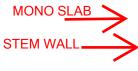
PLANS DESIGNED TO THE **2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE**

MEAN ROOF HEIGHT: 19'-9)"	HEIGHT TO RIDGE: 27'-5"			
CLIMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A		
FENESTRATION U-FACTOR	0.35	0.35	0.35		
SKYLIGHT U-FACTOR	0.55	0.55	0.55		
GLAZED FENESTRATION SHGC	0.30	0.30	0.30		
CEILING R-VALUE	38 or 30ci	38 or 30ci	38 or 30ci		
WALL R-VALUE	15	15	19		
FLOOR R-VALUE	19	19	30		
* BASEMENT WALL R-VALUE	5/13	10/15	10/15		
** SLAB R-VALUE	0	10	10		
* CRAWL SPACE WALL R-VALUE	5/13	10/15	10/19		

 ***INULIZING DEPTH WITH STEM WALL SLAB 24" OR FOO MORECTION OF POUNDATION WITH STEM WITH STEM WALL SLAB 24" OR FOO MINSPECTION GAP TO BOTTOM OF POUNDATION WALL DESIGNED FOR WIND SPEED OF 120 MPH, 3 SECOND GUST (93 FASTEST MILE) EXPOSURE "B"

COMPONENT	* CLADDING DESIGNED FOR THE FOLLOWING LOADS							
MEAN ROOF	UP T	O 30'		TO 35'	35'-1"	TO 40'		
ZONE 1	14.2	-15.0		-15.8				
ZONE 2	14.2	-18.0	14.9	-18.9	15.5	19.6	15.9	-20.2
ZONE 3	14.2	-18.0	14.9	-18.9	15.5	19.6	15.9	-20.2
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	-21.8	17.4	-22.4
DESIGNED FOR WIN	ID SPEED	OF 130 MF	H, 3 SECO	OND GUST	(101 FAS	TEST MILI	EXPOSU	RE "B"
COMPONENT	& CLA	DDING	DESIG	NED FO	R THE	FOLLO	WING I	OADS
MEAN ROOF	UP T	O 30'		TO 35'		TO 40'		
ZONE 1	16.7	-18.0		-18.9		-19.6		-20.2
ZONE 2		-21.0		-22.1				-23.5
ZONE 3		-21.0		-22.1		-22.9		-23.5
ZONE 4	18.2	-19.0	19.1	-20.0				
ZONE 5	18.2	-24.0	19.1	25.2	19.8	26.2	20.4	26.9





NOTE: MONO SLAB - STONE TO RUN TO THE BOTTOM OF WINDOW STEM WALL - STONE TO FOUNDATION HEIGHT ONLY



RATE AS NEEDEL

FRONT ELEVATION - A

SCALE 1/4" = 1'-0"



REAR ELEVATION SCALE 1/8" = 1'-0"

PARGE

SOUARE FOOTAGE HEĂTED FIRST FLOOR

1791 SQ FT. 1791 SQ FT. HEATED OPTIONAL

SUB FLOOR

WEST PRESERVE

LOT - 39

152 OLEANDER LN

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ELEVATION

9'-1 1/2"-

CAROLINA ROOM 148 SQ FT 148 SQ FT

UNHEATED FRONT PORCH 188 SO FT

469 SQ FT. 657 SO FT.

UNHEATED OPTIONAL

160 SQ.FT. 108 SQ.FT. AIR LEAKAGE Section N111124 GARAGE 292 SQ. FT.
N1102.4.1 Building thermal envelope. The building thermal

envelope shall be durably sealed with an air barrier system to limit infiltration. The sealing methods between dissimilar materials shall allow for differential expansion and contraction. For all homes, where present, the following shall be caulked, gasketed, weather stripped or otherwise sealed with an air barrier material or solid material consistent with Appendix E-2.4 of this code:

1. Blocking and sealing floor/celling systems and under knee wall: open to unconditioned or exterior space.

Capping and sealing shafts or chases, including flue shafts.
 Capping and sealing soffit or dropped ceiling areas.



ROOF VENTILATION

SECTION R806

R806.1 Ventilation required. Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain or snow. Ventilation openings shall have a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Ventilation openings having a least dimension larger than 1/4 inch (6.4 mm) shall be provided with corrosion-resistant wire cloth screening, hardware cloth, or similar material with openings having a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Openings in roof framing members shall conform to the requirements of Section R802.7

R806.2 Minimum area. The total net free ventilating area shall not be less than 1/150 of the area of the space ventilated except that reduction of the total area to 1/300 is permitted provided that at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above the eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents. As an alternative, the net free cross-ventilation area may be reduced to 1/300 when a Class I or II vapor retarder is installed on the warm-in-winter side of the ceiling.

Exceptions:

 Enclosed attic/rafter spaces requiring less than 1 square foot (0.0929 m2) of ventilation may be vented with continuous soffit ventilation only.

2. Enclosed attic/rafter spaces over unconditioned space may be vented with continuous soffit vent only.

SQUARE FOOTAGE OF ROOF TO BE VENTED = 2,477 SQ.FT.

NET FREE CROSS VENTILATION NEEDED:

WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 16.51 SQ.FT. WITH 50% TO 80% OF VENTING 3'-0" ABOVE FAVE: OR WITH CLASS LOR II

GUARD RAIL NOTES

SECTION R312

R312.1 Where required. Guards shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or *grade* below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect expension, but he the opening and are applied to the company of the compa

screening shall not be considered as a *guard*. **R312.2 Height.** Required *guards* at open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914 mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads.

Exceptions: Guards on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the

treads.

2. Where the top of the *quard* also serves as a handrail on the open sides of stairs, the top of the *guard* shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading edges of the treads.

R312,3 Opening limitations. Required guards shall not have openings from the walking surface to the required guard height which allow passage of a sphere 4 inches (102 mm)in diameter.

 The triangular openings at the open side of a stair, formed by the riser, tread. and bottom rail of a guard, shall not allow passage of a sphere 6 inches (153

2. Guards on the open sides of stairs shall not have openings which allow passage of a sphere 4 3/8 inches (111 mm) in diameter

SPECIFIED VENEER AS SPECIFIED

LEFT SIDE ELEVATION PARGE

PARGE

INHEATED OPTIONAL © Copyright 2020

SQUARE FOOTAGE HEATED HEATED OPTIONAL 148 SQ.FI 148 SQ.FI

UNHEATED

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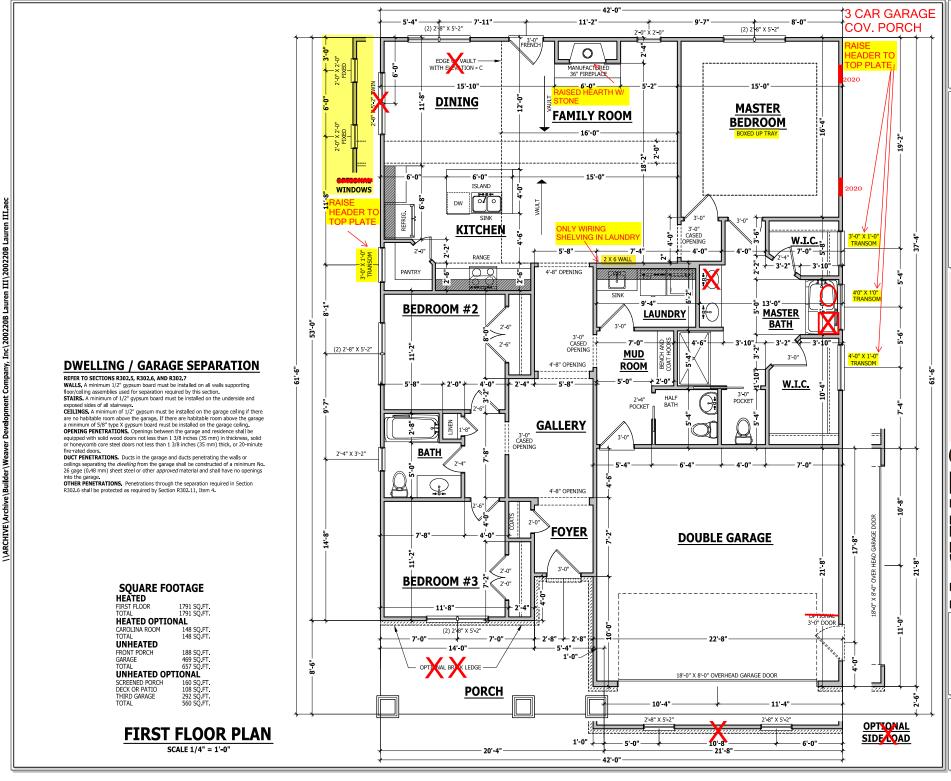
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SQUARE FOOTAGE HEATED FIRST FLOOR 1791 SQ.FT.
TOTAL 1791 SQ.FT.
HEATED OPTIONAL UNHEATED FRONT PORCH GARAGE UNHEATED OPTIONAL

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PROCESSIONS MAY
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FIRST FLOOR PLAN
The Lauren III

SQUARE FOOTAGE
HEATED
PIRST RLOR
1791 SQ.FF.
1077A, 1791 SQ.FF.
1077A, 1791 SQ.FF.
1077A, 1892 SQ.FF.
1077A,

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PAGE 3 OF 6

STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10	10	L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200	-	-
Guardrail in-fill components	50	-	-
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40	-	L/360

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1,9x106 PSI Paralel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Instal all connections per manufacturers instructions

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" upless noted otherwise 3. 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise.

FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4"

PF: Portal fame per figure R602.10.1

ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick.

CONCRETE AND SOILS: See foundation notes.

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brough to Haynes Home Plan, Inc. attention before construction begins.

KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE, All required anchors for trusses due to uplift or bearing

shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights

BRACE WALL PANEL NOTES

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless

GYPSUM: All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method

GB to be fastened per table R602.10.1.

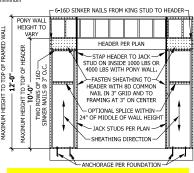
REQUIRED LENGTH OF BRACING: Required brace wall length for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length Method PE contributes 1.5 times its actual length

HD: 800 lbs hold down hold down device fastened to the edge of the brace wall panel dosets to the corner.

Methods Per Table R602.10.1

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or 8d(2 1/2" long x 0.113" diameter). CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing

GB: Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws



EXTERIOR HEADERS (2) 2 X 6 WITH 1 JACK STUD EACH END

PORTAL FRAME AT OPENING (METHOD PF PER FIGURE AND SECTION R602.10.1)

SCALE 1/4" = 1'-0"

UNLESS NOTED OTHERWISE - KING STUDS EACH END PER TABLE BELOW

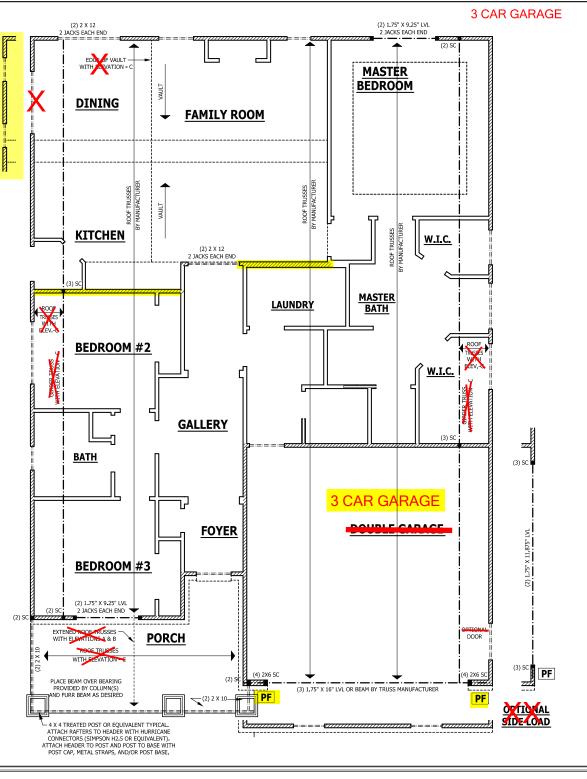
HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16 KING STUD(S) 1 2 3 5 6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE - NON LOAD BEARING HEADERS TO BE LADDER FRAMED

FIRST FLOOR STRUCTURAL

SCALE 1/4" = 1'-0'



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The Lauren

STRUCTURAL

FLOOR

FIRST

SQUARE FOOTAGE HEATED

INHEATED OPTIONAL

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PAGE 4 OF 6

148 SQ.FI 148 SQ.FI

HEATED OPTIONAL

UNHEATED

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins, KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE, All required anchors for trusses due to uplift or bearing

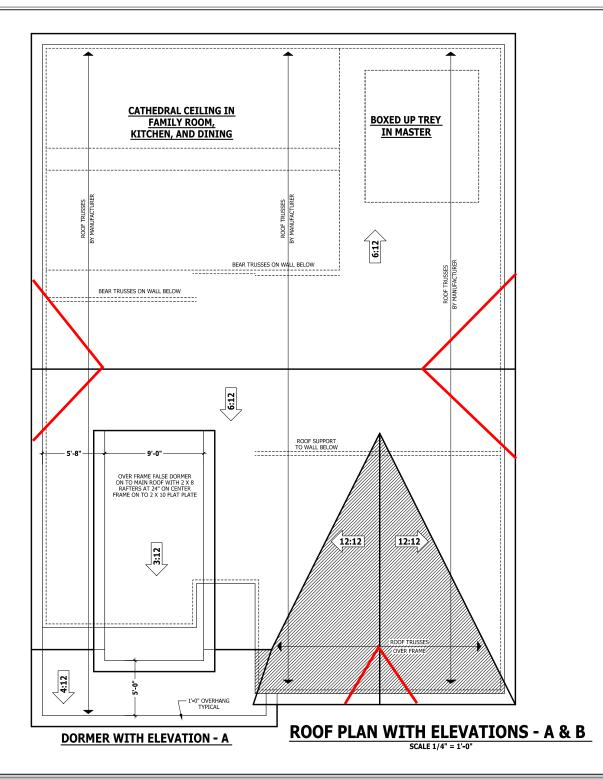
shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

HEEL HEIGHT ABOVE

HEEL HEIGHT ABOVE SECOND FLOOR PLATE

TUDOR HIP



DIMENSIONS AND CONDITIONS
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8 య ROOF PLAN

The Lauren III

SQUARE FOOTAGE HEATED OTAL UNHEATED INHEATED OPTIONAL

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200220B PAGE 5 OF 6 III.aec

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Development

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2 X 4 STUDS AT 16" O.C

SHEATHING

AS SPECIFIED

2 X 4 SILL PLATE

2 X RIM JOIST

- 8" SOLID MASONRY CAP

" CONCRETE BLOCK

4" BRICK

GRADE

1/2" GYPSUM

SHEATHING AS SPECIFIED

SIDING AS SPECIFIED

— 8" SOLID MASONRY CAP

4" BRICK VENEER

TAMPED OF

INDISTURBE

S FARTH

DECK STAIR NOTES

AM110.1 Stairs shall be constructed per Figure AM110. Stringer spans shall be no greater than 7 foot span between supports. Spacing between stringers shall be based upon decking material used per AM107.1. Each Stringer shall have

minimum 3 1/2 inches between step cut and back of stringer If used, suspended headers shall shall be attached with 3/8 inch galvanized bolts with nuts and washers to securely support stringers at the top.

DECK BRACING

SECTION AM109

AM109.1 Deck bracing. Decks shall be braced to provide lateral stability. The following are acceptable means to rovide lateral stability.

AM109.1.1. When the deck floor height is less than 4'-0' above finished grade per Figure AM109 and the deck is attached to the structure in accordance with Section AM104, lateral bracing is not required.

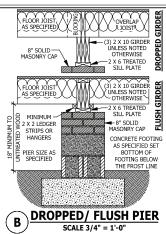
AM109.1.2. 4 x 4 wood knee braces may be provided on each column in both directions. The knee braces shall attach to each post at a point not less than 1/3 of the post length from the top of the post, and the braces shall be angled between 45 degrees and 60 degrees from the horizontal. Knee braces shall be bolted to the post and the girder/double band with one 5/8 inch hot dipped galvanized bolt with nut and washer at both ends of the brace per Figure AM109.1

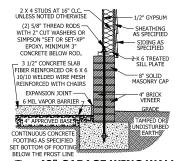
AM109.1.3. For freestanding decks without knee braces or diagonal bracing, lateral stability may be provided by

and the following:										
POST SIZE										
4 X 4	48 SF	4'-0"	2'-6"	1'-0"						
6 X 6	120 SF	6'-0"	3'-6"	1'-8"						

AM109.1.4. 2 x 6 diagonal vertical cross bracing may be provided in two perpendicular directions for freestanding decks or parallel to the structure at the exterior column line for attached decks. The 2 x 6's shall be attached to the posts with one 5/8 inch hot dipped galvanized bolt with nut and washer at each end of each bracing member per Figure AM109 3

AM109.1.5. For embedment of piles in Coastal Regions, see Chapter 45.





<48" GARAGE WING WALL Ε SCALE 3/4" = 1'-0'

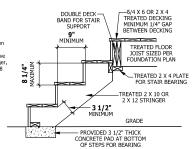
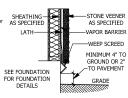


FIGURE AM110 TYPICAL DECK STAIR DETAIL

SCALE 3/4" = 1'-0"

WEEP SCREEDS



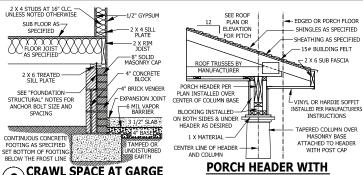
WEEP SCREED SCALE 3/4" = 1'-0"

All weep screeds and stone veneer to be installed ner manufactures instructions and per the 2012 North Carolina Residential

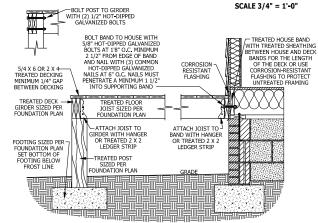
C

Building code. R703.6.2.1 - A minimum 0.019-inch (0.5 mm) (No. 26 galvanized sheet gage), corrosion-resistant weep screed or plastic veep screed, with a minimum vertical attachment flange of 31/2 inches (89 mm) shall be provided at or below the foundation plate line on exterior stud walls in accordance with ASTM C 926. The weep

screed shall be placed a minimum of 4 inches (102 mm) above the earth or 2 inches (51 mm) above paved areas and shall be of a type that will allow trapped water to drain to the exterior of the building. The weather-resistant barrier shal Ian the attachment flance. The exterior lath shall cover and terminate on the attachment flange of the weep screed.



CRAWL SPACE AT GARGE **TAPERED COLUMN** SCALE 3/4" = 1'-0"



DECK ATTACHMENT DETAIL TO FRAMED WALL

SCALE 3/4" TO 1'-0"

SMOKE ALARMS

SECTION R314 R314.1 Smoke detection and notification. All smoke alarms shall be listed in accordance with UL 217 and installed in accordance with the provisions of this code and the household fire warning equipment provisions of NFPA 72.

R314.2 Smoke detection systems. Household fire alarm systems installed in accordance with NFPA 72 that include smoke alarms, or a combination of smoke detector and audible notification device installed as required by this section for smoke alarms, shall be permitted. The household fire alarm system shall provide the same level of smoke detection and alarm as required by this section for smoke alarms. Where a household fire warning system is installed using a combination of smoke detector and audible notification device(s), it shall become a permanent fixture of the occupancy and owned by the homeowner. The system shall be monitored by an approved supervising station and be maintained in accordance with NEDA 72

Exception: Where smoke alarms are provided meeting the requirements of Section R314.4.

R314.3 Location. Smoke alarms shall be installed in the following

 In each sleeping room. 2. Outside each separate sleeping area in the immediate vicinity of

the bedrooms.

3. On each additional *story* of the *dwelling*, including *basements* and habitable attics (finished) but not including crawl spaces. uninhabitable (unfinished) attics and uninhabitable (unfinished) attic-stories. In dwellings or dwelling units with split levels and without an intervening door between the adjacent levels, a smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full *story*

below the upper level. When more than one smoke alarm is required to be installed within an individual dwelling unit the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of

the alarms in the individual unit. **R314.4 Power source.** Smoke alarms shall receive their primary power from the building wiring when such wiring is served from a commercial source, and when primary power is interrupted, shall receive power from a battery. Wiring shall be permanent and without a disconnecting switch other than those required for overcurrent protection. Smoke alarms shall be interconnected.

STAIRWAY NOTES

shall not be less than 6 feet 8 inches (2032 mm) measured vertically from the sloped line adjoining the tread nosing or from the floor surface of the landing or platform on that portion of the stainway.

R311.7.4 Stair treads and risers. Stair treads and risers shall meet the requirements of this section. For the purposes of this section all dimensions and dimensioned surfaces shall be exclusive of carpets, rugs or runners. R311.7.4.1 Riser height. The maximum riser height shall be 8 1/4 inches

(210 mm). The riser shall be measured vertically between leading edges of the adiacent treads. P311 7.4.2 Tread denth. The minimum tread denth shall be 9 inches (229 mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread depth of 9 inches (229 mm) measured as above at a point 12 inches (305 mm) from the side where the treads are narrower. Winder treads shall have a

minimum tread depth of 4 inches (102 mm) at any point.

R311.7.4.3 Profile. The radius of curvature at the nosing shall be no greater than 9/16 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1 1/4 inches (32 mm) shall be provided on stairways with solid

R311.7.7 Handrails. Handrails shall be provided on at least one side of each continuous run of treads or flight with four or more risers.

R311,7,7,1 Height, Handrail height, measured vertically from the sloped plane adjoining the tread nosing, or finish surface of ramp slope, shall be not less than 34 inches (864 mm)and not more than 38 inches (965 mm). Exceptions:

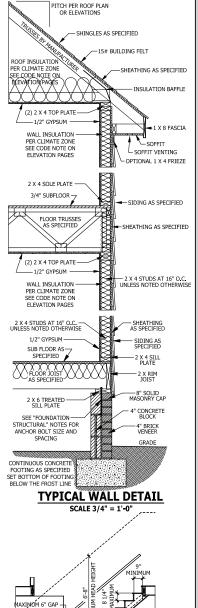
1. The use of a volute, turnout or starting easing shall be allowed over the lowest tread.

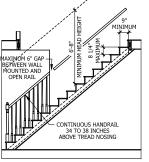
 When handrail fittings or bendings are used to provide continuous transition between flights, the transition from handrall to guardrall, or used at the start of a flight, the handrall height at the fittings or bendings shall be permitted to exceed the maximum height.

R311.7.7.2 Continuity. Handrails for stairways shall be continuous for the full length of the flight, from a point directly above the top riser of the flight to a point directly above the lowest riser of the flight. Handrail ends shall be returned or shall terminate in newel posts or safety terminals. Handrails adjacent to a wall shall have a space of not less than 11/2 inch (38 mm) between the wall and the handrails.

1. Handrails shall be permitted to be interrupted by a newel post.

The use of a volute, turnout, starting easing or starting newel shall be allowed over the lowest tread. 3. Two or more separate rails shall be considered continuous if the termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrail/handrail, the wall-mounted rail must return into the wall.





TYPICAL STAIR DETAIL

HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR ONTRACTORS PRACTICES AN PROCEDURES. CODES AND CONDITIONS MAY ARY WITH LOCATION, A LOCA DESIGNER, ARCHITECT OR SINEER SHOULD BE CONSUL' BEFORE CONSTRUCTION.

THESE DRAWING ARE ISTRUMENTS OF SERVICE AND AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER.

S DETAIL Ε Lauren TYPICAL The

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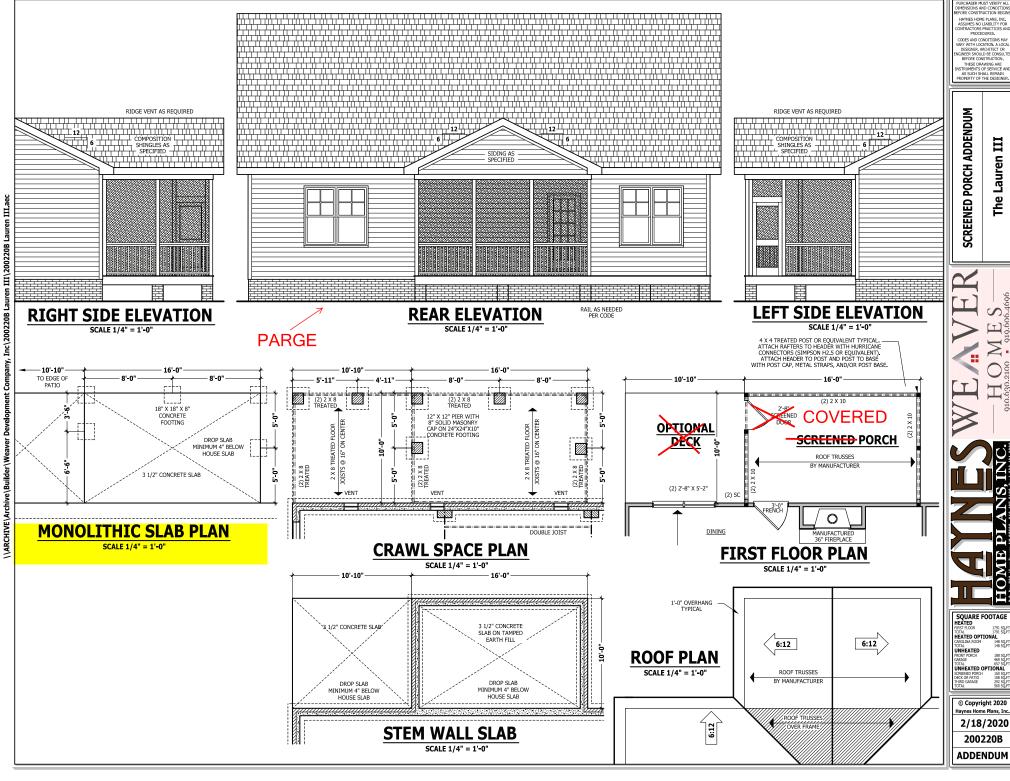
SQUARE FOOTAGE HEATED

HEATED OPTIONAL

Havnes Home Plans, Inc. 2/18/2020

200220B

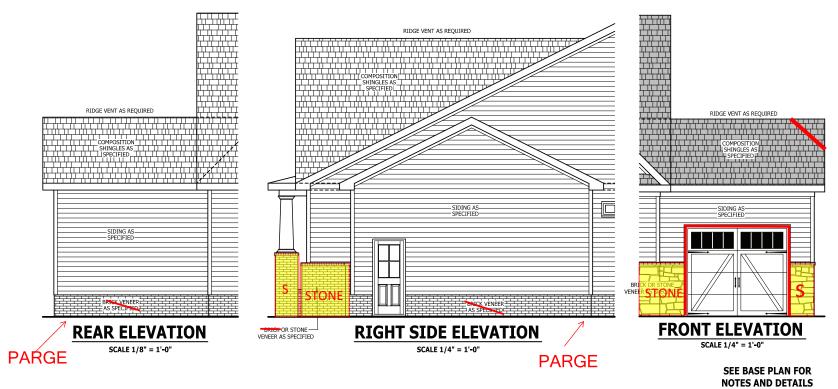
PAGE 6 OF 6

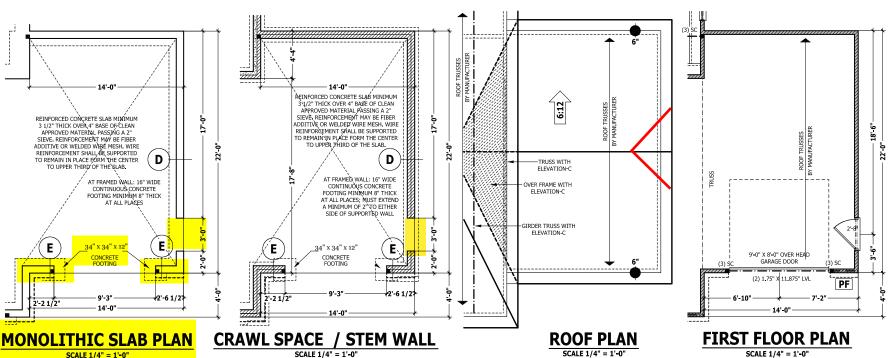


HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AN

SQUARE FOOTAGE HEATED FIRST FLOOR 1791 SQ FT TOTAL 1791 SQ FT HEATED OPTIONAL 148 SQ FI 148 SQ FI INHEATED OPTIONAL

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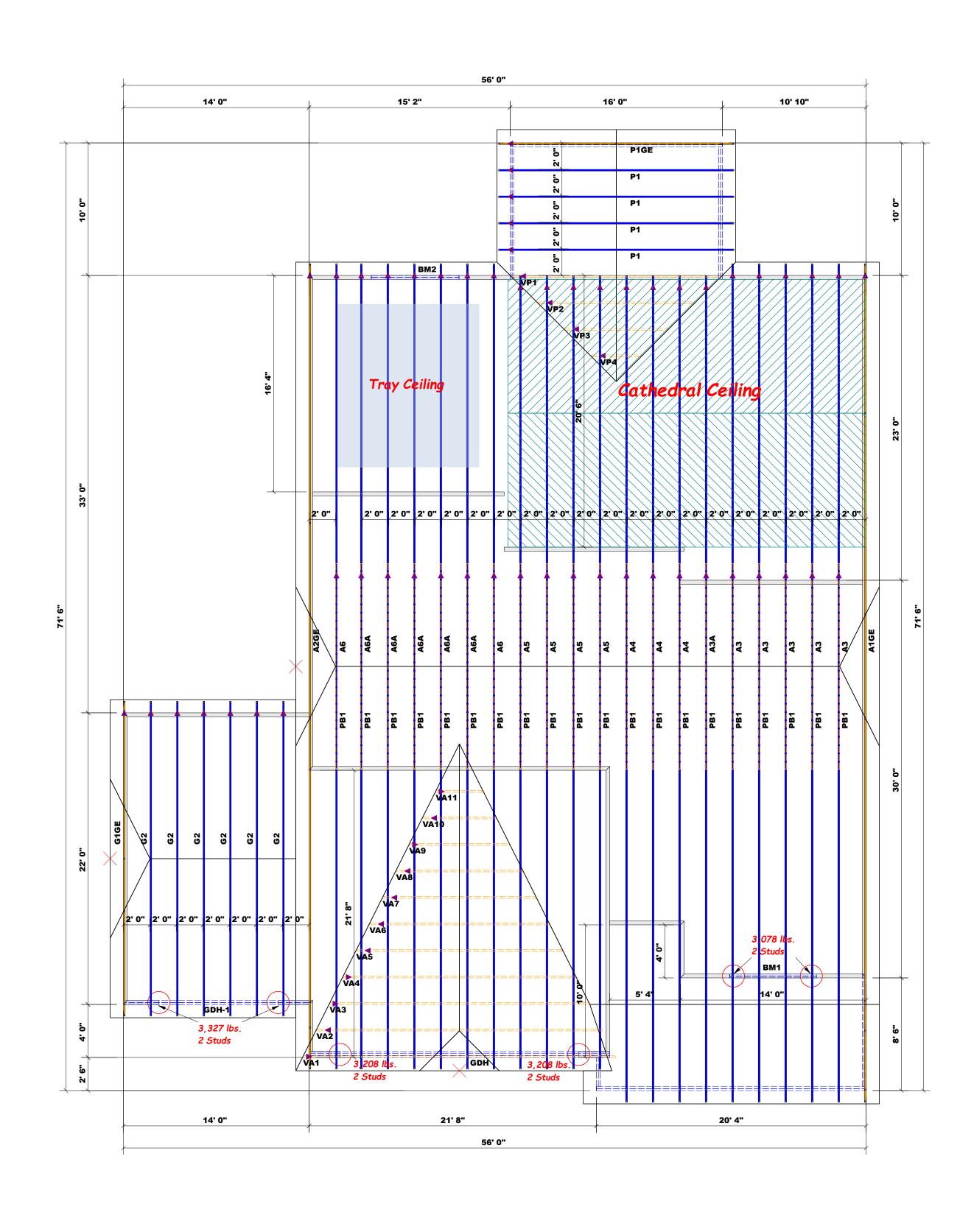
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SOUARE FOOTAGE
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TOTAL
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ADDENDUM



▲ = Denotes Left End of Truss(Reference Engineered Truss Drawing)Do Not Erect Trusses Backwards

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Truss Placement Plan SCALE: 3/16" = 1'

		Beam Legend			
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
BM2	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
GDH	23' 0"	1-3/4"x 16" LVL Kerto-S	3	3	FF

	(B.	ASED O	N TABLES	5 R502	.5(1) & (**			Bl
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER									
NOI (c	DS FOR EADER		NOI (c	DS FOR EADER		NOIT:	IDS FOR EADER		J
END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (4) PLY HEADER		PL
1700	1		2550	1		3400	1		
3400	2		5100	2		6800	2		SI
5100	3		7650	3		10200	3		•
6800	4		10200	4		13600	4		
8500	5		12750	5		17000	5		0
10200	6		15300	6					×
11900	7								
13600	8								.T/
15300	9								"

BUILDER	Weaver Homes, Inc.	CITY / CO.	Sanford / Harnett	THIS IS A TRUSS PLAC These trusses are designed a the building design at the spe sheets for each truss design i	
JOB NAME	Lot 39 West Preserve	ADDRESS	152 Oleander Ln.	is responsible for temporary ar the overall structure. The desig walls, and columns is the resp regarding bracing, consult BCS	
PLAN	Lauren III / Elev. A / 3 Car / CP	/ CP MODEL Roof		or online @ sbcindustry.com Bearing reactions less than prescriptive Code requirements	
SEAL DATE	4/29/20	DATE REV.	01/17/24	(derived from the prescripti foundation size and number than 3000# but not greater t be retained to design the su	
QUOTE#	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tal retained to design the support	
JOB#	J0124-0299	SALES REP.	Lenny Norris	Signature	

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into he building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for he overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables derived from the prescriptive Code requirements.) to determine the minimum

paring reactions less than or equal to 3000# are deemed to comply with the escriptive Code requirements. The contractor shall refer to the attached Tables derived from the prescriptive Code requirements) to determine the minimum undation size and number of wood studs required to support reactions greater an 3000# but not greater than 15000#. A registered design professional shall retained to design the support system for any reaction that exceeds those excified in the attached Tables. A registered design professional shall be tained to design the support system for all reactions that exceed 15000#.

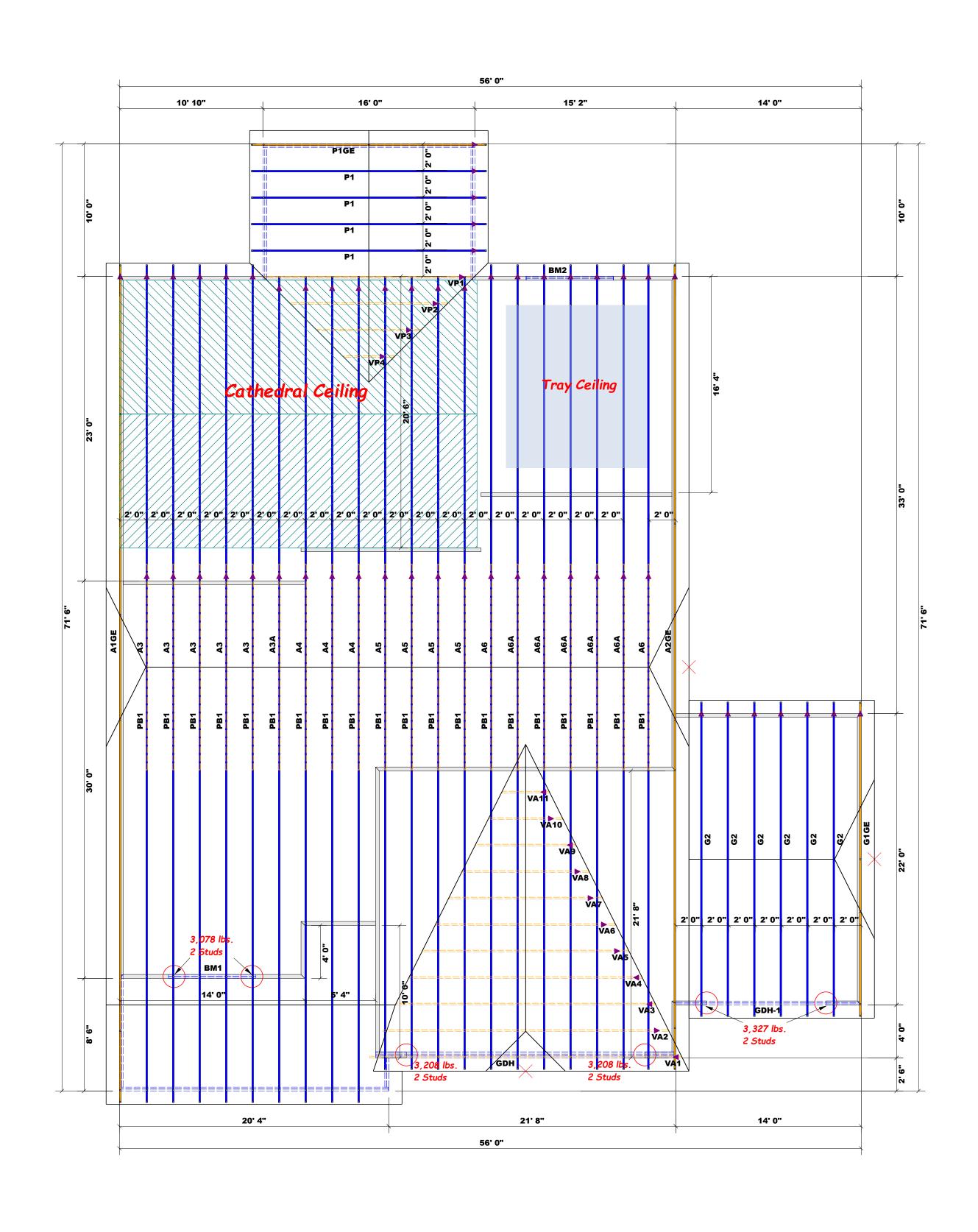
ched Tables. A registered désign professional shall be le support system for all reactions that exceed 15000#.

Curtis Quick

Curtis Quick



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444



▲ = Denotes Left End of Truss(Reference Engineered Truss Drawing)Do Not Erect Trusses Backwards

3400 1

6800 2

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

5100 2

7650 3

10200 4 12750 5

15300 6

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Truss Placement Plan SCALE: 3/16" = 1'

		Beam Legend			
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
BM2	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
GDH	23' 0"	1-3/4"x 16" LVL Kerto-S	3	3	FF

соттесн

ROOF & FLOOR

TRUSSES & BEAMS

Reilly Road Industrial Park

Fayetteville, N.C. 28309

Phone: (910) 864-8787 Fax: (910) 864-4444

BUILDER	Weaver Homes, Inc.	CITY / CO.	Sanford / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer
JOB NAME	Lot 39 West Preserve	ADDRESS	152 Oleander Ln.	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package
PLAN	Lauren III / Elev. A / 3 Car / CP	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables
SEAL DATE	4/29/20	DATE REV.	01/17/24	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those
QUOTE#	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#. Signature
JOB#	J0124-0299	SALES REP.	Lenny Norris	Curtis Quick



Client: Project: Address: Weaver Development

1/18/2024

Input by: Curtis Quick Job Name: The Lauren III Beams Page 1 of 8

Project #:

1.750" X 16.000" Kerto-S LVL 3-Ply - PASSED GDH

Design Method:

Building Code:

Load Sharing:

Deck:

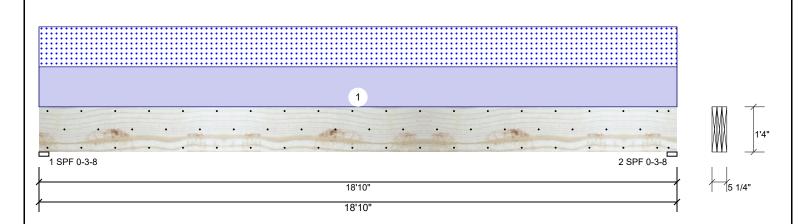
ASD

Yes

IBC 2012

Not Checked

Level: Level



Member Information

Type: Plies: 3 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360

Importance: Normal - II Temperature: Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift) Application:

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1692	1516	0	0
2	Vertical	0	1692	1516	0	0

Bearings

Bearing	Length	Dir.	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	41%	1692 / 1516	3208	L	D+S
2 - SPF	3.500"	Vert	41%	1692 / 1516	3208	L	D+S

Analysis Results

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	14410 ft-lb	9'5"	62010 ft-lb	0.232 (23%)	D+S	L
Unbraced	14410 ft-lb	9'5"	14425 ft-lb	0.999 (100%)	D+S	L
Shear	2679 lb	1'7 1/2"	20608 lb	0.130 (13%)	D+S	L
LL Defl inch	0.125 (L/1765)	9'5 1/16"	0.460 (L/480)	0.272 (27%)	S	L
TL Defl inch	0.265 (L/834)	9'5 1/16"	0.613 (L/360)	0.432 (43%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6". Nail from both sides.
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 13'4 3/8" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

O Editoral biolia	onnece rane bacea en enigie										
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	161 PLF	0 PLF	161 PLF	0 PLF	0 PLF	A6A	
	Self Weight				19 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 2 Damaged Beams must not be used
- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- This design is valid until 6/28/2026



Client: Project: Address:

Weaver Development

1/18/2024

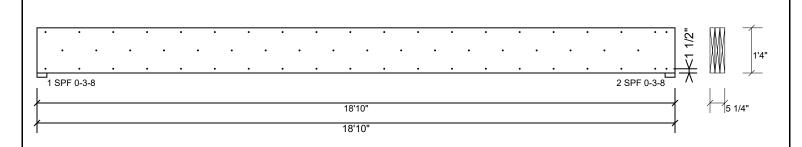
Input by: Curtis Quick Job Name: The Lauren III Beams Page 2 of 8

Project #:

GDH Kerto-S LVL 1.750" X 16.000"

3-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1 00

Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Infoculing & Installation

 I. VIL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info



Client:

Project: Address: Weaver Development

1/18/2024

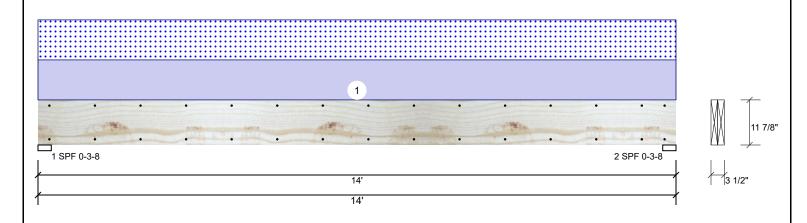
Input by: Curtis Quick Job Name: The Lauren III Beams Page 3 of 8

Project #:

Kerto-S LVL GDH-1

1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Infor	mation			Rea	ctions UNP	ATTER	NED lb (Uplif	t)		
Туре:	Girder	Application:	Floor	Brg	Direction	Live	e Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	Vertical	(1696	1631	0	0
Moisture Condition	n: Dry	Building Code:	IBC 2012	2	Vertical	(1696	1631	0	0
Deflection LL:	480	Load Sharing:	No							
Deflection TL:	360	Deck:	Not Checked							
Importance:	Normal - II									
Temperature:	Temp <= 100°F									
				Bea	rings					
				Ве	aring Length	Dir.	Cap. React D	/L lb Total	Ld. Case	Ld. Comb.
				1 1 -	SPF 3.500"	Vert	64% 1696 /	1631 3327	' 1	D+S

2 - SPF

3.500"

Vert

64%

1696 / 1631

3327 L

D+S

Analysis Results

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10893 ft-lb	7'	22897 ft-lb	0.476 (48%)	D+S	L
Unbraced	10893 ft-lb	7'	10904 ft-lb	0.999 (100%)	D+S	L
Shear	2727 lb	12'8 5/8"	10197 lb	0.267 (27%)	D+S	L
LL Defl inch	0.195 (L/832)	7' 1/16"	0.339 (L/480)	0.577 (58%)	S	L
TL Defl inch	0.398 (L/408)	7' 1/16"	0.451 (L/360)	0.882 (88%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 8'2 11/16" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	233 PLF	0 PLF	233 PLF	0 PLF	0 PLF	G2
	Self Weight				9 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

2 Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood

301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

isDesign

Client:

Project: Address: Weaver Development

1/18/2024 Input by:

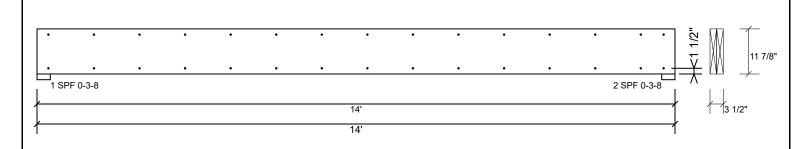
Curtis Quick Job Name: The Lauren III Beams Page 4 of 8

Project #:

GDH-1 **Kerto-S LVL** 1.750" X 11.875"

2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
CM	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851

Manufacturer Info

(800) 622-5850 www.metsawood.com/us



Client:

Address:

Weaver Development Project:

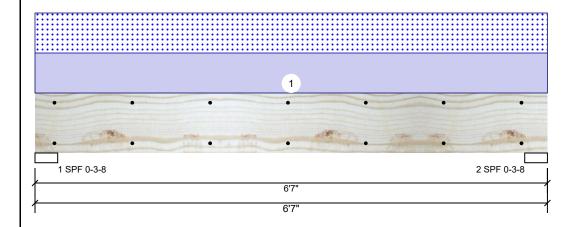
Date: 1/18/2024

Input by: Curtis Quick Job Name: The Lauren III Beams

Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** BM1

Level: Level



Application:

Design Method:

Building Code:

Load Sharing:

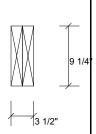
Deck:

ASD

No

IBC 2012

Not Checked



Page 5 of 8

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II

Temperature: Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1551	1527	0	0
2	Vertical	0	1551	1527	0	0

Bearings

Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	59%	1551 / 1527	3078	L	D+S
2 - SPF	3.500"	Vert	59%	1551 / 1527	3078	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4386 ft-lb	3'3 1/2"	14423 ft-lb	0.304 (30%)	D+S	L
Unbraced	4386 ft-lb	3'3 1/2"	10451 ft-lb	0.420 (42%)	D+S	L
Shear	2090 lb	1' 3/4"	7943 lb	0.263 (26%)	D+S	L
LL Defl inch	0.040 (L/1858)	3'3 1/2"	0.153 (L/480)	0.258 (26%)	S	L
TL Defl inch	0.080 (L/922)	3'3 1/2"	0.204 (L/360)	0.391 (39%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	464 PLF	0 PLF	464 PLF	0 PLF	0 PLF	A3
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 2 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

isDesign

Client: Weaver Development

Project: Address:

Date: 1/18/2024 Input by:

Curtis Quick Job Name: The Lauren III Beams

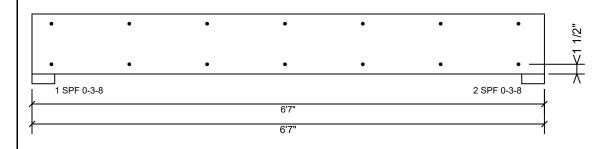
Project #:

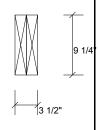
Kerto-S LVL **BM1**

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 6 of 8

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
CM	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us



Client:

Project: Address: Weaver Development

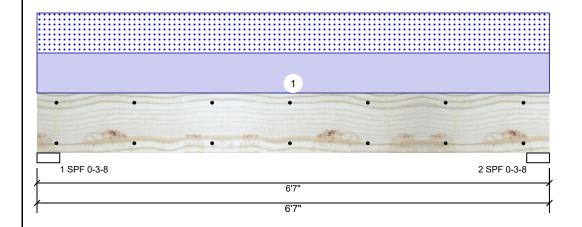
Date: 1/18/2024

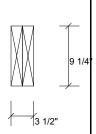
Input by: Curtis Quick Job Name: The Lauren III Beams

Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** BM₂

Level: Level





Page 7 of 8

Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application: Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No

Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)

1	Brg	Direction	Live	Dead	Snow	Wind	Const
ı	1	Vertical	0	458	435	0	0
ı	2	Vertical	0	458	435	0	0

Bearings

Bearing	Length	Dir.	Cap. Re	act D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	17%	458 / 435	893	L	D+S
2 - SPF	3.500"	Vert	17%	458 / 435	893	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1272 ft-lb	3'3 1/2"	14423 ft-lb	0.088 (9%)	D+S	L
Unbraced	1272 ft-lb	3'3 1/2"	10451 ft-lb	0.122 (12%)	D+S	L
Shear	610 lb	5'6 1/4"	7943 lb	0.077 (8%)	D+S	L
LL Defl inch	0.011 (L/6530)	3'3 1/2"	0.153 (L/480)	0.074 (7%)	S	L
TL Defl inch	0.023 (L/3178)	3'3 1/2"	0.204 (L/360)	0.113 (11%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	132 PLF	0 PLF	132 PLF	0 PLF	0 PLF	A6A
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 2 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

This design is valid until 6/28/2026

isDesign

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Project:

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Date: Input by: 1/18/2024

Curtis Quick

Job Name: The Lauren III Beams

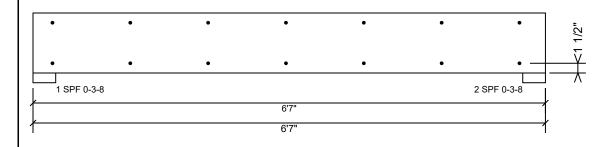
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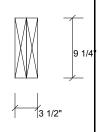
Kerto-S LVL BM2

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 8 of 8

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
CM	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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Manufacturer	Info
Metsä Wood	

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