

RE: MP38

DRHORTON/WILMINGTON; LOT 38 MCKAY PLACE

Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: MP38 Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.6 Wind Speed: 120 mph Floor Load: N/A psf

This package includes 10 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	158690911	A01G	6/1/2023
2	158690912	A02V	6/1/2023
3	158690913	A03	6/1/2023
4	158690914	A04	6/1/2023
5	158690915	A05G	6/1/2023
6	158690916	A06G	6/1/2023
7	158690917	P01	6/1/2023
8	158690918	P02	6/1/2023
9	158690919	P03	6/1/2023
10	158690920	P04G	6/1/2023

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Builders FirstSource-Apex,NC.

Truss Design Engineer's Name: Gilbert, Eric

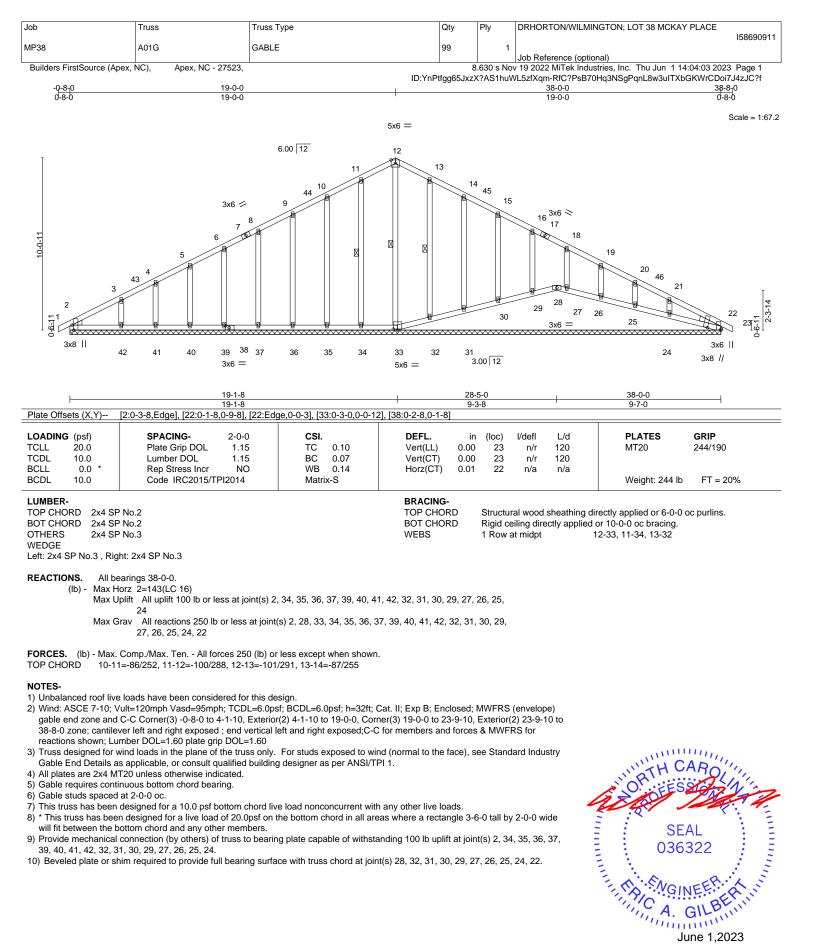
My license renewal date for the state of North Carolina is December 31, 2024

North Carolina COA: C-0844

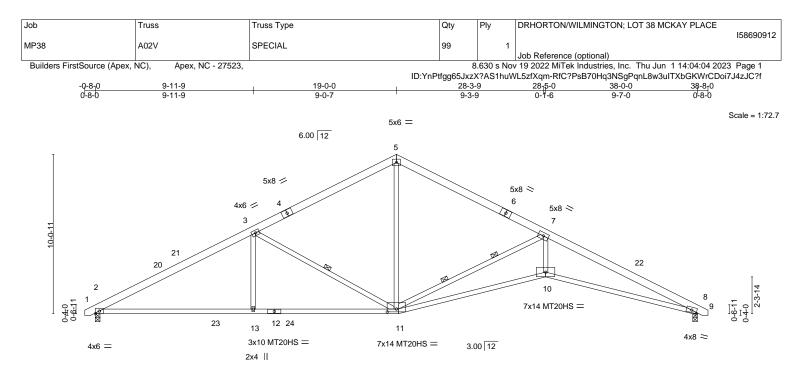
IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



1	9-11-9	19-1-8	28-5-0	38-0-0
Γ	9-11-9	9-1-15	9-3-8	9-7-0
Plate Offsets (X,Y) [1	11:0-7-0,0-2-3]			

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.66 BC 0.97 WB 0.95 Matrix-MS	Vert(LL) -0.38 Vert(CT) -0.85 Horz(CT) 0.33	n (loc) l/defl L/d 5 10-11 >999 360 5 10-11 >535 240 7 8 n/a n/a 7 10-11 >999 240	PLATES MT20 MT20HS Weight: 215 lb	GRIP 244/190 187/143 FT = 20%
	P No.1 *Except*		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing d Rigid ceiling directly applied	2 I I	
WEBS 2x4 SI	2x4 SP SS, 11-12: 2x4 SP No.2 P No.3 *Except* 2x4 SP SS		WEBS		3-11 7-11	

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=135(LC 12) Max Uplift 2=-27(LC 12), 8=-27(LC 13) Max Grav 2=1549(LC 1), 8=1549(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2685/217, 3-5=-1838/249, 5-7=-1845/247, 7-8=-4890/321

BOT CHORD 2-13=-100/2303, 11-13=-100/2303, 10-11=-217/4406, 8-10=-218/4406

WEBS 3-13=0/403, 3-11=-942/155, 5-11=-28/999, 7-11=-3081/264, 7-10=0/2288

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-5-14 to 4-3-12, Interior(1) 4-3-12 to 19-0-0, Exterior(2) 19-0-0 to 25-9-7, Interior(1) 25-9-7 to 38-5-14 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are MT20 plates unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

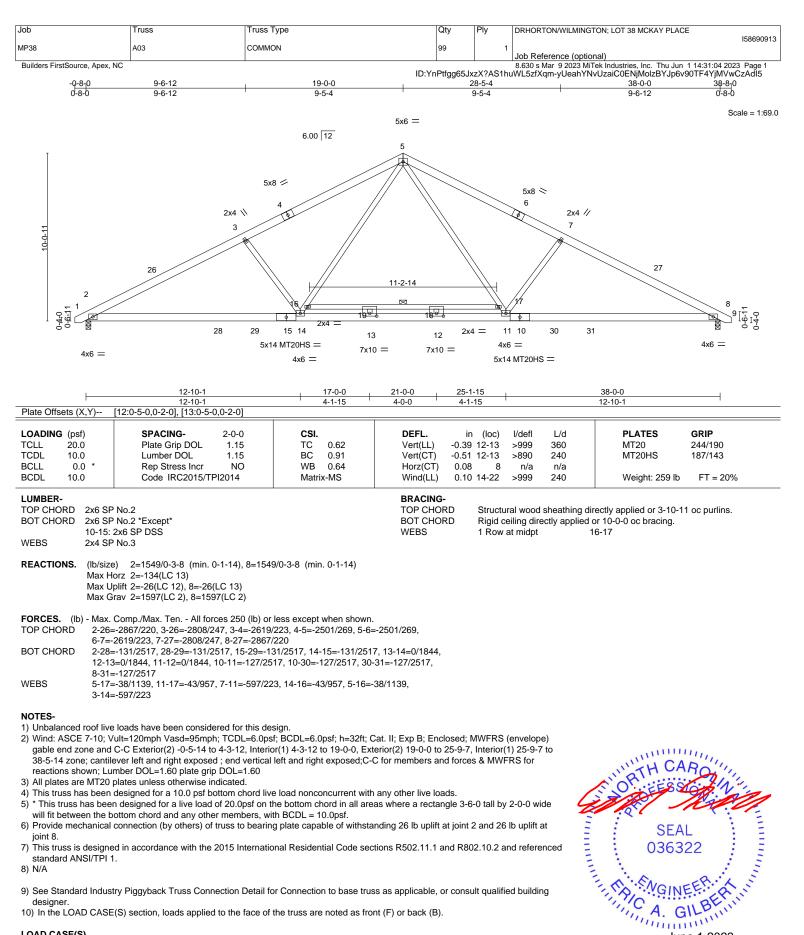
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.
 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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LOAD CASE(S)

nued on page

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

818 Soundside Road

Edenton, NC 27932

June 1,2023

Job	Truss	Truss Type	Qty	Ply	DRHORTON/WILMINGTON; LOT 38 MCKAY PLACE
MP38	A03	COMMON	99	1	158690913
Builders FirstSource				· · ·	Job Reference (optional) 8.630 s Mar 9 2023 MiTek Industries, Inc. Thu Jun 1 14:31:04 2023 Page 2
Builders FirstSource	e, Apex, NC		ID:YnPtfgg65J:	xzX?AS1h	uWL5zfXqm-yUeahYNvUzaiC0ENjMolzBYJp6v90TF4YjMVwCzAdl5
LOAD CASE(S))				
1) Dead + Roof Uniform Load	Live (balanced): Lumber Increase ds (plf)	e=1.15, Plate Increase=1.15			
	: 1-5=-60, 5-9=-60, 20-23=-20				
2) Dead + 0.75 Uniform Load		hab. Attic Storage: Lumber Increase=1.1	b, Plate Increase=1.15		
		-29=-50, 29-30=-20, 30-31=-50, 23-31=-2 umber Increase=1.25, Plate Increase=1.2			
Uniform Load	ds (plf)				
	: 1-5=-20, 5-9=-20, 20-23=-40, 16 C-C Wind (Pos. Internal) Case 1: L	-17=-40(F) .umber Increase=1.60, Plate Increase=1.	60		
Uniform Load	ls (plf) : 1-2=47, 2-26=25, 5-26=14, 5-6=	25 6-8-14 8-0-0 20-2312			
Horz	:: 1-2=-59, 2-26=-37, 5-26=-26, 5-	6=37, 6-8=26, 8-9=21			
5) Dead + 0.6 C Uniform Load	. ,	umber Increase=1.60, Plate Increase=1.	60		
Vert:	: 1-2=9, 2-4=14, 4-5=25, 5-27=14				
	:: 1-2=-21, 2-4=-26, 4-5=-37, 5-27	′=26, 8-27=37, 8-9=59 _umber Increase=1.60, Plate Increase=1.	60		
Uniform Load	ds (plf)		00		
	: 1-2=-12, 2-5=-33, 5-8=-33, 8-9= :: 1-2=-8, 2-5=13, 5-8=-13, 8-9=-8				
		, _umber Increase=1.60, Plate Increase=1.	60		
Uniform Load	ds (plf) : 1-2=-28, 2-5=-33, 5-8=-33, 8-9≕	-12 20-2320			
	:: 1-2=8, 2-5=13, 5-8=-13, 8-9=8	12, 20-2320			
8) Dead + 0.6 M Uniform Load		Lumber Increase=1.60, Plate Increase=1	.60		
	: 1-2=9, 2-5=-2, 5-8=9, 8-9=4, 20-	23=-12			
	:: 1-2=-21, 2-5=-10, 5-8=21, 8-9= M//ERS //ind (Pos. Internal) Pigh	16 t: Lumber Increase=1.60, Plate Increase=	-1 60		
Uniform Load	, , ,	L Lumber increase 1.00, Trate increase	-1.00		
	: 1-2=4, 2-5=9, 5-8=-2, 8-9=9, 20- :: 1-2=-16, 2-5=-21, 5-8=10, 8-9=				
		t: Lumber Increase=1.60, Plate Increase=	=1.60		
Uniform Loa		4 20 22 20			
	rt: 1-2=-15, 2-5=-20, 5-8=-9, 8-9= rz: 1-2=-5, 2-5=-0, 5-8=11, 8-9=1				
,		ht: Lumber Increase=1.60, Plate Increase	e=1.60		
Uniform Loa Ver	rt: 1-2=-4, 2-5=-9, 5-8=-20, 8-9=-1	5, 20-23=-20			
	rz: 1-2=-16, 2-5=-11, 5-8=0, 8-9=	5 Parallel: Lumber Increase=1.60, Plate In	araaaa 1.60		
Uniform Loa	. ,	Parallel: Lumber Increase=1.60, Plate In	crease=1.60		
	rt: 1-2=17, 2-3=22, 3-5=11, 5-8=3				
	rz: 1-2=-29, 2-3=-34, 3-5=-23, 5-8 MWFRS Wind (Pos. Internal) 2nd	l Parallel: Lumber Increase=1.60, Plate Ir	crease=1.60		
Uniform Loa		0 0 47 00 00 40			
	rt: 1-2=-2, 2-5=3, 5-7=11, 7-8=22, rz: 1-2=-10, 2-5=-15, 5-7=23, 7-8				
,	. ,	Parallel: Lumber Increase=1.60, Plate In	crease=1.60		
Uniform Loa Ver	ads (pir) rt: 1-2=7, 2-5=11, 5-8=3, 8-9=-2, 2	20-23=-12			
Hor	rz: 1-2=-19, 2-5=-23, 5-8=15, 8-9	=10			
Uniform Loa		Parallel: Lumber Increase=1.60, Plate In	crease=1.60		
Ver	rt: 1-2=-2, 2-5=3, 5-8=11, 8-9=7, 2				
	rz: 1-2=-10, 2-5=-15, 5-8=23, 8-9 MWFRS Wind (Neg. Internal) 1st	=19 Parallel: Lumber Increase=1.60, Plate In	crease=1.60		
Uniform Loa	ads (plf)		-		
	rt: 1-2=9, 2-3=4, 3-5=-6, 5-8=-15, rz: 1-2=-29, 2-3=-24, 3-5=-14, 5-8				
17) Dead + 0.6	MWFRS Wind (Neg. Internal) 2nd	d Parallel: Lumber Increase=1.60, Plate I	ncrease=1.60		
Uniform Loa Ver	ads (plf) rt: 1-2=-10, 2-5=-15, 5-7=-6, 7-8=	4. 8-9=9. 20-23=-20			
Hor	rz: 1-2=-10, 2-5=-5, 5-7=14, 7-8=	24, 8-9=29			
18) Dead + Unir Uniform Loa	5	Increase=1.25, Plate Increase=1.25			
Ver	rt: 1-5=-20, 5-9=-20, 20-28=-20, 2	8-29=-60, 29-30=-20, 30-31=-60, 23-31=			
19) Dead + 0.75 Increase=1.		Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber	Increase	=1.60, Plate
Uniform Loa					

Uniform Loads (plf)

Vert: 1-2=-46, 2-5=-50, 5-8=-42, 8-9=-38, 20-28=-20, 28-29=-50, 29-30=-20, 30-31=-50, 23-31=-20, 16-17=-30(F) Horz: 1-2=-4, 2-5=-0, 5-8=8, 8-9=12

20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60

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Job	Truss	Truss Type	Qty	Ply	DRHORTON/WILMINGTON; LOT 38 MCKAY PLACE	
MP38	A03	СОММОН	00	1		158690913
IVIP 30	A03	COMMON	99	'	Job Reference (optional)	

Builders FirstSource, Apex, NC

8.630 s Mar 9 2023 MiTek Industries, Inc. Thu Jun 1 14:31:04 2023 Page 3 ID:YnPtfgg65JxzX?AS1huWL5zfXqm-yUeahYNvUzaiC0ENjMolzBYJp6v90TF4YjMVwCzAdI5

LOAD CASE(S)

Uniform Loads (plf)

Vert: 1-2--38, 2-5=-42, 5-8=-50, 8-9=-46, 20-28=-20, 28-29=-50, 29-30=-20, 30-31=-50, 23-31=-20, 16-17=-30(F)

Horz: 1-2=-12, 2-5=-8, 5-8=0, 8-9=4

21) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-28, 2-3=-32, 3-5=-40, 5-8=-46, 8-9=-43, 20-28=-20, 28-29=-50, 29-30=-20, 30-31=-50, 23-31=-20, 16-17=-30(F) Horz: 1-2=-22, 2-3=-18, 3-5=-10, 5-8=4, 8-9=7

22) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-43, 2-5=-46, 5-7=-40, 7-8=-32, 8-9=-28, 20-28=-20, 28-29=-50, 29-30=-20, 30-31=-50, 23-31=-20, 16-17=-30(F)

Horz: 1-2=-7, 2-5=-4, 5-7=10, 7-8=18, 8-9=22

23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-60, 5-9=-20, 20-23=-20

24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-20, 5-9=-60, 20-23=-20

25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

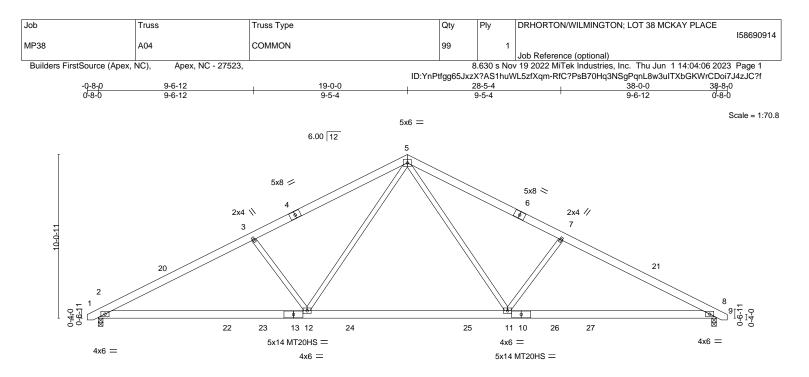
Vert: 1-5=-50, 5-9=-20, 20-28=-20, 28-29=-50, 29-30=-20, 30-31=-50, 23-31=-20, 16-17=-30(F)

26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-5=-20, 5-9=-50, 20-28=-20, 28-29=-50, 29-30=-20, 30-31=-50, 23-31=-20, 16-17=-30(F)

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	12-10-1 12-10-1				25-1-15 12-3-15				38-0-0 12-10-1	
LOADING (psf) TCLL 20.0	SPACING- Plate Grip DOL	2-0-0 1.15	CSI. TC	0.54	DEFL. Vert(LL)	in (loc) -0.27 11-12	l/defl >999	L/d 360	PLATES MT20	GRIP 244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.88	Vert(CT)	-0.42 11-12	>999	240	MT20HS	187/143
BCLL 0.0 BCDL 10.0	* Rep Stress Incr Code IRC2015/TF	YES PI2014	WB Matrix	0.42 -MS	Horz(CT) Wind(LL)	0.08 8 0.10 12-16	n/a >999	n/a 240	Weight: 241 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

```
LUMBER-
```

TOP CHORD2x6 SP No.2BOT CHORD2x6 SP No.2WEBS2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-134(LC 13) Max Uplift 2=-26(LC 12), 8=-26(LC 13) Max Grav 2=1549(LC 1), 8=1549(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2714/247, 3-5=-2465/269, 5-7=-2465/269, 7-8=-2714/247

BOT CHORD 2-12=-132/2381, 11-12=0/1562, 8-11=-127/2381

WEBS 5-11=-38/1008, 7-11=-598/223, 5-12=-38/1008, 3-12=-598/223

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-5-14 to 4-3-12, Interior(1) 4-3-12 to 19-0-0, Exterior(2) 19-0-0 to 25-9-7, Interior(1) 25-9-7 to 38-5-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are MT20 plates unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

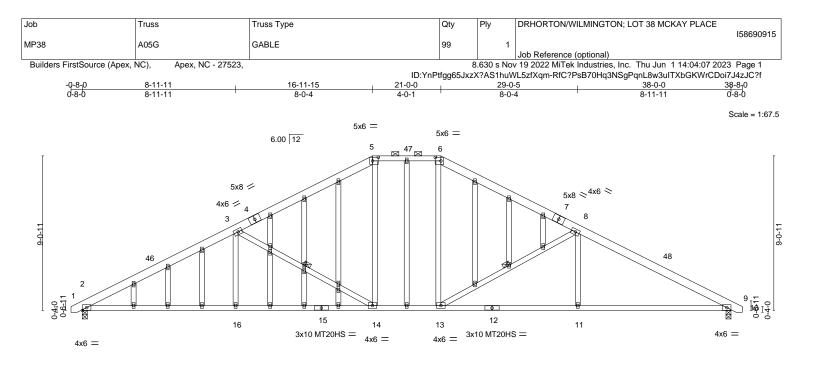


Structural wood sheathing directly applied or 3-11-2 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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A REALE A Mi Tek Affilia 818 Soundside Road Edenton, NC 27932

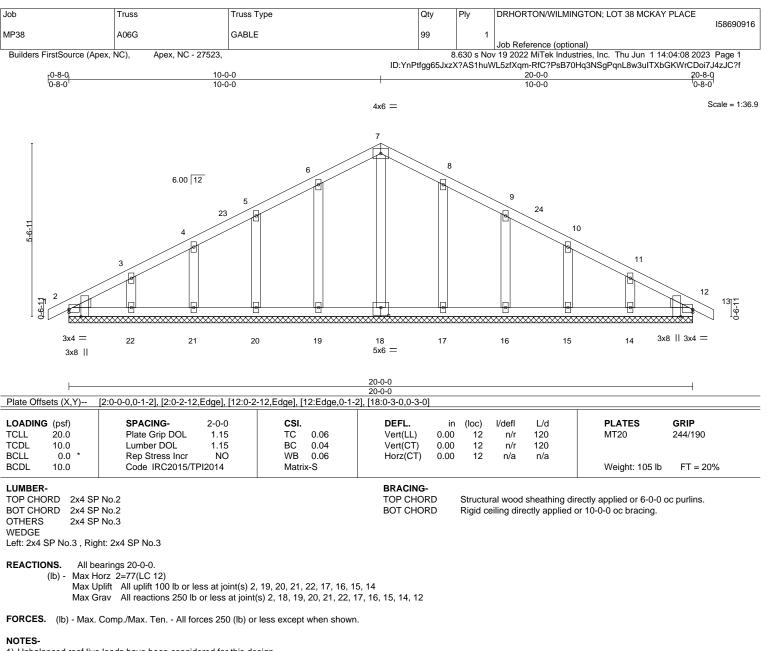


⊢	8-6-11	16-11-15	21-0-0		29-5-5		38-0-0	
Plate Offsets (X,Y)	<u>8-6-11</u> [5:0-4-0,0-2-8], [6:0-4-0,0-2-8], [9:0-0-1	,0-0-0]	4-0-1		8-5-4		8-6-11	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCode IRC2015/TPI2014	CSI. TC 0.51 BC 0.98 WB 0.42 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	-0.34 -0.52 0.12	(loc) l/defl 11-13 >999 11-13 >880 9 n/a 14-16 >999	360 240 n/a	PLATES MT20 MT20HS Weight: 289 lb	GRIP 244/190 187/143 FT = 20%
5-6: 2x4 BOT CHORD 2x4 SP	2x4 SP No.2 No.3	1	BRACING- TOP CHOR BOT CHOR WEBS		2-0-0 oc purli	ns (4-5-13 max lirectly applied	irectly applied or 4-0-9 c): 5-6. or 10-0-0 oc bracing. 3-14, 8-13	c purlins, except
Max Ho Max Ur Max G FORCES. (Ib) - Max. (TOP CHORD 2-3=-2	 2=0-3-8, 9=0-3-8 2=-128(LC 13) plift 2=-106(LC 12), 9=-106(LC 13) rav 2=1549(LC 1), 9=1549(LC 1) Comp./Max. Ten All forces 250 (lb) o 2736(256, 3-5=-1967/272, 5-6=-1647/2) 	72, 6-8=-1967/272, 8-9=-2	2736/256					
 WEBS 3-16= NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; Vi gable end zone and to 38-5-14 zone; can reactions shown; Lur 3) Truss designed for wi Gable End Details as 4) Provide adequate dra 5) All plates are MT20 plate 6) All plates are 2x4 MT 7) Gable studs spaced 8) This truss has been of 9) * This truss has been of 9) * This truss has been of 9) * This truss has been of 10) Provide mechanicaa 2=106, 9=106. 11) See Standard Indust designer. 	designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on ottom chord and any other members, w I connection (by others) of truss to bear stry Piggyback Truss Connection Detail	-11=0/355, 6-13=-12/527 asign. sf; BCDL=6.0psf; h=32ft; or(1) 4-3-12 to 16-11-15, cal left and right exposed For studs exposed to wi g designer as per ANSI/T re load nonconcurrent with the bottom chord in all are ith BCDL = 10.0psf. ing plate capable of withs for Connection to base tr	, 8-13=-891/235 Cat. II; Exp B; Encl Exterior(2) 16-11-1 ;C-C for members nd (normal to the fa Pl 1. h any other live loa eas where a rectan standing 100 lb upli uss as applicable,	5 to 27 and for ace), se ds. gle 3-6 ft at join or cons	7-9-8, Interior(1 rces & MWFRS ee Standard In 6-0 tall by 2-0-0 nt(s) except (jt sult qualified br) 27-9-8 5 for dustry	SE/ 0363	• -
12) Graphical purlin rep	presentation does not depict the size or	the orientation of the purl	in along the top an	d/or bo	ottom chord.		A. (FILBE

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



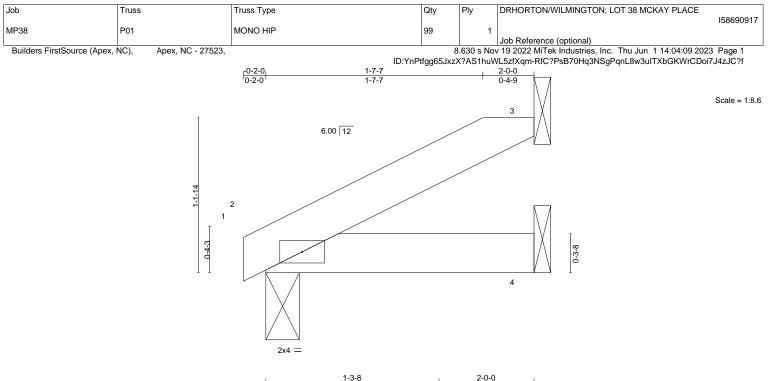
June 1,2023



- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BcDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-0 to 4-0-0, Exterior(2) 4-0-0 to 10-0-0, Corner(3) 10-0-0 to 14-9-10, Exterior(2) 14-9-10 to 20-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 19, 20, 21, 22, 17, 16, 15, 14.



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		1-3-6		0-8-8		
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL) -0.	.00 7	>999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.05	Vert(CT) -0.	.00 7	>999 240	
BCLL 0.0 *	Rep Stress Incr NO	WB 0.00	Horz(CT) 0.	.00 2	n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MP	Wind(LL) 0.	.00 7	>999 240	Weight: 7 lb FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=Mechanical, 3=Mechanical

Max Horz 2=33(LC 8)

Max Uplift 3=-21(LC 8)

Max Grav 2=83(LC 1), 4=36(LC 3), 3=50(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope)

- gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.

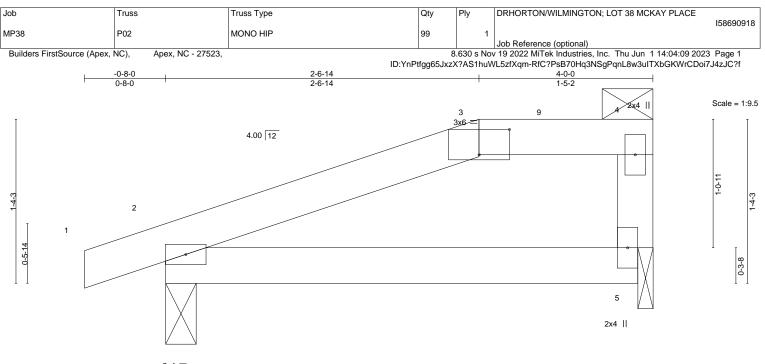
6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

SEAL 036322 June 1,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



Edenton, NC 27932



2x4 =

2-6-14	4-0-0	
2-6-14	1-5-2	

Structural wood sheathing directly applied or 4-0-0 oc purlins,

except end verticals, and 2-0-0 oc purlins: 3-4.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Plate Offsets (X,Y)	[3:0-3-0,0-2-8]				1						
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.26	Vert(LL)	-0.00	5-8	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.14	Vert(CT)	-0.01	5-8	>999	240		
BCLL 0.0 *	Rep Stress Incr	NO	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI	12014	Matrix	-MR	Wind(LL)	0.00	5-8	>999	240	Weight: 14 lb	FT = 20%
LUMBER-					BRACING-						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

REACTIONS. (size) 2=0-3-0, 5=0-1-8

Max Horz 2=40(LC 5) Max Uplift 2=-41(LC 4), 5=-20(LC 4)

Max Grav 2=206(LC 1), 5=159(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.

9) Girder carries hip end with 0-0-0 right side setback, 0-0-0 left side setback, and 2-6-0 end setback.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

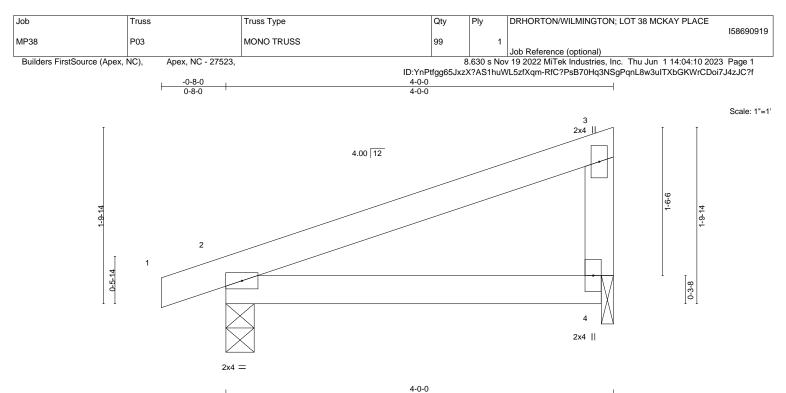
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-63(F=-3), 3-4=-63(F=-3), 5-6=-21(F=-1)



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						4-0-0						
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	-0.01	4-7	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.15	Vert(CT)	-0.02	4-7	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2	014	Matrix	k-MP	Wind(LL)	0.01	4-7	>999	240	Weight: 15 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS

2x4 SP No.3

REACTIONS. 2=0-3-8, 4=0-1-8 (size) Max Horz 2=55(LC 11) Max Uplift 2=-38(LC 8), 4=-22(LC 12) Max Grav 2=198(LC 1), 4=151(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



Structural wood sheathing directly applied or 4-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

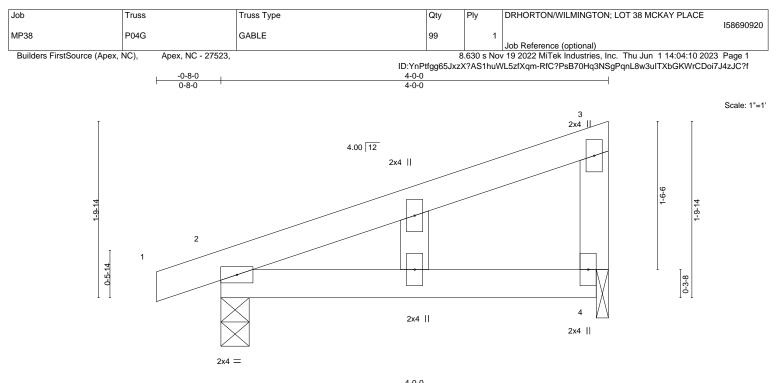
except end verticals.

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818 Soundside Road

Edenton, NC 27932



			4-0-0				
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. ir	()	l/defl L/d	PLATES	GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15	TC 0.21 BC 0.15	Vert(LL) -0.01 Vert(CT) -0.02	4-9	>999 360 >999 240	MT20	244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.00 Matrix-MP	Horz(CT) 0.00 Wind(LL) 0.01		n/a n/a >999 240	Weight: 16 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

OTHERS 2x4 SP No.3 REACTIONS. (size) 2=0-3-8, 4=0-1-8

Max Horz 2=55(LC 11) Max Uplift 2=-38(LC 8), 4=-22(LC 12) Max Grav 2=198(LC 1), 4=151(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



Structural wood sheathing directly applied or 4-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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