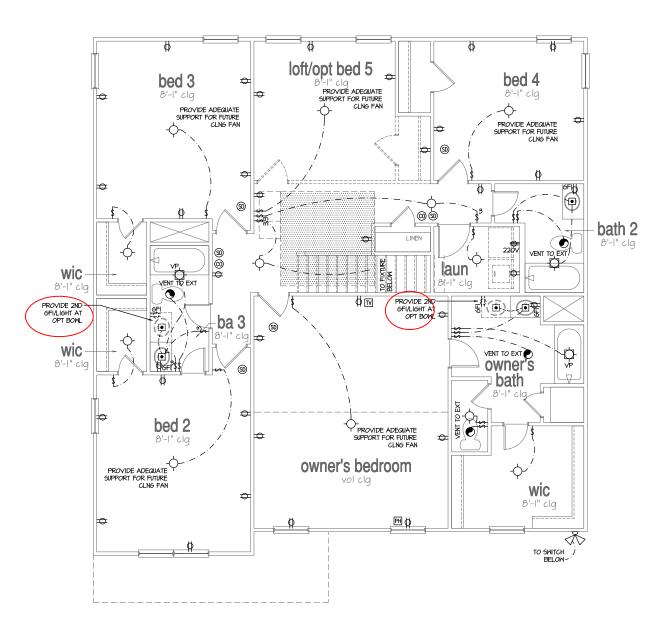
PLAN REV DATE

06.06.23

SHEET NUMBER

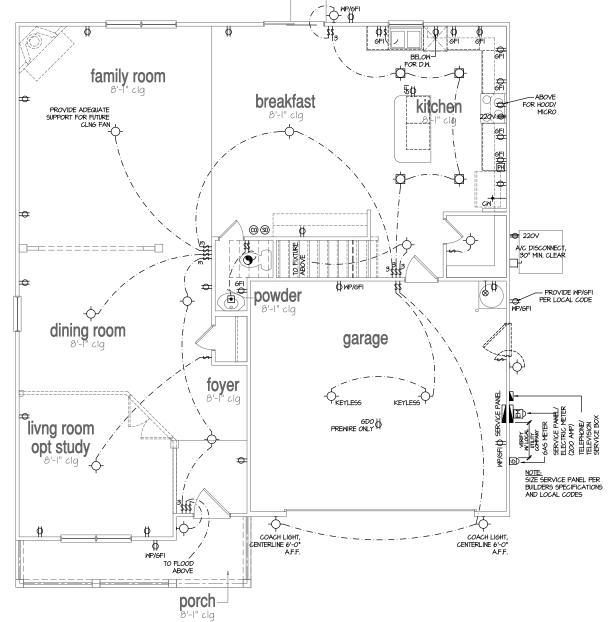






2nd Floor Plan 'A'

ALL ELEVATIONS ARE SIMILAR



Ist Floor Plan 'A'

PLAN REV DATE

FLOOR PLANS

'COLUMBIA'

06.06.23

D.R.HORTON America's Builder

NOTES:	LEG	END:						
- PROVIDE GROUNDING ELECTRICAL ROD PER LOCAL CODES.			₩.	FLUSH-MOUNT LED CEILING FIXTURE				
 PROVIDE AND INSTALL ARC FAULT CIRCUIT-INTERRUPTERS (AFCI) AS REQUIRED BY NATIONAL ELECTRICAL CODE (NEC.) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. 	Ф	DUPLEX OUTLET	Ψ	TESSIFICATI EED CEIENC TIXIONE	CH	CHIMES		
- ALL EXHAUST FANS SHALL HAVE BACKDRAFT DAMPERS.	ΦwP/6FI	I WEATHERPROOF GFI DUPLEX OUTLET	-ф-	HANGING FIXTURE	9	PUSHBUTTON SWITCH		
- FAN/LIGHTS IN WET/DAMP LOCATIONS SHALL BE LABLED "SUITABLE FOR WET OR DAMP LOCATIONS."	(f) GEI	GROUND-FAULT CIRCUIT-INTERRUPTER				IIOV SMOKE DETECTOR		CEILING FAN
- ELECTRICAL SYSTEMS ARE SHOWN FOR INTENT ONLY. THESE SYSTEMS SHALL BE ENGINEERED BY OTHERS. THE	H OI	DUPLEX OUTLET		FLUSH-MOUNT LED CEILING FIXTURE (PROVIDE CEILING FAN SUPPORT)	99	W BATTERY BACKUP		(PROVIDE ADEQUATE SUPPORT)
CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER INSTALLATION AND PLACEMENT.	0	HALF-SWITCHED DUPLEX OUTLET	OIT		6	CO2 DETECTOR		
 PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. 	*****	200 (4) 7 (4) 7	-\$	2-LIGHT VANITY FIXTURE		THERMOSTAT	⊢⊗	GAS SUPPLY WITH VALVE
- PROVIDE AND INSTALL GROUND FAULT CIRCUIT-INTERRUPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRICAL	₩2200	220 VOLT OUTLET			L W	INERPOSIAI		
CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.	0	REINFORCED JUNCTION BOX	-3	3-LIGHT VANITY FIXTURE	PH	TELEPHONE	→	, HOSE BIBB
- ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS.	1		-4	4-LIGHT VANITY FIXTURE	TV	TELEVISION		*
- HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS.	ş	WALL SWITCH	Ψ_	TEIGHT VARITT I KIDKE				N 1/4" WATER STUB OUT
- ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FURNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUMP PITS,	\$ 3	THREE-WAY SWITCH	1 -	WALL MOUNT FIXTURE		ELECTRIC METER	·cr	N 174 TANIER STOD GOT
DRAIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS.			\vdash			ELECTRIC PANEL	Ж	
 PROVIDE POWER, LIGHT AND SWITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S WRITTEN INSTRUCTIONS. 	\$ 4	FOUR-WAY SMITCH	•	EXHAUST FAN (VENT TO EXTERIOR)	4	DISCONNECT SWITCH	₹	WALL SCONCE

DESIGN SPECIFICATIONS:

Construction Type: Commerical □ Residential ⊠

Applicable Building Codes:

• 2018 North Carolina Residential Building Code with All Local Amendments

ASCE 7-10: Minimum Design Loads for Buildings and Other Structures

esign	Lo	pads:		
- 1	L	Roof	Live Loads	
		1.1.	Conventional 2x	20 PSF
		1.2.	Truss	20 PSF
			12.1. Attic Truss	60 PS
	2.	Roof	Dead Loads	
		2.1.	Conventional 2x	10 PSF
		2.2.	Truss	20 PSF
	3.	Snow		15 PSF
		3.1.	Importance Factor	10
	4.	Floor	Live Loads	

4.1. Typ. Dwelling 4.2. Sleeping Areas ... 4.3. Decks .. 40 PSF 4.4. Passenger Garage 5. Floor Dead Loads

10 PSF 5.1. Conventional 2x 6. Ultimate Design Wind Speed (3 sec. gust) 130 MPH 6.1. Exposure ______ 62. Importance Factor.

63. Wind Base Shear 6.3.1. Vx = 6.32.Vy = 7. Component and Cladding (in PSF)

	~			
MEAN ROOF HT.	UP TO 30'	3Ø'1"-35'	35'1"-4Ø'	40'1"-45'
ZONE 1	16.7,-18.0	17.5,-18.9	18.2,-19.6	18.7,-202
ZONE 2	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 3	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 4	182,-19.0	19.2,-20.0	19.9,-20.7	20.4,-21.3
ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.1	20.4,-26.9

8 Seismin

06121111	<u> </u>	
8.1.	Site Class	D
8.2.	Design Category	С
8.3.	Importance Factor	1.0
8.4.	Seismic Use Group	1
85.	Spectral Response Acceleration	

85. Spectral Response 85.1. Sms = %g 85.2. Sml = %g 8.6. Seismic Base Shear

8.6.1. Vx = 8.6.2. Vy = 87. Basic Structural Sustem (check one)

□ Dual w/ Special Moment Frame □ Dual w/ Intermediate R/C or Special Steel □ Inverted Pendulum

Wind 🖂

The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade

Any fill shall be placed under the direction or recommendation of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95%

maximum dry density.

Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation

No concrete shall be placed against any subgrade containing water, ice, frost, or loose material

Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design" latest editions.

Structural steel shall receive one coat of shop applied rust-inhibitive paint.

All steel shall have a minimum yield stress (F,,) of 36 ksi unless otherwise noted.

Welding shall conform to the latest edition of the American

Welding Society's Structural Welding Code AWS D1.1. Electrodes for shop and field welding shall be class ETOXX. All welding shall be performed by a certified welder per the above standards.

NCNCIE:
Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.

Concrete shall be proportioned mixed and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings"

Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to 42% of target values as follows:
3.l. Footings: 5%
3.2. Exterior Glabs: 5%

written permission of the SER.



STRUCTURAL PLANS PREPARED FOR:

COLUMBIA - RH

PROJECT ADDRESS

DR Horton, Inc. 8001 Arrowridge Blvd. Charlotte, NC 28273

GMD Design Group 102 Fountain Brook Circle, Suite C Cary, NC 27511

These drawings are to be coordinated with the architectural, mechanical, plumbing, Inese aratumgs are to be coordinated with the architectural, mechanical, pulmoing electrical, and civil dratumings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory 4 Testing, INC. before construction begins.

PLAN ABBREVIATIONS

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	R9	ROOF SUPPORT
CJ	CEILING JOIST	5C	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
DSP	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
EE	EACH END	SYP	SOUTHERN YELLOW PINE
ΕW	EACH WAY	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
oc	ON CENTER	TYP	TYPICAL
P6F	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
PSI	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

SHEET LIST:

Sheet No.	Description
C5I	Cover Sheet, Specifications, Revisions
SI.Øm	Monolithic Slab Foundation
S1.Øs	Stem Wall Foundation
51.0c	Crawl Space Foundation
S1.0b	Basement Foundation
52.Ø	Basement Plan
53.Ø	First Floor Plan
54.0	Second Floor Plan
95.0	Roof Framing Plan

DR HORTON PROJECT SIGN-OFF

Manager	Signature	
Operations		
Operations System		
Operations Product Development		

SUMMIT

REVISION LIST:

Revision No.	Date	Project No.	Description
1	2.10.17	12388R	Created LH and RH versions
2	5.24.17	12388R2	Revised garage slab. Verified floor (4.23.15)
3	6.13.17	12388R3	Added stem wall foundation
4	11.15.17	12388R4	Revised SYP and pressure treated member notes
5	4.23.18	17869	Added crawl space foundation
6	7.10.18	17869R	Revised per new architectural plans (4,24,18)
7	8.3Ø.18	17869R2	Added dimensions to tapered porch columns
8	10.5.18	17869R3	Added stick framed options at extended porch
9	11.28.18	17869R4	Revised NC version only for 2018 NCRC
10	3.14.19	21844	Revised TN version only for 2018 IRC
- 11	12,27,19	21844R	Revised SC version only for 2018 IRC
			Verified roof (3.28.11), added interior bearing
12	Ø3.IØ.21	528.TØØ6Ø	Added OX-15 Structural Insulated Sheathing Option
13	5.6.21	TØ060	Added SPF note option
14	6.30.21	TØØ6Ø	Added OX-15 bracing for framing
15	8.9.23	TØØ6Ø	Changed the direction of the floor system over the garage & updated garage header sizes
16	Ø9.15.23	TØ060	Added Girder to Crawl Space Foundation

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Added OX-16 bracing for framing
nged the direction of the floor system over e garage 4 updated garage header sizes
Added Girder to Crawl Space Foundation

The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.

The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to

The trusses shall be designed fabricated and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Blood Trusses

The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments fo

the trusses.

Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

EXTERIOR WOOD FRAMED DECKS:

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

WOOD STRUCTURAL PANELS:

- Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide
 "Residential and Commercial," and all other applicable APA standards.
- $\ensuremath{\mathsf{All}}$ structurally required wood sheathing shall bear the mark of

Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.

Roof sheathing shall be APA rated sheathing exposure 1 or 2.

Roof sheathing shall be an A tated steading expected in 12.

Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as

required by the state Building Code.

Wood floor sheathing shall be APA rated sheathing exposure I or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshark nall at 6°o/c at panel edges and at 12°o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing, like suitable edge support by use of T4G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
Sheathing shall have a 1/8" gap at panel ends and edges as

recommended in accordance with the APA.

STRUCTURAL FIBERBOARD PANELS:

Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.

All structurally required fiberboard sheathing shall bear the mark of the AFA.

mark or the AFA.

Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more

Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.



RAUNG DATE: *09/15/20*23 9CALE: 22x34 |/4"∗1"-Ø" |k/∏ |/8"∗1"-Ø" PROJECT 9 528.10060 DRAWN BY: EO

CHECKED BY: JOEF

REFER TO COVER SHEET FOR A

GENERAL STRUCTURAL NOTES:

1. The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without unitien permission of SUPMIT Engineering, Laboratory & Testing, INC. (SUPMIT) or the SER. For the purposes of these construction documents the SER and SUPMIT

shall be considered the same entity. The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction

to stabilize the structure.

The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents should any non-conformities occur.

Anu structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions

is not the responsibility of the SER or SWMMIT.

Verification of assumed field conditions is not the responsibilit of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.

The SER is not responsible for any secondary structural elements

or non-structural elements, except for the elements specifically noted on the structural drawings. This structure and all construction shall conform to all

FOUNDATIONS:

I. The structural engineer has not performed a subsurface

contacted before proceeding.

investigation. Verification of this assumed value is the

responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be

applicable sections of the international residential code. This structure and all construction shall conform to all applicable sections of local building codes. All structural assemblies are to meet or exceed to requirements of the current local building code.

No admixtures shall be added to any structural concrete without

Concrete slabs-on-grade shall be constructed in accordance with ACI 302.IR-96: "Guide for Concrete Slab and Slab

The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from urreported conditions not in accordance with the above assumptions.

Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted Control or saw cut joints shall be produced using conventional

process within 4 to 12 hours after the slab has been finished nforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut joint.

10. All welded wire fabric (WWF.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The WWF. shall be securely supported during the concrete pour.

CONCRETE REINFORCEMENT:

Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.

Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcemen

Application of filpermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (15 pounds per cubic yard)
Filbermesh shall comply with ASTM CIII6, any local building code equirements, and shall meet or exceed the current industry

Steel reinforcing bars shall be new billet steel conforming to

ASTM A615, grade 60.
Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"

Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same

size/spacing as the horizontal reinforcement with a class B Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.

Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters

into the footing.

Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.

WOOD FRAMING:

I. Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Southern-Yellow-Pine (SYP) 2 or Southrn-Spruce Pine (SPF) 2. LVL or PSL engineered wood shall have the following minimum

design values: 2.1. E = 1,900,000 psi 2.2. Fb = 2600 psi

2.4.Fc = 700 psi Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance

with AUPA standard C-2 Nalls shall be common wire nalls unless otherwise noted.

Lag screws shall conform to ANSI/ASME standard B182.1-1981 Lead holes for lag screws shall be in accordance with NDS

specifications.

All beams shall have full bearing on supporting framing members

unless otherwise noted. Exterior and load bearing stud walls are to be 2x4 SYP *2 *9 l6 *
O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimu of one king stud shall be placed at each end of the header. King studs shall be continuous.

Ring stude shall be continuous. Individual stude forming a column shall be attached with one 100 nail 10 6° 100. Staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfer, Multi-ply beams shall have each ply attached with (3) lod nails a

10. Four and five ply beams shall be bolted together with (2) rows of 1/2" diameter through bolts staggered @ 16" O.C. unless noted otherwise.

WOOD TRUSSES:

- FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL
- STRUCTURAL CONCRETE TO BE Fc = 3000 PSI, PREPARED AND PLACED IN
- ACCORDANCE WITH ACI STANDARD 318.
 FOOTINGS TO BE PLACED ON UNDISTURBED EARTH, BEARING A MINIMUM OF 12" BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE CODE ENFORCEMENT OFFICIAL.
 FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF
- 2000 PSF. CONTRACTOR IS SOLELLY RESPONSIBLE FOR VERRYING THE SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION. FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE
- ELEMENTS. PROVIDE 2" MINIMUM FOOTING PROJECTION FROM THE FACE OF
- MASONRY.

 MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE 46 SPECIFIED IN SECTION R404.1 OF THE 2018 NORTH CAROLINA RESIDENTIAL
- SPECIFIED IN SECTION RADALOF THE 200 NORTH CAROLINA RESIDENTIAL BUILDING CODE.

 PILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL.

 PROVIDE FOUNDATION WATERPROOFING, AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS.

 PROVIDED PERIMETER INSULATION FOR ALL FOUNDATIONS PER 2018 NORTH
- CAROLINA RESIDENTIAL BUILDING CODE.
- 10. CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK
- VENEERS.

 CRAUL SPACE TO BE GRADED LEVEL, AND CLEARED OF ALL DEBRIS.
- CRAIL SPACE (D BE GRADE) LEVEL, AND LEARED OF ALL DEBRIS. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2016 NORTH CAROLINA RESIDENTIAL CODE SECTION R403.16, MINIMUM 1/2" DIA BOLTS SPACED AT 6'-0" ON CENTER WITH A "I" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE, ANCHOR BOLTS SHALL BE 12" FROM THE END OF EACH PLATE SECTION MINIMUM (2) ANCHOR BOLTS FER PLATE SECTION, ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.
- 9. ABBREVIATIONS:
- DJ = DOUBLE JOIST SJ = SINGLE JOIST FT = FLOOR TRUSS GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END DR = DOUBLE RAFTER
 TR = TRIPLE RAFTER TJ = TRIPLE JOIST OC = ON CENTER
- 10. ALL PIERS TO BE 16 "X16" MASONRY AND ALL PILASTERS TO BE 8 "X16"
- MASONRY, TYPICAL. (UNO)
 WALL FOOTINGS TO BE CONTINUOUS CONCRETE, SIZES PER STRUCTURAL PLAN.
- A FOUNDATION EXCAVATION OBSERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER, OR HIS QUALIFIED REPRESENTATIVE. IF ISOLATED AREAS OF YIELDING MATERIALS AND/OR POTENTIALLY EXPANSIVE SOILS ARE OBSERVED IN THE FOOTING EXCAVATIONS AT THE TIME OF CONSTRUCTION, SUMMIT ENGINEERING, LABORATORY 4 TESTING, INC. MUST BE PROVIDED THE OPPORTUNITY TO REVIEW THE FOOTING DESIGN PRIOR TO CONCRETE PLACEMENT.
- ALL FOOTINGS & SLABS ARE TO BEAR ON UNDISTURBED SOIL OR 95% COMPACTED FILL, VERIFIED BY ENGINEER OR CODE OFFICIAL.

REFER TO BRACED WALL PLAN FOR PANEL LOCATIONS AND ANY REQUIRED HOLDOWNS, ADDITIONAL INFORMATION PER SECTION R602:10.8 AND FIGURES R602:10.6.5, R602,10,7, R602,10,8(1) AND R602,10,8(2) OF THE 2015 IRC

NOTE: ALL EXTERIOR FOUNDATION DIMENSIONS ARE TO FRAMING AND NOT BRICK VENEER, UNO

NOTE: A 4" CRUSHED STONE BASE COURSE IS NOT REQUIRED WHEN SLAB IS INSTALLED ON WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS CLASSIFIED AS GROUP LIPER TABLE R4051

REINFORCE GARAGE PORTAL WALLS PER FIGURE R602.10.9 OF THE 2015 IRC.

BEAM POCKETS MAY BE SUBSTITUTED FOR MASONRY PILASTERS AT GIRDER ENDS, BEAM POCKETS SHALL HAVE A MINIMUM 4" SOLID MASONRY BEARING.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL, PLANS PROVIDED BY DR HORTON COMPLETED REVISED ON 2/28/20, IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY & TESTING, INC., IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS.

ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO
BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, INC. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

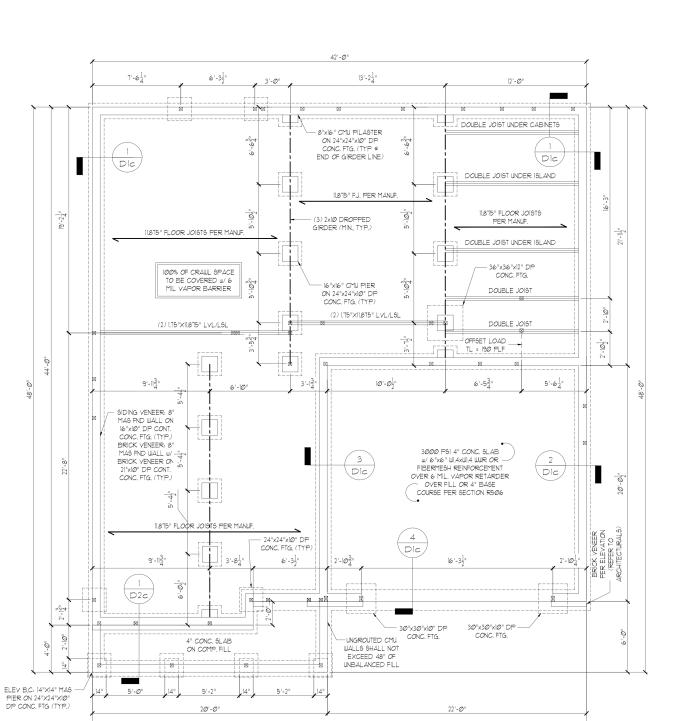
STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

CRAWL SPACE FOUNDATION PLAN

9CALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"

18"x24" MIN, CRAWL SPACE ACCESS DOOR TO BE LOCATED IN FIELD PER BUILDER, PROVIDE MIN. (2) 2XIØ HEADER OVER DOOR W/ MIN. 4" BEARING EACH END. AVOID SHOWN POINT LOADS.

DECK FLOOR JOISTS SHALL BE SPACED AT MAX. 12" ON CENTER WHEN DECKING INSTALLED DIAGONALLY



CRAWL SPACE FOUNDATION - ALL ELEVATIONS





TO 00 \$ **₹**



DR**AUNG** DATE: *09/15/20*23 8CALE: 22x34 1/4"+1"-0" lk:|1 1/8"+1"-0" PROJECT 4 528,70060 CHECKED BY: JOEF



REQUIRED BRACED WALL PANEL CONNECTIONS						
			REQUIRED CONNECTION			
METHOD	MATERIAL	MIN. THICKNESS	@ PANEL EDGES	© INTERMEDIATE SUPPORTS		
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS © 12" O.C.		
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS** © 7" O.C.	5d COOLER NAILS** © 7" O.C.		
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.		
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.6.4	PER FIGURE R602.10.6.4		
"OR EQUIVALENT PER TABLE R102.3.5						

GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING
- CODE WITH ALL LOCAL AMENDMENTS,
 CONTRACTOR SHALL VERIEY ALL DIMENSIONS, CONTRACTOR SHALL COMPLY WITH
 THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT, ENGINEER IS NOT
 RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.
- CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED.

- CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION.
 PROPERTIES USED IN THE DESIGN ARE AS POLLOUS:
 MICROLLAM (LVL), F_B = 2600 PSI, Fv = 285 PSI, E = 1.9x10° PSI
 PARALLAM (PSL), F_B = 3900 PSI, Fv = 390 PSI, E = 1.9x10° PSI
 ALL WOOD MEMBERS SHALL BE "\$ 5YP", SPF (MLESS NOTED ON PLAN, ALL STUD COLUMN AND JOINTS SHALL BE "\$ 5YP", SPF (MLS)
 ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 "\$ 5YP.72 SPF (SUD COLUMN AT EXCLUSION) IN SEA WITHOUT ONLY SUPPORTED WITH A (2) 2x4 "\$ 5YP.72 SPF (SUD COLUMN AT EXCLUSION) IN SEA WITHOUT ONLY SUPPORTED WITH A (2) 2x4 "\$ 5YP.72 SPF (MLS).
- EACH END UNLESS NOTED OTHERWISE.

 ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM A615

 AND SHALL HAVE A MINIMUM COVER OF 3".

 FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH FOUNDATION AND CONTROLLE SECTION RADIALS HIS MINIMUM IN!" DIA BOLITÓ SPACED AT 6'-0" ON CENTER WITH A 1" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE. ANCHOR BOLITÓ SHALL BE 12" FROM THE END OF EACH PLATE
- SECTION, MINIMUM (2) ANCHOR BOLTS PER PLATE SECTION, ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE. CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- PERFENDICULAR TO RAFIERS. FLITCH BEAMS, 4-PLY LVIS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER UITH 1/2" DIA THAI BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/D31. MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
- ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2'x4 STP "2'SFF" "2', DROPPED, FOR NON-LOAD BEARING HEADERS EXCEEDING 8'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2'x4 SYP 12/SPF 12, DROPPED. (UNLESS NOTED OTHERWISE)
- ABBREVIATIONS:

DJ = DOUBLE JOIST SJ = SINGLE JOIST GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END TJ = TRIPLE JOIST FT = FLOOR TRUSS DR = DOUBLE RAFTER TR = TRIPLE RAFTER OC = ON CENTER CL = CENTER LINE PL = POINT LOAD

NOTE:

DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE, PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

JOIST & BEAM SITES SHOUN ARE MINIMUMS BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

INSTALL ANY REQUIRED HOLDOWNS PER SECTION R602.10.8 AND FIGURES R602.10.6.5, R602.10.7, R602.10.8(1) AND R602.10.8(2) OF THE 2015 IRC

NOTE: MEMBER NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL, PLANS PROVIDED BY DR. HORTON COMPLETED/REVISED ON 2/28/20, IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY & TESTING, CLIEN TO KOTIFT SUMMIT ENGINEERING, LABORATIONE & TESTING, INC, IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS HEND WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

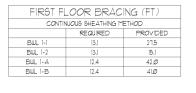
STRUCTURAL MEMBERS ONLY

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

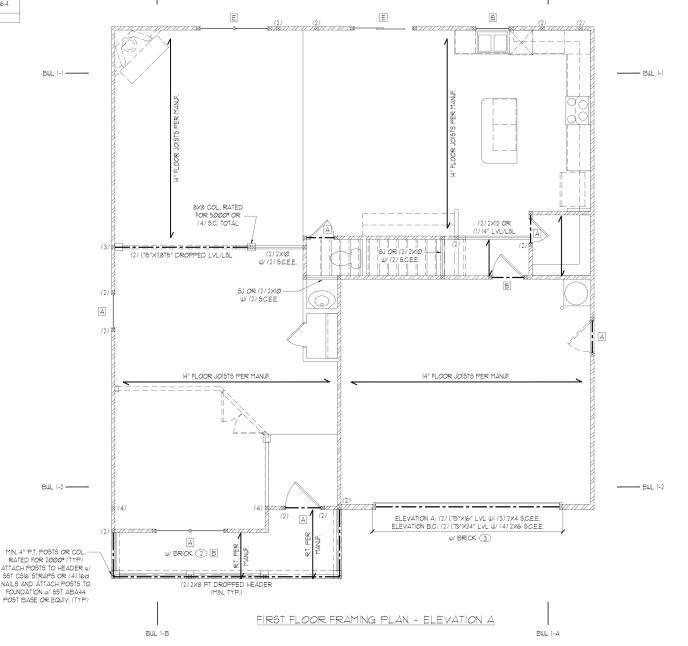
FIRST FLOOR FRAMING PLAN

5CALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"



BWL 1-A

BWL 1-B



HEADER SCHEDULE			
TAG	SIZE	JACKS (EACH END	
А	(2) 2x6	(1)	
В	(2) 2x8	(2)	
С	(2) 2xlØ	(2)	
D	(2) 2xl2	(2)	
E	(2) 9-1/4" LSL/LVL	(3)	
F	(3) 2x6	(1)	
G	(3) 2x8	(2)	
Н	(3) 2xlØ	(2)	
	(3) 2x12	(2)	

HEADER SITES SHOUN ON PLANS ARE MINIMUMS GREATER HEADER SIZES SHOWN ON FLAMS ARE THIN IN IN GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION. ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE. SC NOTED ON PLAN OVERRIDE SC LISTED ABOVE

LINTEL SCHEDULE				
TAG	TAG SIZE			
	L3x3x1/4"	LESS THAN 6'-0"		
2	6'-0" TO 10'-0"			
3	L5x3-1/2"x5/16"	GREATER THAN 10'-0"		
4	ALL ARCHED OPENINGS			

SECURE LINTEL TO HEADER W/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED & 16" O.C. (TYP FOR 3)

ALL HEADERS WHERE BRICK IS USED, TO BE: (1)(UNO)

WALL STUD SCHEDULE

| 151 # 2ND FLOOR LOAD BEARING STUDS: 2x4 STUDS @ 16" OC. OR 2x6 STUDS @ 24" OC. | 151 FLOOR LOAD BEARING STUDS @ 16" OC. 2x4 STUDS @ 12" OC. OR 2x6 STUDS @ 16" OC. 2x4 STUDS @ 12" OC. OR 2x6 STUDS @ 16" OC. 2x4 STUDS @ 12" OC. OR 2x6 STUDS @ 16" OC. NON-LOAD BEARING STUDS (ALL FLOORS) 2x4 STUDS @ 24" O.C. TWO STORY WALLS: 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED W/ CROSS BRACING @ 6'-0" O.C. VERTICALLY

KING STUD R	EQUIREMENTS
OPENING WIDTH	KINGS (EACH END)
LESS THAN 3'-0"	(1)
3'-Ø TO 4'-Ø"	(2)
4'-0" TO 8'-0"	(3)
8'-0" TO 12'-0"	(5)
12'-0" TO 16'-0"	(6)
KING STUD DECULIDEN	TENTS ABOVE DO NO

APPLY TO PORTAL FRAMED OPENINGS

BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10
- FROM THE 2015 INTERNATIONAL RESIDENTIAL CODE AS ALLOWED PER SECTION RE02/10 OF THE 2018 NO RESIDENTIAL CODE. UALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE WIND SPEEDS UP TO 13/0 MPH.
- REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING
- 3. BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN
- ACCORDANCE WITH IRC TABLE R602.104.
 4. ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEDE OF RET FOR ISOLATED PANEL METHOD AND IZ FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL
- ENGINEERING CALCULATIONS.
 MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.10.5.
- THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD (UNO).
- 1/27 3 FROM BOARD (MO).
 FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFILL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS.
- FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- A BRACED WALL PANEL SHALL BE LOCATED WITHIN 10 FEET OF
- EACH END OF A BRACED WALL LINE.

 10. THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 20 FEET.
- SHALL NOT EXCEED 20 FEET.

 MASONRY OR CONCRETE STEM WALLS WITH A LENGTH OF 48" OR
 LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN
 ACCORDANCE WITH FIGURE R602 10.9 OF THE 2015 IRC.
- 12. BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE
- CONSTRUCTED IN ACCORDANCE WITH SECTION R602:108
 BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE
 CONSTRUCTED IN ACCORDANCE WITH SECTION R602:1082 AND FIGURES R602.10.8(1)4(2)4(3).
- HOUSES FACILIZATION (2415).

 4. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION RE02.10.11

 15. PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE
- R6021064 (UNO)
- ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS. ABBREVIATIONS:

CS-XXX = CONT. SHEATHED ENG = ENGINEERED SOLUTION
PF = PORTAL FRAME PF-ENG = ENG. PORTAL FRAME





CLIENT: DR Horton, Inc. 8001 Arrowridge Blvc Charlotte, NC 28213

Ω Ø Ţ 0



DR**AUNG** DATE: *09/15/20*23 8CALE: 22x34 1/4"+1"-0" lk:|1 1/8"+1"-0" PROJECT 9 528,10060 CHECKED BY: JOEF

RIGINAL INFORMA PROJECT * T0060 DATE Ø3/IØ/2Ø2

REQUIRED BRACED WALL PANEL CONNECTIONS				
		L MIN. THICKNESS	REQUIRED CONNECTION	
METHOD	MATERIAL 1		@ PANEL EDGES	@ INTERMEDIATE SUPPORTS
C5-W5P	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS	6d COMMON NAILS © 12" O.C.
GB	GYP9UM BOARD	1/2"	5d COOLER NAILS** @ 7" O.C.	5d COOLER NAILS" @ 7" O.C.
W6P	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS 9 6" O.C.	6d COMMON NAILS 9 12" O.C.
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.6.4	PER FIGURE R602.10.6.4
#OP FOLIVALENT PER TABLE PTØ235				

SECOND FLOOR BRACING (FT)			
CONTINUOUS SHEATHING METHOD			
REQUIRED PROVIDED			
BWL 2-1	7.2	28.6	
BWL 2-2	7.2	25.2	
BWL 2-A	6.9	37.6	
BWL 2-B	6.9	36.7	

BWL 2-B

GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AMENDMENTS.

 CONTRACTOR SHALL VERIFY ALL DIMENSIONS. CONTRACTOR SHALL COMPLY WITH
- THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT. ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.
 CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED
- TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION.

- TO RESIST ALL FORCES ENCONTERED DURING ERECTION.

 PROPERTIES USED IN THE DESIGN ARE AS FOLLOUS:

 MICROLLAM (LVL): F₀ = 2600 PSI, F₁ = 285 PSI, E = 1.9x/0° PSI

 PARALLAM (PSI): F₀ = 2900 PSI, F₁ = 290 PSI, E = 1.25x/0° PSI

 ALL WOOD MEMBERS SHALL BE "2 SYP/"2 SPF WILESS NOTED ON PLAN. ALL STUD

 COLUMNS AND JOISTS SHALL BE "2 SYP/"2 SPF (WO).

 ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 "2 SYP/"2 SPF STUD COLUMN AT

 EACH END WILESS NOTED QUIEDWING.
- EACH END UNLESS NOTED OTHERWISE.
- ALL RENFORCING STELL SHALL BE GRADE 60 BARS CONFORMING TO ASTM A615
 AND SHALL HAVE A MINIMUM COVER OF 3".
 FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION R403.16. MINIMIM 1/2" DIA. BOLTS SPACED AT 6 -0" ON CENTER WITH A 1" MINIMIM BYBEDMENT INTO MASONRY OR CONCRETE. ANCHOR BOLTS SHALL BE 10" FROM THE BOD OF EACH PLATE SECTION. MINIMIM (2) ANCHOR BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL
- SECTION, MINIMUM (27 ANCHOR BOLLS FER PLATE SECTION, ANCHOR BOLLS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.

 9. CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN PERFENDICULAR TO RAFTERS.

 10. FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH 1/2" DIA, THRU BOLTS SPACED AT 24" OC. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL (102) MINIMUM 6" FROM EACH END OF THE BEAM.

 AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
- ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP 7/5FF 9, DROPPED, FOR NON-LOAD BEARING HEADERS EXCEEDING 8-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 SYP *2/SPF *2, DROPPED, (UNLESS NOTED OTHERWISE) 12. ABBREVIATIONS:

SJ = SINGLE JOIST GT = GIRDER TRUSS FT = FLOOR TRUSS SC = STUD COLUMN EE = EACH END DR = DOUBLE RAFTER TR = TRIPLE RAFTER TJ = TRIPLE JOIST OC = ON CENTER PL = POINT LOAD

CL = CENTER LINE

DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

JOIST & BEAM SIZES SHOWN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

INSTALL ANY REQUIRED HOLDOWNS PER SECTION R602.10.8 AND FIGURES R602.10.6.5, R602.10.1, R602 I08(1) AND R602 I08(2) OF THE 2015 IRC

NOTE: MEMBER NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>DR. HORTON</u>
COMPLETED/REVISED ON 2/28/28. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY & TESTING, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, INC. AND CONTROL AND TESTING AND CANNOT CHANGES THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, INC. AND CONTROL AND THE TIME AND CANNOT CHANGES THE SET. TESTING, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

SECOND FLOOR FRAMING PLAN

SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"

	BWL 2-B		BWL 2-A	
(A)	1			A BWL 2-1
A	ROOT TREGGES FER MANIF.	OPT. VAULT		
BWL 2-2	GIRDER TRUSS PER MANUF. W/ (4) S.C.E.E.	(2) (2) LE O LE CONTROL (2) LE		——— BUL 2-2
	[[A		1	

SECOND FLOOR FRAMING PLAN - ALL ELEVATIONS

BWL 2-A

HEADER SCHEDULE			
TAG	SIZE	JACKS (EACH END)	
Д	(2) 2x6	(1)	
B (2) 2x8		(2)	
С	(2) 2xlØ	(2)	
D	(2) 2x12	(2)	
E	(2) 9-1/4" LSL/LVL	(3)	
F	(3) 2x6	(1)	
G	(3) 2x8	(2)	
Н	(3) 2xlØ	(2)	
	(3) 2xl2	(2)	

HEADER SIZES SHOWN ON PLANS ARE MINIMUMS, GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION.
ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE.
SC NOTED ON PLAN OVERRIDE SC LISTED ABOVE.

LINTEL SCHEDULE				
TAG	SIZE	OPENING SIZE		
1	L3x3x1/4"	LESS THAN 6'-0"		
2	L5x3xl/4"	6'-0" TO 10'-0"		
3	L5x3-1/2"x5/16"	GREATER THAN 10'-0"		
4	L5x3-1/2"x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS		

SECURE LINTEL TO HEADER W/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED ® 16" O.C. (TYP FOR (3))

ALL HEADERS WHERE BRICK IS USED, TO BE: (1)(UNO)

WALL STUD SCHEDULE

16T & 2ND FLOOR LOAD BEARING STUDS: 2x4 STUDS @ 16" O.C. OR 2x6 STUDS @ 24" O.C. 195 FLOOR LOAD BEARING STUDS W WALK-UP ATTIC: 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BASEMENT LOAD BEARING STUDS: 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. NON-LOAD BEARING STUDS (ALL FLOORS): 2x4 STUDS @ 24" O.C. TWO STORY WALLS:
2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED w/ CROSS BRACING @ 6'-0" O.C. VERTICALLY

KING STUD R	EQUIREMENTS
OPENING WIDTH	KINGS (EACH END)
LESS THAN 3'-Ø"	(1)
3'-0 TO 4'-0"	(2)
4'-0" TO 8'-0"	(3)
8'-0" TO 12'-0"	(5)
12'-Ø" TO 16'-Ø"	(6)

KING STUD REQUIREMENTS ABOVE DO NOT APPLY TO PORTAL FRAMED OPENINGS

BRACED WALL NOTES:

- 1) WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2015 INTERNATIONAL RESIDENTIAL CODE AS ALLOWED PER SECTION R60210 OF THE 2018 NC RESIDENTIAL CODE.
 WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE WIND
- SPEEDS UP TO 130 MPH.
- 2. REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING
- 3. BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN
- ACCORDANCE WITH IRC TABLE R60/204.

 4. ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED UP FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL
- ENGINEERING CALCULATIONS.
 MINIMUM PANEL LENGTH SHALL BE PER TABLE REGOLIOS.
 THE INTERIOR BIOD OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM
- INZ GYPSUM BOARD (UNO).
 FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFILL AREAS
- BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL
 OPENINGS, AND ON GABLE END WALLS.
 8. FLOORS HALL NOT BE CANTILEVERED MORE THAN 24" BEYOND
 THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL
- ENGINEERING CALCULATIONS.

 9. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 10 FEET OF EACH END OF A BRACED WALL LINE.

 10. THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS
- MALONOT EXCEED 20 FEET.
 MASONRY OR CONCRETE STEM WALLS WITH A LENGTH OF 48" OR
 LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN
- ACCORDANCE WITH FIGURE R602 ID 9 OF THE 2015 IRC.
- 2. BRACED WALL PANEL CONNECTIONS TO FIGURE REPLIES OF THE 2005 INC.

 13. BRACED WALL PANEL CONNECTIONS TO FLOOR(CEILING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION RE02.108

 13. BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.82 AND
- FIGURES REGOLIO & (1)4(2)4(3).

 14. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE
- CEMPTE WALLS AND WALL OUT DASPINEN WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUIRED.

 B. PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE REGO! 106.4 (UNO)

 B. ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.

 J. ABBREVIATIONS:

GB = GYPSUM BOARD WSP = WOOD STRUCTURAL PANEL





CLIENT: DR Horton, Inc. 8001 Arrowridge Blv Charlotte, NC 28213

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DR**AUNG** DATE: *09/15/20*23 8CALE: 22x34 1/4"+1"-0" lk:|1 1/8"+1"-0" PROJECT 9 528,10060 CHECKED BY: JOEF

DATE Ø3/IØ/2Ø21

NOTE: 1ST PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNO)

NOTE: ROOF TRUSSES SHALL BE SPACE TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

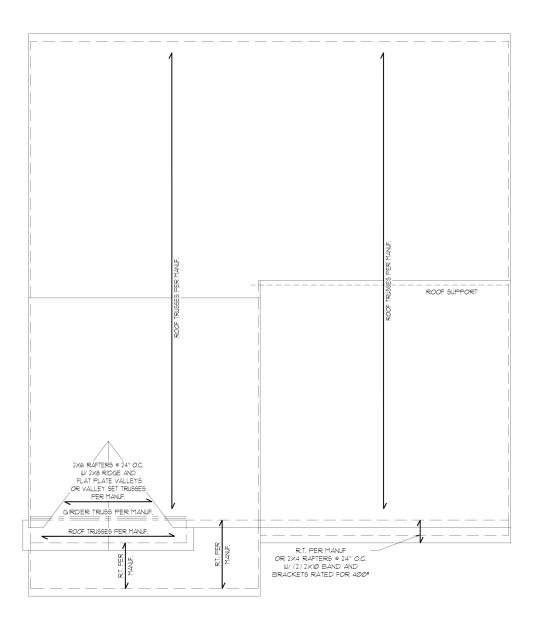
THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DR HORTON COMPLETED/REVISED ON 2/28/28. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY 4 TESTING, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY 4 TESTING, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, INC. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

ROOF FRAMING PLAN SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"



ROOF FRAMING PLAN - ELEVATION A





F Framing PROJECT: Columbia -ROOF



DRAUNG DATE: 09/5/2023 PROJECT \$ 528.10060 CHECKED BY: JOEF

ORIGINAL INFORMATION
PROJECT • DATE
T0060 03/10/2021