

RE: J0124-0294

Weaver Homes/10 West Preserve/Harnett

## Site Information:

Customer: Project Name: J0124-0294 Lot/Block: Address: City:

Model: Subdivision: State:

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

Trenco

818 Soundside Rd

Edenton, NC 27932

This package includes 35 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	163547618	A1	2/12/2024	21	163547638	G1GE	2/12/2024
2	163547619	A1GE	2/12/2024	22	163547639	VB1	2/12/2024
3	163547620	A2	2/12/2024	23	163547640	VB2	2/12/2024
4	163547621	A3	2/12/2024	24	163547641	VB3	2/12/2024
5	163547622	A4	2/12/2024	25	163547642	VB4	2/12/2024
6	163547623	A5	2/12/2024	26	163547643	VB5	2/12/2024
7	163547624	A7	2/12/2024	27	163547644	VB6	2/12/2024
8	163547625	A7GE	2/12/2024	28	163547645	VB7	2/12/2024
9	163547626	B1	2/12/2024	29	163547646	VB8	2/12/2024
10	163547627	B2	2/12/2024	30	163547647	VC1	2/12/2024
11	163547628	B3GR	2/12/2024	31	163547648	VC2	2/12/2024
12	163547629	C1	2/12/2024	32	163547649	VD1	2/12/2024
13	163547630	C1GE	2/12/2024	33	163547650	VD2	2/12/2024
14	163547631	C2	2/12/2024	34	163547651	VD3	2/12/2024
15	163547632	C3	2/12/2024	35	163547652	VD4	2/12/2024
16	163547633	C4	2/12/2024				
17	163547634	D1	2/12/2024				
18	163547635	D1GE	2/12/2024				
19	163547636	D1GR	2/12/2024				
20	163547637	G1	2/12/2024				

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Tony Miller

My license renewal date for the state of North Carolina is December 31, 2024

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Tony Miller



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<sup>818</sup> Soundside Road Edenton, NC 27932



- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 11) will fit between the bottom chord and any other members.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 23, 30, 31, 32, 33, 34, 35, 37, 38, 39, 40, 42, 29, 27, 26, 25, 24 except (jt=lb) 41=121.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=17ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) 0-0-0 to 5-8-6, Exterior(2) 5-8-6 to 20-7-0, Corner(3) 20-7-0 to 26-3-6, Exterior(2) 26-3-6 to 33-6-12 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6) will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 14=123



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#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=17ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-4 to 5-9-10, Interior(1) 5-9-10 to 20-7-0, Exterior(2) 20-7-0 to 26-3-6, Interior(1) 26-3-6 to 33-6-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 13=189.

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2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=17ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) 0-1-4 to 5-9-10, Exterior(2) 5-9-10 to 20-7-0, Corner(3) 20-7-0 to 26-3-6, Exterior(2) 26-3-6 to 33-6-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

Unbalanced snow loads have been considered for this design.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6) will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 12=161



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	1	0-9-12	20-7-0			30-4-4	1		4	1-0-4		49-10-0	
	1	0-9-12	9-9-4			9-9-4	1		1	0-8-0		8-9-12	
Plate Offsets (	X,Y) [13:0-4-0,	0-4-0]											
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 15.4/20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/I	2-0-0 1.15 1.15 YES PI2014	<b>CSI.</b> TC BC WB Matri	0.85 0.41 0.49 x-S	DEFL Vert(I Vert( Horz(	.L) -0 CT) -0 CT) 0	in ( .11 15 .17 15 .02	(loc) 5-17 5-17 13	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 343 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-						BRACING-							
TOP CHORD	2x6 SP No.1 *Ex	cept*				TOP CHORD	Stru	ctural v	wood	sheathin	g directly	applied or 4-5-12 oc purlir	IS.
	9-12: 2x4 SP No	.1					Exce	ept:			• •		
BOT CHORD	2x6 SP No.1						1 Rc	w at n	nidpt		9-13		
WEBS	2x4 SP No.2					BOT CHORD	Rigio	d ceilin	ng dire	ctly appl	ied or 6-0	-0 oc bracing.	
						WEBS	1 Ř.	w at n	nidpt		4-19, 6	6-17	
REACTIONS.	(size) 1=Me	echanical, 19=0-3-8, 13	=0-3-8										

Max Horz 1=-297(LC 14) Max Uplift 19=-144(LC 16), 13=-224(LC 13) Max Grav 1=323(LC 42), 19=2019(LC 30), 13=2028(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown TOP CHORD 2-4=-43/445, 4-5=-842/306, 5-6=-815/312, 6-8=-1376/124, 8-9=-1438/106,

9-13=-2664/1524, 9-10=-1679/1687, 10-11=-1409/1186

BOT CHORD 17-19=0/332, 15-17=0/1037, 13-15=0/1203, 11-13=-1098/1446 WEBS 2-19=-480/310, 4-19=-1406/318, 4-17=0/658, 5-17=-106/432, 6-17=-701/186, 6-15=0/395, 10-13=-723/490

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) 0-1-4 to 5-9-10, Exterior(2) 5-9-10 to 20-7-0, Corner(3) 20-7-0 to 26-3-6, Exterior(2) 26-3-6 to 50-8-8 zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10; Min. flat roof snow load governs.

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7) will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 19=144, 13=224.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7) will fit between the bottom chord and any other members.
- 8) Bearing at joint(s) 26 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 44, 45, 46, 47, 42, 41, 40, 38, 37, 36, 35, 34, 29, 28, 26 except (jt=lb) 49=151, 31=199.
- 10) N/A

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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February 12,2024



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Refer to airder(s) for truss to truss connections

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 1=170.8=322.



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roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 8.



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#### Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	Weaver Homes/10 West Preserve/Harnett	
						163547628
J0124-0294	B3GR	Common Girder	1	2		
				_	Job Reference (optional)	
Comtech, Inc, Fayett	eville, NC - 28314,			8.430 s J	an 6 2022 MiTek Industries, Inc. Fri Feb 9 10:02:32 2024	Page 2
		ID: Z	fildaqjrs	ztBHN?xT	fO4zmc0I-RfC?PsB70Hq3NSqPqnL8w3uITXbGKWrCDoi7	J4zJC?f

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 10=-339(B) 12=-254(B) 13=-253(B) 14=-253(B) 16=-253(B) 17=-253(B) 18=-253(B) 19=-253(B) 20=-339(B) 21=-339(B) 22=-242(B)



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Unbalanced snow loads have been considered for this design.

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7) All plates are 2x4 MT20 unless otherwise indicated.

8) Gable requires continuous bottom chord bearing.

9) Gable studs spaced at 2-0-0 oc.

10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 11) will fit between the bottom chord and any other members.

12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 16, 26, 27, 28, 29, 30, 24, 22, 21, 20, 19, 18 except (jt=lb) 31=117.



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February 12,2024

a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oblapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



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Job	Truss	Truss Type	Qty	Ply	Weaver Homes/10 West Preserve/Harnett	
						163547636
J0124-0294	D1GR	COMMON	1	2		
				-	Job Reference (optional)	
Comtech, Inc, Fayette	ville, NC - 28314,			8.430 s J	an 6 2022 MiTek Industries, Inc. Fri Feb 9 10:02:45 2024	Page 2

ID:\_ZfiIDAQJRSztBHN?xTfO4zmc0I-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 9=-738(F) 10=-619(F) 11=-619(F) 12=-619(F) 13=-2048(F)



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LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 15.4/20.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.21 BC 0.16 WB 0.17 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in ( n/a n/a 0.00	(loc) - - 7	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 125 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD OTHERS	2x4 SP No.1 2x4 SP No.1 2x4 SP No.2		BI TC BC W	RACING- DP CHORD S DT CHORD F EBS 1	Structural Rigid ceilir Row at n	wood s ng dire nidpt	sheathin ctly appl	g directly app ied or 10-0-0 4-11	blied or 6-0-0 oc purlins oc bracing.	

REACTIONS. All bearings 26-9-0. (lb) - Max Horz 1=208(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 1, 12, 9 except 13=-121(LC 14), 8=-121(LC 15)

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 11=444(LC 28), 12=520(LC 25), 13=497(LC 25), 9=519(LC 26), 8=497(LC 26)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

WEBS 3-12=-284/203, 2-13=-369/258, 5-9=-284/203, 6-8=-369/258

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) 0-5-15 to 6-2-5, Exterior(2) 6-2-5 to 13-5-1, Corner(3) 13-5-1 to 19-1-7, Exterior(2) 19-1-7 to 26-4-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

4) All plates are 2x4 MT20 unless otherwise indicated.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 12, 9 except (jt=lb) 13=121, 8=121.

8) N/A



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**REACTIONS.** All bearings 23-3-0.

(lb) - Max Horz 1=-181(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 1, 13, 8 except 12=-105(LC 14), 9=-105(LC 15)

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 11=453(LC 28), 12=452(LC 25), 13=327(LC 25), 9=452(LC 26), 8=327(LC 26)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 3-12=-314/219, 2-13=-284/202, 5-9=-314/220, 6-8=-284/201

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=16ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-15 to 6-2-5, Interior(1) 6-2-5 to 11-8-1, Exterior(2) 11-8-1 to 17-4-7, Interior(1) 17-4-7 to 22-10-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 13, 8 except (jt=lb) 12=105, 9=105.
- 8) N/A



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A MiTek Af 818 Soundside Road



TOP CHORD

BOT CHORD

# TOP CHORD 2x4 SP No.1

BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

**REACTIONS.** All bearings 19-9-0.

(lb) - Max Horz 1=154(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 13, 8 except 12=-110(LC 14), 9=-110(LC 15)

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 11=445(LC 25), 12=464(LC 25), 13=262(LC 25), 9=464(LC 26), 8=262(LC 26)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 3-12=-323/227, 5-9=-323/227

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=17ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-15 to 5-11-1, Interior(1) 5-11-1 to 9-11-1, Exterior(2) 9-11-1 to 15-7-7, Interior(1) 15-7-7 to 19-4-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

4) All plates are 2x4 MT20 unless otherwise indicated.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 13, 8 except (jt=lb) 12=110, 9=110.

8) N/A



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=17ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-15 to 6-2-5, Interior(1) 6-2-5 to 8-2-1, Exterior(2) 8-2-1 to 13-10-7, Interior(1) 13-10-7 to 15-10-3 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=116, 6=116.

7) N/A



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4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 8, 6. 7) N/A



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818 Soundside Road



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=18ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10
This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

7) N/A



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REACTIONS. (size) 1=5-9-0, 3=5-9-0, 4=5-9-0 Max Horz 1=41(LC 13) Max Uplift 1=-19(LC 14), 3=-23(LC 15) Max Grav 1=106(LC 2), 3=106(LC 2), 4=177(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=19ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10
This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This trust has been designed for a live load of 30.0ps on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

7) N/A



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TRENCO A Mitek Affiliate

818 Soundside Road



0<u>-0-9</u> 2-3-9 Plate Offsets (X,Y)-- [2:0-2-0,Edge] LOADING (psf) SPACING-2-0-0 CSI. DEFL in (loc) l/defl L/d PLATES GRIP TCLL (roof) 20.0 Plate Grip DOL 1.15 тс 0.01 Vert(LL) 999 MT20 244/190 n/a n/a Snow (Pf/Pg) 15.4/20.0 Lumber DOL 1.15 BC 0.01 Vert(CT) n/a n/a 999 TCDL 10.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 3 n/a n/a BCLL 0.0 \* Code IRC2015/TPI2014 FT = 20% Matrix-P Weight: 6 lb

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

BCDL

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. (size) 1=2-3-0, 3=2-3-0 Max Horz 1=-12(LC 10) Max Uplift 1=-4(LC 14), 3=-4(LC 15) Max Grav 1=54(LC 2), 3=54(LC 2)

10.0

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=19ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

7) N/A



Structural wood sheathing directly applied or 2-4-2 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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Max Grav 1=143(LC 2), 3=143(LC 2), 4=258(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=16ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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2x4 💋

2x4 \lt

Rigid ceiling directly applied or 10-0-0 oc bracing.

Plate Offsets (X,Y) [2:0-2-0,F	Edge]	<u>3-10-1</u> 3-10-1	0				<u>3-11</u> -4 0-0-10	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.02 BC 0.07 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	l/defl n/a n/a n/a	L/d 999 999 n/a	<b>PLATES</b> MT20 Weight: 11 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.1		BR. TO	ACING- P CHORD Str	ructural wood	sheathing	directly appl	lied or 3-11-4 oc purli	ns.

BOT CHORD

# TOP CHORD 2x4 SP No.1

BOT CHORD 2x4 SP No.1

REACTIONS. 1=3-10-0, 3=3-10-0 (size) Max Horz 1=-21(LC 12) Max Uplift 1=-7(LC 16), 3=-7(LC 17) Max Grav 1=113(LC 2), 3=113(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=17ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7) will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 6.

8) N/A



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Unbalanced snow loads have been considered for this design.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 6, 7.

8) N/A



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#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (ground sn

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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2x4 💋

2x4 📎

Rigid ceiling directly applied or 10-0-0 oc bracing.

3-6-12 0-0-10 3-6-2 3-6-2 Plate Offsets (X,Y)-- [2:0-2-0,Edge] LOADING (psf) SPACING-2-0-0 CSI. DEFL in (loc) l/defl L/d PLATES GRIP TCLL (roof) 20.0 Plate Grip DOL 244/190 1.15 тс 0.02 Vert(LL) 999 MT20 n/a n/a Snow (Pf/Pg) 15.4/20.0 Lumber DOL 1.15 BC 0.05 Vert(CT) n/a n/a 999 TCDL 10.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 3 n/a n/a BCLL 0.0 \* Code IRC2015/TPI2014 FT = 20% Matrix-P Weight: 9 lb BCDL 10.0 LUMBER-BRACING-TOP CHORD 2x4 SP No.1 TOP CHORD Structural wood sheathing directly applied or 3-6-12 oc purlins.

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. (size) 1=3-5-8, 3=3-5-8 Max Horz 1=18(LC 13) Max Uplift 1=-6(LC 16), 3=-6(LC 17) Max Grav 1=98(LC 2), 3=98(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=16ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
TCLL: ASCE 7-10; PI=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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