DESIGN LOADS	LIVE LOAD	DEAD LOAD
TABLE R301.4	(PSF)	(PSF)
DWELLING UNITS	40	10
SLEEPING ROOMS	30	10
ATTICS WITH STORAGE	20	10
ATTICS WITHOUT STORAGE	10	10
ROOF SNOW	20	10
STAIRS	40	10
DECKS	40	10
EXTERIOR BALCONIES	60	10
PASSENGER VEHICLE GARAGES	50	_
FIRE ESCAPES	40	10
CHADDDALC AND HANDDALC	200	_

MATERIALS

1. FRAMING LUMBER SHALL BE #2 SPRUCE PINE FIR (SPF) WITH THE FOLLOWING DESIGN PROPERTIES: F9 = 875 PSI Fv = 70 PSI E = 1.466 PSI

2, FRANING LUNBER EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND, CONCRETE OR MASONRY SHALL BE #2 SOUTHERN YELLOW PINE (SYP) TREATED IN ACCORDANCE WITH AWPA C22 WITH THE FOLLOWING DESIGN PROPERTIES: PD = 105 PS IF YES PS IF E = 1,66 PS I

3. ENSINEERED WOOD BEAMS SHALL BE LAWINATED VENEER LUMBER (LVL.) OR PARALLEL STRAND LUMBER (PSL.) WITH THE FOLLOWING MINIMUM DESIGN PROPERTIES: FD = 200 PSL | Ps. 285 PSL | E = 1966 PSL

4. STRUCTURAL STEEL SHALL CONFORM TO ASTM A-36 MINIMUM GRADE.

5. BOLTS SHALL CONFORM TO A307 MINIMUM GRADE.

6. REBAR SHALL BE DEFORMED STEEL CONFORMING TO ASTM A615 GRADE 60.

7. POURED CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. MATERIALS USED TO PRODUCE CONCRETE SHALL COMPLY WITH THE APPLICABLE STANDARDS LISTED IN ACI 316 OR ASTM C 1157.

8. CONCRETE LOCATED PER TABLE R402.2 SHALL BE AIR ENTRAINED WITH THE TOTAL AIR CONTENT NOT LESS THAN 5 PERCENT OR MORE THAN 7 PERCNET.

9. MASONRY UNITS SHALL CONFORM TO ACI 530/ASCE 5/TMS 402 AND MORTAR SHALL COMPLY WITH ASTM C 270.

10. ALLOWABLE SOIL BEARING PRESSURE 2000 PSF.

ENGINEER'S SEAL APPLIES TO STRUCTURAL COMPONENTS ONLY AND DOES NOT CERTIFY ARCHITECTURAL LAYOUT OR DIMENSIONAL ACCURACY, ENGINEER IS NOT RESPONSIBLE FOR CONSTRUCTION METHODS OR ANY DEVIATION FROM THE

ALL CONSTRUCTION, WORKMANSHIP, MATERIAL QUALITY AND SELECTION SHALL BEIN ACCORDANCE WITH THE <u>NORTH CAROLINA STATE BUILDING CODE - RESIDENTIAL CODE 2016 EDITION</u> FROM THE INTERNATIONAL RESIDENTIAL CODE 2016 (RIC), AND LOCAL CODES AND REGULATIONS, DIMENSIONS SHALL GOVERN OVER SCALE AND CODE SHALL GOVERN OVER STALL EXPREDICTIONS. DIMENSIONS.

ADDITIONAL LOADS

FIGURE R301.2(4) - ULTIMATE DESIGN WIND SPEEDS 115-120 MPH

FIGURE R301.2(2) - SEISMIC DESIGN CATEGORY B

TABLE R301.2(4) - DESIGN POSITIVE AND NEGATIVE PRESSURE FOR DOORS AND WINDOW FOR A MEAN ROOF HEIGHT OF 35 FEET OR LESS SHALL BE 25 PSF

TABLE R301.2(2) - COMPONENT AND CLADDING LOADS FOR A MEAN ROOF HEIGHT OF 30 FEET OR LESS LOCATED IN EXPOSURE B

EXPOSURE B
ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE DESIGNED BASED ON ROOF PITCHES AS FOLLOWS:
45.4 PSF FOR 0:12 TO 2.25:12, 34.8 PSF FOR 2:25:12 TO 7:12 AND 21 PSF FOR 7:12 TO 12:12
WALL CLADDING IS DESIGNED FOR A 24.1 PSF POSITIVE AND NEGATIVE PRESSURE

TABLE N1102.1 - REFER TO TABLE N1101.1 TO DETERMINE THE CLIMATE ZONE BY COUNTY AND REFER TO TABLE N1102.1 FOR R VALUE INSULATION REQUIREMENTS LISTED BY ZONE.

 $\begin{array}{l} \underline{\text{TABLE N1102.1-ZONE 7-MAX, GLAZING U FACTOR; \underline{0.40}. \ MIN. \ INSULATION R VALUES; CEILING \underline{R-30}, WALLS \underline{R-13}, \\ \underline{\text{FLOORS }}\underline{\text{R-19}}, \underline{\text{BASEMENT WALLS }}\underline{\text{R-7}}, \underline{\text{SLAB PERIMETER }}\underline{\text{R-0}}, \underline{\text{CRAWL SPACE WALLS }}\underline{\text{R-7}}. \end{array}$

 $\frac{\text{TABLE N1102.1-ZONE 8-MAX, GLAZING U FACTOR; 0.40, MIN, INSULATION R VALUES; CEILING R-30, WALLS \underline{R-13, FLOORS R-19, BASEMENT WALLS \underline{R-3}, SLAB PERIMETER R-5 (2 FT DEEP), CRAWL SPACE WALLS \underline{R-10}.}$

1. STEEL FLITCH BEAMS SHALL BE FASTENED TOGETHER WITH 1/2" DIAMETER BOLTS WITH WASHERS PLACED UNDER THE THREADED END OF THE BOLT. BOLTS SHALL BE SPACED AT MAXIMUM 24" OC. STAGGERED TOP AND BOTTOM OF BEAM WITH A MINIMUM 2" EDGE DISTANCE. TWO BOLTS SHALL BE LOCATED AT 6" FROM EACH END OF FLITCH BEAM.

STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3 1/2" AND FULL FLANGE WIDTH.
 BEAMS MUST BE ANCHORED AT EACH END WITH A MINIMUM OF FOUR 16d NAILS OR TWO 1/2" x 4" LAG SCREWS.

3. ENGINEERED WOOD BEAMS SHALL BE INSTALLED WITH ALL CONNECTIONS PER MANUFACTURER'S INSTRUCTIONS.

4. ALL BEAMS SHALL BE CONTINUOUSLY SUPPORTED LATERALLY AND SHALL BEAR FULL WIDTH ON THE SUPPORTING WALLS OR COLUMNS INDICATED WITH A MINIMUM OF THREE STUDS.

6. ENGINEERED WOOD FLOOR SYSTEMS AND ROOF TRUSS SYSTEMS SHALL BE PROVIDED FOR REVIEW AND COORDINATED WITH THE ENGINEER OF RECORD, INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURERS INSTRUCTIONS.

7. WALL BRACING REQUIREMENTS SHALL BE IN ACCORDANCE WITH SECTION R602, 10 OF THE NORTH CAROLINA RESIDENTIAL CODE.

8. BRICK LINTELS SHALL BE 3 1/2 x 3 1/2 x 1/4 STEEL ANGLE FOR UP TO 60° MAXIMUM SPAN AND 6 x 4 x 5/16 FOR SPANS GREATER THAN 60° .

9. BRICK LINTELS AT SLOPED AREAS SHALL BE 4 x 3 1/2 x 1/4 STEEL ANGLE WITH 164 NAILS IN 3/16" HOLES IN 4" ANGLE LEG AT 12" oc. TO DOUBLE RAFTER. WHEN THE SLOPE EXCEEDS 4:12 A MINIMUM OF 3 x 3 x 1/4 PLATES SHALL BE WELDED AT 24" oc. ALONG THE STEEL ANGLE.

Lot 17 Cotton Farms

MEAN ROOF HEIGHT 1 STORY = 11'-0"

CLADDING POSITIVE & NEGATIVE PRESSURE = 21

1 1/2 STORY = 19'-0"

CLADDING POSITIVE & NEGATIVE PRESSURE =

2 STORY = 19'-0"

CLADDING POSITIVE & NEGATIVE PRESSURE =

ANCHOR BOLTS INSTALL ANCHOR BOLTS, NUTS, AND WASHERS PER CODE AT ALL EXTERIOR WALL TREATED PLATES AND AT INTERIOR BEARING

TREATED PLATES ON SLAB

FOUNDATIONS, TO BE A MINIMUM OF 6' O.C. AND WITHIN 12" FROM THE ENDS OF EACH PLATE.

MLWINDOWS 3500 SERIES

SQL	JARE FOOT	4GE
	HEATED S.F.	UNHEATED S.F.
FIRST FLOOR	1571	
SECOND FLOOR	1008	
HVAC /STORAGE		177
SCREEN PORCH		144
FRONT PORCH		129
GARAGE		764
TOTAL	2579	1214
	OPTION\$	
	HEATED S.F.	UNHEATED S.F.
OPT. PATIO		118

	REVISION LOG							
Rev	Description	Drawn By	Date	Sheets Affected	Brochure Required	Engineering Required		
1								
2								
3								
4								
5								
6								
7								
8								
9								
10								
-11								
12								
13								
14								

TABLE N1102 1 CLIMATE ZONES 3-5

EACH
FLAT PLATE
FOOTING
HANGER
LAMINATED VENEER LUMBER

TYPICAL UNLESS NOTED OTHERWISE

CLIMATE ZONES	FENESTRATION U-FACTOR b	SKYLIGHT ^b U-FACTOR	GLAZED FENESTRATION SHGC b,e	CELLING ^k R-VALUE	WOOD FRAMED WALL R-VALUE	MASS WALL R-VALUE I	FLOOR R-VALUE	BASEMENT C WALL R-VALUE	SLAB ^d R-VALUE AND DEPTH	CRAWL SPACE C WALL R-VALUE
3	0.35	0,55	0.30	30	43	5/13	-10	5/12	0	5/13
4	0.35	0.55	0.30	38 OR 30 CONT J	15 OR 13+2.5 ^h	5/13	19	10/15	10	10/15
-5	0.95	0.55	NR	38 OR 30 CONT	19 OR 13+5 OR 15+3e,ii	13/17 OR 13/12.50	90 C	10/15	10	10/10

a R-VALUES ARE MINIMUMNS THEACTORS AND SHGC ARE MAXIMUMS.

b. THE FENESTRATION U-FACTOR COLUMN EXCLUDED SKYLIGHTS. THE SHGC COLUMN APPLIES TO ALL GLAZED FENESTRATION.

- . "10/13" MEANS R-10 CONT. INSULATED SHEATHING ON THE INTERIOR OR EXTERIOR OF THE HOME OR R-13 CAVITY INSULATION AT THE INTERIOR OF THE BASEMENT WALL OR
- d. FOR INDIGITHIC SLABS, INSULATION SHALL BE APPLIED FROM THE INSPECTION GAP DOWNWARD TO THE BOTTOM OF THE FOOTING OR A MAXIMUM OF 18 INCHES BELOW GRADE, WHICHEVER IS LESS. FOR FLOATING SLABS, INSULATION SHALL EXTEND TO THE BOTTOM OF THE FOUNDATION WALL OR 24 INCHES, WHICHEVER IS LESS. R-S SHALL BE ADDED TO THE
- REQUIRED SLAB EDGE R-VALUE FOR HEATED SLABS.

 e. R-19 FIBERGLASS BATTS COMPRESSED AND INSTALLED IN A NOMINAL 2:6 CAVITY IS DEEMED TO COMPLY. FIBERGLASS BATTS RATED R-19 OR HIGHER COMPRESSED AND INSTALLED IN A 2x4 WALL IS NOT DEEMED TO COMPLY
- f. BASEMENT WALL INSULATION IS NOT REQUIRED IN WARM-HUMD LOCATIONS AS DEFINED BY FIGURE M101.2 (1 AND 2) AND TABLE M1101.2.
 g. OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY, R-19 MINIMUM.
- "13-6" MEANS R-13 CAUTTY INSULATION PLUS R-5 INSULATED SHEATHING 15-3 MEANS R-15 CAUTTY INSULATION PLUS R-3 INSULATED SHEATHING, IF STRUCTURAL SHEATHING COVERS 25
 PERCENT OR LESS OF THE EXTERIOR, INSULATING SHEATHING IS NOT REQUIRED WHERE STRUCTURAL SHEATHING IS USED, IF THE STRUCTURAL SHEATHING COVERS MORE THAN 25 PERCENT
 OF THE EXTERIOR, STRUCTURAL SHEATHING SHALL BE SUPPLEMENTED WITH INSULATED SHEATHING OF AT LEAST R-2, 13-2.5 MEANS R-13 CAUTTY INSULATION PLUS R-2,5 SHEATHING.
- FOR MASS WALLS, THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR OF THE MASS WALL.
- R-30 SHALL BE DEFINED TO SATISFY THE CEILING INSULATION REQUIREMENT WHEREVER THE FULL HEIGHT OF THE UNCOMPRESSED R-30 INSULATION EXTENDS OVER THE WALL TOP PLATE AT THE EAVES, OTHERWISE R-38 INSULATION IS REQUIRED WHERE ADEQUATE CLEARANCE EXISTS OR INSULATION MUST EXTEND TO EITHER THE INSULATION BAFFLE OR WITHIN 1
- k. TABLE VALUE REQUIRED EXCEPT FOR ROOF EDGE WHERE THE SPACE IS LIMITED BY THE PITCH OR THE ROOF. THERE THE INSULATION MUST FILL THE SPACE UP TO THE AIR BAFFLE.

NOTE: A MINIMUM OPENING MEASURING 18 INCHES BY 24

OPT #1: WHERE AN APPROVED VAPOR BARRIER IS INSTALLED OVER GROUND SURFACE, THE REQUIRED VENTILATION MAY BE REDUCED BY 50%.

OPT #2 - CLOSED CRAWL SPACE PER R409.

			A	TTIC	vent s	CHEDU	LE		
MAIN	MAIN HOUSE SQ FTG 2608 AT / NEAR RIDGE AT / NEAR EAVE					AR EAVE			
VENT TYPE		. FT. JIRED	SQ. FT.	PERCENT OF TOTAL	POT LARGE	POT SMALL	RIDGE VENT (SQ. H. FURU)	EAVE VENT	CONT. VENT
		√GE	SUPPLIED	SUPPLIED	0.4236	0.2778	0.125	0.1944	0.0625
RIDGE VENT	3.48	4.35	5.25	55.26	0	0	42.00		
SOFFIT VENTS	5.22	4.35	4.25	44.74				0	68.00
TOTAL (MIN)	8.69	8.69	9.50	100.00	POT VENTS MAY BE REQUIRED IF THERE IS INSUFFICIENT MOGEAVAILABLE				

* SCHEDULE HAS BEEN CALCULATED ASSUMING EAVE VENTILATION AT 50-60% OF TOTAL AND RIDGE AT 40-50% OF TOTAL REQUIRED VENTILATION

(C) COPYRIGHT 2024 SOUTHERN DESIGNS, INC.



DESIGNS

P.O. Box 688 Wake Forest, NC 27588 (O) 919-556-2226 (F) 919-556-2228 www.southdesigns.com

Drawn By:	RWB
Checked By:	RWB
1-19	-2024
Revision No.	Revision Date
	•

Client:			

COVER SHEET

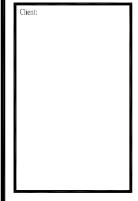




SOUTH

DESIGNS
P.O. Box 688
Wake Forest, NC 27588
(0) 919-556-2226
(F) 919-556-2228 www.southdesigns.com

Drawn By: RWB
Checked By: RWB
1-19-2024
Revision No. Revision Date



ELEVATIONS

2579





SOUTH

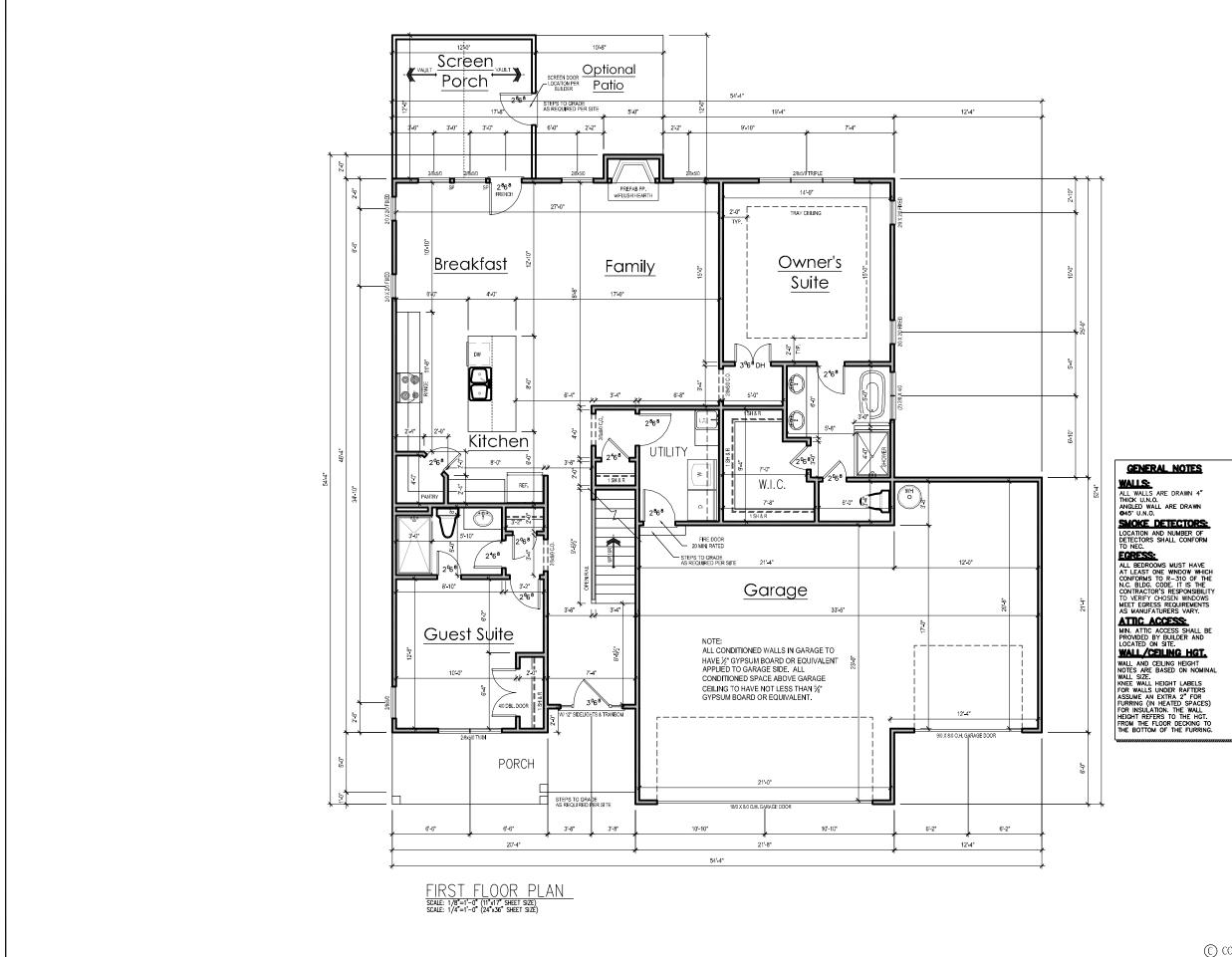
DESIGNS
P.O. Box 688
Wake Forest, NC 27588
(0) 919-556-2226
(F) 919-556-2228 www.southdesigns.com

RWB
RWB
-2024
Revision Date



ELEVATIONS

2579 EL-2



SOUTH DESIGNS

P.O. Box 688 Wake Forest, NC 27588 (O) 919-556-2226 (F) 919-556-2228 www.southdesigns.com

RWB
RWB
2024
Revision Date

This plan is the property of Southern Designs, inc. and may not be reproduced without the expressed writte cansent of Southern Designs, inc. These drawings are offered to the named client for a conditional one time. The conditional use is limited to the lat or property as specified herein and only for solid location.

Southern Designs, Inc. assumes no inbibility for any home constructed from these plans. Contractor or Builder shall werify all dimensions and conditions prior to construction. Caution must be exercised when making changes to these drawings. If changes are made to these drawings, contact Southern Designs,

Title:

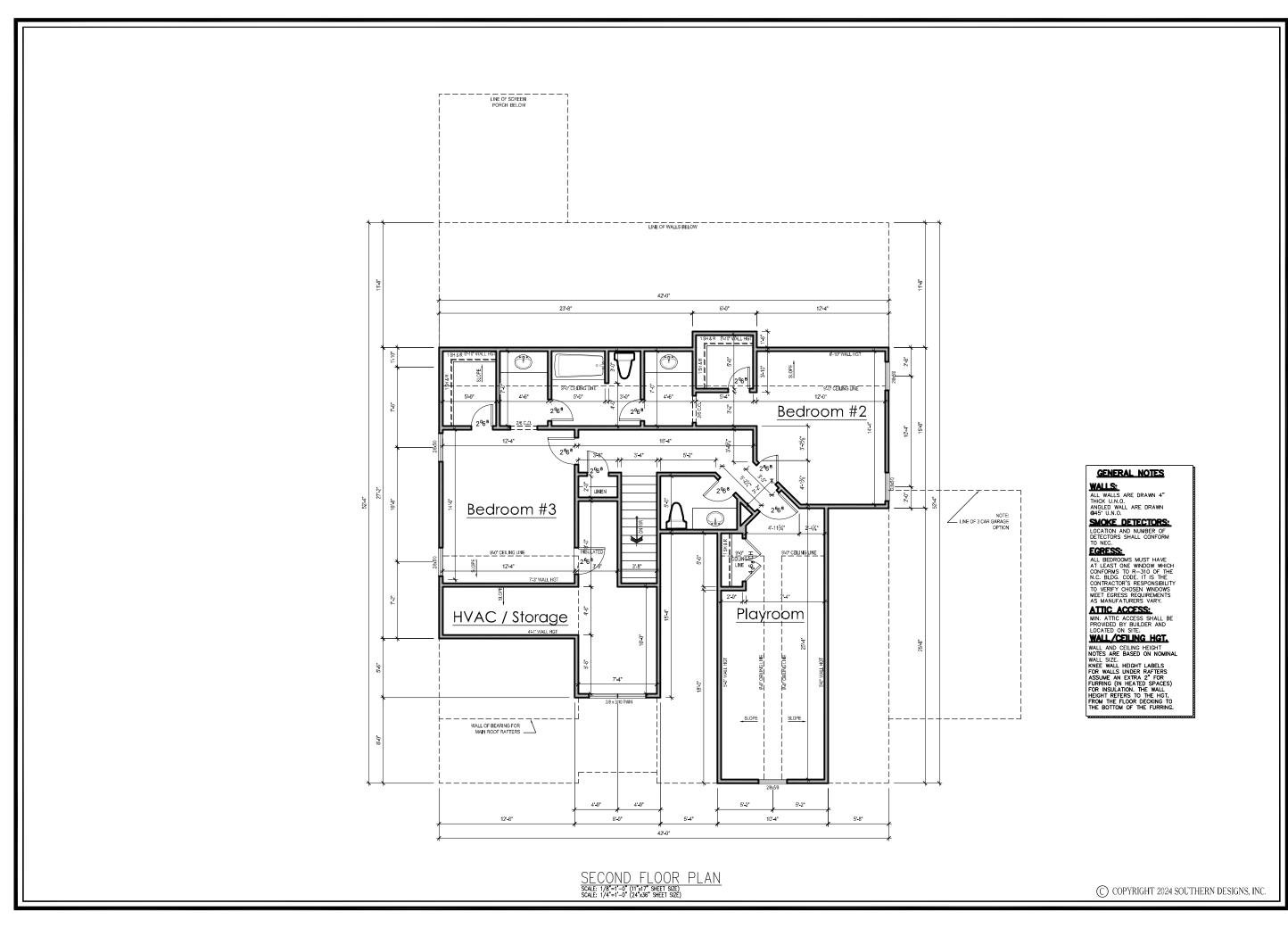
FIRST FLOOR PLAN

Plan No.

2579

A-1

(C) COPYRIGHT 2024 SOUTHERN DESIGNS, INC.





SOUTH DESIGNS

P.O. Box 688 Wake Forest, NC 27588 (O) 919-556-2226 (F) 919-556-2228 www.southdesigns.com

Drawn By: I	RWB
Checked By:	RWB
1-19-	2024
Revision No.	Revision Date

This plan is the property of Southern Designs, inc. and may not be reproduced without the expressed write consent of Southern Designs, inc.
These drawings are offered to the named client for a conditional one timuse. The conditional use is limited to the lot or property as specified herein, and only for said location.

Southern Designs, Inc. assumes no liability for any home constructed from these plans. Contractor or Builder shall werify all dimensions and conditions prior to construction. Caution must be exercised when making changes to the drawings. If changes are made to these drawings, contact Southern Designs,

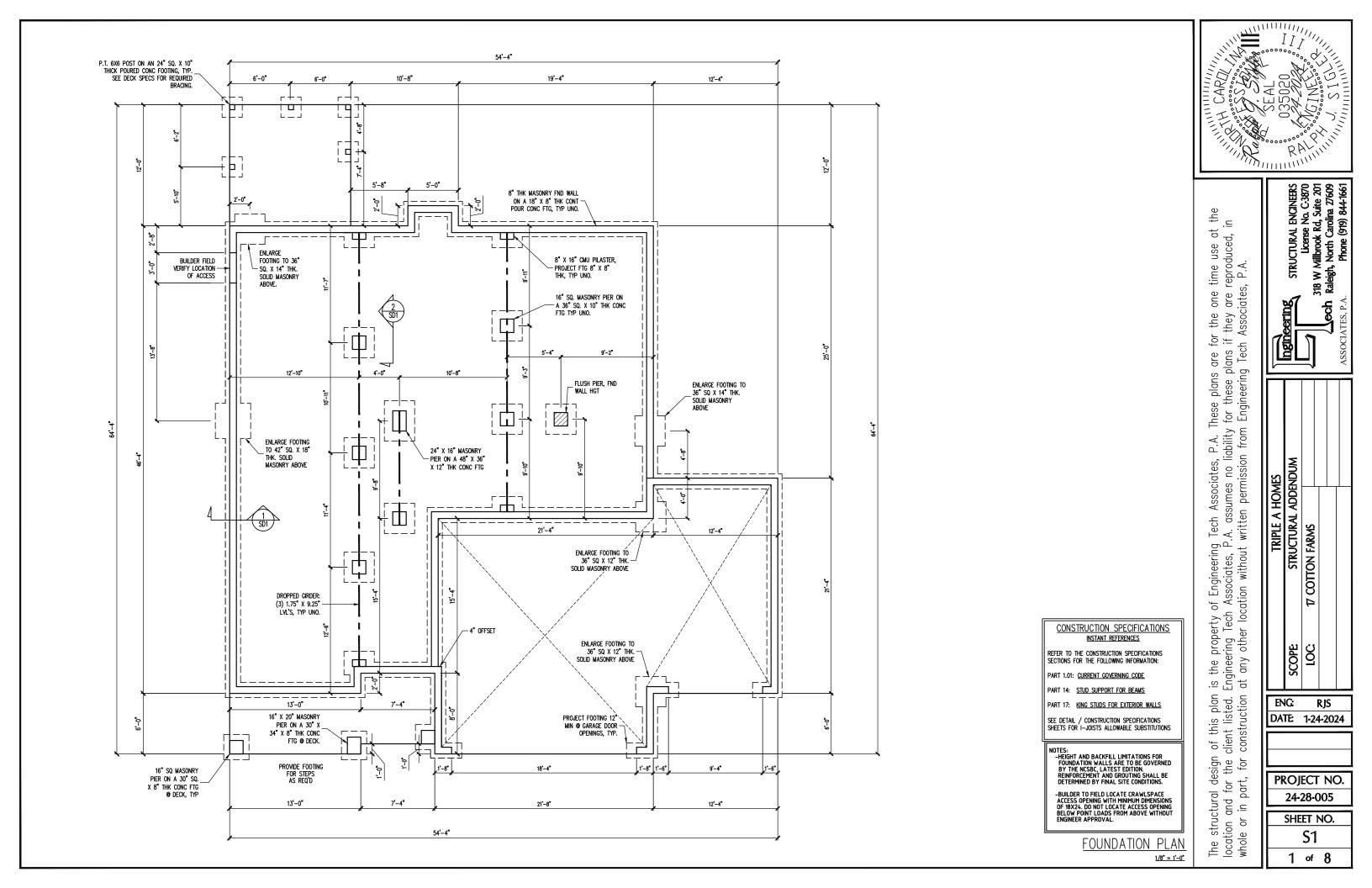


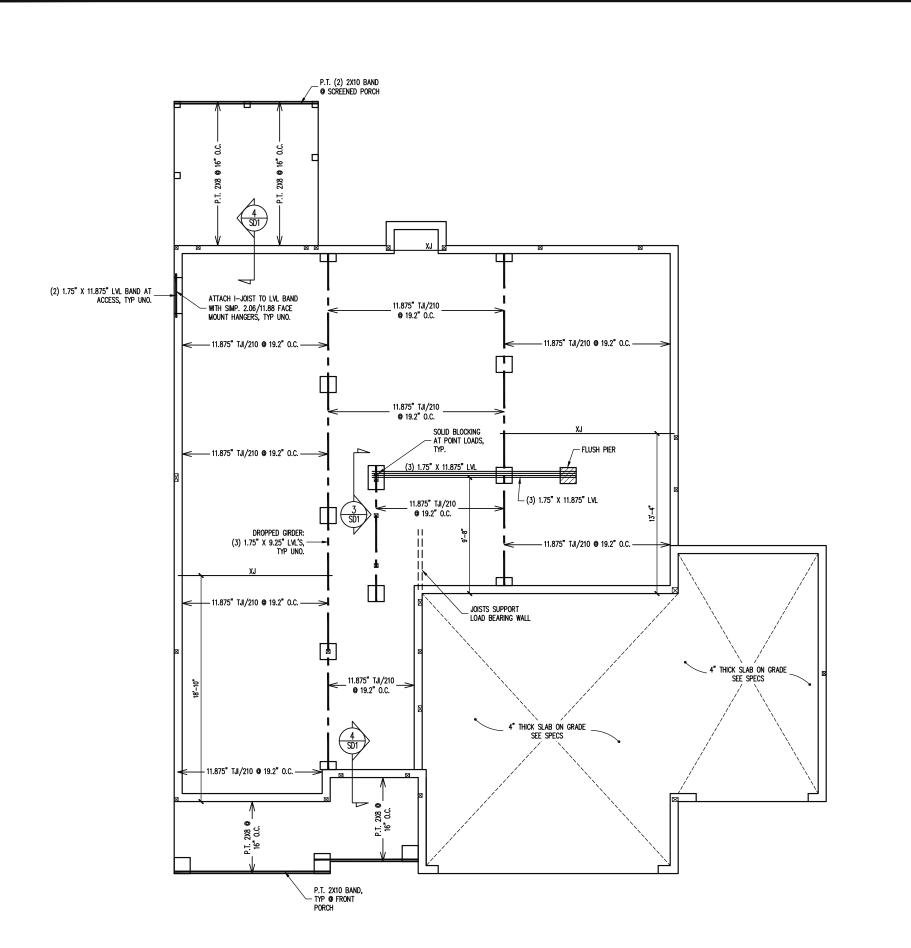
SECOND FLOOR PLAN

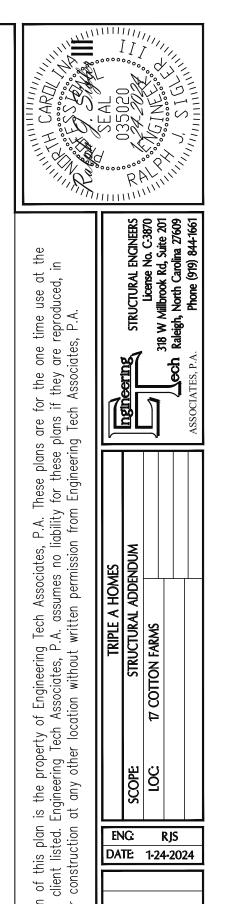
Plan No.

2579

. A-2







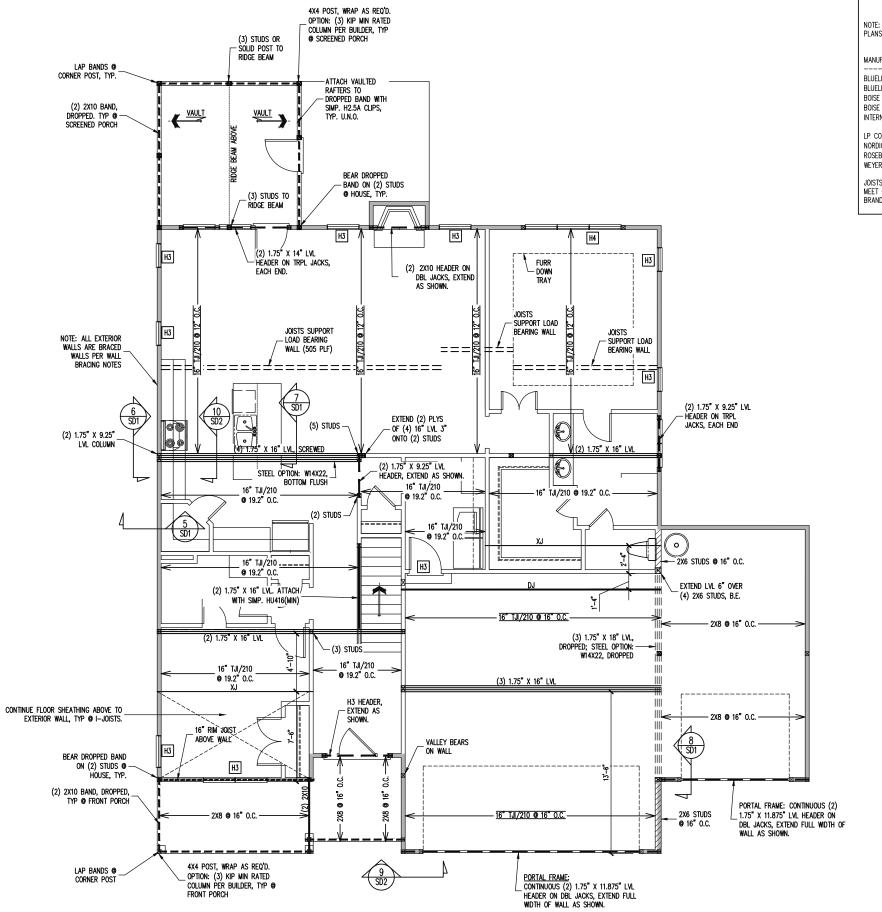
CRAWL SPACE FRAMING PLAN

1/8" = 1'-0"

The structural design c location and for the cl whole or in part, for c

PROJECT NO. 24-28-005 SHEET NO. **S2**

2 of 8



ALLOWABLE I-JOIST SUBSTITUTION

NOTE: MAINTAIN JOIST DEPTH, DIRECTION, AND SPACING SPECIFIED ON

MANUFACTURER	DEPTH	SERIES	SIMPSON FACE MOUNT HGR	SIMPSON TOF FLANGE HGR
BLUELINX BLUELINX BOISE CASCADE BOISE CASCADE NTERNATIONAL BEAMS	16" 16" 16" 16" 16"	BLI 40 BLI 60 BCI 5000s BCI 6000S IB 600	IUS2.56/16 IUS2.56/16 IUS2.06/16 IUS2.37/16 IUS2.56/16	ITS2.56/16 ITS2.56/16 ITS2.06/16 ITS2.37/16 ITS2.56/16
LP CORP NORDIC ROSEBURG MEYERHAEUSER	16" 16" 16" 16"	LPI 20+ NI 40X RFPI 60S TJI 210	IUS2.56/16 IUS2.56/16 IUS2.56/16 IUS2.06/16	ITS2.56/16 ITS2.56/16 ITS2.56/16 ITS2.06/16

JOISTS NOT LISTED IN THE ABOVE TABLE MAY BE USED PROVIDED THEY MEET OR EXCEED THE PROPERTIES OF THOSE LISTED, SUBSTITUTE USP BRAND HANGERS WITH EQUIVALENT VALUES AS DESIRED.

TRUSS SUBSTITUTION

16" I-JOISTS PERMITTED TO BE SUBSTITUTED WITH 16" FLOOR TRUSSES. MAINTAIN MINIMUM SPACING AS CALLED OUT ON PLANS. CONNECTIONS PER TRUSS MANU.

WALL BRACING

SHADED WALLS:

ALL EXTERIOR STUD WALLS, EXTERIOR SIDE, ARE TO BE CONTINUOUSLY SHEATHED WITH 7/16 APA RATED OSB NAILED TO STUDS WITH 8d NAILS @ 6" O.C. AT PANEL EDGES, 12" O.C.

- WSP ONE SIDE OF INTERIOR WALL OR INSIDE OF EXTERIOR WALL WITH 3/8" MIN. THICKNESS WOOD STRUCTURAL PANELING. ATTACH WSP TO STUD WALL WITH 8d NAILS @ 4" O.C. AT PANEL EDGES, 8" O.C. IN PANEL FIELD.
- GB INTERIOR BRACED WALL. 1/2" GB SECURED PER TABLE R602.10.2 OF THE 2018 NCRBC. (FASTENERS @ 7" O.C.) BOTH SIDES OF WALL, OR (FASTENERS @ 4" O.C.) ONE SIDE OF WALL AT STAIRS
- 2X SHEATH BOTH SIDES OF STUD WALL WITH 76
 APA RATED OSB, NAILED TO STUDS WITH 8d NAILS @ 6" O.C. AT PANEL EDGES, 12" O.C. IN PANEL FIELD.

PROVIDED CONTINUOUS SHEATHING = 222' MIN.

REFERENCE PART 16.02 OF CONSTRUCTION SPECIFICATIONS FOR GENERAL WIND BRACING INFORMATION.

HEADER SCHEDULE

- H1 SINGLE 2X4 TURNED FLAT (A)
- H2 (2) 2X4'S ON SINGLE JACKS (B)
- H3 (2) 2X10'S ON SINGLE JACKS (C)
- H4 (2) 1.75" X 9.25" LVL'S ON DBL JACKS
- H5 (3) 2X10'S ON SINGLE JACKS
- (A) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPENING 38" MAX.
- (B) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPNG 38" TO 74" MAX.
- (C) TYPICAL FOR ALL CONDITIONS NOT LISTED IN (A) OR (B) UNO.

-HEADERS IN NON LOAD BEARING INTERIOR WALLS ARE NOT LABELED.

1ST FLOOR FRAMING PLAN

WALLS AND CEILING $1/8^{\circ} = 1'-0"$

the

at i

oroduced,

repr

<u></u>

one time a

re for uns if Tech

These plans y for these p m Engineering

s, P.A. Ti liability on from

Tech Associates,

Engineering 7

ty of Enginee ech Associate Iocation with

property o ering Tech

umes no liat permission

written

without

eľ

ing othe

is the p Engineeri at any c

of this plan client listed. E

design or the c art, for

for

The struc location whole or

and in p

structural

assumes

hese plans neering Tech

the

are

plans

STRUCTURAL ENGINEERS
License No. C-3870
318 W Milbrook Rd, Suite 201
Raleigh, North Carolina 27609
A. Phone (919) 844-1661

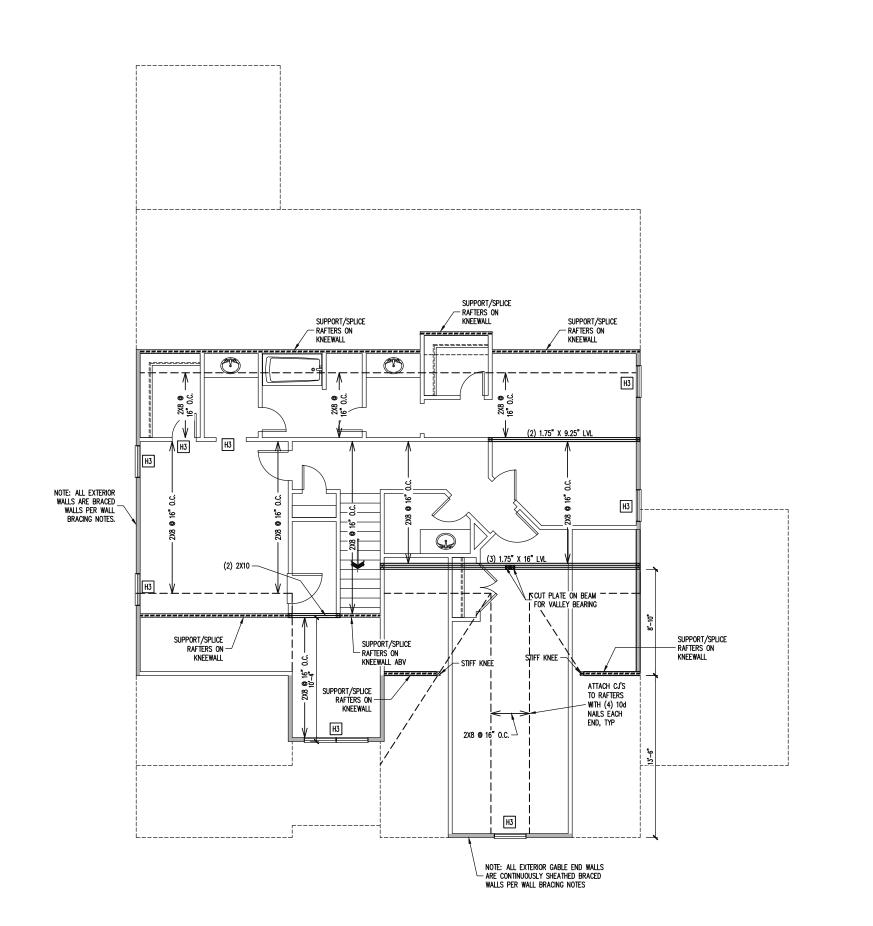
TRIPLE A HOMES RUCTURAL ADDENDUM

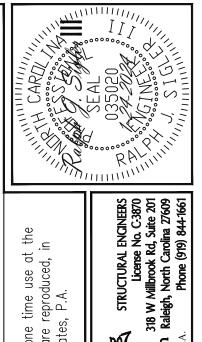
ENG: RJS DATE: 1-24-2024

PROJECT NO. 24-28-005

SHEET NO.

S3 3 of 8





the

WALL BRACING

SHADED WALLS:

ALL EXTERIOR STUD WALLS, EXTERIOR SIDE, ARE TO BE CONTINUOUSLY SHEATHED WITH 7/16 APA RATED OSB NAILED TO STUDS WITH 8d NAILS @ 6" O.C. AT PANEL EDGES, 12" O.C. IN PANEL FIELD.

PROVIDED CONTINUOUS SHEATHING = 71' MIN.

REFERENCE PART 16.02 OF CONSTRUCTION SPECIFICATIONS FOR GENERAL WIND BRACING INFORMATION.

HEADER SCHEDULE

- H1 SINGLE 2X4 TURNED FLAT (A)
- H2 (2) 2X4'S ON SINGLE JACKS (B)
- H3 (2) 2X10'S ON SINGLE JACKS (C)
- H4 (2) 1.75" X 9.25" LVL'S ON DBL JACKS
- H5 (3) 2X10'S ON SINGLE JACKS
- (A) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPENING 38" MAX.
- (B) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPNG 38" TO 74" MAX.
- (C) TYPICAL FOR ALL CONDITIONS NOT LISTED IN (A) OR (B) UNO.

-HEADERS IN NON LOAD BEARING INTERIOR WALLS ARE NOT LABELED.

2ND FLOOR FRAMING PLAN WALLS AND CEILING

 $1/8^{\circ} = 1'-0''$

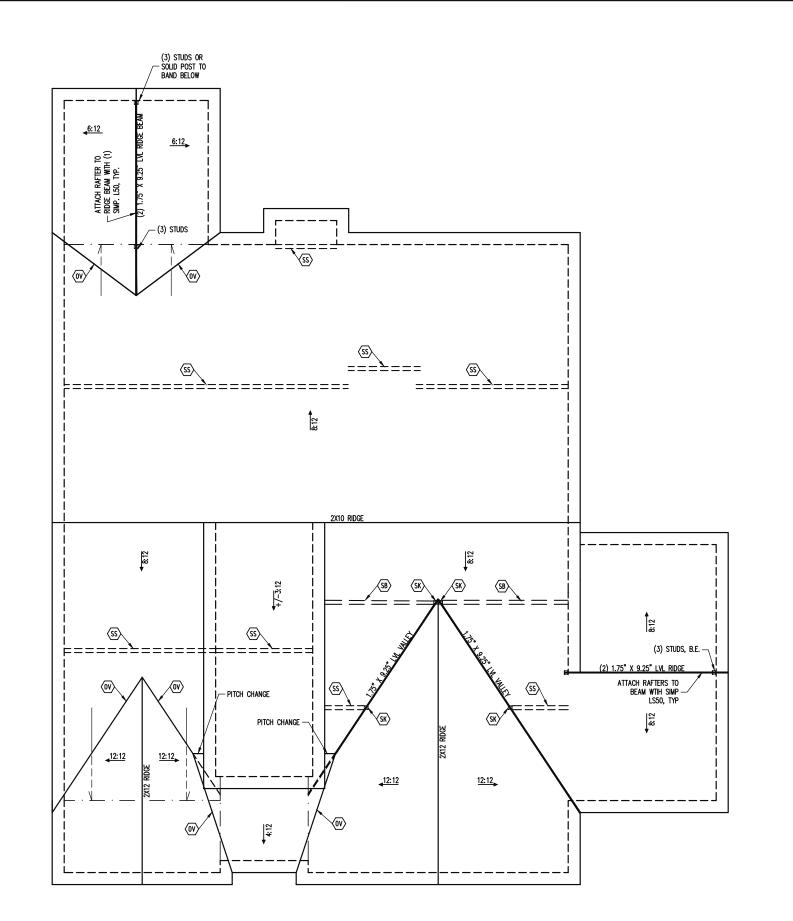
or the one time use at t f they are reproduced, in Associates, P.A. ngineering . These plans are for tl ty for these plans if th m Engineering Tech Ass s, P.A. Ti liability on from umes no liat permission g Tech Associates, F P.A. assumes no lia written permission is the property of Engineering Engineering Tech Associates, P. at any other location without of this plan i client listed. E construction o The structural design c location and for the cl whole or in part, for c PROJECT NO.

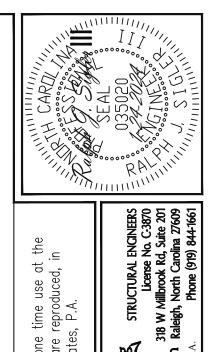
NES	WDQN:					
TRIPLE A HOMES	SCOPE STRUCTURAL ADDENDUM	TOC 4 COLLON FARMS				
\equiv			_			
EN			R	J:	<u>S</u>	
DA.	TE:	1-2	4	-2	202	24

24-28-005 SHEET NO.

S4

4 of 8





ngineering

TRIPLE A HOMES STRUCTURAL ADDENDUM

the

is the property of Engineering Tech Associates, P.A. These plans are for the one time use at Engineering Tech Associates, P.A. assumes no liability for these plans if they are reproduced, in at any other location without written permission from Engineering Tech Associates, P.A.

n of this plan i client listed. E

The structural design c location and for the cl whole or in part, for c

FRAMING NOTES

ROOF ONLY

-COMMON RAFTERS 2X8 © 16" O.C. TYP U.N.O.

-CULAR TIES 2X4 EVERY 3RD SET OF RAFTERS
TYP U.N.O.

-VERIEV ALL PARTIES

- Verify all kneewall heights, Architectural overhangs, and roof pitches Prior to construction

- OVERFRAME VALLEY (2X10 SLEEPER)
- SUPPORT/SPLICE RAFTERS ON KNEEWALL BELOW

- DBL 2X4 STIFF KNEE
- SUPPORT/SPLICE RAFTERS ON BEAM BELOW

ENG: RJS DATE: 1-24-2024

SCOPE LOC

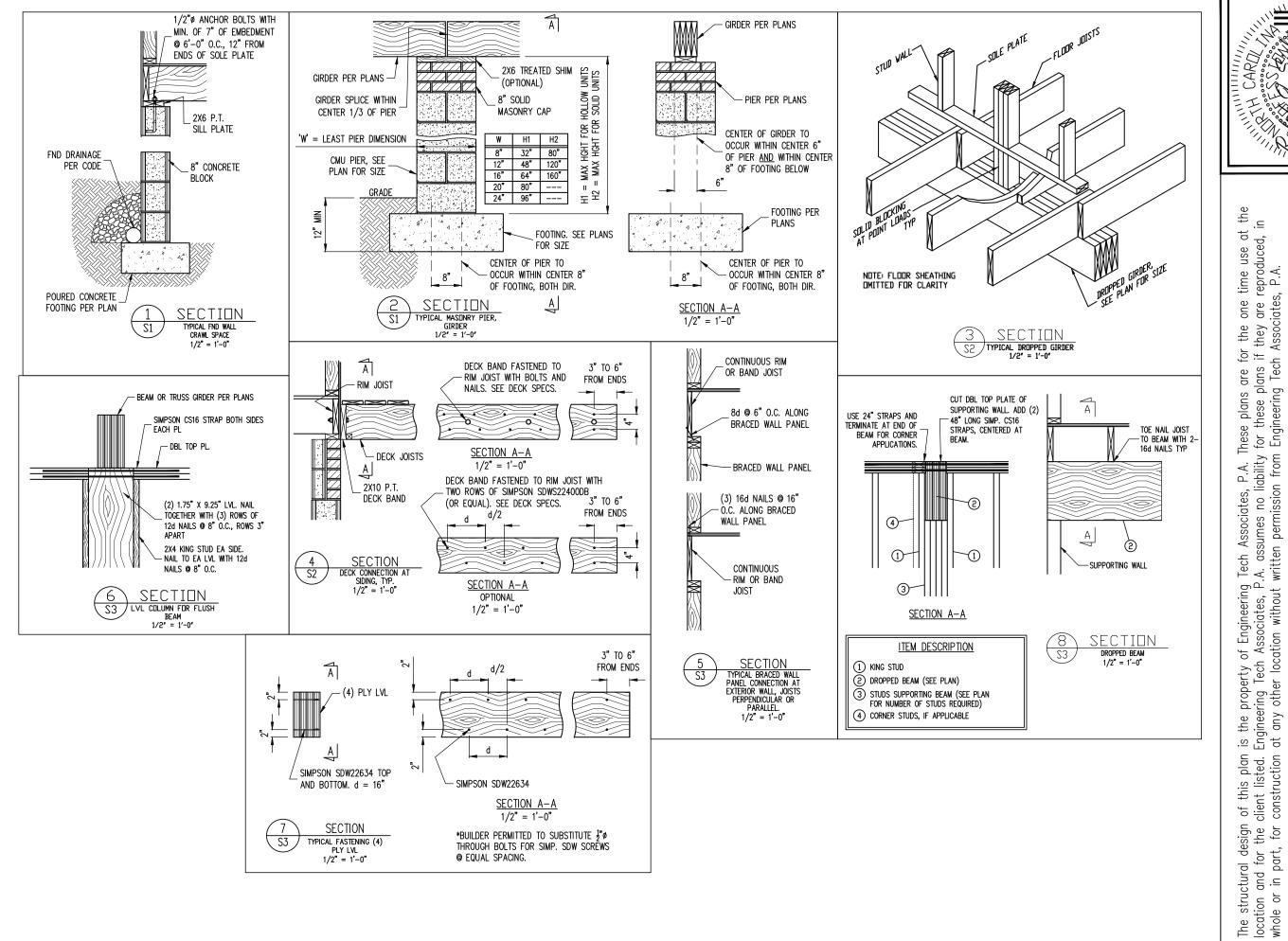
COTTON FARMS

4

PROJECT NO. 24-28-005

> SHEET NO. **S5** 5 of 8

ROOF FRAMING PLAN 1/8" = 1'-0"



CARALLINIA CARALLINIA

ssociates, P.A. assumes no liability for these plans if they are reproduced, in on without written permission from Engineering Tech Associates, P.A.

TRIPLE A HOMES

STRUCTURAL ADDENDUM

COTTON FARMS

License No. C-3870

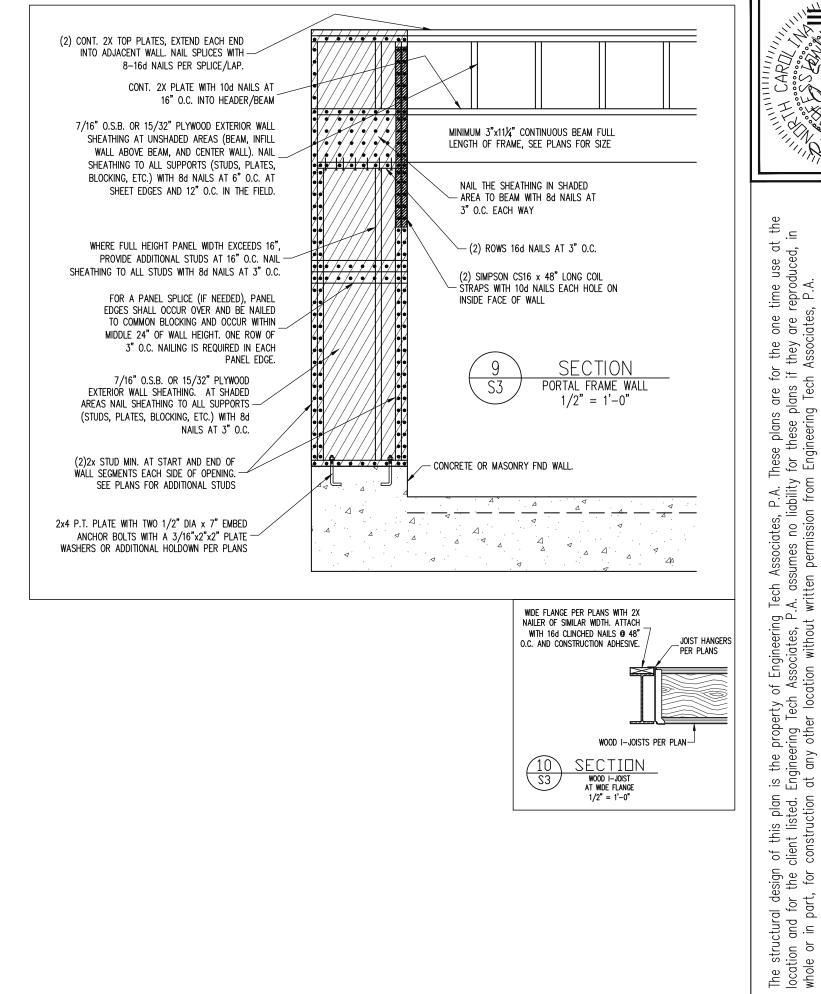
3.18 W Millbrook Rd, Suire 201

ASSOCIATES, P.A. Phone (919) 844-1661

ENG: RJS DATE: 1-24-2024

PROJECT NO. 24-28-005

SHEET NO.
SD1
6 of 8



ne time use at the re reproduced, in ites, P.A.

STRUCTURAL ENGINEERS

License No. C.3870

318 W Millbrook Rd, Suite 201

Raleigh, North Carolina 27609

A. Phone (919) 844-1661

umes no liat permission TRIPLE A HOMES RUCTURAL ADDENDUM ⁵.A. assumes written permi COTTON 7 eľ ring othe ENG: RJS DATE: 1-24-2024 PROJECT NO.

SHEET NO.

SD1

7 of 8

CONSTRUCTION SPECIFICATIONS PART 1: GENERAL fM = 1.500 PSI MINCONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION. 1.02 DIMENSIONS SHOWN SHALL GOVERN OVER SCALE ON THESE DRAWINGS.

LIVE LOAD (PSF) DEAD LOAD (PSF)

1.05 METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR, WHO SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND INSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.

7.04

PART 2: DESIGN LOADS

2.01 DESIGN LOADS SHALL CONFORM WITH THE TABLE BELOW:

BALCONIES, DECKS, ATTICS WITH FIXED STAIR ACCESS, DWELLING UNITS INCLUDING ATTICS WITH FIXED STAIR ACCESS, STAIRS, FIRE ESCAPES	40	10
GARAGES (PASSENGER CARS ONLY)	50	
ATTICS (NO STORAGE, LESS THAN 5' HEADROOM)	10	10
ATTICS (WITH STORAGE)	20	10
ROOF	20	10 (15 FOR VAULTS)

NOTES: — INDIVIDUAL STAIR TREADS ARE TO BE DESIGNED FOR THE UNIFORMLY DISTRIBUTED LIVE LOAD OF 40 PSF OR A 300 LB. CONCENTRATED LOAD ACTING OVER AN AREA OF 4 SQ. WHICHEVER PRODUCES THE GREATER STRESS.

BUILDER TO VERIFY DEAD LOAD DOES NOT EXCEED 10 PSF WHEN HEAVY FLOOR OR ROOF FINISHES SUCH AS TILE OR SLATE ARE UTILIZED. NOTIFY ENGINEERING UNDER

2.02 INTERIOR WALLS: 5 PSF LATERAL.

2.03 BASIC WIND DESIGN VELOCITY OF 120 MPH.

2.04 SOIL BEARING CAPACITY 2000 PSF (PRESUMPTIVE).

PART 3: STRUCTURAL STEEL

3.01 WIDE_FLANGE BEAMS AND TEE SECTIONS SHALL CONFORM TO ASTM A992 MINIMUM

3.02 SQUARE AND RECTANGULAR TUBING SHALL CONFORM TO ASTM A500 GRADE B MINIMUM GRADE.

3.03 STEEL PIPE SHALL CONFORM TO ASTM A53 GRADE B. TYPE S. MINIMUM GRADE 3.04 ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 MINIMUM GRADE

STRUCTURAL STEEL CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE AISC 3.05 SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.

PART 4: WELDING

4.01 WELDING ELECTRODES SHALL BE E70XX AND ALL WELDING SHALL BE PERFORMED BY AN AWS CERTIFIED WELDER

PART 5: CONCRETE AND SLABS ON GRADE

CAST IN PLACE CONCRETE SHALL BE OF NORMAL WEIGHT, 4-6% AIR ENTRAINMENT, FOR EXTERIOR CONCRETE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS TYP UNO. ALL ITEMS NOTED AS 'CONCRETE' ARE TO BE CAST IN PLACE,

5.02 REINFORCED CAST IN PLACE CONCRETE SHALL BE PROPORTIONED, MIXED AND PLACED IN ACCORDANCE WITH THE SPECIFICATIONS OF ACI 318, LATEST EDITION.

5.03 SLABS ON GRADE, IF ANY, SHALL BE CAST IN PLACE, CONTAIN SYNTHETIC POLYPROPYLENE FIBRILLATED MICRO FIBERS, FIBER LENGTH 1 1/2", DOSAGE RATE 1 1/2 LBS/CU YD. SLAB TO BE PLACED ON A 6 MIL VAPOR BARRIER ON 4" MIN GRANULAR FILL ON SOIL WITH 90% MIN STANDARD PROCTOR DENSITY. VAPOR BARRIER MAY BE OMITTED FOR SLABS NOT IN ENCLOSED AREAS

PART 6: REBAR AND WIRE REINFORCEMENT

REBAR SHALL BE DEFORMED STEEL CONFORMING TO ASTM A615 GRADE 60 TYP UNO 6.01

LAP SPLICES SHALL BE CLASS B AS DEFINED BY ACI 318, TYP UNO

6.03 WIRE REINFORCEMENT SHALL BE 9 GA AND SHALL CONFORM TO ASTM A1064.

PART 7: MASONRY

7.01 CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 AND C55, NORMAL WEIGHT,

7.02 CLAY MASONRY UNITS SHALL CONFORM TO ASTM C62-17 GRADE SW

MORTAR SHALL BE TYPE S. MORTAR AND GROUT SHALL CONFORM TO ASTM C476, MIN COMPRESSIVE STRENGTH OF 2000 PSI.

MASONRY CONSTRUCTION SHALL CONFORM TO THE SPECIFICATIONS OF ACI 530

LADDER WIRE REINFORCEMENT SHALL CONFORM TO ASTM A951. 6" MIN LAPS FOR CONTINUOUS WALL APPLICATIONS

PART 8: BOLTS AND LAG SCREWS

BOLTS SHALL CONFORM TO ASTM A307 MINIMUM GRADE TYP UNO. INSTALL STANDARD STEEL WASHERS (ASTM F844-07a) FOR THE NUT / BOLT HEAD WHEN BOLTING WOOD MEMBERS. HOLES FOR BOLTS SHALL BE AISC STANDARD HOLES UNO

8.02 LAG SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1-1981. PILOT HOLES SHALL BE USED FOR LAG SCREW INSTALLATION AND SHALL BE BORED ACCORDING TO NDS SPECIFICATIONS. INSTALL STANDARD STEEL WASHERS (ASTM F844-070) FOR

8.03 ANCHOR RODS AND BOLTS SHALL CONFORM TO ASTM F1554-15 GRADE 36 UNO. BENT ANCHOR BOLTS SHALL HAVE A 2" MIN HOOK UNO

PART 9: DRIVEN FASTENERS

NAILS, SPIKES AND STAPLES SHALL CONFORM TO ASTM F 1667- 05. NAILS ARE TO BE COMMON WIRE OR BOX

PART 10: DIMENSIONAL LUMBER

10.01 Solid sawn wood framing design is based on no. 2 spruce pine fir \underline{or} syp #2 for joists, rafters, girders, beams, studs, etc.

PART 11: ENGINEERED LUMBER

LVL OR PSL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS: E= 1.9 X 10E6 PSI, Fb = 2600 PSI, Fv = 285 PSI, Fc = 750 PSI LSL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS: E= 1.3 X 10E6 PSI, Fb = 1700 PSI, Fv = 400 PSI, Fc = 680 PSI

LVL OR PSL MEMBERS MAY BE RIPPED FROM DEEPER MEMBERS TO MATCH THE MEMBER DEPTH SPECIFIED IN THE PLANS

PART 12: PRESSURE TREATED LUMBER

LUMBER IN CONTACT WITH THE GROUND, CONCRETE OR MASONRY SHALL BE PRESSURE TREATED IN ACCORDANCE WITH AWPA STANDARD C-15. ALL OTHER EXPOSED LUMBER SHALL BE TREATED IN ACCORDANCE WITH AWPA STANDARD C-2 OR BY ANY METHOD GIVING EQUAL PROTECTION. THE BUILDING CODE OFFICE MAY ALSO APPROVE A NATURAL DECAY RESISTANT WOOD PER SECTION 19-6(A)

PART 13: STEEL FLITCH PLATE BEAMS

FLITCH PLATE BEAMS SHALL CONSIST OF A CONTINUOUS STEEL PLATE BOLTED BETWEEN TWO PIECES OF CONTINUOUS LUMBER AS SIZED ON THE PLANS. BOLT PIECES TOGETHER USING 1/2" @ BOLTS SPACED AT 16" O.C. STAGGERED TOP TO BOTTOM OF THE BEAM. 13.01 MAINTAIN A 2" EDGE DISTANCE. PLACE TWO BOLTS, ONE ABOVE THE OTHER, 16" MAX FROM EACH END OF THE BEAM. TYP UNO

PART 14: STUD SUPPORTS FOR BEAMS

14.01 STEEL, ENGINEERED LUMBER, AND FLITCH PLATE BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS:

1-WHEN THE REAM IS PERPENDICULAR TO OR SKEWED RELATIVE TO THE WALL. THE REAM SHALL BEAR SUL WITH ON THE SUPPORTING WALL INDICATED AND SHALL BE SUPPORTED BY A MINIMUM OF THREE GANGED STUDS, OR A GANGED STUD COLUMN WITH A NUMBER OF STUDS SUCH THAT THE STUD COLUMN IS AT LEAST AS WIDE AS THE TRUE WIDTH OF THE BEAM BEING SUPPORTED, WHICHEVER IS GREATER, TYP UND. FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLUMN IS CENTERED ON THE BEAM

2-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR A MINIMUM OF 4 1/2" ONTO THE WALL AND BE SUPPORTED BY A TRPL STUD GANGED

4.02 DIMENSIONAL LUMBER BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS:

1-when the beam is perpendicular to, or skewed relative to the wall, the beam shall bear <u>full width</u> on the supporting wall indicated (less 1 1/2" to allow for a continuous rim joist where applicable) and shall be supported by a GANGED STUD COLUMN THE SAME WIDTH AS THE BEAM TYP UNO. (E.G. A TRIPLE 2X10 IS TO BE SUPPORTED BY (3) STUDS), FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLUMN IS CENTERED ON THE BEAM 2-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR

MINIMUM OF 3" ONTO THE WALL AND BE SUPPORTED BY A DBL STUD GANGED COLUMN

NOTES

THE BUILDER IS RESPONSIBLE FOR REVIEWING PLANS PRIOR TO CONSTRUCTION. THE BUILDER

SHALL IMMEDIATELY CONTACT THE ENGINEER OF RECORD (FOR) REFORE PROCEEDING IF THE

ANY ERRORS DUE TO A FAILURE TO FOLLOW THE ABOVE PROCEDURES SHALL NOT BE THE

ENSURE THAN ANY REVISIONS ISSUED BY THE EOR ARE PROMPLY DISTRIBUTED TO THE

THE EOR DOES NOT PERFORM FENESTRATION OR VENTING CALCULATIONS OR ANY OTHER

ROOF AND FLOOR TRUSSES TO BE DESIGNED BY AN ENGINEER REGISTERED BY THE STATE, FINAL

CALCULATIONS THAT ARE NOT DIRECTLY RELATED TO STRUCTURAL ENGINEERING.

RESPONSIBILITY OF THE EOR. FURTHERMORE, IT IS THE RESPONSIBILITY OF THE BUILDER TO

FOLLOWING CONDITIONS ARE NOTED BEFORE OR DURING CONSTRUCTION:

THE WORKING PLANS DO NOT BEAR THE SEAL OF THE EOR

2) THE PLANS CONTAIN DISCREPANT OR INCOMPLETE INFORMATION

TRUSS DRAWING SHOULD BE SUBMITTED TO THE EOR FOR REVIEW

14.03 EXTRA JOISTS BEARING ON A STUD WALL PERPENDICULAR TO OR SKEWED RELATIVE TO THE BEAM SHALL BE SUPPORTED BY ONE ADDITIONAL STUD.

STUDS THAT ARE CANCED TO FORM A COLLIMN SHALL HAVE ADJACENT STUDS WITHIN STUDIS THAT ARE GANGED TO FORM A COLUMN SHALL HAVE ADJACENT STUDS MITHIN THE COLUMN NALLED TOCETHER WITH ONE ROW OF 10d NAILS AT 8" O.C., (TWO ROWS OF 10d NAILS @ 8" O.C., 3" APART, FOR 2X8 OR 2X10 STUDS) ALL COLUMNS SHALL BE CONTINUOUS DOWN TO THE FOUNDATION OR OTHER PROPERLY DESIGNED STRUCTURAL ELEMENT SUCH AS A BEAM. COLUMNS TRANSFERRING LOADS THROUGH FLOOR LEYELS SHALL BE SOLIDLY BLOCKED FOR THE FULL WIDTH OF THE STUD COLUMN MATCH THE CANTY CONTINUES BY THE STUD COLUMN. THE CAVITY FORMED BY THE

PART 15: NAILING OF MULTI PLY WOOD BEAMS

SOLID SAWN LUMBER JOISTS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM NAILED TOGETHER WITH THREE ROWS OF 10d NAILS 15.01 9 16" O.C. FOR 2X10 OR LARGER, TWO ROWS OF 10d NAILS @ 16" O.C. FOR 2X8, ONE ROW OF 10d NAILS @ 16" O.C. FOR 2X6 OR SMALLER, STAGGER ROWS 5" MIN.

LVL MEMBERS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM FASTENED TOGETHER PER MANUFACTURERS RECOMMENDATIONS, TYP 15.02

PART 16: WALL FRAMING AND BRACING

STUD WALLS SHALL CONSIST OF 2X4 STUDS SPACED AT 16" O.C. UNO. STUDS SHALL BE CONTINUOUS FROM SOLE PLATE AT FLOOR TO DOUBLE TOP PLATE AT THE CEILING OR ROOF. NO INTERMEDIATE BANDS OR PLATES SHALL CAUSE DISCONTINUITES IN A STUD WALL EXCEPT AS REQUIRED FOR DOOR OR WINDOW OPENINGS. THE KING STUDS FOR SUCH OPENINGS SHALL BE CONTINUOUS, TYP UNO.

MAX ALLOWABLE WALL HEIGHTS FOR EXTERIOR STUD WALLS, INCLUSIVE OF SOLE IN ATTEMAND DOIL TO BE ATTEMAND. THE CONTINUOUS OF STUD WALLS, INCLUSIVE OF SOLE IN A TEMAND DOIL TO BE ATTEMAND. THE CONTINUOUS OF SOLE IN A TEMAND DOIL TO BE ATTEMAND. 16.01

MAX ALLOWABLE WALL HEIGHTS FOR EXTENDOR STOLD WALLS, INCLUSIVE OF SOLE PLATE AND DEL TOP PLATE AND 7/16" OSB EXTERIOR BRACING AND ROW OF 2X4 2X6 PURLINS AT 8" HEIGHT (AND AT 16" HEIGHT FOR TALL WALLS), TYP UNO: 2X4 @ 16" O.C.: 11"-11/2" 2X6 @ 16" O.C.: 17"-0" 2X4 @ 12" O.C.: 12"-11/2" 2X6 @ 16" O.C.: 18"-8" DBL 2X4 @ 16" O.C.: 13"-4" DBL 2X6 @ 16" O.C.: 21"-0"

16.02 FOR WALL BRACING THE FOLLOWING SHALL APPLY:

-BLOCKING AT UNSUPPORTED PANEL EDGES IS REQUIRED TYP UNO.

-WALL BRACING IS BY ENGINEERED DESIGN AND NOT PRESCRIPTIVE PER SECTION
602.10 OF THE 2018 NCRC. CONTINUOUS SHEATHING HAS BEEN PROVIDED, ALONG
WITH ALTERNATIVE METHODS TO INSURE THE MINIMUM INTENT OF SECTION 602.10
OF THE 2018 NCRC HAS BEEN MET AND EXCEEDED.

-BRACED WALL PANELS SHALL BE FASTENED IN ACCORDANCE WITH TABLE 602.3(1) TO
EXPANDE CONTINUOUS PANEL INJURIE TO SECTANCE AND CAMBILIANCE WITH MCRED.

PROVIDE CONTINUOUS PANEL UPLIFT RESISTANCE AND COMPLIANCE WITH NCRBC R602.3.5 AND R802.11 UNLESS NOTED OTHERWISE ON STRUCTURAL PLANS.

-MAY SUBSTITUTE WSP FOR GB
-SINGLE JOIST, CONTINUOUS RIM JOIST, OR BLOCKING OF EQUAL DEPTH IS REQUIRED
ABOVE AND BELOW ALL BRACED WALLS. NAIL BLOCKING ABOVE WALL TO TOP PLATE
WITH 16d TOE NAILS @ 6" O.C. NAIL SOLE PLATE OF BRACED WALL TO BLOCKING BELOW WITH (3) 16d NAILS @ 16" O.C. BLOCKING AT HORIZONTAL JOINTS IN BRACED WALL LINES ONLY REQUIRED AT SHADED WALLS, UNO.

17.01 KING STUDS FOR OPENINGS IN EXTERIOR WALLS SHALL BE AS FOLLOWS:

			NUMBE	r of Kin	ig studs	
MAX OPENING	3 WIDTH	5'-0"	9'-0"	13'-0"	17'-0"	21'-0'
STUD SIZE	2X4 2X6 2X8	1 1 1	2 1 1	3 2 1	4 2 1	5 2 2

PART 18: SUBSTITUTIONS

ARV AROVE

RTWN RFTWFFN

CONC

CS

DBL DOUBLE

DSD

L PL

EQ

BOTH

BOTH ENDS

CONCRETE

DIAMETER

EQUAL

EA EACH FLG FLANGE

FLR FLOOR

DOUBLE JOIST

FLITCH PLATE

DBL STUD POCKET

CAST IN PLACE

CONTINUOUS SHEATHING

18.01 MATERIAL OR MEMBER SIZE SUBSTITUTIONS OR PLAN DEVIATIONS REQUIRE THE WRITTEN AUTHORIZATION OF THE DESIGNERS. UNAUTHORIZED DEVIATIONS ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

PART 19: OWNERSHIP OF STRUCTURAL DESIGN

THE STRUCTURAL DESIGN OF THIS PLAN IS THE PROPERTY OF ENGINEERING TECH 19.01 ASSOCIATES (ETA). THESE PLANS ARE FOR THE ONE TIME USE AT THE LOCATION INDICATED AND FOR THE CLIENT LISTED. ETA ASSUMES NO LIABILITY FOR THESE PLANS IF THEY ARE REPRODUCED, IN WHOLE OR IN PART, FOR CONSTRUCTION AT ANY OTHER LOCATION WITHOUT WRITTEN PERMISSION FROM ETA

ABBREVIATIONS

TJ TRIPLE JOIST

TYPICAL

UNO UNLESS NOTED

XJ EXTRA JOIST

OTHERWISE

TRPL TRIPLE TSP TRIPLE STUD POCKET

FND FOUNDATION

HDG HOT DIPPED

GAI VANIZED HGR HANGER

LVL LAMINATED VENEER

PSL PARALLEL STRAND

PT PRESSURE TREATED

SP SPACE (OR SPACING

SSP SINGLE STUD POCKET

LUMBER

NTS NOT TO SCALE

LUMBER

QJ QUAD JOIST

SQ SQUARE

O.C. ON CENTER

FTG FOOTING

DECK SPECIFICATIONS

A DECK IS AN EXPOSED EXTERIOR WOOD FLOOR STRUCTURE WHICH MAY BE ATTACHED TO A STRUCTURE OR BE FREE STANDING. ROOFED PORCHES, OPEN OR SCREENED IN, MAY BE CONSTRUCTED LISING THESE PROVISIONS

SUPPORT POSTS SHALL BE SUPPORTED BY A FOOTING.

WHEN ATTACHED TO A STRUCTURE, THE STRUCTURE TO WHICH ATTACHED SHALL HAVE A TREATED WOOD BAND FOR THE LENGTH OF THE DECK, OR CORROSION RESISTANT FLASHING SHALL BE USED TO PREVENT MOISTURE FROM COMING IN CONTACT WITH THE UNTREATED FRAMING OF THE STRUCTURE. THE DECK BAND AND THE STRUCTURE BAND SHALL BE CONSTRUCTED IN CONTACT WITH FACH OTHER EXCEPT AT BRICK VENEER AND WHERE PLYWOOD SHEATHING IS REQUIRED AND PROPERLY FLASHED. SIDING SHALL NOT BE INSTALLED BETWEEN THE STRUCTURE AND THE DECK BAND. IF ATTACHED TO A BRICK STRUCTURE, NEITHER FLASHING NOR A TREATED BAND FOR THE BRICK STRUCTURE IS REQUIRED. IN ADDITION, THE TREATED DECK BAND SHALL BE CONSTRUCTED IN CONTACT

WHEN THE DECK IS SUPPORTED AT THE STRUCTURE BY ATTACHING THE DECK TO THE STRUCTURE. THE FOLLOWING ATTACHMENT SCHEDULES SHALL APPLY FOR ATTACHING THE

A. ALL STRUCTURES EXCEPT BRICK STRUCTURES

	JOIST LENGTH		
	UP TO 8' MAX.	UP TO 16' MAX.	
REQUIRED FASTENERS	ONE- 5/8" Ø BOLT @ 42" O.C. AND (2) ROWS OF 12d NAILS @ 8" O.C. OR TWO ROWS OF SIMPSON SDWS22400DB @ d = 32" O.C. STAGGERED	ONE- 5/8" Ø BOLT @ 20" O.C. AND (3) ROWS OF 12d NAILS @ 6" O.C. OR TWO ROWS OF SIMPSON SDWS22400DB @ d = 16" O.C. STAGGERED	

A . BRICK VENEER STRUCTURES

UP TO 8' MAX.	UP TO 16' MAX.
REQUIRED ONE— 5/8" Ø BOLT @ 28" O.C.	ONE- 5/8" Ø BOLT @ 16" O.C.

5. IF THE DECK BAND IS SUPPORTED BY A 1/2" MINIMUM MASONRY LEDGE ALONG THE FOUNDATION WALL, 5/8" Ø BOLTS SPACED @ 48" O.C. MAY BE USED FOR SUPPORT.

OTHER MEANS OF SUPPORT, SUCH AS JOIST HANGERS, MAY BE USED TO CONNECT DECK JOISTS TO A TREATED STRUCTURE BAND

GIRDERS SHALL BEAR DIRECTLY ON POSTS OR BE BE CONNECTED TO THE SIDES OF POSTS

FLOOR DECKING SHALL BE NO. 2 GRADE TREATED SOUTHERN PINE OR EQUIVALENT. THE MINIMUM FLOOR DECKING THICKNESS SHALL BE AS FOLLOWS:

JOIST SPAN	DECKING
12" O.C.	1" S4S
16" O.C.	1" T&G
24" O.C.	1 1/4" S4S
32" O.C.	2" S4S

MAXIMUM HEIGHT OF DECK SUPPORT POSTS IS AS FOLLOWS:

POST SIZE	MAX POST HEIGHT
4X4	8'
6X6	20'
ENGINEERED	20' +

NOTES: 1) THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS. 2) THIS TABLE IS BASED ON A MAXIMUM TRIBUTARY AREA OF 128 SQ. FT. 3) POST HEIGHT IS FROM TOP OF FOOTING TO BOTTOM OF GIRDER.

10. DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF THE FOLLOWING

A. WHEN THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION 4, LATERAL BRACING IS NOT REQUIRED.

B. 4X4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 45" AND 60" FROM THE HORIZONTAL, KNEE BRACES SHALL BE ATTACHED A THE ENDS TO THE GIRDER AND THE POST WITH ONE - 5/8" BOLT

C. FOR FREE STANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN CONCRETE IN ACCORDANCE

POST SIZE	TRIBUT. AREA	POST HEIGHT	EMB. DEPTH	CONC. DIAM.
4X4	48 SQ. FT.	4'-0"	2'-6"	1'-0"
6X6	120 SQ. FT.	6'-0"	3'-6"	1'-8"

D. 2X6 DIAGONAL VERTICAL CROSS BRACING SHALL BE PROVIDED IN TWO PERPENDICULAR DIRECTIONS FOR FREE STANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE BRACES SHALL BE ATTACHED TO THE POSTS WITH ONE -5/8" ϕ BOLT AT EACH END OF THE BRACE.

NOTES: 1) ALL NAILS AND BOLTS ARE TO BE HOT DIPPED GALVANIZED.

2) MINIMUM EDGE DISTANCE FOR BOLTS IS 2 1/2".

3) nails must penetrate the supporting structure band a minimum of 1 1/2".

무 at roduced, .A. nse time repr one are Ö the Asso for Tech plans are g plans These p y for the Engin s, P.A. T liability from umes no liat permission Associates, samuss written Tech Ä Ф<u>Ф</u>. out with Engin io. of locati roperty c ng Tech ing pr igineer t any the Ë. at plan sted. of this pl client liste constructi design or the art, for for structural and .⊑ tion e or

ADDENDUM UCTURAL RIPLE \triangleright E١ DA

NC:	RJS
NTE:	1-24-2024

PROJECT NO. 24-28-005

SPECS

ngli

SEAL OSSOCI

급

STRUCTURAL ENGINEERS
License No. C-3870
8 W Millbrook Rd, Suite 201
aleigh, North Carolina 27609
Phone (919) 844-1661

0

SHEET NO.

8 of 8