

Job	Truss	Truss Type	Qty	Ply	3202 Leaflet Church-Roof-Abby GRH	164834839
24020093-01	B1GR	Hip Girder	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

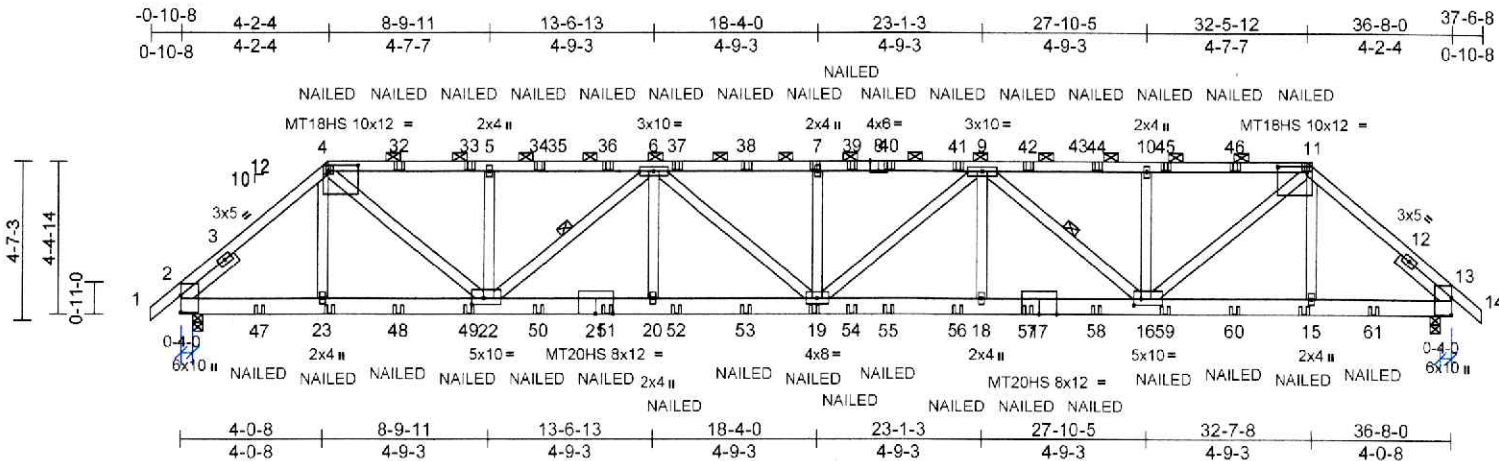
Run: 8:73 S Mar 21 2024 Print: 8:730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:35:55

Page: 1

ID:CCXjv5bmLobJuf_ohZrCvXzaLPE-RfC?PsB70Hq3NSgPqnl8w3uTXbCKWrcDol7J4zJC7f

CANTILEVER EACH END 4'

NO REPAIR REQUIRED



Scale = 1/66.3

Plate Offsets (X, Y): [4:0-10-4,0-2-0], [8:0-3-0,Edge], [11:0-10-4,0-2-0], [16:0-2-8,0-2-4], [22:0-4-0,0-2-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.84	Vert(LL)	-0.41	19	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.54	Vert(CT)	-0.63	19	>701	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	NO	WB	0.80	Horz(CT)	0.12	13	n/a	n/a	MT18HS	244/190
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 243 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.1 *Except* 4-8,8-11:2x4 SP 2400F 2.0E
BOT CHORD 2x6 SP 2400F 2.0E
WEBS 2x4 SP No.3 *Except* 22-4,22-6,19-6,19-9,16-9,16-11:2x4 SP No.2
SLIDER Left 2x4 SP No.3 -- 2-0-0, Right 2x4 SP No.3 -- 2-0-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-15 oc purlins, except 2-0-0 oc purlins (2-4-15 max.): 4-11.
BOT CHORD Rigid ceiling directly applied or 7-0-10 oc bracing.
WEBS 1 Row at midpt 6-22, 9-16

REACTIONS (size) 2=0-3-8, 13=0-3-8
Max Horiz 2=100 (LC 11)
Max Uplift 2=-969 (LC 12), 13=-967 (LC 13)
Max Grav 2=3016 (LC 19), 13=3025 (LC 20)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-4=-3787/1247, 4-5=-5300/1733, 5-6=-5300/1733, 6-7=-7268/2397, 7-9=-7268/2397, 9-10=-5317/1739, 10-11=-5317/1739, 11-13=-3799/1244, 13-14=0/59
BOT CHORD 2-23=-946/2837, 22-23=-948/2840, 20-22=-2218/6737, 19-20=-2218/6737, 18-19=-2215/6777, 16-18=-2215/6777, 15-16=-877/2850, 13-15=-873/2846

WEBS 4-23=-102/119, 4-22=-1087/3265, 5-22=-755/307, 6-22=-1914/661, 6-20=0/282, 6-19=-287/727, 7-19=-736/338, 9-19=-269/676, 9-18=0/295, 9-16=-1943/675, 10-16=-743/301, 11-16=-1093/3275, 11-15=-104/119

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at Jt(s) 2 and 13. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.



Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss B1GR	Truss Type Hip Girder	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	I64834839
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Carter Components (Sanford, NC), Sanford, NC - 27332

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Page: 2

14) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (lb/ft)

Vert: 1-4=-60, 4-11=-60, 11-14=-60, 24-28=-20

Concentrated Loads (lb)

Vert: 23=-20 (F), 4=-143 (F), 19=-20 (F), 7=-143 (F),
11=-143 (F), 15=-20 (F), 32=-143 (F), 33=-143 (F),
34=-143 (F), 36=-143 (F), 37=-143 (F), 38=-143 (F),
39=-143 (F), 40=-143 (F), 41=-143 (F), 42=-143 (F),
44=-143 (F), 45=-143 (F), 46=-143 (F), 47=-181 (F),
48=-20 (F), 49=-20 (F), 50=-20 (F), 51=-20 (F),
52=-20 (F), 53=-20 (F), 54=-20 (F), 55=-20 (F),
56=-20 (F), 57=-20 (F), 58=-20 (F), 59=-20 (F),
60=-20 (F), 61=-181 (F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbccomponents.com)



818 Soundside Road
Edenton, NC 27932

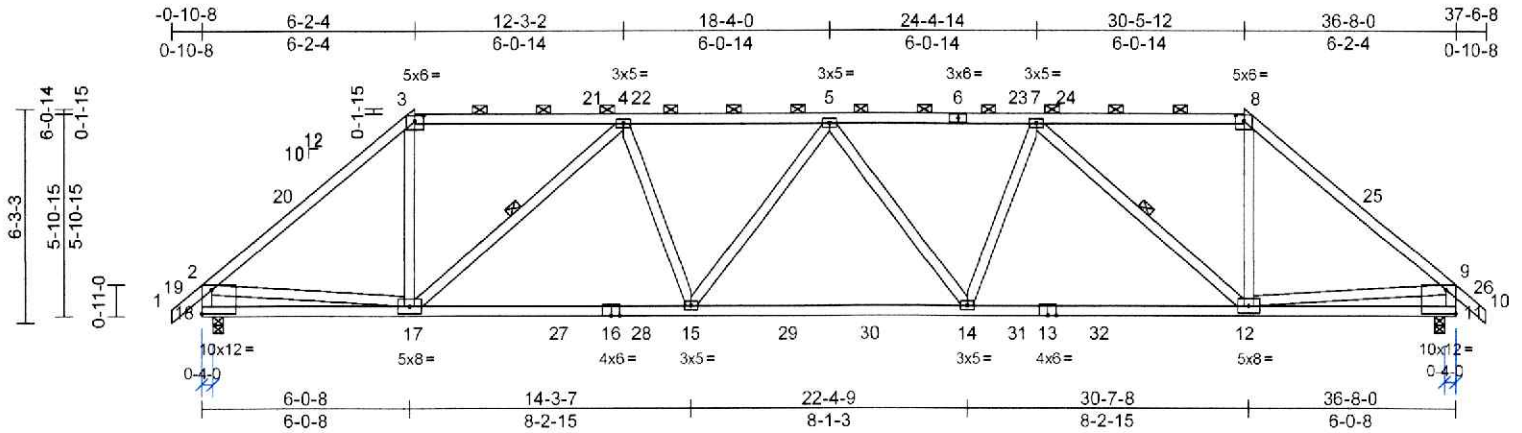
Job 24020093-01	Truss B2	Truss Type Hip	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834840
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Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1

CANTILEVER EACH END 4"
NO REPAIR REQUIRED



Scale = 1/67.3

Plate Offsets (X, Y): [3:0-3-0,0-2-1], [8:0-3-0,0-2-1], [11:Edge,0-8-6], [18:Edge,0-8-6]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.85	Vert(LL)	-0.22	14-15	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.96	Vert(CT)	-0.39	15-17	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(CT)	0.10	11	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 214 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except* 16-13:2x4 SP No.1
WEBS 2x4 SP No.3 *Except* 18-2,11-9:2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-12 oc purlins, except end verticals, and 2-0-0 oc purlins (2-9-3 max.); 3-8.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 4-17, 7-12

REACTIONS (size) 11=0-3-8, 18=0-3-8
Max Horiz 18=160 (LC 13)
Max Uplift 11=-172 (LC 15), 18=-172 (LC 14)
Max Grav 11=1684 (LC 44), 18=1684 (LC 44)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/67, 2-3=-2129/205, 3-4=-1598/224, 4-5=-2765/290, 5-7=-2765/290, 7-8=-1598/223, 8-9=-2129/205, 9-10=0/67, 2-18=-1597/201, 9-11=-1597/201
BOT CHORD 17-18=-250/600, 15-17=-324/2598, 14-15=-342/2860, 12-14=-278/2598, 11-12=-184/491
WEBS 3-17=-50/1028, 4-17=-1361/243, 4-15=-12/546, 5-15=-272/136, 5-14=-272/136, 7-14=-12/546, 7-12=-1361/242, 8-12=49/1028, 2-17=-191/1401, 9-12=-198/1401

NOTES
1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-9-8, Exterior(2R) 2-9-8 to 11-4-8, Interior (1) 11-4-8 to 25-3-8, Exterior(2R) 25-3-8 to 33-10-8, Exterior(2E) 33-10-8 to 37-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at j(s) 18 and 11. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbccomponents.com)

ENGINEERING BY
TRENCO
A MITek Affiliate

818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss B3	Truss Type Hip	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834841
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Carter Components (Sanford, NC), Sanford, NC - 27332,

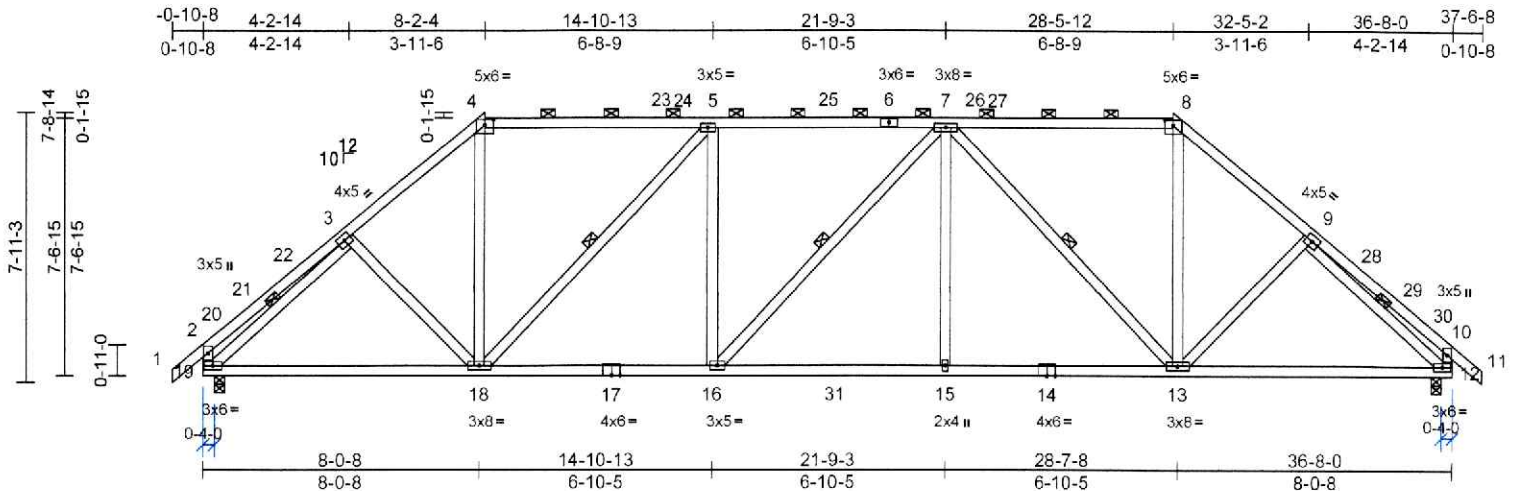
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CANTILEVER EACH END 4"

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NO REPAIR REQUIRED



Scale = 1/8" = 1'-0"

Plate Offsets (X, Y): [4:0-3-0,0-2-1], [8:0-3-0,0-2-1]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP		
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.90	Vert(LL)	-0.16	15-16	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.90	Vert(CT)	-0.29	15-16	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(CT)	0.11	12	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
										Weight: 238 lb	FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.2 *Except* 4-6,6-8:2x4 SP No.1
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-10-11 oc purlins, except end verticals, and 2-0-0 oc purlins (2-2-0 max.): 4-8.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 5-18, 7-16, 7-13, 3-19, 9-12

REACTIONS

(size) 12=0-3-8, 19=0-3-8
 Max Horiz 19=200 (LC 12)
 Max Uplift 12=-167 (LC 15), 19=-167 (LC 14)
 Max Grav 12=1693 (LC 6), 19=1688 (LC 5)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/39, 2-3=-517/117, 3-4=-2001/246, 4-5=-1528/236, 5-7=-2236/248, 7-8=-1533/236, 8-9=-2008/246, 9-10=-518/117, 10-11=0/39, 2-19=-474/130, 10-12=-475/130
 BOT CHORD 18-19=-220/1446, 16-18=-235/2236, 15-16=-200/2246, 13-15=-200/2246, 12-13=-82/1451
 WEBS 3-18=-177/215, 4-18=-26/936, 5-18=-1035/192, 5-16=0/350, 7-16=-106/107, 7-15=0/367, 7-13=-1043/192, 8-13=-26/939, 9-13=-177/216, 3-19=-1629/120, 9-12=-1635/120

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-9-8, Exterior(2R) 2-9-8 to 13-4-8, Interior (1) 13-4-8 to 23-3-8, Exterior(2R) 23-3-8 to 33-10-8, Exterior(2E) 33-10-8 to 37-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at Jt(s) 19 and 12. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbccomponents.com)



816 Soundside Road
 Edenton, NC 27932

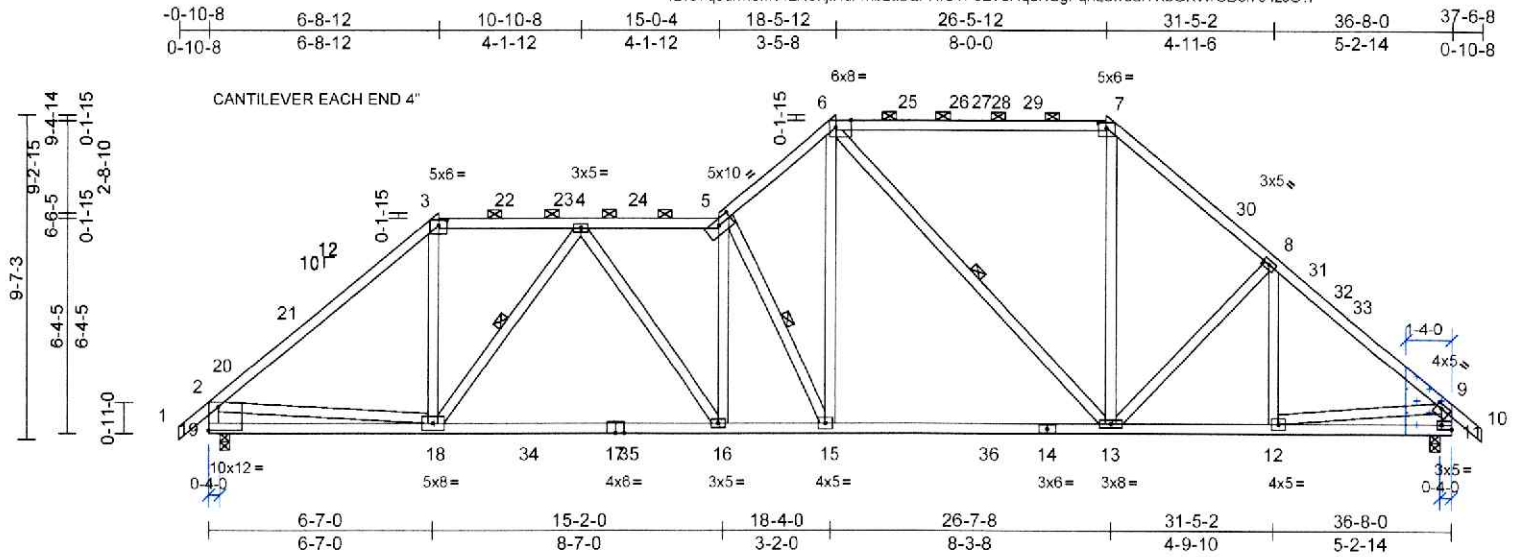
Job 24020093-01	Truss B4	Truss Type Roof Special	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834842
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Carter Components (Sanford, NC), Sanford, NC - 27332

Run: 8:73 S Mar 21 2024 Print: 8:730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:35:56

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ATTACH 1/2" PLYWOOD OR OSB GUSSET (15/32" RATED SHEATHING 32/16 EXP 1)
TO EACH FACE OF TRUSS WITH (0 131" X 2 5" MIN.) NAILS PER THE FOLLOWING NAIL SCHEDULE:
2 X 3'S - 2 ROWS, 2 X 4'S - 3 ROWS, 2 X 6'S AND LARGER - 4 ROWS SPACED @ 4" O.C.
NAILS TO BE DRIVEN FROM BOTH FACES. STAGGER SPACING FROM FRONT TO BACK FACE
FOR A NET 2" O.C. SPACING IN EACH COVERED TRUSS MEMBER. USE 2" MEMBER END DISTANCE.

Scale = 1:67.9

Plate Offsets (X, Y): [3:0-3-0,0-2-1], [5:0-5-0,0-2-0], [6:0-5-11,Edge], [7:0-3-0,0-2-1], [9:0-2-0,0-1-8], [11:Edge,0-1-8], [19:Edge,0-8-6]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.97	Vert(LL)	-0.26	16-18	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.77	Vert(CT)	-0.45	16-18	>966	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(CT)	0.07	11	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 250 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except* 6-7:2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP No.1
WEBS 2x4 SP No.3 *Except* 13-6,19-2,11-9:2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (2-2-0 max.): 3-5, 6-7.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-18, 5-15, 6-13

REACTIONS (size) 11=0-3-8, 19=0-3-8
Max Horiz 19=-239 (LC 12)
Max Uplift 11=-82 (LC 15), 19=-162 (LC 14)
Max Grav 11=1702 (LC 53), 19=1720 (LC 53)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/67, 2-3=-2144/228, 3-4=-1595/240, 4-5=-2610/322, 5-6=-2431/353, 6-7=-1429/280, 7-8=-1886/294, 8-9=-2033/230, 9-10=0/39, 2-19=-1633/220, 9-11=-1603/214
BOT CHORD 18-19=-345/756, 16-18=-162/2242, 15-16=-130/2601, 13-15=-42/1845, 12-13=-43/1498, 11-12=-72/261
WEBS 3-18=-12/1110, 4-18=-1139/147, 4-16=-27/691, 5-16=-373/90, 5-15=-1531/231, 6-15=-135/1600, 6-13=-620/107, 7-13=-33/792, 8-13=-328/182, 8-12=-91/57, 2-18=-107/1347, 9-12=-26/1273

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E)-0-10-8 to 2-9-8, Exterior(2R) 2-9-8 to 10-4-12, Interior (1) 10-4-12 to 15-0-4, Exterior(2R) 15-0-4 to 22-1-12, Interior (1) 22-1-12 to 22-9-12, Exterior(2R) 22-9-12 to 30-1-12, Interior (1) 30-1-12 to 33-10-8, Exterior(2E) 33-10-8 to 37-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 19 and 11. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

NOTES
1) Unbalanced roof live loads have been considered for this design.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

ENGINEERING BY
TRENCO
A MITEK AFFILIATE

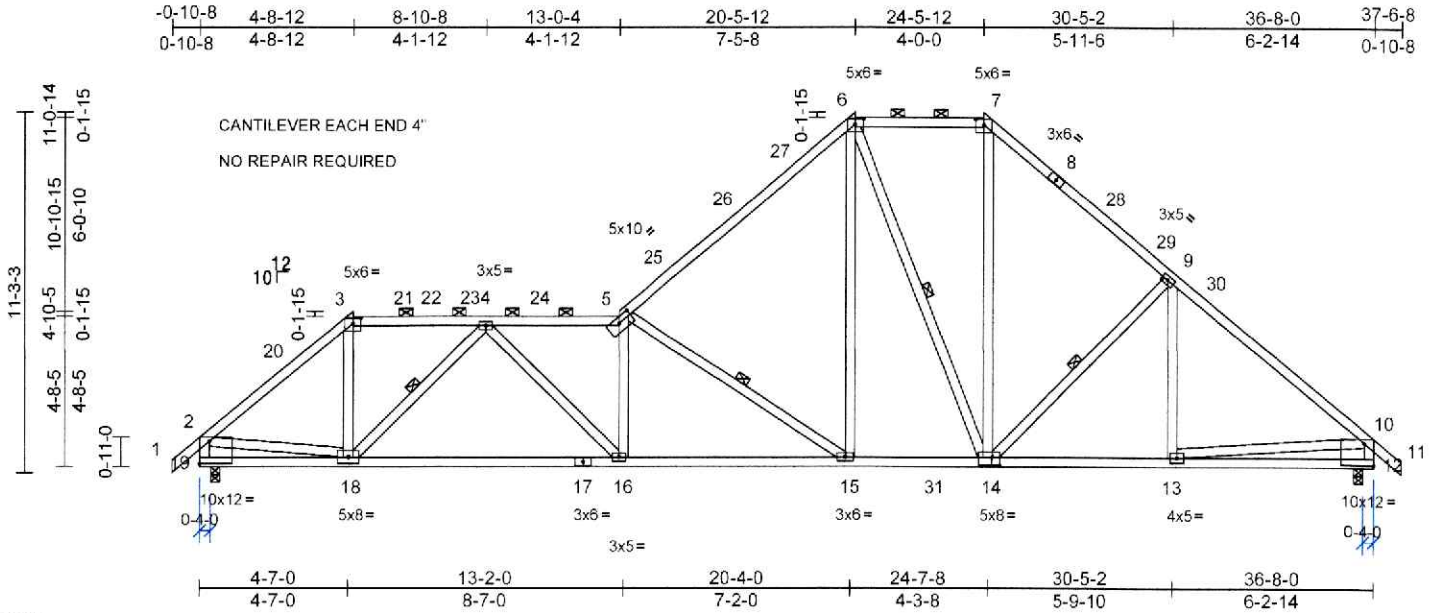
818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss B5	Truss Type Roof Special	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834843
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8:73 S Mar 21 2024 Print: 8:730 S Mar 21 2024 MITek Industries, Inc. Thu Apr 11 07:35:56
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Page: 1



Scale = 1:72

Plate Offsets (X, Y): [3:0-3-0,0-2-1], [5:0-5-4,0-2-0], [6:0-3-0,0-2-1], [7:0-3-0,0-2-1], [12:Edge,0-8-6], [14:0-3-0,0-3-0], [19:Edge,0-8-6]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.95	Vert(LL)	-0.22	16-18	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.92	Vert(CT)	-0.43	16-18	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.80	Horz(CT)	0.09	12	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 250 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except* 5-6:2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP No.2 *Except* 17-14:2x4 SP No.1
WEBS 2x4 SP No.3 *Except* 15-6,14-6,14-7:2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (2-9-12 max.): 3-5, 6-7.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
2-2-0 oc bracing: 16-18.
WEBS 1 Row at midpt 4-18, 5-15, 6-14, 9-14

REACTIONS (size) 12=0-3-8, 19=0-3-8
Max Horiz 19=278 (LC 13)
Max Uplift 12=-97 (LC 15), 19=-170 (LC 14)
Max Grav 12=1771 (LC 53), 19=1750 (LC 53)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/68, 2-3=-2060/195, 3-4=-1490/198, 4-5=-3213/335, 5-6=-2113/287, 6-7=-1305/284, 7-9=-1877/298, 9-10=-2156/214, 10-11=0/39, 2-19=-1699/190, 10-12=-1667/201
BOT CHORD 18-19=-281/445, 16-18=-274/2438, 15-16=-279/3195, 13-15=-18/1586, 12-13=-115/334
WEBS 3-18=-20/1037, 4-18=-1377/143, 4-16=-62/1175, 5-16=-642/146, 5-15=-2004/314, 6-15=-97/1260, 6-14=-509/130, 7-14=-111/847, 9-14=-470/210, 9-13=-31/161, 2-18=0/1395, 10-13=0/1315

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-9-8, Exterior(2R) 2-9-8 to 8-4-12, Interior (1) 8-4-12 to 16-9-12, Exterior(2R) 16-9-12 to 28-1-12, Interior (1) 28-1-12 to 33-10-8, Exterior(2E) 33-10-8 to 37-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- The Fabrication Tolerance at joint 5 = 8%
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at j(s) 19 and 12. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

- NOTES**
- Unbalanced roof live loads have been considered for this design.



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sboccomponents.com)

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818 Soundside Road
Edenton, NC 27932

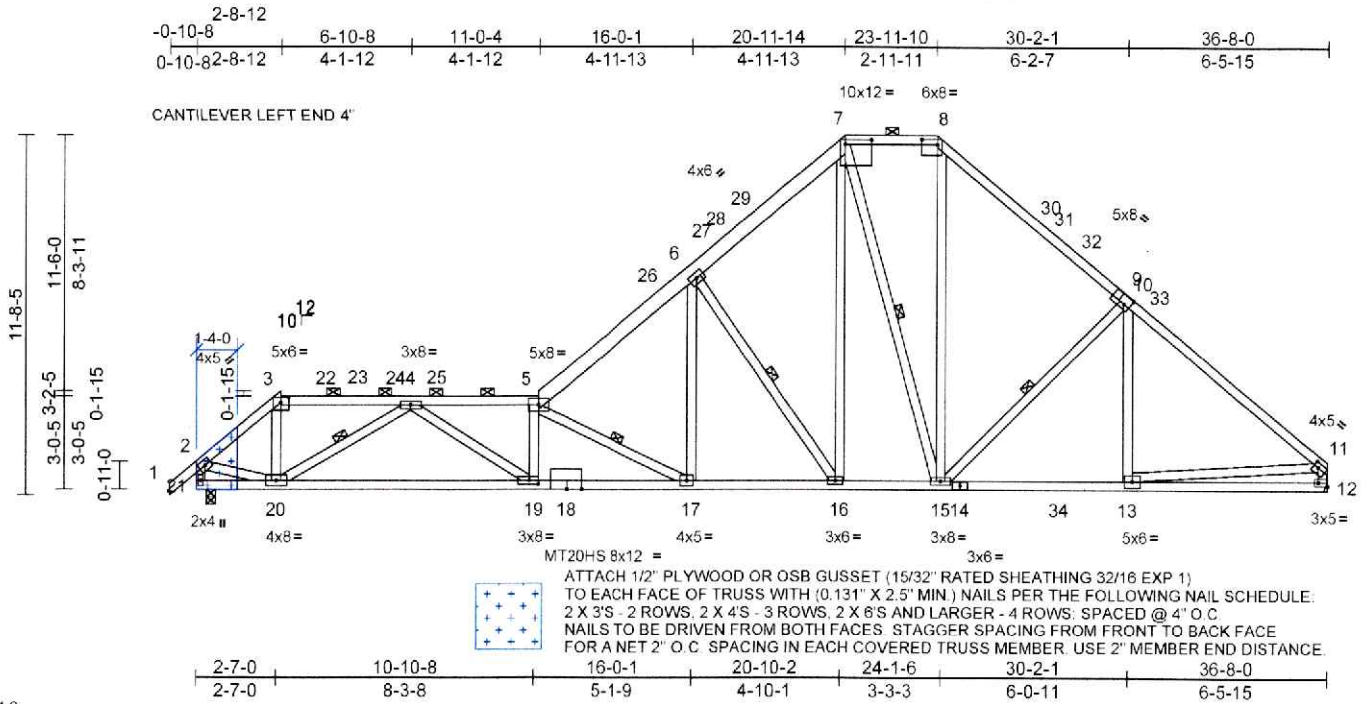
Job 24020093-01	Truss B6	Truss Type Piggyback Base	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834844
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Carter Components (Sanford, NC), Sanford, NC - 27332.

Run: 8.73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:35:56

Page: 1

ID:wCo3_jz9bc2fHg0oeev0PzzaLQ2-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC7f



ATTACH 1/2" PLYWOOD OR OSB GUSSET (15/32" RATED SHEATHING 32/16 EXP 1) TO EACH FACE OF TRUSS WITH (0.131" X 2.5" MIN) NAILS PER THE FOLLOWING NAIL SCHEDULE:
 2 X 3'S - 2 ROWS, 2 X 4'S - 3 ROWS, 2 X 6'S AND LARGER - 4 ROWS; SPACED @ 4" O.C.
 NAILS TO BE DRIVEN FROM BOTH FACES. STAGGER SPACING FROM FRONT TO BACK FACE FOR A NET 2" O.C. SPACING IN EACH COVERED TRUSS MEMBER. USE 2" MEMBER END DISTANCE.

Plate Offsets (X, Y): [3:0-3-0,0-2-1], [7:0-10-0,0-1-12], [8:0-6-4,0-2-0], [9:0-2-8,0-3-4], [11:0-2-0,0-1-12], [12:Edge,0-1-8], [19:0-3-8,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.94	Vert(LL)	-0.30	19-20	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.89	Vert(CT)	-0.56	19-20	>786	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.75	Horz(CT)	0.11	12	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 269 lb	FT = 20%

LUMBER
 TOP CHORD 2x4 SP No.2 *Except* 5-7:2x6 SP No.2
 BOT CHORD 2x4 SP No.1
 WEBS 2x4 SP No.3 *Except* 16-7,15-7,15-8:2x4 SP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (2-4-2 max.): 3-5, 7-8.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 4-20, 6-16, 7-15, 10-15, 5-17

REACTIONS (size) 12= Mechanical, 21=0-3-8
 Max Horiz 21=284 (LC 13)
 Max Uplift 12=-80 (LC 15), 21=-171 (LC 14)
 Max Grav 12=1761 (LC 53), 21=1736 (LC 53)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/69, 2-3=-1932/159, 3-4=-1398/151, 4-5=-4735/373, 5-6=-3095/301, 6-7=-2036/316, 7-8=-1317/284, 8-10=-1905/293, 10-11=-2221/205, 2-21=-1766/157, 11-12=-1655/159
 BOT CHORD 20-21=-280/272, 19-20=-420/3238, 17-19=-439/4777, 16-17=-185/2356, 15-16=-8/1440, 13-15=-51/1641, 12-13=-83/272
 WEBS 3-20=-24/1035, 4-20=-2183/229, 4-19=-60/1814, 5-19=-866/123, 6-16=-1563/304, 7-16=-200/1398, 7-15=-457/109, 8-15=-99/835, 10-15=-543/224, 10-13=-32/210, 2-20=-38/1476, 11-13=-10/1412, 6-17=-88/1514, 5-17=-2801/289

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-8-12, Exterior(2R) 2-8-12 to 6-4-12, Interior (1) 6-4-12 to 17-3-14, Exterior (2R) 17-3-14 to 27-7-10, Interior (1) 27-7-10 to 32-10-4, Exterior(2E) 32-10-4 to 36-6-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 12.
- H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 21. This connection is for uplift only and does not consider lateral forces.

- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- LOAD CASE(S)** Standard



NOTES

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2023 BEFORE USE
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbccomponents.com)



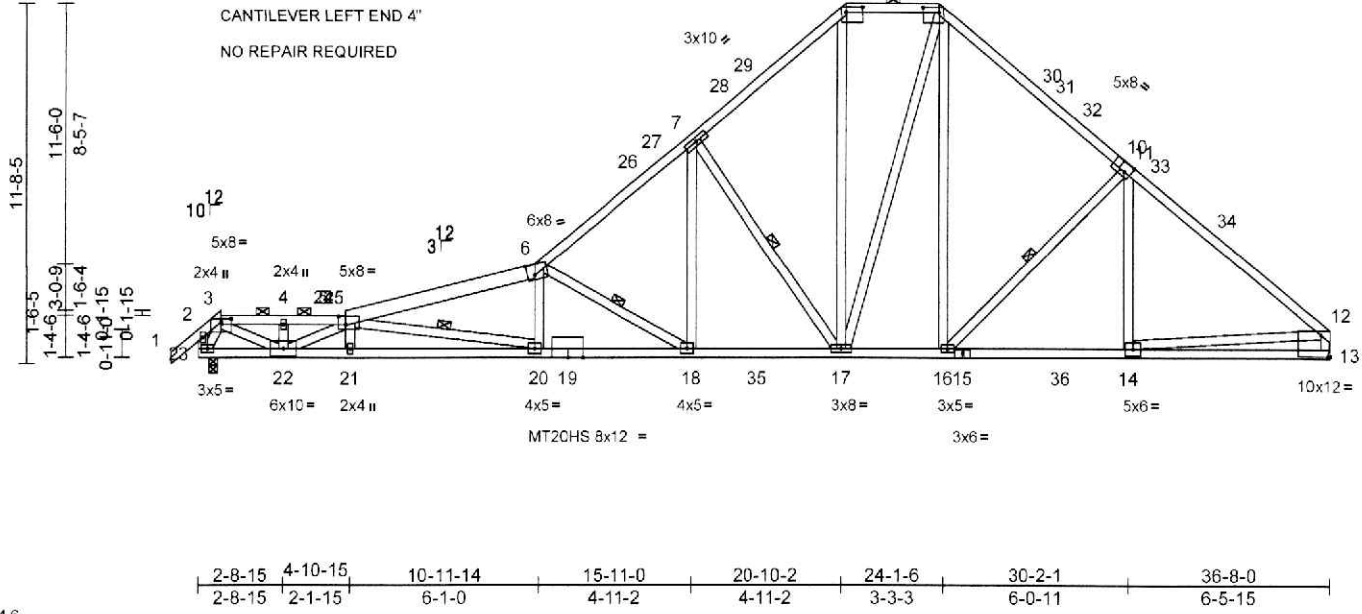
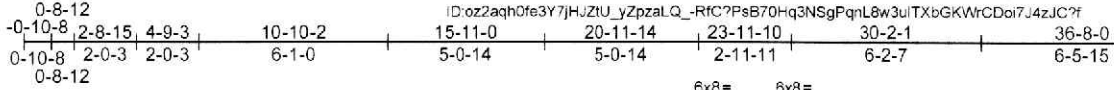
818 Soundside Road
 Edenton, NC 27932

Job 24020093-01	Truss B7	Truss Type Piggyback Base	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834845
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Carter Components (Sanford, NC), Sanford, NC - 27332.

Run: 8.73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MITek Industries, Inc. Thu Apr 11 07:35:57

Page: 1



Scale = 1/74.6

Plate Offsets (X, Y): [3:0-4-0,0-1-4], [5:0-2-12,0-3-4], [6:0-2-12,0-4-4], [8:0-6-4,0-2-0], [9:0-6-4,0-2-0], [10:0-2-8,0-3-4], [13:Edge,0-8-6]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.97	Vert(LL)	-0.41	20-21	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.85	Vert(CT)	-0.72	20-21	>609	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(CT)	0.13	13	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 261 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except* 5-6:2x6 SP No.2
BOT CHORD 2x4 SP 2400F 2.0E *Except* 19-15:2x4 SP No.1
WEBS 2x4 SP No.3 *Except* 17-8,17-9,16-9,22-3,22-5:2x4 SP No.2, 23-2:2x4 SP No.1

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (2-8-14 max.); 3-5, 8-9.
BOT CHORD Rigid ceiling directly applied or 7-10-10 oc bracing.
WEBS 1 Row at midpt 5-20, 6-18, 7-17, 11-16

REACTIONS
(size) 13= Mechanical, 23=0-3-8
Max Horiz 23=289 (LC 11)
Max Uplift 13=80 (LC 15), 23=171 (LC 14)
Max Grav 13=1787 (LC 57), 23=1776 (LC 57)

FORCES
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/70, 2-3=-293/102, 3-4=-3711/605, 4-5=-3710/604, 5-6=-5227/775, 6-7=-3161/540, 7-8=2064/499, 8-9=-1458/446, 9-11=-1938/466, 11-12=-2258/381, 2-23=358/162, 12-13=-1681/322
BOT CHORD 22-23=-232/919, 21-22=-882/6361, 20-21=-872/6375, 18-20=-642/5038, 17-18=-211/2401, 16-17=0/1359, 14-16=-148/1670, 13-14=-107/277

WEBS
5-21=0/161, 5-20=-1400/232, 6-20=0/466, 6-18=-3017/489, 7-18=-170/1608, 7-17=-1563/361, 8-17=-201/1052, 9-17=-118/482, 9-16=-106/544, 11-16=-548/226, 11-14=-30/213, 12-14=-41/1434, 3-22=-502/3234, 4-22=-263/73, 5-22=-3001/393, 3-23=-1508/211

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 4-4-12, Interior (1) 4-4-12 to 17-3-14, Exterior(2R) 17-3-14 to 27-7-10, Interior (1) 27-7-10 to 32-10-4, Exterior(2E) 32-10-4 to 36-6-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf

- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 13.
- H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at Jt(s) 23. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED WITH REFERENCE PAGE WITH 4% RED IN 2023 BEFORE USE.
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbccomponents.com)

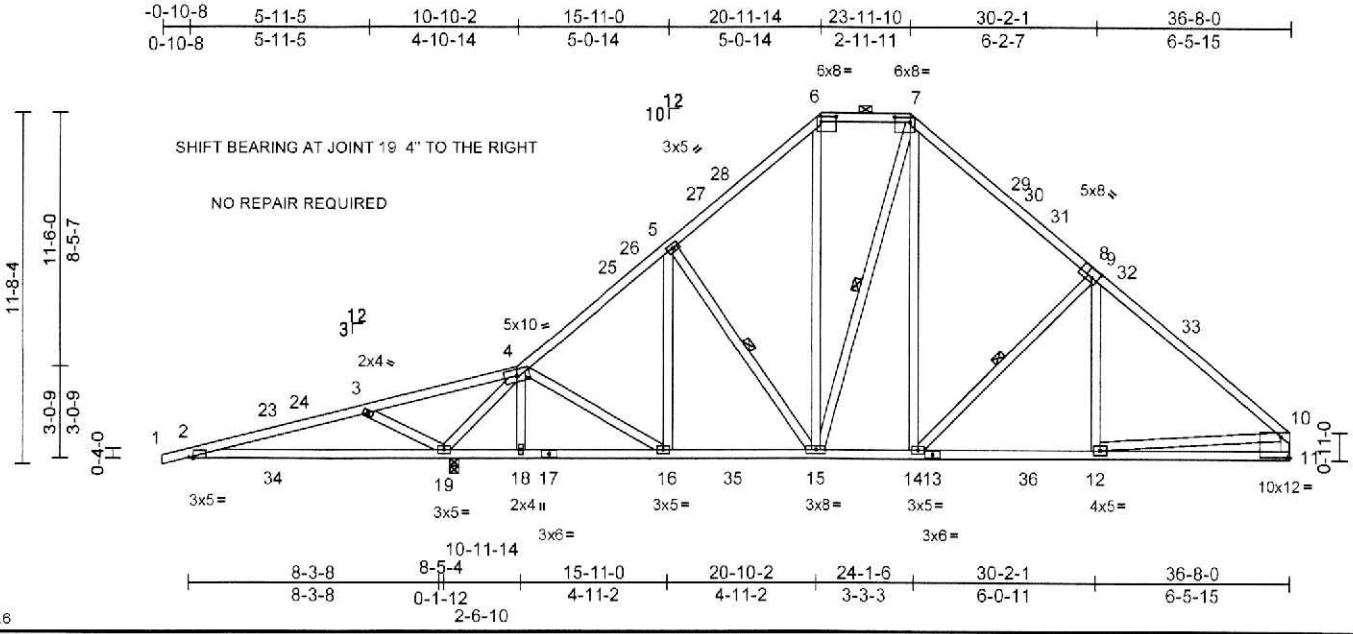
TRENCO ENGINEERING BY A MITEK AFFILIATE
818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss B8	Truss Type Piggyback Base	Qty 4	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834846
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Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1/76.6

Plate Offsets (X, Y): [2:0-1-11,Edge], [6:0-6-4,0-2-0], [7:0-6-4,0-2-0], [8:0-2-8,0-3-4], [11:Edge,0-8-6]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.76	Vert(LL)	-0.08	12-14	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.63	Vert(CT)	-0.13	12-14	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(CT)	0.03	11	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 245 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except* 1-4:2x4 SP No.1
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3 *Except* 15-6,15-7,14-7:2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-5-10 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-7.
 BOT CHORD Rigid ceiling directly applied or 5-1-10 oc bracing.
 WEBS 1 Row at midpt 5-15, 7-15, 9-14.

REACTIONS

(size) 11= Mechanical, 19=0-3-8
 Max Horiz 19=283 (LC 13)
 Max Uplift 11=78 (LC 15), 19=223 (LC 10)
 Max Grav 11=1312 (LC 53), 19=2127 (LC 5)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/13, 2-3=-1190/1149, 3-4=-1447/1856, 4-5=-1375/134, 5-6=-1190/207, 6-7=-812/196, 7-9=-1252/204, 9-10=-1602/136, 10-11=-1211/145
 BOT CHORD 2-19=-1050/1176, 18-19=-605/1042, 16-18=-605/1052, 15-16=-65/1164, 14-15=0/890, 12-14=0/1189, 11-12=-94/244
 WEBS 3-19=-849/369, 4-19=-2098/708, 4-18=0/179, 4-16=-835/945, 5-16=-327/448, 5-15=-448/159, 6-15=105/503, 7-15=-284/174, 7-14=-117/579, 9-14=-590/243, 9-12=0/252, 10-12=0/1011

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-9-8, Interior (1) 2-9-8 to 17-3-14, Exterior(2R) 17-3-14 to 27-7-10, Interior (1) 27-7-10 to 32-10-4, Exterior(2E) 32-10-4 to 36-6-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 78 lb uplift at joint 11.
- H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at J(s) 19. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPIH Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



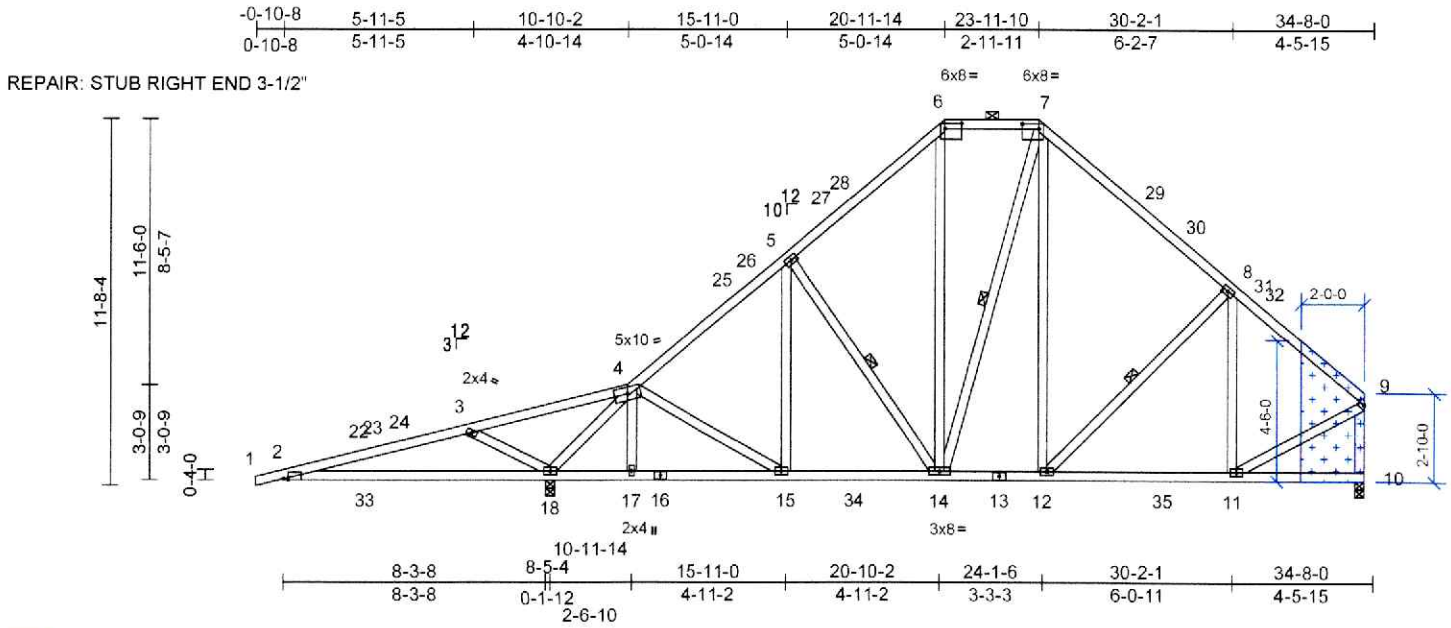
818 Soundside Road
 Edenton, NC 27932

Job 24020093-01	Truss B9	Truss Type Piggyback Base	Qty 4	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	I65002238
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Carter Components (Sanford, NC), Sanford, NC - 27332.

Run: 8:73 S Apr 3 2024 Print: 8:730 S Apr 3 2024 MITek Industries, Inc. Thu Apr 18 09:37:43
ID: Lb3UJnc1tQr1z8Ap_lgbMzALPC-RFC?PsB70Hq3NSgPqnLw3uITxbGKwRCDci7J4zJC?#

Page: 1



INSTALL 2 X 4 SPF/DF/SP NO.2
CUT TO FIT TIGHT.



ATTACH 1/2" PLYWOOD OR OSB GUSSET (15/32" RATED SHEATHING 32/16 EXP 1)
TO EACH FACE OF TRUSS WITH (0 131" X 2.5" MIN.) NAILS PER THE FOLLOWING NAIL SCHEDULE:
2 X 3'S - 2 ROWS, 2 X 4'S - 3 ROWS, 2 X 6'S AND LARGER - 4 ROWS, SPACED @ 4" O.C.
NAILS TO BE DRIVEN FROM BOTH FACES. STAGGER SPACING FROM FRONT TO BACK FACE
FOR A NET 2" O.C. SPACING IN EACH COVERED TRUSS MEMBER. USE 2" MEMBER END DISTANCE.

Scale = 1/73.2

Plate Offsets (X, Y): [2:0-11,Edge], [6:0-6-4,0-2-0], [7:0-6-4,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.80	Vert(LL)	-0.06	11-12	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.62	Vert(CT)	-0.11	11-12	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.63	Horz(CT)	0.02	10	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 238 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except* 1-4:2x4 SP No.1
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 14-6,14-7,12-7:2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-4-4 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-7.

BOT CHORD Rigid ceiling directly applied or 5-2-0 oc bracing.

WEBS 1 Row at midpt 5-14, 7-14, 8-12

REACTIONS

(size) 10=0-3-8, 18=0-3-8
Max Horiz 18=304 (LC 13)
Max Uplift 10=-63 (LC 15), 18=-236 (LC 10)
Max Grav 10=1232 (LC 51), 18=2058 (LC 5)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/13, 2-3=-1181/1138, 3-4=-1436/1824, 4-5=-1245/121, 5-6=-1053/193, 6-7=-709/190, 7-8=-1080/190, 8-9=-1086/126, 9-10=-1177/109
BOT CHORD 2-18=-1041/1167, 17-18=-621/1026, 15-17=-621/1036, 14-15=-98/1062, 12-14=-22760, 11-12=-31842, 10-11=-2726
WEBS 3-18=830/365, 4-18=2007/697, 4-17=0/178, 4-15=-826/921, 5-15=-314/445, 5-14=-456/159, 6-14=-100/420, 7-14=-210/188, 7-12=-97/374, 8-12=-263/220, 8-11=-310/78, 9-11=-13/941

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-7-2, Interior (1) 2-7-2 to 17-6-5, Exterior(2R) 17-6-5 to 27-5-3, Interior (1) 27-5-3 to 31-0-10, Exterior(2E) 31-0-10 to 34-6-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) All plates are 3x5 MT20 unless otherwise indicated.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 236 lb uplift at joint 18 and 63 lb uplift at joint 10.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



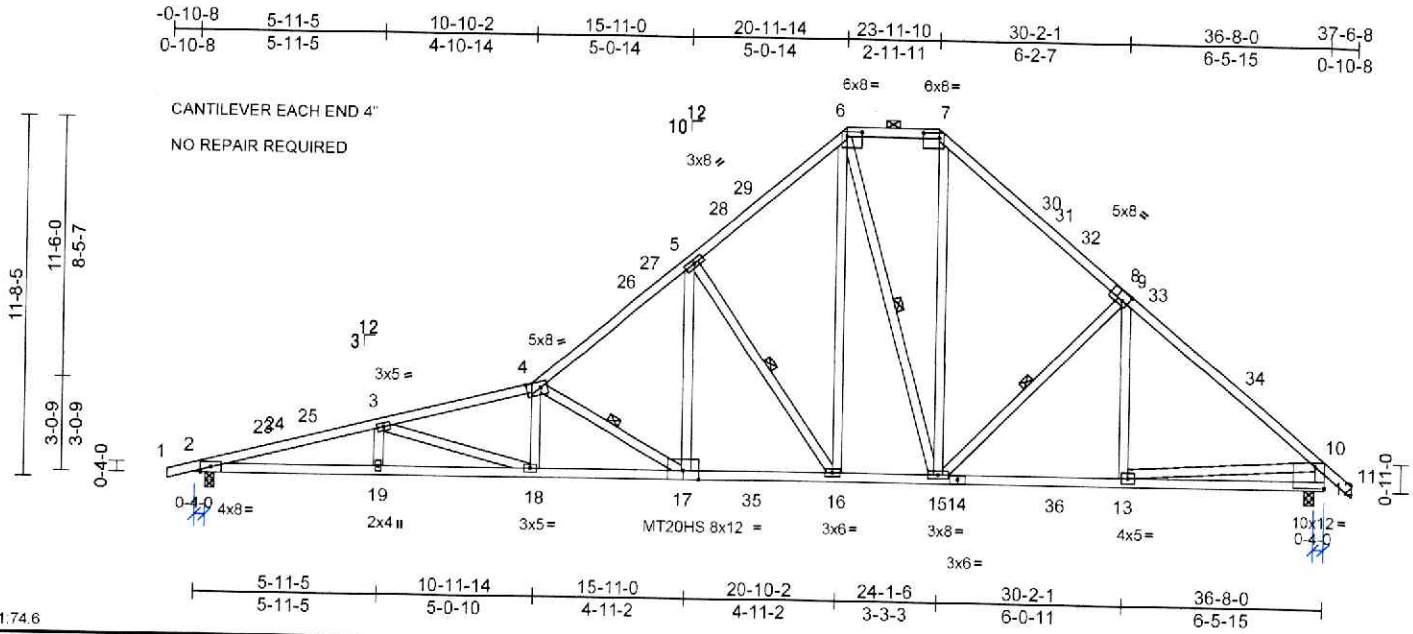
April 19, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPH Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road
Edenton, NC 27932



Scale = 1.746

Plate Offsets (X, Y): [4:0-5-8,0-2-4], [6:0-6-0,0-1-12], [7:0-6-4,0-2-0], [8:0-2-8,0-3-4], [12:Edge,0-8-6]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in (loc)		l/defl	L/d	PLATES	GRIP	
						in	(loc)					
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.98	Vert(LL)	-0.38	18-19	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.69	Vert(CT)	-0.69	18-19	>634	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(CT)	0.13	12	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											

Weight: 246 lb FT = 20%

- LUMBER**
- TOP CHORD 2x4 SP No.2
 - BOT CHORD 2x4 SP No.2 *Except* 2-17:2x4 SP 2400F 2.0E
 - WEBS 2x4 SP No.3 *Except* 16-6,15-6,15-7:2x4 SP No.2
- BRACING**
- TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (4-10-5 max.); 6-7.
 - BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 - WEBS 1 Row at midpt 4-17, 5-16, 6-15, 9-15
- REACTIONS** (size) 2=0-3-8, 12=0-3-8
 Max Horiz 2=290 (LC 13)
 Max Uplift 2=-169 (LC 14), 12=-100 (LC 15)
 Max Grav 2=1636 (LC 5), 12=1800 (LC 49)
- FORCES** (lb) - Maximum Compression/Maximum Tension
- TOP CHORD 1-2=0/13, 2-3=-5424/837, 3-4=-4699/748, 4-5=-3021/530, 5-6=-1988/492, 6-7=-1301/427, 7-9=-1880/455, 9-10=-2204/375, 10-11=0/39, 10-12=-1695/372
 - BOT CHORD 2-19=-688/5240, 18-19=-688/5240, 16-18=-555/4564, 15-16=0/1429, 13-15=-75/1622, 12-13=-126/346, 3-19=0/201, 3-18=-1144/198, 4-18=0/488, 4-17=-2667/463, 5-17=-162/1505, 5-16=-1469/359, 6-16=-246/1326, 6-15=-471/117, 7-15=-141/831, 9-15=-538/221, 9-13=-19/212, 10-13=0/1353

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-9-8, Interior (1) 2-9-8 to 17-3-14, Exterior(2R) 17-3-14 to 27-7-10, Interior (1) 27-7-10 to 33-10-8, Exterior(2E) 33-10-8 to 37-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) All plates are MT20 plates unless otherwise indicated.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 12. This connection is for uplift only and does not consider lateral forces.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinet.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbccomponents.com)

ENGINEERING BY
TRENCO
A MiTek Affiliate

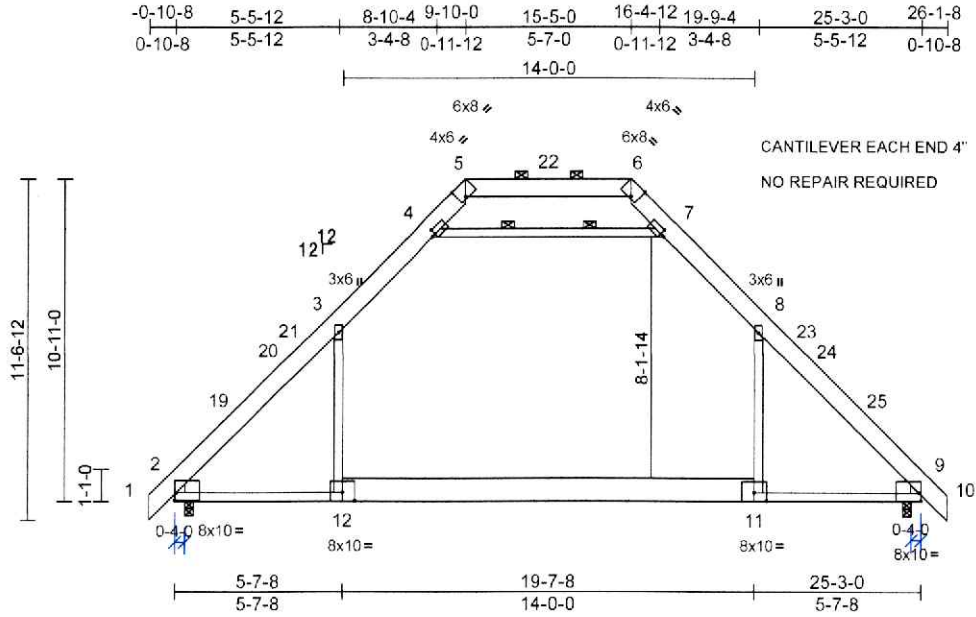
818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss C1	Truss Type Attic	Qty 3	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834849
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8:73 S Mar 21 2024 Print: 8:730 S Mar 21 2024 MITek Industries, Inc. Thu Apr 11 07:35:58
ID: OgutmTBRLMJ8PROFIQEE8mzaLPrm-RfC?PsB70Hq3NSqPqnL8w3ulTxbGKWrCDo7J4zJC7f

Page: 1



Scale = 1.77:8

Plate Offsets (X, Y): [2:Edge,0-3-4], [4:0-1-9,0-2-4], [5:0-2-14,Edge], [6:0-2-14,Edge], [7:0-1-9,0-2-4], [9:Edge,0-3-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.36	Vert(LL)	-0.39	11-12	>775	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.85	Vert(CT)	-0.57	11-12	>536	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.46	Horz(CT)	0.03	2	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH		Attic	-0.31	11-12	>546	360		
BCDL	10.0											
											Weight: 215 lb	FT = 20%

LUMBER

TOP CHORD 2x8 SP 2400F 2.0E
 BOT CHORD 2x4 SP No.1 *Except* 12-11:2x10 SP 2400F 2.0E
 WEBS 2x4 SP No.3 *Except* 4-7:2x4 SP No.2
 WEDGE Left: 2x4 SP No.3
 Right: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.); 5-6.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 2 Rows at 1/3 pts 4-7

REACTIONS

(size) 2=0-3-8, 9=0-3-8
 Max Horiz 2=254 (LC 13)
 Max Grav 2=1671 (LC 46), 9=1671 (LC 46)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/37, 2-3=-1945/0, 3-4=-1184/138, 4-5=-239/563, 5-6=-2/804, 6-7=-239/565, 7-8=-1184/138, 8-9=-1944/0, 9-10=0/37
 BOT CHORD 2-9=-86/1218
 WEBS 3-12=0/949, 8-11=0/949, 4-7=-1935/82

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-10-8 to 2-1-8, Interior (1) 2-1-8 to 5-4-15, Exterior(2R) 5-4-15 to 19-10-1, Interior (1) 19-10-1 to 23-1-8, Exterior(2E) 23-1-8 to 26-1-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Ceiling dead load (5.0 psf) on member(s), 3-4, 7-8, 4-7; Wall dead load (5.0psf) on member(s), 3-12, 8-11
- Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room, 11-12
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Attic room checked for L/360 deflection.

LOAD CASE(S) Standard



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

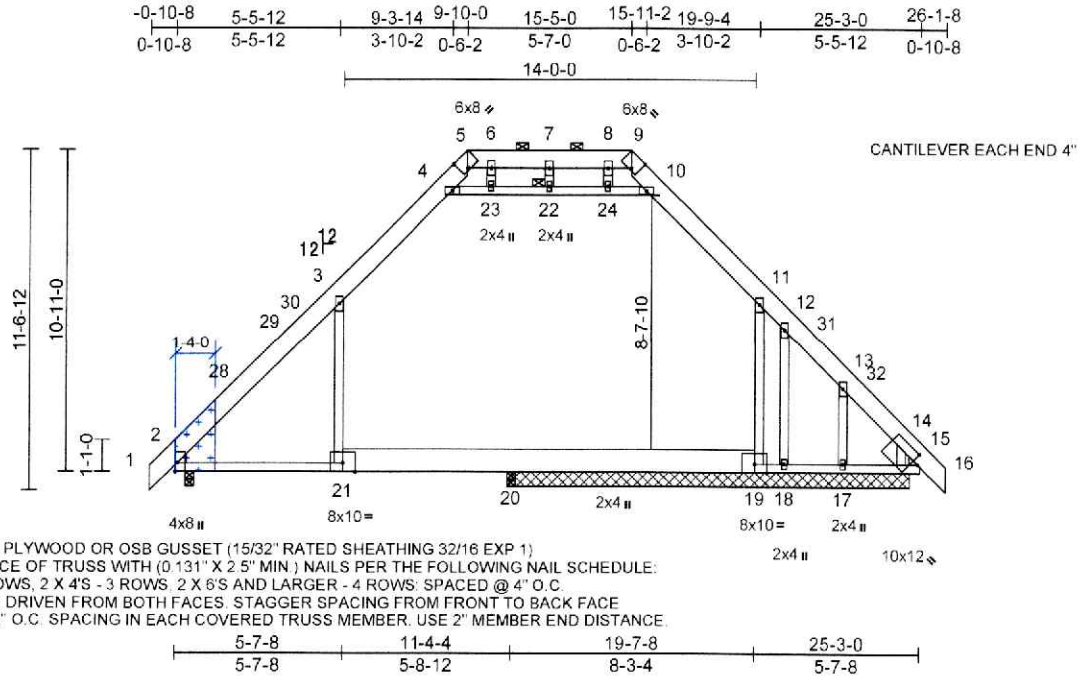


818 Soundside Road
 Edenton, NC 27932

Job 24020093-01	Truss C1E	Truss Type Attic Structural Gable	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	I64834850
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MITek Industries, Inc. Thu Apr 11 07:35:58
ID: 15_jm7XqhrJfVite3RizaLPr-RfC?PsB70Hq3NSgPqnL6w3ulTxbGKwRcDoi7JazJC?r



ATTACH 1/2" PLYWOOD OR OSB GUSSET (15/32" RATED SHEATHING 32/16 EXP 1)
TO EACH FACE OF TRUSS WITH (0.131" X 2.5" MIN.) NAILS PER THE FOLLOWING NAIL SCHEDULE:
2 X 3'S - 2 ROWS, 2 X 4'S - 3 ROWS, 2 X 6'S AND LARGER - 4 ROWS, SPACED @ 4" O.C.
NAILS TO BE DRIVEN FROM BOTH FACES. STAGGER SPACING FROM FRONT TO BACK FACE
FOR A NET 2" O.C. SPACING IN EACH COVERED TRUSS MEMBER. USE 2" MEMBER END DISTANCE.

Scale = 1:78

Plate Offsets (X, Y): [2:Edge,0-0-12], [5:0-2-14,Edge], [9:0-2-14,Edge], [15:0-2-8,0-3-9]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.17	Vert(LL)	0.10	21-27	>999	240
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.41	Vert(CT)	-0.15	21-27	>886	180
TCDL	10.0	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.02	2	n/a	n/a
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH		Attic	-0.08	20-21	>999	360
BCDL	10.0									

Weight: 228 lb FT = 20%

LUMBER
TOP CHORD 2x8 SP 2400F 2.0E
BOT CHORD 2x4 SP No.2 *Except* 21-19:2x10 SP 2400F 2.0E
WEBS 2x4 SP No.3 *Except* 4-10:2x4 SP No.2
OTHERS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3
Right: 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 5-9.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

JOINTS
1 Brace at Jt(s): 22
REACTIONS (size) 2=0-3-8, 15=14-0-8, 17=14-0-8, 18=14-0-8, 19=14-0-8, 20=0-3-8
Max Horiz 2=255 (LC 13)
Max Uplift 2=42 (LC 14), 15=76 (LC 11), 17=127 (LC 15), 18=121 (LC 14), 19=401 (LC 42)
Max Grav 2=1107 (LC 40), 15=904 (LC 40), 17=219 (LC 50), 18=277 (LC 40), 19=451 (LC 51), 20=1121 (LC 21)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-1130/35, 3-4=-877/182, 4-5=437/231, 5-6=-269/418, 6-7=-269/418, 7-8=-269/418, 8-9=-269/418, 9-10=451/176, 10-11=903/184, 11-12=-1123/246, 12-13=-1080/189, 13-14=-988/145, 14-15=-1007/130, 15-16=0/34
BOT CHORD 2-20=192/741, 18-20=99/735, 17-18=90/708, 15-17=94/727

WEBS
3-21=69/199, 11-19=-211/428, 4-23=-1052/357, 22-23=-1050/356, 22-24=-1050/356, 10-24=-1052/357, 7-22=17/19, 6-23=6/69, 8-24=-16/92, 12-18=-204/70, 13-17=-237/131

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-10-8 to 2-1-8, Interior (1) 2-1-8 to 5-4-15, Exterior(2R) 5-4-15 to 19-10-1, Interior (1) 19-10-1 to 23-1-8, Exterior(2E) 23-1-8 to 26-1-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - 5) Unbalanced snow loads have been considered for this design.
 - 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - 7) Provide adequate drainage to prevent water ponding.
 - 8) All plates are 3x6 MT20 unless otherwise indicated.
 - 9) Gable studs spaced at 2-0-0 oc.
 - 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 12) Ceiling dead load (5.0 psf) on member(s). 3-4, 10-11, 4-23, 22-23, 22-24, 10-24; Wall dead load (5.0psf) on member(s).3-21, 11-19
- 13) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 20-21, 19-20
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 15.
- 15) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 16) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 19, 18, and 17. This connection is for uplift only and does not consider lateral forces.



April 12, 2024

Continued on page 2

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ENGINEERING BY
TRENCO
A MITEK AFFILIATE
818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss C1E	Truss Type Attic Structural Gable	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional) I64834850
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Carter Components (Sanford, NC), Sanford, NC - 27332

Run: 8.73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:35:58
ID:1f5_jm7JXqhrJfVt3RizalPr-RIC?PsB70Hq3NSgPqL8w3uITXbGKWrCDol7J4zJC?f

Page: 2

- 17) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R502.10.2 and referenced standard ANSI/TPI 1.
- 18) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 19) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard

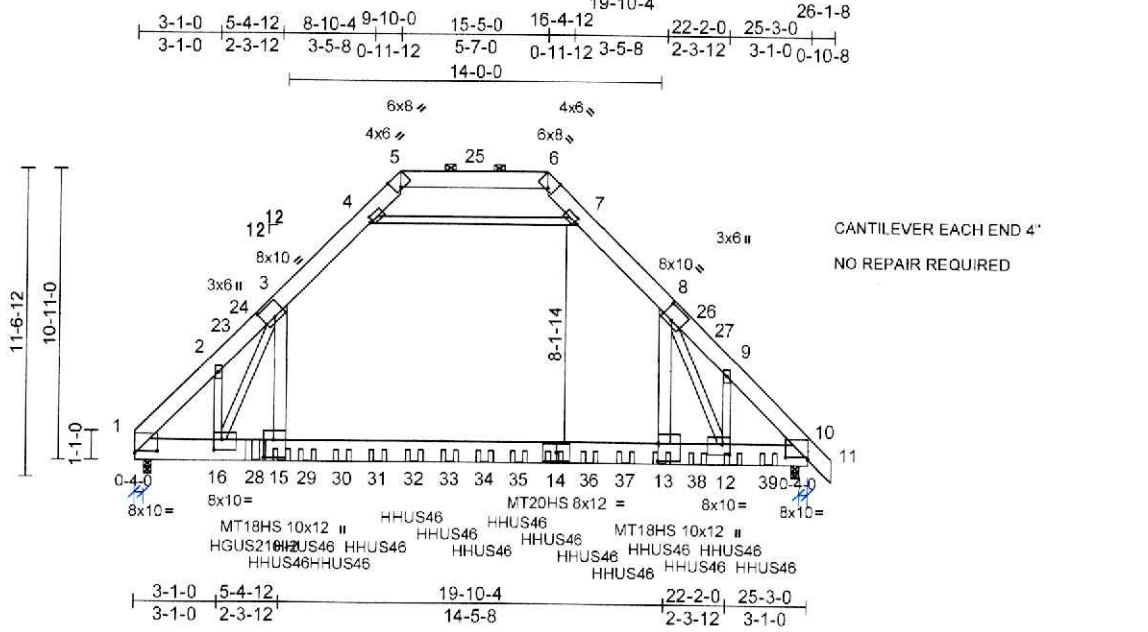
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE

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818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss C2G2	Truss Type Attic Girder	Qty 2	Ply 4	3202 Leaflet Church-Roof-Abby GRH	164834851
Carter Components (Sanford, NC), Sanford, NC - 27332,					Run: 8:73 S Mar 21 2024 Print 8:730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:35:58 ID:K20cB9CitzZseXKexRiGDBzALPk-RIC?PsB70Hq3NSgPqnlaw3uitXbGKWicDoi7J4zJC?F	Page: 1



Scale = 1/86.3

Plate Offsets (X, Y): [1:0-10-0-0-1-0], [3:0-5-0-0-6-0], [4:0-1-9-0-2-4], [5:0-2-14,Edge], [6:0-2-14,Edge], [7:0-1-9-0-2-4], [8:0-5-0-0-6-0], [10:0-10-0-0-1-0], [12:0-3-8,0-4-12], [13:0-8-0,Edge], [15:0-8-0-0-4-8], [16:0-3-8,0-4-12]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/def	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.84	Vert(LL)	-0.36	13-15	>847	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.51	Vert(CT)	-0.56	13-15	>542	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	NO	WB	0.70	Horz(CT)	0.01	10	n/a	n/a	MT18HS	244/190
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH	Attic	-0.22	13-15	>778	360			
BCDL	10.0											

Weight: 1071 lb FT = 20%

LUMBER
TOP CHORD 2x8 SP 2400F 2.0E
BOT CHORD 2x10 SP 2400F 2.0E
WEBS 2x4 SP No.3 *Except* 3-15,8-13:2x6 SP No 2, 4-7:2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (10-0-0 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=0-3-8, 10=0-3-8
Max Horiz 1=247 (LC 10)
Max Grav 1=9965 (LC 47), 10=8396 (LC 19)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-11831/0, 2-3=-15233/0, 3-4=-4756/0, 4-5=0/3967, 5-6=0/5676, 6-7=0/3686, 7-8=-5018/0, 8-9=-12546/0, 9-10=-9436/0, 10-11=0/37
BOT CHORD 1-16=-46/8419, 15-16=0/6515, 13-15=0/6515, 12-13=0/6515, 10-12=0/6697
WEBS 2-16=-5088/0, 3-16=-1234/5819, 3-15=0/9731, 8-13=0/9778, 8-12=-1378/1814, 9-12=-4537/0, 4-7=-12104/0

NOTES
1) 4-ply truss to be connected together as follows:
Top chords connected with 10d (0.131"x3") nails as follows: 2x8 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected with Simpson SDS 1/4 x 6 screws as follows: 2x10 - 3 rows staggered at 0-4-0 oc.
Web chords connected with 10d (0.131"x3") nails as follows: 2x4 - 2 rows staggered at 0-6-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.

- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat II; Exp B: Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL; Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- The Fabrication Tolerance at joint 15 = 4%, joint 13 = 4%
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Ceiling dead load (5.0 psf) on member(s). 3-4, 7-8, 4-7; Wall dead load (5.0psf) on member(s).3-15, 8-13
- Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 13-15

- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - Use Simpson Strong-Tie HGUS210-2 (46-10d Girder, 16-10d Truss) or equivalent at 4-6-0 from the left end to connect truss(es) to front face of bottom chord.
 - Use Simpson Strong-Tie HHUS46 (14-10d Girder, 6-10d Truss, Single Ply Girder) or equivalent spaced at 1-4-0 oc max. starting at 5-5-12 from the left end to 23-9-4 to connect truss(es) to front face of bottom chord.
 - Fill all nail holes where hanger is in contact with lumber.
 - Attic room checked for L/360 deflection.
- LOAD CASE(S)** Standard
- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)



April 12, 2024

Continued on page 2

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TRENCO
A MITTEK AFFILIATE

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	3202 Leaflet Church-Roof-Abby GRH	I64834851
24020093-01	C2G2	Attic Girder	2	4	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 2

Vert: 1-3=-60, 3-4=-70, 4-5=-60, 5-6=-60, 6-7=-60,
 7-8=-70, 8-11=-60, 15-17=-20, 13-15=-30,
 13-20=-20, 4-7=-10
 Drag: 3-15=-10, 8-13=-10

Concentrated Loads (lb)

Vert: 14=-143 (F), 15=-143 (F), 13=-551 (F), 12=-551 (F),
 28=-6706 (F), 29=-143 (F), 30=-143 (F), 31=-143 (F),
 32=-143 (F), 33=-143 (F), 34=-143 (F), 35=-143 (F),
 36=-143 (F), 37=-143 (F), 38=-551 (F), 39=-551 (F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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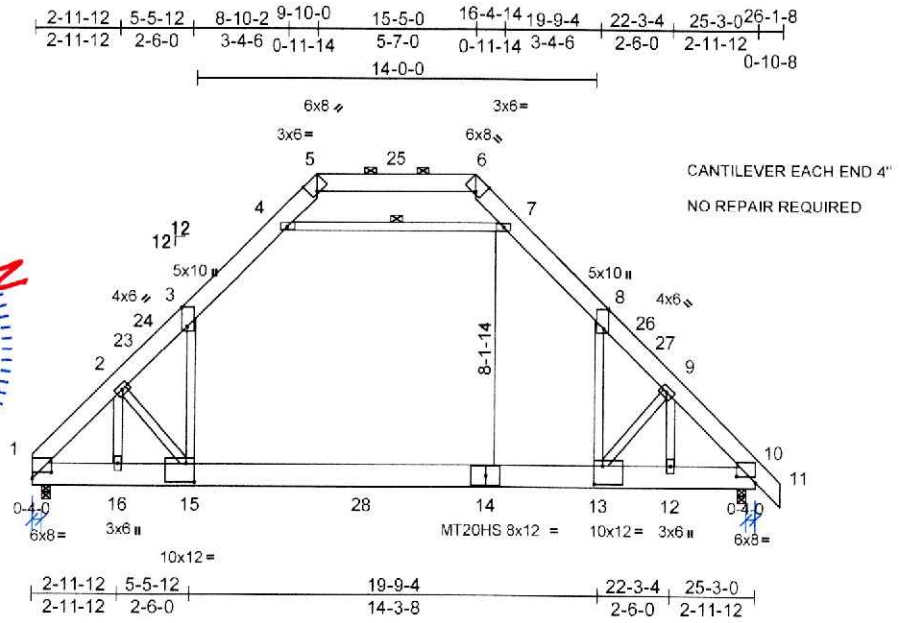
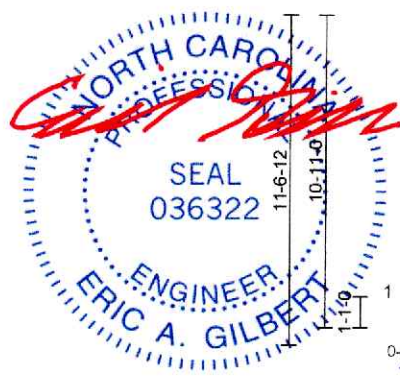


818 Soundside Road
 Edenton, NC 27932

Job 24020093-01	Truss C2G3	Truss Type Attic Girder	Qty 1	Ply 2	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834852
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Carter Components (Sanford, NC), Sanford, NC - 27332

Run: 8:73 S Mar 21 2024 Print: 8:730 S Mar 21 2024 MITek Industries, Inc. Thu Apr 11 07:35:59
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Scale = 1:80.3

Plate Offsets (X, Y): [1:0-8-0,0-2-8], [3:0-8-2,Edge], [5:0-2-14,Edge], [6:0-2-14,Edge], [8:0-8-2,Edge], [10:0-8-0,0-2-8], [13:0-3-8,0-7-12], [15:0-3-8,0-7-12]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.74	Vert(LL)	-0.46	13-15	>663	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.79	Vert(CT)	-0.68	13-15	>449	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	NO	WB	0.66	Horz(CT)	0.01	1	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH		Attic	-0.30	13-15	>564	360		
BCDL	10.0											

Weight: 508 lb FT = 20%

- LUMBER**
- TOP CHORD 2x8 SP 2400F 2.0E
 - BOT CHORD 2x10 SP 2400F 2.0E
 - WEBS 2x4 SP No.3 *Except* 3-15,8-13,4-7:2x4 SP No.2
- BRACING**
- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (10-0-0 max.): 5-6.
 - BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 - WEBS 1 Row at midpt 4-7
- REACTIONS**
- (size) 1=0-3-8, 10=0-3-8
 - Max Horiz 1=-247 (LC 10)
 - Max Uplift 1=-98 (LC 12), 10=-130 (LC 13)
 - Max Grav 1=3418 (LC 46), 10=3626 (LC 48)
- FORCES**
- (lb) - Maximum Compression/Maximum Tension
 - TOP CHORD 1-2=-4016/125, 2-3=-6213/291, 3-4=-2664/215, 4-5=-20/1745, 5-6=-43/2618, 6-7=-23/1777, 7-8=-2635/212, 8-9= 6305/300, 9-10=-4227/143, 10-11=0/37
 - BOT CHORD 1-16=-188/3062, 15-16=-176/3062, 13-15=-80/3352, 12-13=-32/3040, 10-12=-32/3040
 - WEBS 2-16=-3361/279, 2-15=-251/795, 3-15=-217/4901, 8-13=-232/5076, 9-13=-233/558, 9-12=-3154/267, 4-7=-5883/321
- NOTES**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x8 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x10 - 4 rows staggered at 0-7-0 oc.
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Ceiling dead load (5.0 psf) on member(s). 3-4, 7-8, 4-7; Wall dead load (5.0psf) on member(s).3-15, 8-13
 - Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 13-15
 - LGT2 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 10. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - LGT2 Hurricane ties must have two studs in line below the truss.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2317 lb down and 210 lb up at 11-5-8, and 1462 lb down and 132 lb up at 15-9-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - Attic room checked for L/360 deflection.
- LOAD CASE(S)** Standard
- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-3=-60, 3-4=-70, 4-5=-60, 5-6=-60, 6-7=-60, 7-8=-70, 8-11=-60, 15-17=-20, 13-15=-30, 13-20=-20, 4-7=-10

April 12, 2024

Continued on page 2

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ENGINEERING BY
TRENCO
A MITek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	3202 Leaflet Church-Roof-Abby GRH	I64834852
24020093-01	C2G3	Attic Girder	1	2	Job Reference (optional)	

Carler Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:35:59
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Page: 2

Drag: 3-15=-10, 8-13=-10
 Concentrated Loads (lb)
 Vert: 14=-836 (F), 28=-1325 (F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

ENGINEERING BY
TRENCO
 A MiTek Affiliate

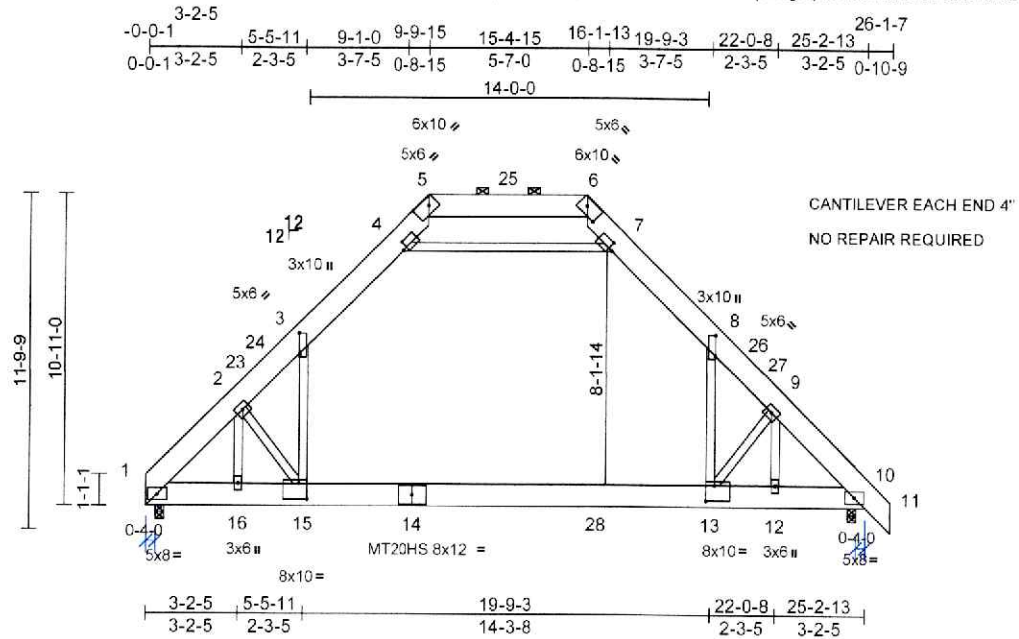
818 Soundside Road
 Edenton, NC 27932

Job 24020093-01	Truss C2G4	Truss Type Attic Girder	Qty 1	Ply 2	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834853
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Carter Components (Sanford, NC), Sanford, NC - 27332.

Run: 8.73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:35:59
ID:9CNURCHTTPK?MP?oA6N6SSzaLPe-RfC?PsB70hQ3NSgPqnL8w3ulTXbGKWvCD0i7J4zJC?F

Page 1



Scale = 1:80.6

Plate Offsets (X, Y): [3:0-8-8,0-0-4], [4:0-1-13,0-3-0], [5:0-6-7,0-3-0], [6:0-6-7,0-3-0], [7:0-1-13,0-3-0], [8:0-8-8,0-0-4], [13:0-3-8,0-6-8], [15:0-3-8,0-6-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.34	Vert(LL)	-0.20	13-15	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.40	Vert(CT)	-0.31	13-15	>991	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	NO	WB	0.74	Horz(CT)	0.01	10	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH		Attic	-0.14	13-15	>999	360		
BCDL	10.0											

Weight: 568 lb FT = 20%

LUMBER

TOP CHORD 2x10 SP 2400F 2.0E
BOT CHORD 2x10 SP 2400F 2.0E
WEBS 2x4 SP No.3 *Except* 4-7:2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (10-0-0 max.); 5-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 1=0-3-8, 10=0-3-8
Max Horiz 1=-248 (LC 10)
Max Uplift 10=-37 (LC 13)
Max Grav 1=2154 (LC 46), 10=2571 (LC 48)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2648/0, 2-3=-3648/37, 3-4=-1830/145, 4-5=-8917, 5-6=0/1491, 6-7=0/1053, 7-8=-1710/134, 8-9=-4046/73, 9-10=-3090/17, 10-11=0/37
BOT CHORD 1-16=-100/1955, 15-16=-64/1955, 13-15=0/2089, 12-13=0/2113, 10-12=0/2113
WEBS 2-16=-1668/101, 2-15=-235/574, 3-15=0/2479, 8-13=-23/3240, 9-13=-448/266, 9-12=-1443/87, 4-7=-3493/104

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-7-0 oc.
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.

- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Ceiling dead load (5.0 psf) on member(s). 3-4, 7-8, 4-7; Wall dead load (5.0psf) on member(s). 3-15, 8-13
- Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 13-15
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at Jt(s) 10. This connection is for uplift only and does not consider lateral forces.

- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1462 lb down and 132 lb up at 15-9-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- Attic room checked for L/360 deflection.

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-3=-60, 3-4=-70, 4-5=-60, 5-6=-60, 6-7=-60, 7-8=-70, 8-11=-60, 15-17=-20, 13-15=-30, 13-20=-20, 4-7=-10
Drag: 3-15=-10, 8-13=-10
Concentrated Loads (lb)



April 12, 2024

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomp.com)



818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss C2G4	Truss Type Attic Girder	Qty 1	Ply 2	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834853
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:35:59
ID 9CNuRCHTTpK?Mf?oA6N6SSzaLPe-RfC?PsB70Hq3NSgPqnLw3ulTxbGKwCDoi7J4zJC?f

Page: 2

Vert: 28=836 (F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 1/2/2023 BEFORE USE.

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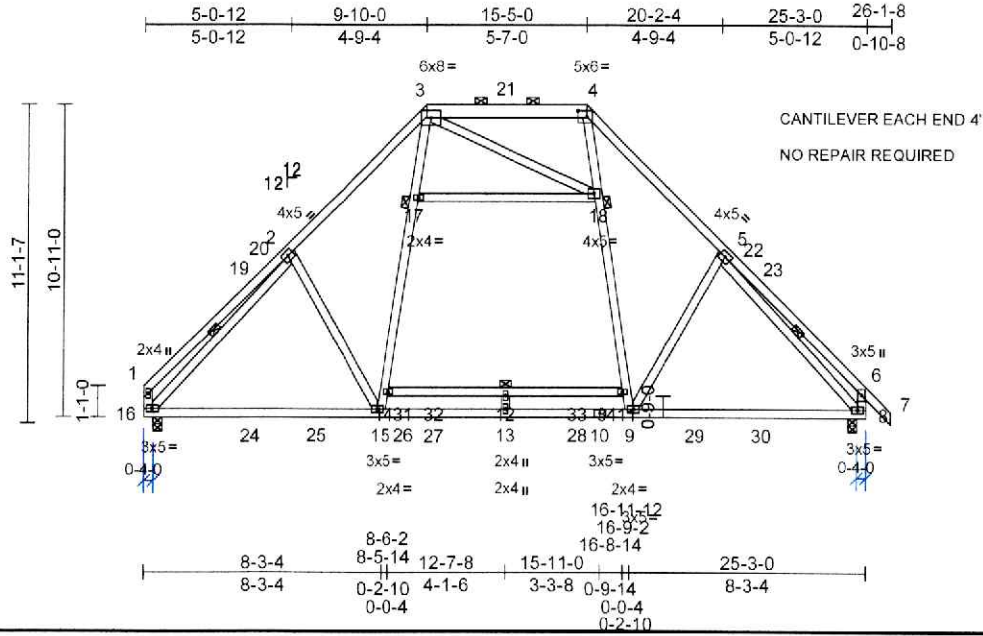
818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss C3	Truss Type Piggyback Base	Qty 7	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834854
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8 73 S Mar 21 2024 Print: 8 730 S Mar 21 2024 MITek Industries, Inc. Thu Apr 11 07:35:59
ID:OxQIKHO6LaSkx2BxBV1DKLzLaLPV-RfC?PsB70Hq3NSgPqnL6w3uITXbGKwVCDoi7J4zJC?7

Page: 1



Scale = 1/80.4

Plate Offsets (X, Y): [4-0-3-12,0-2-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.62	Vert(LL)	-0.34	12	>879	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.98	Vert(CT)	-0.63	12	>478	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(CT)	0.04	8	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 199 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except* 3-4:2x6 SP No.2
BOT CHORD 2x4 SP No.1 *Except* 14-11:2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 15-3,9-4:2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-4 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 3-4.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 2-16, 5-8
JOINTS 1 Brace at Jt(s): 17, 18

REACTIONS (size) 8=0-3-8, 16=0-3-8
Max Horiz 16=273 (LC 12)
Max Grav 8=1623 (LC 45), 16=1571 (LC 45)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-384/167, 2-3=-1704/31, 3-4=-989/114, 4-5=-1698/31, 5-6=-440/212, 6-7=0/43, 1-16=-345/148, 6-8=-449/203
BOT CHORD 15-16=-2/1302, 13-15=0/1024, 9-13=0/1024, 8-9=0/1164, 12-14=-6/10, 11-12=-6/10
WEBS 2-15=-245/286, 5-9=-236/285, 2-16=-1549/0, 5-8=-1510/0, 14-15=-9/772, 14-17=0/922, 3-17=0/915, 4-18=0/858, 11-18=0/912, 9-11=-7/761, 12-13=-127/1, 17-18=-65/68, 3-18=-161/159

NOTES
1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior (1) 3-1-12 to 5-7-1, Exterior(2R) 5-7-1 to 19-7-15, Interior (1) 19-7-15 to 23-1-8, Exterior(2E) 23-1-8 to 26-1-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 200.0lb AC unit load placed on the bottom chord, 12-7-8 from left end, supported at two points, 5-0-0 apart.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



April 12, 2024

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Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpiinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbccomponents.com)



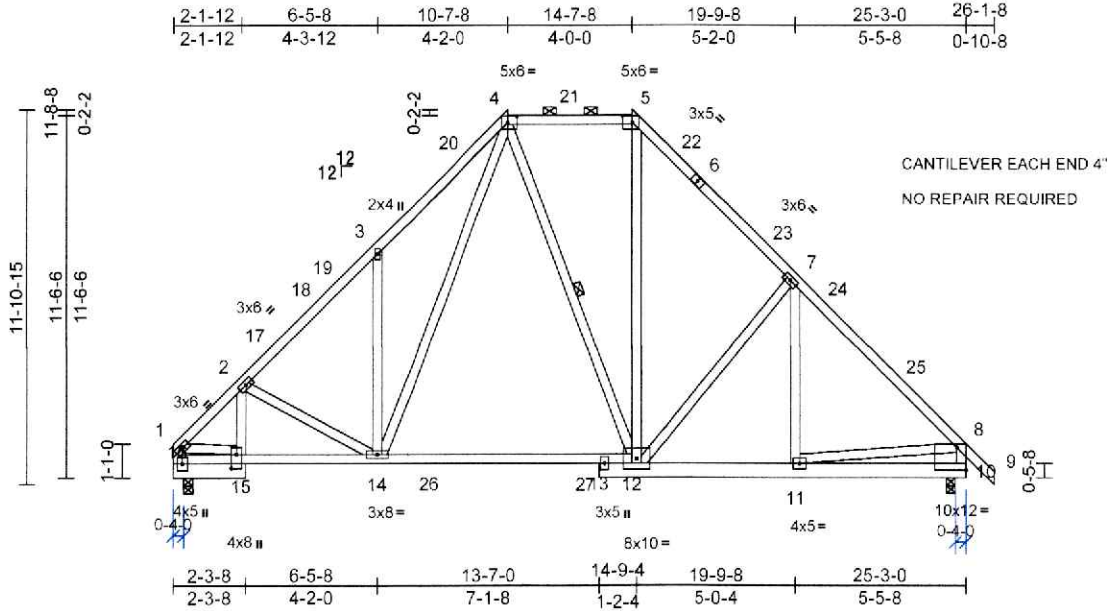
818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss C4	Truss Type Hip	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834855
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Carter Components (Sanford, NC), Sanford, NC - 27332

Run: 8.73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MITek Industries, Inc. Thu Apr 11 07:35:59
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Page 1



Scale = 1/73.2

Plate Offsets (X, Y): [4:0-3-9,0-2-8], [5:0-3-9,0-2-8], [10:Edge,0-8-9], [15:0-5-4,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.48	Vert(LL)	-0.14	12-14	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.68	Vert(CT)	-0.24	12-14	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.69	Horz(CT)	0.04	10	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 208 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x6 SP No.2 *Except* 15-2:2x4 SP No.3,
 16-12:2x4 SP No.2
 WEBS 2x4 SP No.3 *Except* 14-4,12-4,12-5:2x4 SP
 No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or
 4-5-11 oc purlins, except end verticals, and
 2-0-0 oc purlins (6-0-0 max.): 4-5.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc
 bracing.
 WEBS 1 Row at midpt 4-12

REACTIONS

(size) 10=0-3-8, 16=0-3-8
 Max Horiz 16=-287 (LC 10)
 Max Uplift 10=-90 (LC 15), 16=-70 (LC 14)
 Max Grav 10=1284 (LC 45), 16=1243 (LC 45)

FORCES

(lb) - Maximum Compression/Maximum
 Tension
 TOP CHORD 1-2=-1260/108, 2-3=-1419/159,
 3-4=-1499/366, 4-5=-745/225,
 5-7=-1204/227, 7-8=-1344/149, 8-9=0/43,
 1-16=-1171/90, 8-10=-1172/152
 BOT CHORD 15-16=-251/297, 14-15=-184/1088,
 12-14=-68/799, 11-12=0/908, 10-11=-97/231,
 2-15=-285/48
 WEBS 2-14=-66/142, 4-14=-288/817, 4-12=-96/209,
 5-12=-81/522, 7-12=322/237, 7-11=-104/58,
 8-11=0/731, 1-15=-45/850, 3-14=-424/276

NOTES

1) Unbalanced roof live loads have been considered for
 this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust)
 Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.
 II; Exp B; Enclosed; MWFRS (envelope) exterior zone
 and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior (1) 3-1-12
 to 6-4-9, Exterior(2R) 6-4-9 to 18-10-7, Interior (1)
 18-10-7 to 23-1-8, Exterior(2E) 23-1-8 to 26-1-8 zone;
 cantilever left and right exposed; end vertical left and
 right exposed; C-C for members and forces & MWFRS
 for reactions shown; Lumber DOL=1.60 plate grip
 DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15
 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate
 DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9;
 Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this
 design.
- This truss has been designed for greater of min roof live
 load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on
 overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom
 chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf
 on the bottom chord in all areas where a rectangle
 3-06-00 tall by 2-00-00 wide will fit between the bottom
 chord and any other members, with BCDL = 10.0psf.
- H10A Simpson Strong-Tie connectors recommended to
 connect truss to bearing walls due to UPLIFT at j(s) 16
 and 10. This connection is for uplift only and does not
 consider lateral forces.
- This truss is designed in accordance with the 2018
 International Residential Code sections R502.11.1 and
 R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size
 or the orientation of the purlin along the top and/or
 bottom chord.

LOAD CASE(S) Standard



April 12, 2024

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Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



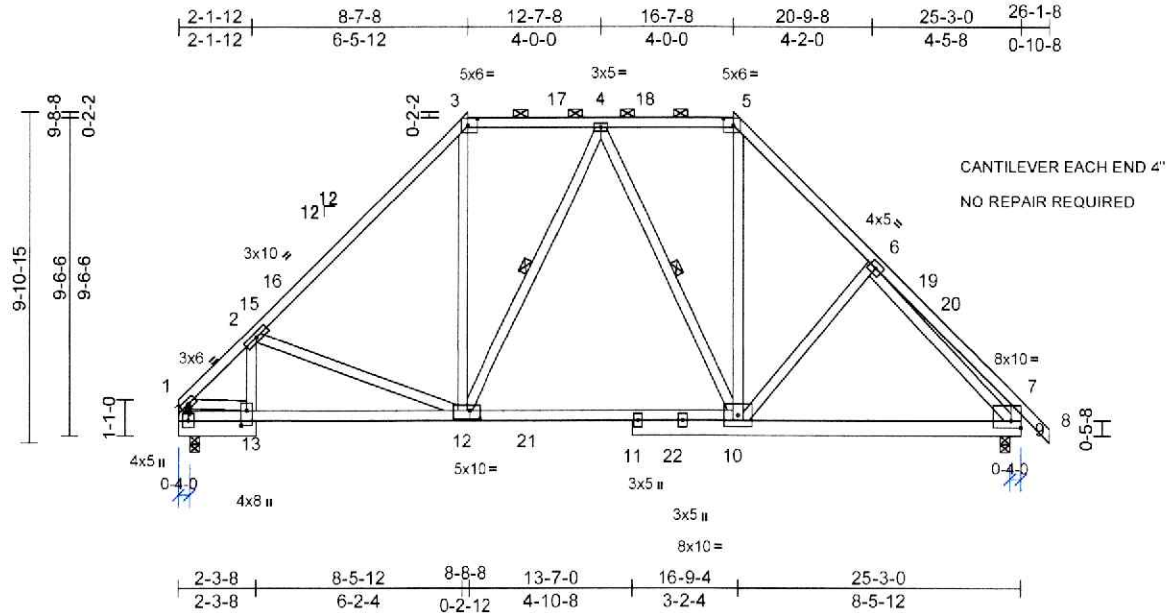
818 Soundside Road
 Edenton, NC 27932

Job 24020093-01	Truss C5	Truss Type Hip	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834856
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:36:00
ID vglDmYqbNmWARGPv11ozalQa-RC?PsB70Hq3NSgPqnL8w3uITXbGKwRCDoI7J4zJC7f

Page: 1



Scale = 1/64

Plate Offsets (X, Y): [3:0-3-9,0-2-8], [5:0-3-9,0-2-8], [7:Edge,0-2-8], [12:0-3-12,0-3-0], [13:0-5-4,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.87	Vert(LL)	-0.14	10-12	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.64	Vert(CT)	-0.22	10-12	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.83	Horz(CT)	0.04	9	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 197 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x6 SP No.2 *Except* 13-2:2x4 SP No.3,
 14-12,12-10:2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or
 2-2-0 oc purlins, except end verticals, and
 2-0-0 oc purlins (6-0-0 max.); 3-5.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc
 bracing, Except:
 6-0-0 oc bracing: 13-14.
 WEBS 1 Row at midpt 4-12, 4-10

REACTIONS

(size) 9=0-3-8, 14=0-3-8
 Max Horiz 14=240 (LC 10)
 Max Uplift 9=103 (LC 15), 14=83 (LC 14)
 Max Grav 9=1243 (LC 45), 14=1190 (LC 45)

FORCES

(lb) - Maximum Compression/Maximum
 Tension
 TOP CHORD 1-2=-1249/92, 2-3=-1282/169, 3-4=-769/199,
 4-5=-773/181, 5-6=-1211/188, 6-7=-394/166,
 7-8=0/43, 1-14=-1146/70, 7-9=408/169
 BOT CHORD 13-14=-208/227, 10-13=-206/1126,
 9-10=0/865, 2-13=-271/113
 WEBS 2-12=-317/227, 3-12=-24/529,
 4-12=-246/149, 5-10=-53/565,
 6-10=-217/206, 6-9=-1042/15,
 4-10=260/157, 1-13=102/944

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust)
 Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II, Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior (1) 3-1-12 to 4-4-9, Exterior(2R) 4-4-9 to 20-11-7, Interior (1) 20-11-7 to 23-1-8, Exterior(2E) 23-1-8 to 26-1-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14 and 9. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcccomponents.com)



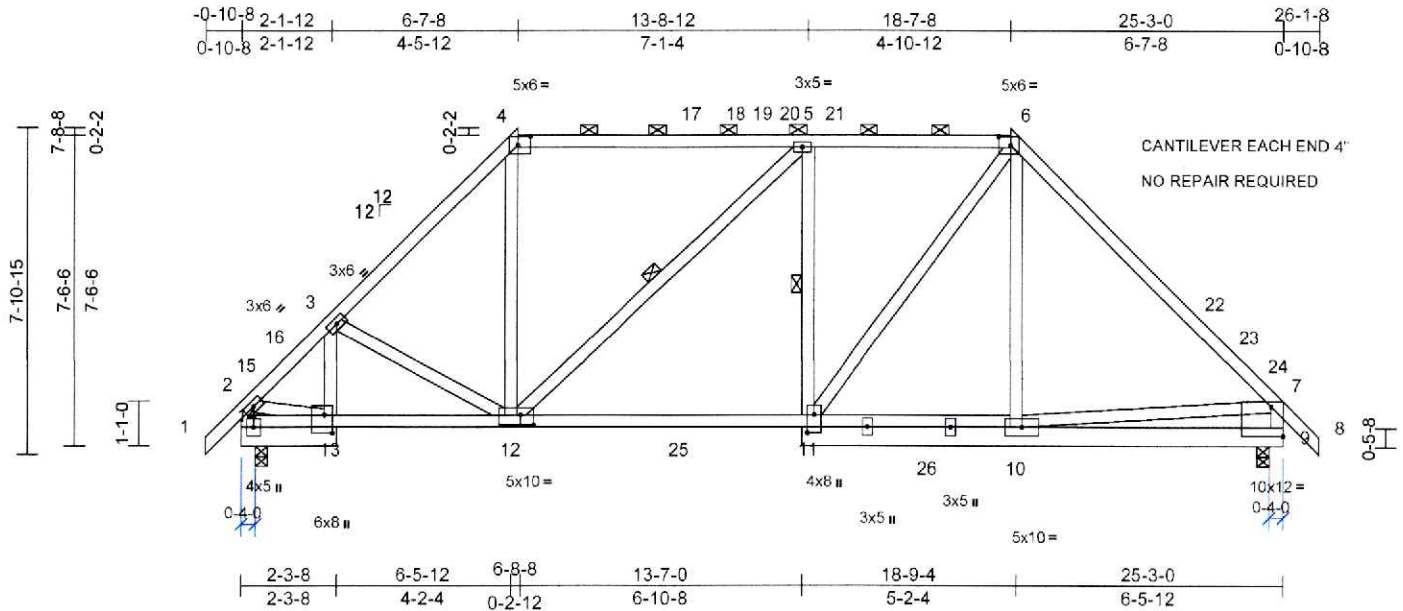
818 Soundside Road
 Edenton, NC 27932

Job 24020093-01	Truss C6	Truss Type Hip	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834857
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Carter Components (Sanford, NC), Sanford, NC - 27332.

Run: 8:73 S Mar 21 2024 Print: 8:730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:36 00
ID: V534DFWXISOXJ_hhzNIKP9zaLQd-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcDdoi7J4zJC7?

Page: 1



Scale = 1/55.8

Plate Offsets (X, Y): [4:0-3-9,0-2-8], [6:0-3-9,0-2-8], [9:Edge,0-8-9], [11:0-5-4,0-2-0], [12:0-3-12,0-3-0], [13:0-5-4,0-2-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.89	Vert(LL)	-0.09	11-12	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.69	Vert(CT)	-0.16	11-12	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.34	Horz(CT)	0.03	9	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 189 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except* 4-6:2x4 SP No.1
BOT CHORD 2x6 SP No.2 *Except* 13-3,5-11:2x4 SP No.3, 14-12,12-10:2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 14-2,9-7:2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals, and 2-0-0 oc purlins (4-7-10 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 13-14.
6-0-0 oc bracing: 5-11
WEBS 1 Row at midpt 5-12

REACTIONS (size) 9=0-3-8, 14=0-3-8
Max Horiz 14=201 (LC 13)
Max Uplift 9=113 (LC 15), 14=113 (LC 14)
Max Grav 9=1212 (LC 45), 14=1201 (LC 45)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-1198/128, 3-4=-1310/186, 4-5=-911/191, 5-6=-1254/208, 6-7=-1295/168, 7-8=0/43, 2-14=-1139/140, 7-9=-1104/175
BOT CHORD 13-14=-174/185, 11-13=-178/1265, 10-11=-34/873, 9-10=-243/539, 3-13=-223/61, 5-11=-404/192
WEBS 3-12=-176/181, 4-12=-6/568, 5-12=-482/126, 6-11=-137/669, 6-10=-4/176, 7-10=-132/749, 2-13=-63/825

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E)-0-10-8 to 2-1-12, Exterior(2R) 2-1-12 to 10-10-7, Interior (1) 10-10-7 to 14-4-9, Exterior (2R) 14-4-9 to 23-1-8, Exterior(2E) 23-1-8 to 26-1-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- The Fabrication Tolerance at joint 7 = 16%
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14 and 9. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpiinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbccomponents.com)

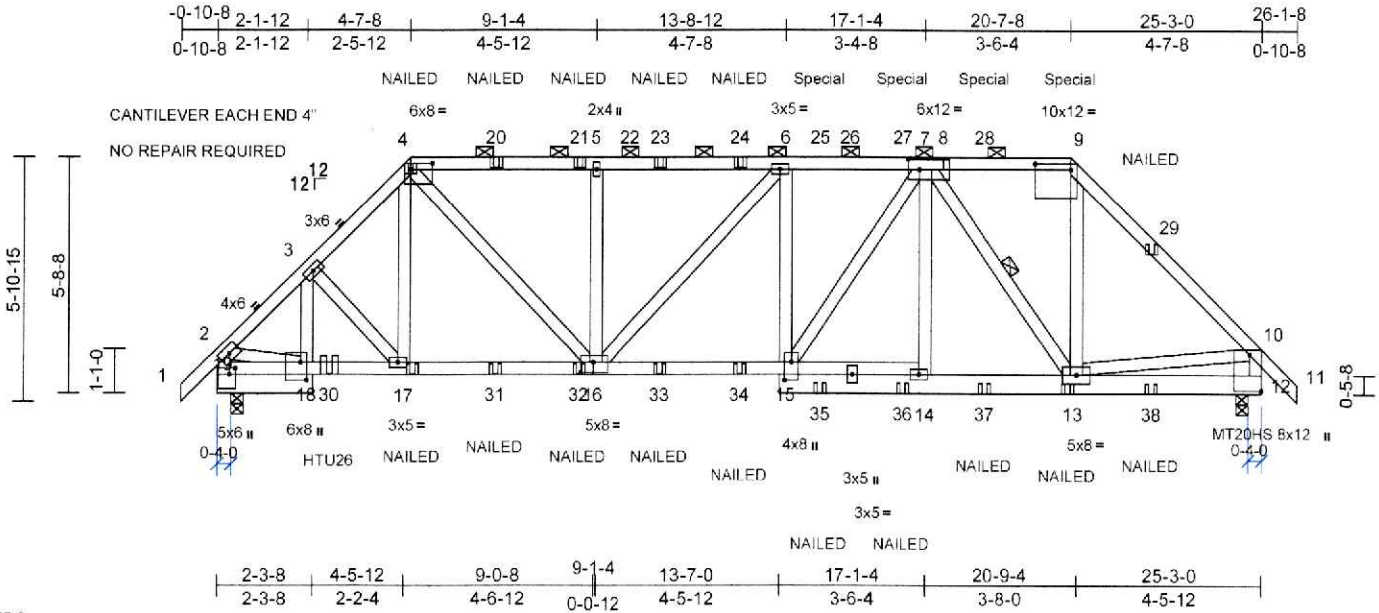
TRENCO ENGINEERING BY A MITEK AFFILIATE
618 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss C7	Truss Type Hip Girder	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834858
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8 73 S Mar 21 2024 Print: 8 730 S Mar 21 2024 MITek Industries, Inc. Thu Apr 11 07:36:00
ID: MK9IFLD2f494aSyWMUWJzaLPY-RfC?PsB70Hq3NSgPqnLbW3uITXbGKWrCDor7J4zJC?F

Page: 1



Scale = 1.55:6	Plate Offsets (X, Y): [4:0-6-4,0-1-12], [8:0-3-4,0-3-0], [9:0-10-4,0-1-12], [12:Edge,0-3-8], [15:0-5-4,0-2-0], [16:0-3-12,0-3-0], [18:0-5-4,0-1-12], [19:0-1-12,0-1-12]
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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.83	Vert(LL)	-0.13	15-16	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.89	Vert(CT)	-0.20	15-16	>999	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	NO	WB	0.64	Horz(CT)	0.07	12	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 194 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2 *Except* 18-3,6-15:2x4 SP No.3, 19-16,16-14:2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins, except end verticals, and 2-0-0 oc purlins (2-7-6 max.): 4-9.
BOT CHORD Rigid ceiling directly applied or 5-5-13 oc bracing.
WEBS 1 Row at midpt 7-13

REACTIONS (size) 12=0-3-8, 19=0-3-8
Max Horiz 19=158 (LC 11)
Max Uplift 12=-849 (LC 13), 19=-863 (LC 12)
Max Grav 12=2211 (LC 37), 19=2280 (LC 37)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/72, 2-3=-2327/955, 3-4=-2519/1051, 4-5=-2788/1147, 5-6=-2797/1151, 6-7=-2897/1175, 7-9=-1611/725, 9-10=-2387/950, 10-11=0/72, 2-19=-2232/879, 10-12=-2087/811
BOT CHORD 18-19=-174/221, 17-18=-738/1650, 15-17=-1170/2908, 14-15=-967/2437, 13-14=-964/2426, 12-13=-171/313, 3-18=-335/97, 6-15=-460/262
WEBS 3-17=-112/252, 4-17=-105/313, 4-16=-636/1547, 5-16=-734/355, 6-16=-199/114, 7-15=-365/860, 7-14=-97/99, 7-13=-1464/595, 9-13=-456/1181, 10-13=-600/1438, 2-18=-587/1542

NOTES
1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 19 and 12. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Use Simpson Strong-Tie HTU26 (10-10d Girder, 14-10dx1 1/2 Truss, Single Ply Girder) or equivalent at 2-8-4 from the left end to connect truss(es) to back face of bottom chord.
- Fill all nail holes where hanger is in contact with lumber.

- "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 222 lb down and 125 lb up at 14-6-12, 222 lb down and 125 lb up at 16-6-12, and 222 lb down and 125 lb up at 18-6-12, and 202 lb down and 125 lb up at 20-7-8 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- LOAD CASE(S)** Standard
- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-2=-60, 2-4=-60, 4-9=-60, 9-10=-60, 10-11=-60, 18-19=-20, 15-18=-20, 12-15=-20
Concentrated Loads (lb)



April 12, 2024

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbccomponents.com)

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A MITEK AFFILIATE

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)
24020093-01	C7	Hip Girder	1	1	I64834858

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:36:00
ID: _Mk9IFLD2f494aSyWMUWjzaLPY-RfC?PsB70Hq3NSgPqL8w3uITxbGKWCDoi7J4zJC?f

Page: 2

Vert: 9=-174 (B), 17=-55 (B), 4=-143 (B), 13=-25 (B),
20=-143 (B), 21=-143 (B), 23=-143 (B), 24=-143 (B),
25=-174 (B), 27=-174 (B), 28=-174 (B), 29=-34 (B),
30=-246 (B), 31=-55 (B), 32=-55 (B), 33=-55 (B),
34=-55 (B), 35=-25 (B), 36=-25 (B), 37=-25 (B),
38=-160 (B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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TRENCO
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818 Soundside Road
Edenton, NC 27932

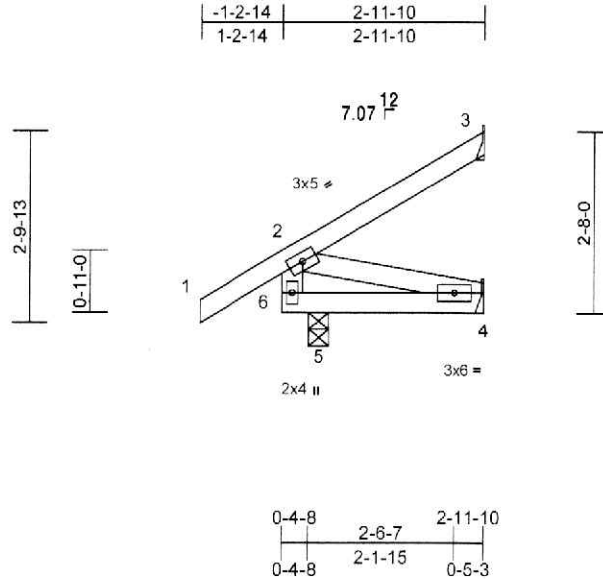
Job 24020093-01	Truss CJ1	Truss Type Jack-Open	Qty 2	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834859
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Carter Components (Sanford, NC), Sanford, NC - 27332.

Run: 8:73 E Nov 16 2023 Print: 8:730 E Nov 16 2023 M/Tek Industries, Inc. Fri Apr 12 09:44:42
ID: Ck8RIGRYyJVxzveL3P7hdhzaLQk-a1?uNB5xFEIIDDZgErVPGA9h9KtZlypWRdz1RjzRVGZ

Page: 1

CANTILEVER LEFT END 4"
NO REPAIR REQUIRED



Scale = 1/32.8

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	0.01	4-5	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.27	Vert(CT)	0.01	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 17 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-11-10 oc purlins. except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS

(lb/size) 3=62/ Mechanical, 4=-5/ Mechanical, 5=248/0-3-8
Max Horiz 5=77 (LC 14)
Max Uplift 3=41 (LC 14), 4=-22 (LC 21), 5=-20 (LC 14)
Max Grav 3=99 (LC 21), 4=38 (LC 7), 5=375 (LC 21)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-6=-297/130, 1-2=0/67, 2-3=-84/42
BOT CHORD 5-6=-12/7, 4-5=-160/38
WEBS 2-4=-39/164

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II: Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner (3) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Bearings are assumed to be: Joint 3 User Defined crushing capacity of 425 psi, Joint 4 User Defined crushing capacity of 425 psi, Joint 5 SP No.2 crushing capacity of 565 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 41 lb uplift at joint 3, 22 lb uplift at joint 4 and 20 lb uplift at joint 5.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbccomponents.com)

ENGINEERING BY
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818 Soundside Road
Edenton, NC 27932

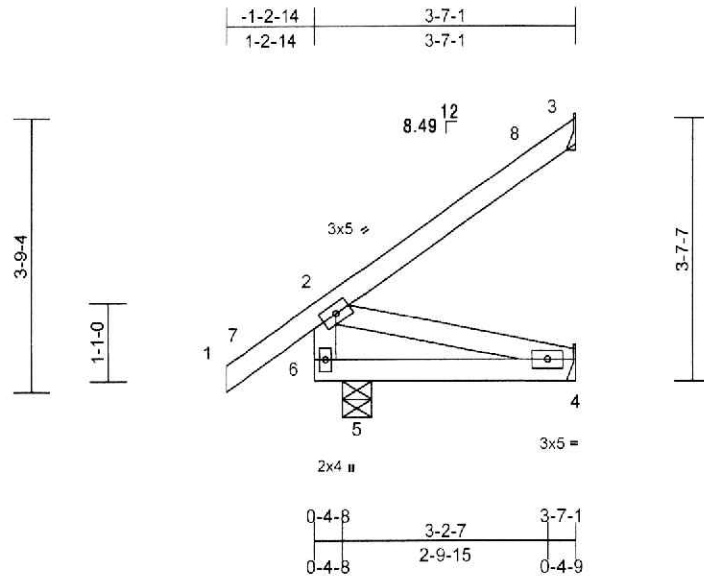
Job 24020093-01	Truss CJ2	Truss Type Jack-Open	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834860
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8:73 E Nov 16 2023 Print: 8:730 E Nov 16 2023 MiTek Industries, Inc. Fri Apr 12 09:45:14
ID: gxpjycRBJcdob3DXd6ew9uzalQj-eBx_57U0j3KK38?q6ZUvZwU88DnNnQLDobjjaszRVG3

Page: 1

CANTILEVER LEFT END 4"
NO REPAIR REQUIRED



Scale = 1:31.6

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.27	Vert(LL)	0.01	4-5	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.34	Vert(CT)	0.01	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(CT)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MP								
BCDL	10.0											
										Weight: 20 lb	FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-7-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=84/ Mechanical, 4=2/ Mechanical, 5=269/0-4-9
Max Horiz 5=107 (LC 14)
Max Uplift 3=63 (LC 14), 4=15 (LC 21)
Max Grav 3=148 (LC 21), 4=50 (LC 7), 5=405 (LC 21)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-6=-321/122, 1-7=0/12, 2-7=0/73,
2-8=-113/39, 3-8=-70/66
BOT CHORD 5-6=-15/8, 4-5=-240/57
WEBS 2-4=-58/248

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed: MWFRS (envelope) exterior zone and C-C Corner (3) -1-2-14 to 3-0-1, Exterior(2R) 3-0-1 to 3-6-5 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Bearings are assumed to be: Joint 3 User Defined crushing capacity of 425 psi, Joint 5 SP No.2 crushing capacity of 565 psi, Joint 4 User Defined crushing capacity of 425 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 3 and 15 lb uplift at joint 4.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)



818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss CJ2T	Truss Type Jack-Open	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	I64834861
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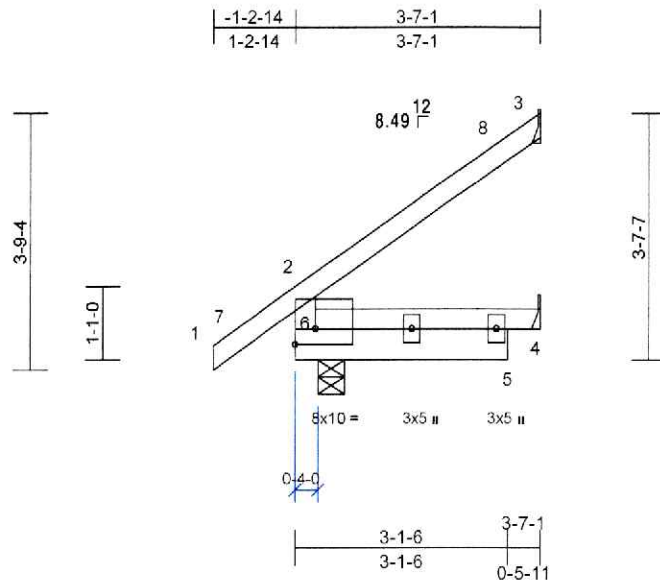
Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1

CANTILEVER LEFT END 4"

NO REPAIR REQUIRED



Scale = 1.33.7

Plate Offsets (X, Y): [6:Edge,0-2-12]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.50	Vert(LL)	0.00	4-6	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.06	Vert(CT)	0.00	4-6	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(C1)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MR								
BCDL	10.0											
											Weight: 22 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x6 SP No.2 *Except* 6-4:2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-7-1 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size) 3= Mechanical, 4= Mechanical, 6=0-4-9
 Max Horiz 6=109 (LC 14)
 Max Uplift 3=-64 (LC 14), 4=-5 (LC 14), 6=-1 (LC 14)
 Max Grav 3=137 (LC 21), 4=68 (LC 7), 6=355 (LC 21)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 2-6=-332/172, 1-2=0/73, 2-3=-119/59
 BOT CHORD 4-6=-9/0

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft, Cat II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner (3) -1-2-14 to 3-0-1, Exterior(2R) 3-0-1 to 3-6-5 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be User Defined .
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1 lb uplift at joint 6, 64 lb uplift at joint 3 and 5 lb uplift at joint 4.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 12, 2024

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinl.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com).



818 Soundside Road
 Edenton, NC 27932

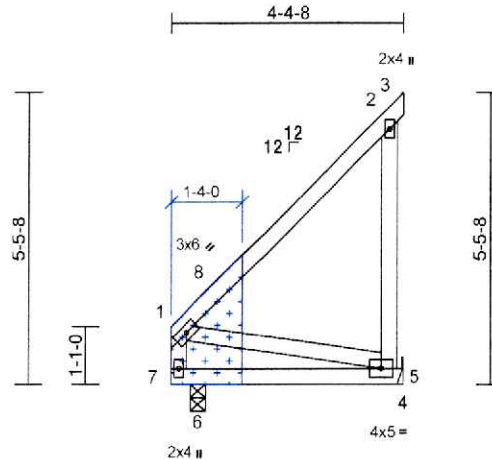
Job 24020093-01	Truss D1	Truss Type Jack-Closed	Qty 4	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834862
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Carter Components (Sanford, NC), Sanford, NC - 27332,

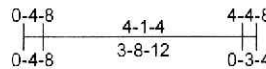
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Page: 1

CANTILEVER LEFT END 4"



ATTACH 1/2" PLYWOOD OR OSB GUSSET (15/32" RATED SHEATHING 32/16 EXP 1) TO EACH FACE OF TRUSS WITH (0 131" X 2 5" MIN) NAILS PER THE FOLLOWING NAIL SCHEDULE
2 X 3'S - 2 ROWS, 2 X 4'S - 3 ROWS, 2 X 6'S AND LARGER - 4 ROWS: SPACED @ 4" O.C.
NAILS TO BE DRIVEN FROM BOTH FACES, STAGGER SPACING FROM FRONT TO BACK FACE
FOR A NET 2" O.C. SPACING IN EACH COVERED TRUSS MEMBER USE 2" MEMBER END DISTANCE



Scale = 1:42.9

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.35	Vert(LL)	0.01	5-6	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.14	Vert(CT)	-0.01	5-6	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.10	Horz(CT)	0.00	5	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 30 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-4-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=164/ Mechanical, 6=174/0-3-8
Max Horiz 6=176 (LC 11)
Max Uplift 5=-115 (LC 11), 6=-25 (LC 10)
Max Grav 5=260 (LC 20), 6=241 (LC 20)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-7=-189/64, 1-8=-166/140, 2-8=-104/185,
2-3=-20/0, 2-5=-237/90
BOT CHORD 6-7=-51/82, 5-6=-197/380, 4-5=0/0
WEBS 1-5=-304/160

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 6) Bearings are assumed to be: Joint 6 SP No.2 crushing capacity of 565 psi, Joint 5 User Defined crushing capacity of 425 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 115 lb uplift at joint 5 and 25 lb uplift at joint 6.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 12, 2024

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ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

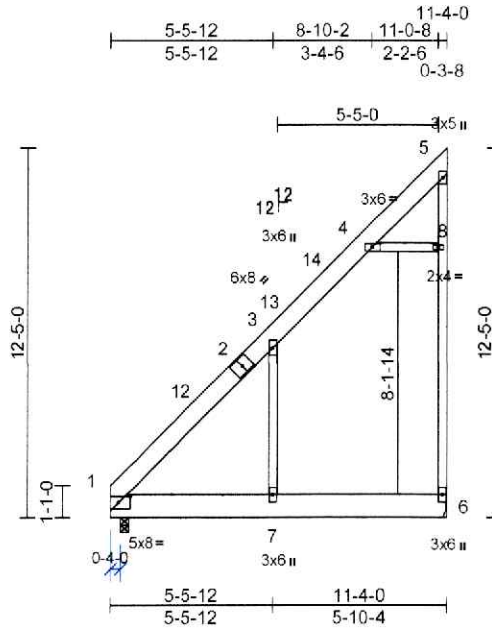
Job 24020093-01	Truss F1	Truss Type Roof Special	Qty 3	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834863
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Carter Components (Sanford, NC), Sanford, NC - 27332.

Run: 8.73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:36:01
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Page: 1

CANTILEVER LEFT END 4"
NO REPAIR REQUIRED



Scale = 1/77.1

Plate Offsets (X, Y): [1:0-4-12,0-2-8]

Loading	(psf)	Spacing	1-11-4	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.36	Vert(LL)	-0.08	6-7	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.23	Vert(CT)	-0.16	6-7	>866	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.01	1	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH		Attic	-0.04	6-7	>999	360		
BCDL	10.0										Weight: 123 lb	FT = 20%

LUMBER

TOP CHORD 2x8 SP 2400F 2.0E
BOT CHORD 2x10 SP 2400F 2.0E
WEBS 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 1=0-3-8, 6= Mechanical
Max Horiz 1=395 (LC 13)
Max Uplift 6=-110 (LC 11)
Max Grav 1=632 (LC 25), 6=883 (LC 24)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-3=-440/420, 3-4=-268/266, 4-5=-94/248, 6-8=-362/103, 5-8=-347/114
BOT CHORD 1-7=-240/315, 6-7=-99/133
WEBS 3-7=-370/218, 4-8=-219/290

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 11-2-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 6) Ceiling dead load (5.0 psf) on member(s). 3-4, 4-8; Wall dead load (5.0psf) on member(s). 3-7
- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 6-7
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 6.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard



April 12, 2024

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818 Soundside Road
Edenton, NC 27932

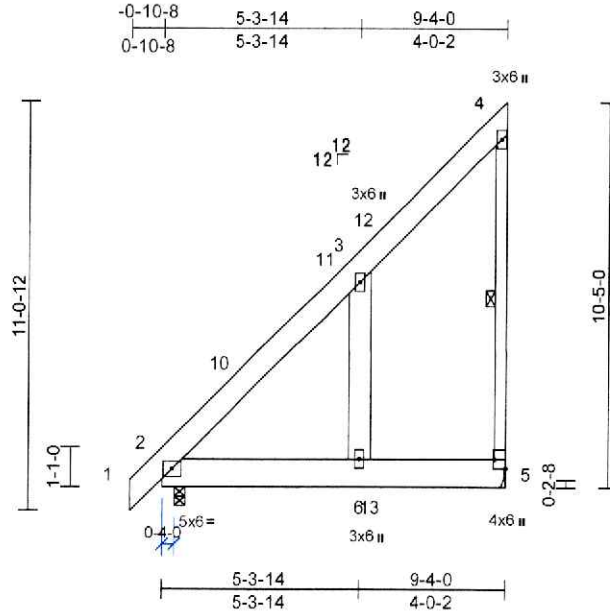
Job 24020093-01	Truss G1	Truss Type Roof Special	Qty 5	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834864
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Carter Components (Sanford, NC), Sanford, NC - 27332.

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Page: 1

CANTILEVER LEFT END 4"
NO REPAIR REQUIRED



Scale = 1:62.2

Plate Offsets (X, Y): [5:Edge,0-3-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.92	Vert(LL)	0.04	6-9	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.13	Vert(CT)	-0.06	6-9	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.01	2	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 112 lb	FT = 20%

LUMBER

TOP CHORD 2x8 SP 2400F 2.0E
BOT CHORD 2x10 SP 2400F 2.0E
WEBS 2x4 SP No.3 *Except* 3-6;2x8 SP 2400F 2.0E

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-5

REACTIONS

(size) 2=0-3-8, 5= Mechanical
Max Horiz 2=349 (LC 13)
Max Uplift 5=159 (LC 11)
Max Grav 2=531 (LC 26), 5=557 (LC 25)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-310/186, 3-4=-187/151, 4-5=-249/105
BOT CHORD 2-6=-210/163, 5-6=-149/163
WEBS 3-6=-287/263

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed: MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-10-8 to 2-1-8, Interior (1) 2-1-8 to 4-11-5, Exterior(2R) 4-11-5 to 9-2-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 159 lb uplift at joint 5.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard



April 12, 2024

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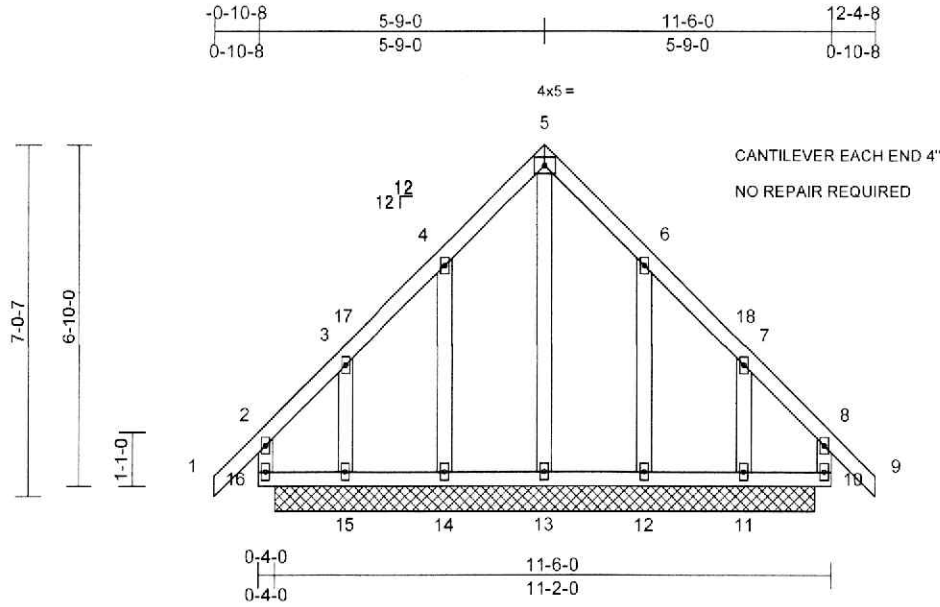
818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss H1	Truss Type Common Supported Gable	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834865
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 E Nov 16 2023 Print: 8.730 E Nov 16 2023 MITek Industries, Inc. Fri Apr 12 09:19:01
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Page 1



Scale = 1.46

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.25	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.17	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.22	Horz(CT)	0.00	11	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MR								
BCDL	10.0											
											Weight: 76 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size)
11=219/10-10-0, 12=143/10-10-0, 13=295/10-10-0, 14=143/10-10-0, 15=219/10-10-0
Max Horiz 15=-186 (LC 12)
Max Uplift 11=-129 (LC 14), 12=-150 (LC 15), 14=-151 (LC 14), 15=-130 (LC 15)
Max Grav 11=283 (LC 22), 12=239 (LC 26), 13=309 (LC 28), 14=240 (LC 25), 15=283 (LC 21)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-16=-32/55, 1-2=0/43, 2-3=-63/172, 3-17=-60/125, 4-17=-55/174, 4-5=-125/260, 5-6=-124/261, 6-18=-53/173, 7-18=-59/124, 7-8=-62/171, 8-9=0/43, 8-10=-32/53
BOT CHORD 15-16=-130/126, 14-15=-129/188, 13-14=-129/188, 12-13=-129/188, 11-12=-129/188, 10-11=-129/188
WEBS 5-13=-279/69, 4-14=-226/158, 3-15=-187/223, 6-12=-226/166, 7-11=-191/218

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCCL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 2-1-8, Exterior(2N) 2-1-8 to 2-9-0, Corner(3R) 2-9-0 to 8-9-0, Exterior(2N) 8-9-0 to 9-4-8, Corner(3E) 9-4-8 to 12-4-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- The Fabrication Tolerance at joint 2 = 15%, joint 8 = 15%
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14, 15, 12, and 11. This connection is for uplift only and does not consider lateral forces.
- Non Standard bearing condition. Review required.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpin.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbccomponents.com)



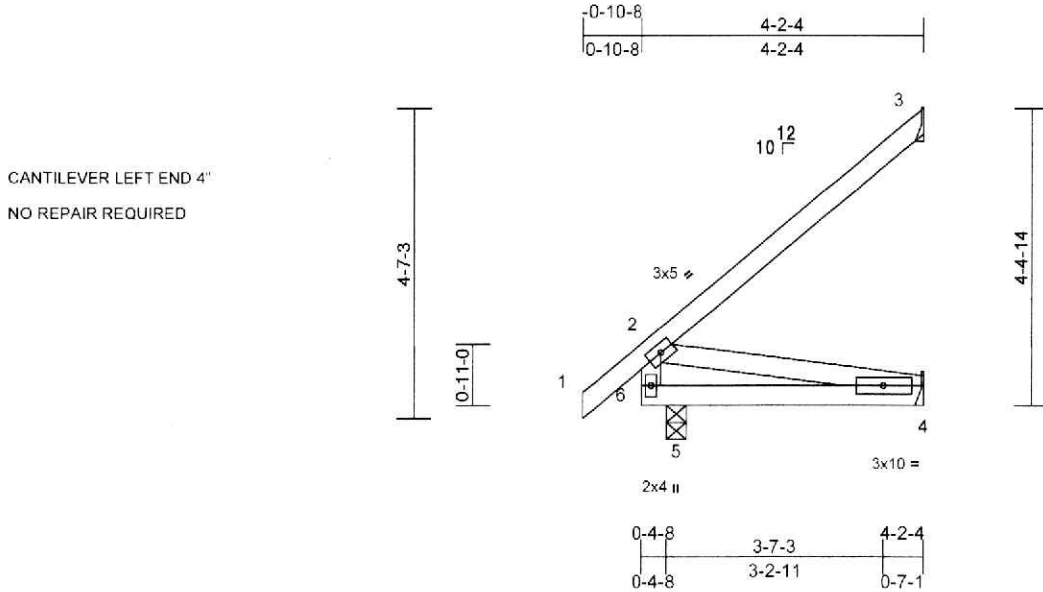
818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss J1	Truss Type Jack-Open	Qty 16	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834866
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 E Nov 16 2023 Print: 8.730 E Nov 16 2023 MITek Industries, Inc. Fri Apr 12 09:20:38
ID:gxipycRBjcdob3DXd6w9uzaLQJ-rEI?JRdAuzNm_JA8ea?nlfmsyE3mysvzCzAcMqzRVd7

Page: 1



Scale = 1.34.1

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.51	Vert(LL)	0.02	4-5	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.23	Vert(CT)	-0.02	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(CT)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 23 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-2-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=112/ Mechanical, 4=17/ Mechanical, 5=252/0-3-8
Max Horiz 5=139 (LC 14)
Max Uplift 3=-94 (LC 14), 4=-3 (LC 14)
Max Grav 3=203 (LC 21), 4=66 (LC 7), 5=353 (LC 21)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-6=-280/50, 1-2=0/39, 2-3=-132/103
BOT CHORD 5-6=-12/7, 4-5=-252/79
WEBS 2-4=-80/256

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Bearings are assumed to be: Joint 3 User Defined crushing capacity of 425 psi, Joint 4 User Defined crushing capacity of 425 psi, Joint 5 SP No.2 crushing capacity of 565 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 3 and 3 lb uplift at joint 4.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 1/2/2023 BEFORE USE

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbccomponents.com)



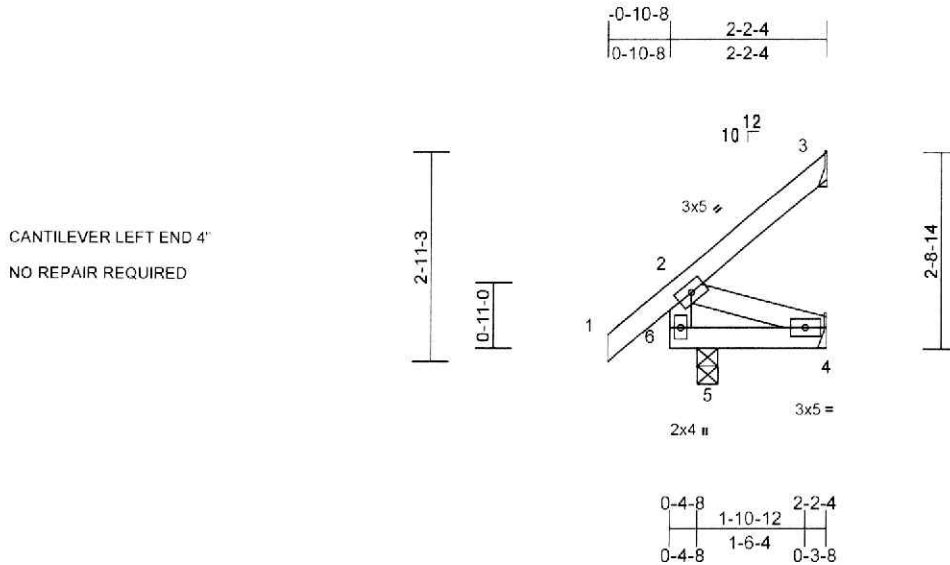
818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss J2	Truss Type Jack-Open	Qty 2	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834867
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Carter Components (Sanford, NC), Sanford, NC - 27332.

Run: 8.73 E Nov 16 2023 Print: 8.730 E Nov 16 2023 MiTek Industries, Inc. Fri Apr 12 09:21:26
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Page: 1



Scale = 1:32.1

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.12	Vert(LL)	0.00	4-5	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.19	Vert(CT)	0.00	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(CT)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 13 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=44/ Mechanical, 4=-15/ Mechanical, 5=192/0-3-8
Max Horiz 5=78 (LC 14)
Max Uplift 3=-39 (LC 14), 4=-33 (LC 21)
Max Grav 3=70 (LC 21), 4=21 (LC 7), 5=292 (LC 21)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-6=-218/58, 1-2=0/53, 2-3=-77/44
BOT CHORD 5-6=-12/7, 4-5=-175/39
WEBS 2-4= 41/185

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed: MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ci=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Bearings are assumed to be: Joint 3 User Defined crushing capacity of 425 psi, Joint 5 SP No.2 crushing capacity of 565 psi, Joint 4 User Defined crushing capacity of 425 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 3 and 33 lb uplift at joint 4
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 12, 2024

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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	3202 Leaflet Church-Roof-Abby GRH	164834868
24020093-01	J2GR	Half Hip Girder	2	1	Job Reference (optional)	

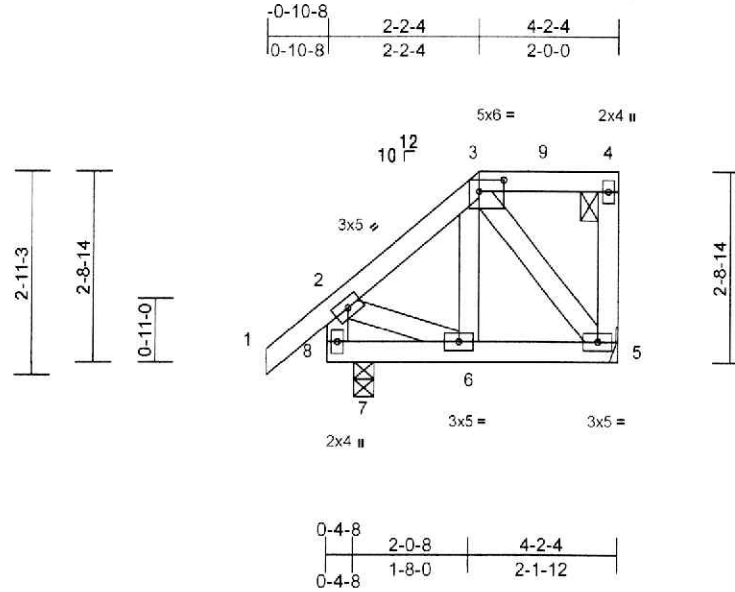
Carter Components (Sanford, NC), Sanford, NC - 27332

Run: 8.73 E Nov 16 2023 Print: 8.730 E Nov 16 2023 MITek Industries, Inc. Fri Apr 12 09:22:05

Page: 1

ID:9xrTt04cTbAQc2CXg1Z7GszaLPV-ozVuaAgkqKpWgJsEL3Bn?SpJmArExnO?jJtZRVbm

CANTILEVER LEFT END 4"
NO REPAIR REQUIRED



Scale = 1:33
Plate Offsets (X, Y): [3:0-4-4-0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.17	Vert(LL)	0.00	5-6	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.16	Vert(CT)	0.00	5-6	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.04	Horz(CT)	0.00	5	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 29 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-4 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 5=141/ Mechanical, 7=269/0-3-8
Max Horiz 7=100 (LC 9)
Max Uplift 5=-82 (LC 9), 7=-72 (LC 12)
Max Grav 5=182 (LC 33), 7=358 (LC 34)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/63, 2-3=-148/61, 3-9=-33/25, 4-9=-33/25, 4-5=-89/27, 2-8=-282/70
BOT CHORD 7-8=-64/14, 6-7=-121/64, 5-6=-71/87
WEBS 3-6=-65/37, 3-5=-122/85, 2-6=-60/143

- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Bearings are assumed to be: Joint 7 SP No.2 crushing capacity of 565 psi.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 82 lb uplift at joint 5 and 72 lb uplift at joint 7.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 114 lb down and 89 lb up at 2-2-4 on top chord, and 22 lb down and 48 lb up at 2-2-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- LOAD CASE(S)** Standard
- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-2=-60, 2-3=-60, 3-4=-60, 5-8=-20
Concentrated Loads (lb)
Vert: 6=-1 (B), 3=-36 (B)

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.



April 12, 2024

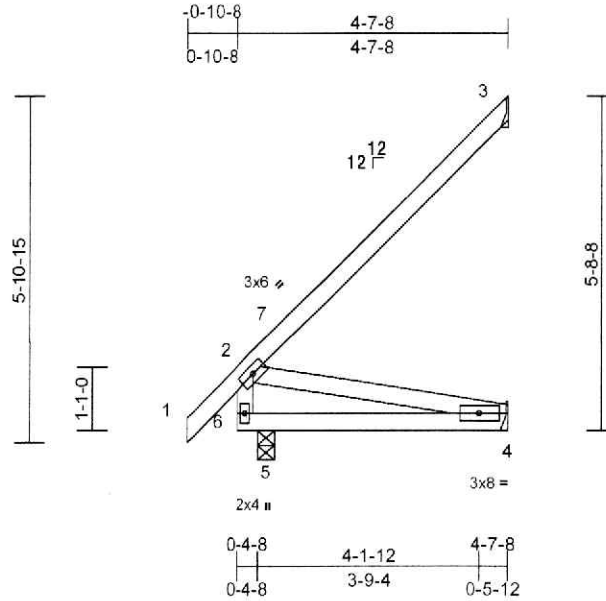
Job 24020093-01	Truss J3	Truss Type Jack-Open	Qty 4	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834869
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 E Nov 16 2023 Print: 8.730 E Nov 16 2023 MiTek Industries, Inc. Fri Apr 12 09:22:36
ID: gxpircRBjcdob3DXd6ew9uzaLQj-Oxtc4n2BXqE08RBqZM7SXYE_epKMcEILWklsvazRVbH

Page: 1

CANTILEVER LEFT END 4"
NO REPAIR REQUIRED



Scale = 1/32

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.67	Vert(LL)	0.02	4-5	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.23	Vert(CT)	-0.03	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.10	Horz(CT)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MP								
BCDL	10.0											
										Weight: 26 lb	FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=125/ Mechanical, 4=22/ Mechanical, 5=268/0-3-8
Max Horiz 5=183 (LC 14)
Max Uplift 3=-130 (LC 14), 4=-16 (LC 14)
Max Grav 3=234 (LC 21), 4=75 (LC 7), 5=356 (LC 21)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-6=-282/0, 1-2=0/43, 2-7=-196/57, 3-7=-176/131
BOT CHORD 5-6=-15/8, 4-5=-337/105
WEBS 2-4=-107/344

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 3 User Defined crushing capacity of 425 psi, Joint 5 SP No.2 crushing capacity of 565 psi, Joint 4 User Defined crushing capacity of 425 psi.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 130 lb uplift at joint 3 and 16 lb uplift at joint 4.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 12, 2024

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbccomponents.com)



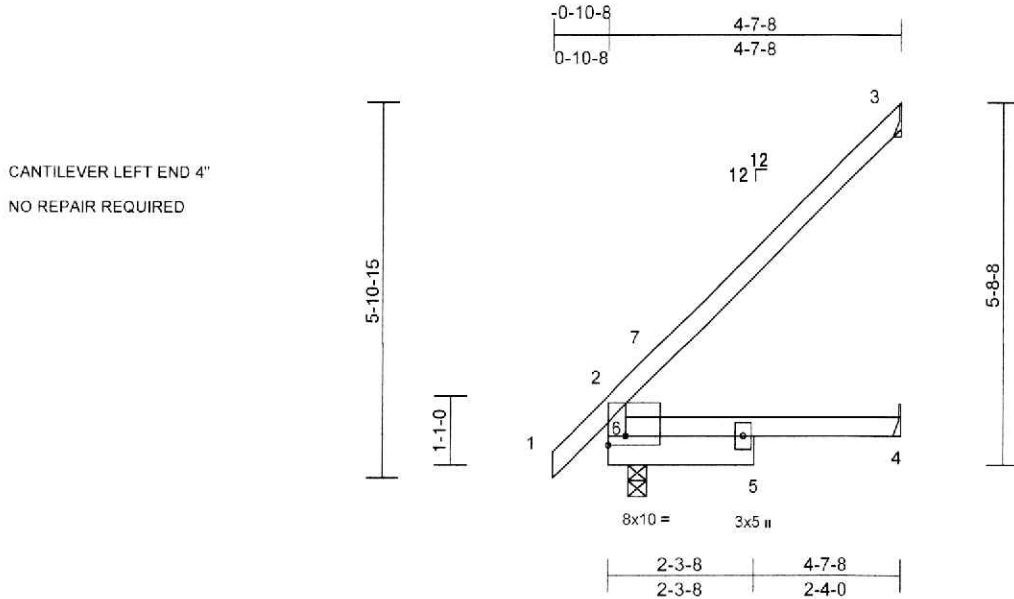
818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss J3T	Truss Type Jack-Open	Qty 5	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	I64834870
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:36:02
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Page: 1



Scale = 1/32.2

Plate Offsets (X, Y): [6:Edge,0-1-12]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.94	Vert(LL)	0.03	4-6	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.32	Vert(CT)	-0.03	4-6	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.03	3	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MR								
BCDL	10.0											
											Weight: 25 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2 *Except* 6-4:2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 3= Mechanical, 4= Mechanical,
6=0-3-8
Max Horiz 6=184 (LC 14)
Max Uplift 3=118 (LC 14), 4=-19 (LC 14)
Max Grav 3=203 (LC 21), 4=90 (LC 7), 6=326 (LC 21)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-6=-313/73, 1-2=0/43, 2-3=-186/109
BOT CHORD 4-6=-18/1

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - 7) All bearings are assumed to be User Defined.
 - 8) Refer to girder(s) for truss to truss connections.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 lb uplift at joint 3 and 19 lb uplift at joint 4.
 - 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- LOAD CASE(S)** Standard



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbccomponents.com)



818 Soundside Road
Edenton, NC 27932

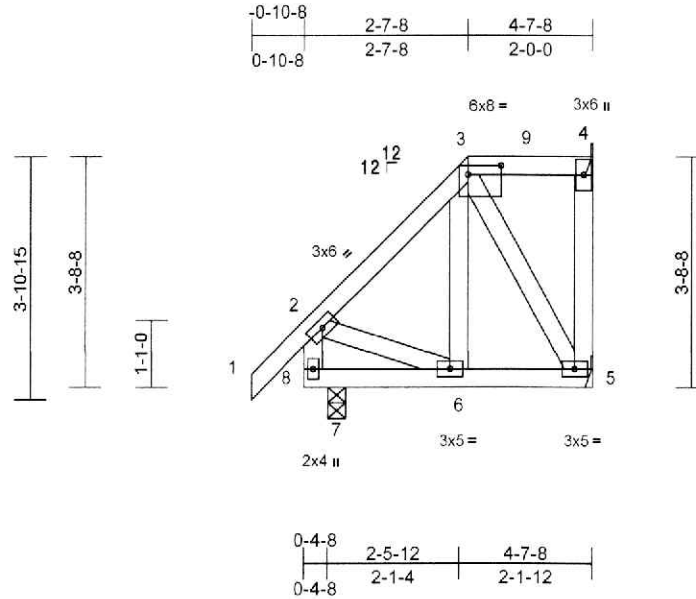
Job 24020093-01	Truss J4	Truss Type Half Hip Girder	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834871
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 E Nov 16 2023 Print: 8.730 E Nov 16 2023 MITek Industries, Inc. Fri Apr 12 09:23:20
ID: hH5g23AIH2ZCudK6K2ukfzaLPw-6PXFJoanALnmQCYH8bvmJp3deYLpW9lpeAQBR1zRVav

Page: 1

CANTILEVER LEFT END 4"
NO REPAIR REQUIRED



Scale = 1:36.9

Plate Offsets (X, Y): [3:0-6-4, 0-1-12]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.15	Vert(LL)	0.00	5-6	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.31	Vert(CT)	-0.01	5-6	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.06	Horz(CT)	-0.01	4	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 36 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=56/ Mechanical, 5=149/ Mechanical, 7=319/0-3-8
Max Horiz 7=137 (LC 9)
Max Uplift 4=-23 (LC 8), 5=-123 (LC 9), 7=-92 (LC 12)
Max Grav 4=93 (LC 33), 5=196 (LC 37), 7=429 (LC 34)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/72, 2-3=-208/94, 3-9=-44/33, 4-9=-44/33, 4-5=0/0, 2-8=-323/83
BOT CHORD 7-8=-11/8, 6-7=-133/83, 5-6=-95/102
WEBS 3-6=-65/68, 3-5=-191/147, 2-6=-64/137

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 9) Bearings are assumed to be: Joint 7 SP No.2 crushing capacity of 565 psi.
- 10) Refer to girder(s) for truss to truss connections.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 4, 123 lb uplift at joint 5 and 92 lb uplift at joint 7.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
- 15) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 177 lb down and 135 lb up at 2-7-8 on top chord, and 42 lb down and 62 lb up at 2-7-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 16) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)

Vert: 1-2=-60, 2-3=-60, 3-4=-60, 5-8=-20
Concentrated Loads (lb)
Vert: 6=-11 (B), 3=-105 (B)



April 12, 2024

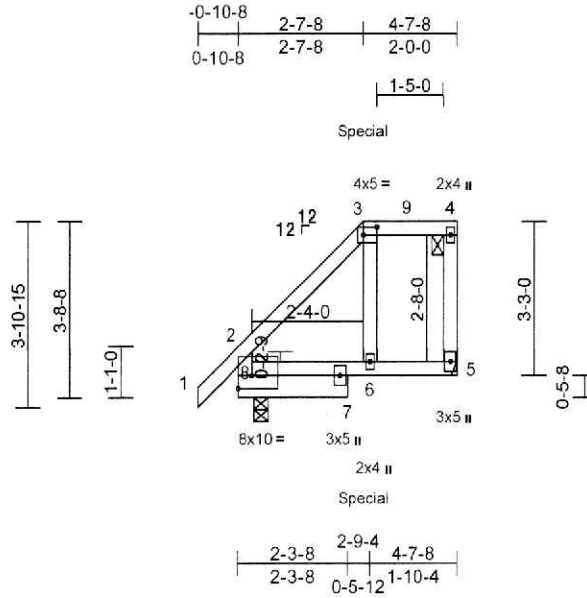
Job 24020093-01	Truss J4T	Truss Type Half Hip Girder	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834872
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:36:02
ID: 9xrTt04oTbAQa2CkXg1Z7GszalPv-RfC?PsB70Hq3NSgPonLew3uITXbGKWCDoi7J4zJC7f

Page: 1

CANTILEVER LEFT END 4"
NO REPAIR REQUIRED



Scale = 1:48.4

Plate Offsets (X, Y): [3:0-3-8,0-2-0], [8:Edge,0-3-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.32	Vert(LL)	0.03	6	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.37	Vert(CT)	-0.03	6	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.03	Horz(CT)	0.01	5	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MR								
BCDL	10.0											
										Weight: 31 lb	FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2 *Except* 8-5:2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 5= Mechanical, 8=0-3-8
Max Horiz 8=128 (LC 9)
Max Uplift 5=-145 (LC 9), 8=-78 (LC 12)
Max Grav 5=269 (LC 37), 8=391 (LC 34)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/72, 2-3=-210/85, 3-4=-64/55, 4-5=-139/57, 2-8=-315/77
BOT CHORD 6-8=-61/67, 5-6=-56/65
WEBS 3-6=-110/90

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.

- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 5.
- H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 164 lb down and 142 lb up at 2-7-8 on top chord, and 59 lb down and 52 lb up at 2-7-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-2=-60, 2-3=-60, 3-4=-60, 5-8=-20
Concentrated Loads (lb)

Vert: 3=-91 (F), 6=-25 (F)



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbccomponents.com)



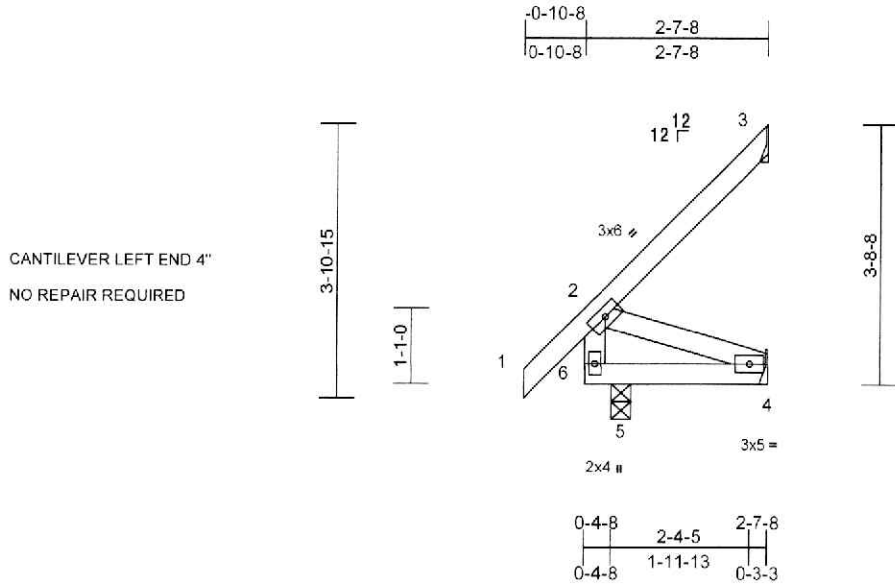
818 Soundside Road
Edenon, NC 27932

Job	Truss	Truss Type	Qty	Ply	3202 Leaflet Church-Roof-Abby GRH	164834873
24020093-01	J5	Jack-Open	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8 73 E Nov 16 2023 Pmt: 8 730 E Nov 16 2023 MiTek Industries, Inc. Fri Apr 12 09:23:59
 ID: Ck8RIGRYyJVxzvL3P7hdhzaLQk-TvO_V93F_KiEbC1nwl6WO2AH3GfWYmcYckMIKGzRVa_

Page: 1



Scale = 1:32.7

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.13	Vert(LL)	0.00	4-5	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.22	Vert(CT)	0.01	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(CT)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix	MP							
BCDL	10.0											
											Weight: 16 lb	FT = 20%

LUMBER
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-7-8 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=60/ Mechanical, 4=-6/ Mechanical, 5=202/0-3-8
 Max Horiz 5=110 (LC 14)
 Max Uplift 3=-63 (LC 14), 4=-33 (LC 14)
 Max Grav 3=101 (LC 21), 4=32 (LC 7), 5=323 (LC 21)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 2-6=-249/11, 1-2=0/74, 2-3=-106/67
 BOT CHORD 5-6=-15/8, 4-5=-251/59
 WEBS 2-4=-62/266

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Bearings are assumed to be: Joint 3 User Defined crushing capacity of 425 psi, Joint 5 SP No.2 crushing capacity of 565 psi, Joint 4 User Defined crushing capacity of 425 psi.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 3 and 33 lb uplift at joint 4.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- LOAD CASE(S)** Standard

- NOTES**
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed: MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.



April 12, 2024

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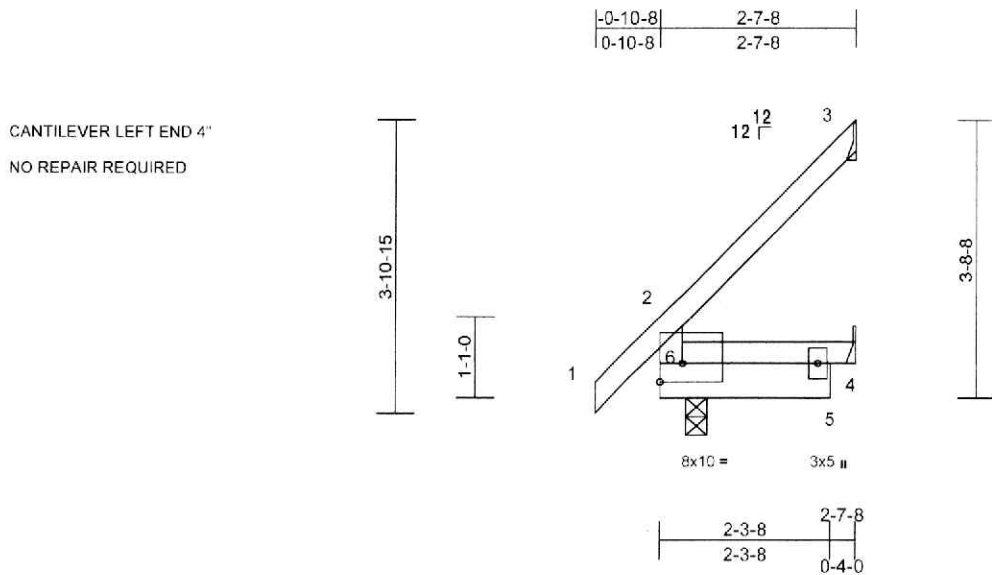
818 Soundside Road
 Edenton, NC 27932

Job 24020093-01	Truss J5T	Truss Type Jack-Open	Qty 1	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834874
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8 73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:36:02
ID: Ck8RIGRyYJVxzveL3P7hdhzaLQk-RfC?PsB70Hq3NSgPqnl.8w3uITxbGKwvCDoi7J4zJC7f

Page 1



Scale = 1:30.7

Plate Offsets (X, Y): [6:Edge,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.47	Vert(LL)	0.00	4-6	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.06	Vert(CT)	0.00	4-6	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MR								
BCDL	10.0											
											Weight: 17 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x6 SP No.2 *Except* 6-4:2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-7-8 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size) 3= Mechanical, 4= Mechanical, 6=0-3-8
 Max Horiz 6=111 (LC 14)
 Max Uplift 3=-68 (LC 14), 4=-18 (LC 14)
 Max Grav 3=97 (LC 21), 4=49 (LC 7), 6=273 (LC 21)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 2-6=-254/80, 1-2=0/74, 2-3=-120/56
 BOT CHORD 4-6=-15/2

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - 7) All bearings are assumed to be User Defined .
 - 8) Refer to girder(s) for truss to truss connections.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 68 lb uplift at joint 3 and 18 lb uplift at joint 4.
 - 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- LOAD CASE(S)** Standard



April 12, 2024

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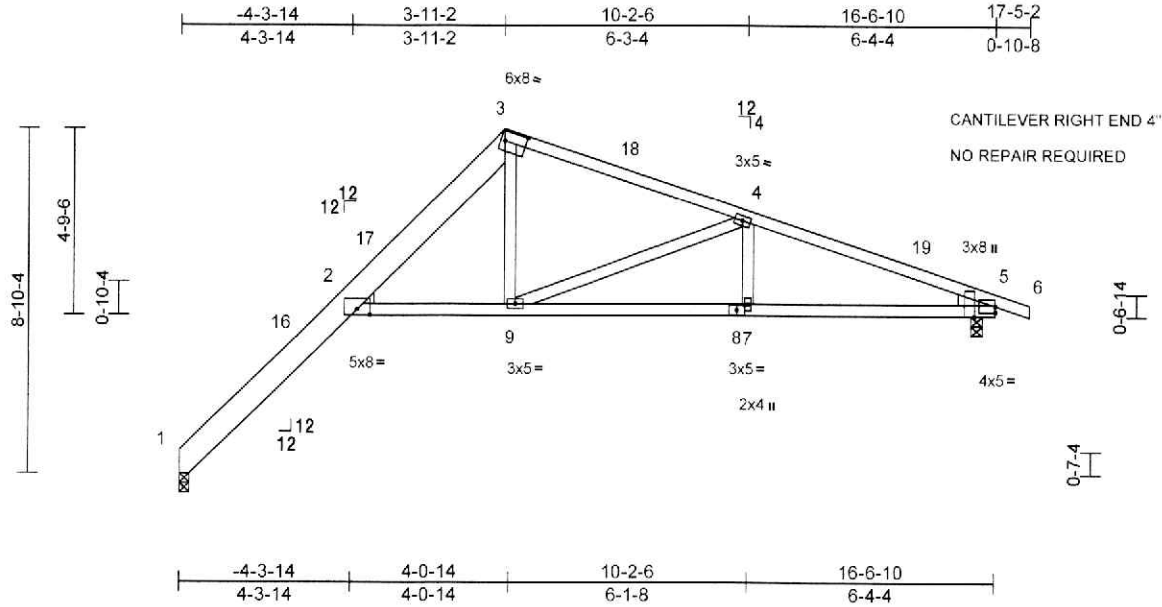
818 Soundside Road
 Edenton, NC 27932

Job 24020093-01	Truss L1	Truss Type Roof Special	Qty 4	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834875
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Mar 21 2024 Print: 8.730 S Mar 21 2024 MiTek Industries, Inc. Thu Apr 11 07:36:02
ID: DW6kILDnVdNpIMJTBG1CyFzB_KL-RfC?PsB70Hq3NSgPqnl8w3ulTXbGKWrCdoi7J4zJC?f

Page: 1



Scale = 1.59

Plate Offsets (X, Y): [3:0-6-9,0-3-0], [5:Edge,0-1-13], [5:0-3-3,Edge], [8:0-2-7,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.74	Vert(LL)	0.36	9-12	>696	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.86	Vert(CT)	-0.70	9-12	>354	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(CT)	0.68	5	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 104 lb	FT = 20%

LUMBER
TOP CHORD 2x8 SP 2400F 2.0E *Except* 3-6:2x4 SP No.2
BOT CHORD 2x4 SP No.1 *Except* 8-5:2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3
Right: 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-8-12 oc bracing.

REACTIONS (size) 1=0-3-0, 5=0-3-8
Max Horiz 1=216 (LC 14)
Max Uplift 1=-19 (LC 11), 5=-122 (LC 11)
Max Grav 1=887 (LC 21), 5=943 (LC 22)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-601/95, 2-3=-1711/516, 3-4=-1189/407, 4-5=-1908/471, 5-6=0/17
BOT CHORD 2-9=-383/1162, 7-9=-361/1765, 5-7=-361/1765
WEBS 4-9=-773/175, 4-7=0/214, 3-9=-42/514

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 1.
- H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft. Cat. II: Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-8 to 3-1-8, Interior (1) 3-1-8 to 5-3-0, Exterior(2R) 5-3-0 to 11-3-0, Interior (1) 11-3-0 to 18-9-0, Exterior(2E) 18-9-0 to 21-9-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

ENGINEERING BY
TRENCO
A MITEK AFFILIATE

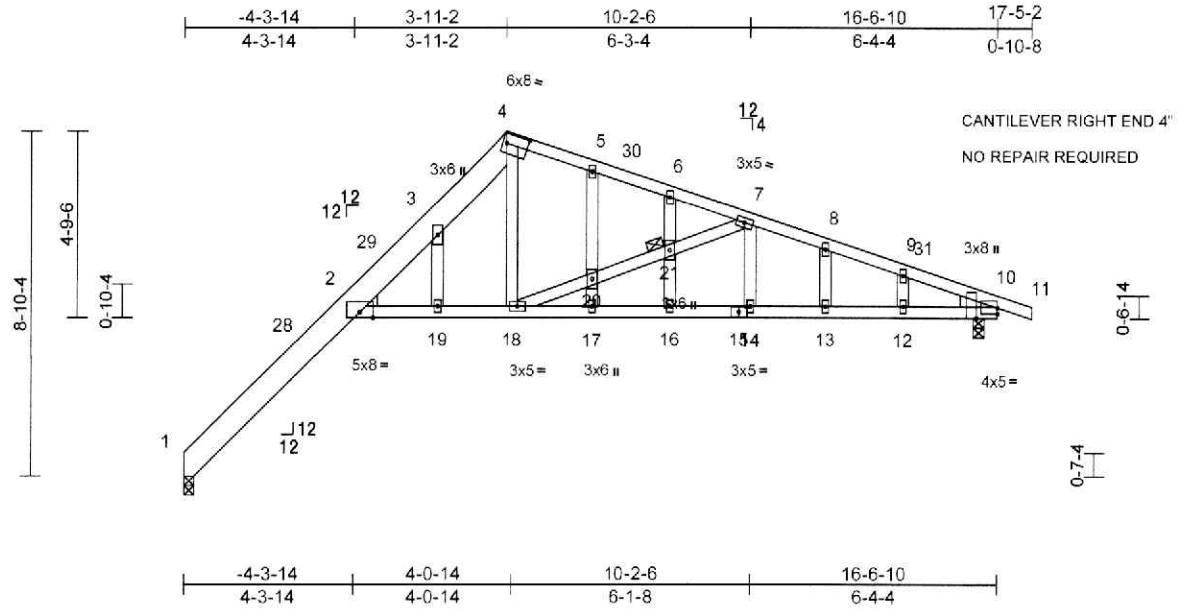
818 Soundside Road
Edenton, NC 27932

Job 24020093-01	Truss L2	Truss Type Roof Special Structural Gable	Qty 2	Ply 1	3202 Leaflet Church-Roof-Abby GRH Job Reference (optional)	164834876
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Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1.59

Plate Offsets (X, Y): [4:0-6-9,0-3-0], [10:Edge,0-1-13], [10:0-3-3,Edge], [15:0-2-7,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.74	Vert(LL)	0.36	19-24	>691	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.75	Vert(CT)	-0.71	19-24	>350	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.42	Horz(CT)	0.69	10	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 119 lb	FT = 20%

LUMBER

TOP CHORD 2x8 SP 2400F 2.0E *Except* 4-11:2x4 SP No. 2
 BOT CHORD 2x4 SP No. 1 *Except* 15-10:2x4 SP No. 2
 WEBS 2x4 SP No. 3
 OTHERS 2x4 SP No. 3
 WEDGE Left: 2x4 SP No. 3
 Right: 2x4 SP No. 3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-0-8 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 9-10-14 oc bracing.
 JOINTS 1 Brace at Jt(s): 21

REACTIONS

(size) 1=0-3-0, 10=0-3-8
 Max Horiz 1=216 (LC 14)
 Max Uplift 1=-19 (LC 11), 10=-122 (LC 11)
 Max Grav 1=887 (LC 21), 10=943 (LC 22)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-601/95, 2-3=-1752/540, 3-4=-1115/345,
 4-5=-1117/434, 5-6=-1138/405,
 6-7=-1170/383, 7-8=-1837/479,
 8-9=-1860/458, 9-10=-1878/438, 10-11=0/17
 BOT CHORD 2-19=-352/1109, 18-19=-227/1040,
 17-18=-354/1744, 16-17=-354/1744,
 14-16=-354/1744, 13-14=-354/1744,
 12-13=-354/1744, 10-12=-354/1744
 WEBS 4-18=-144/663, 18-20=-741/168,
 20-21=-737/164, 7-21=-719/165,
 7-14=-15/176, 5-20=-93/62, 17-20=-86/66,
 3-19=-109/87, 6-21=-101/47, 16-21=-50/43,
 8-13=-39/40, 9-12=-44/42

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCCL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-8 to 3-1-8, Interior (1) 3-1-8 to 5-3-0, Exterior(2R) 5-3-0 to 11-3-0, Interior (1) 11-3-0 to 18-9-0, Exterior(2E) 18-9-0 to 21-9-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 7) All plates are 2x4 MT20 unless otherwise indicated.
 8) Gable studs spaced at 2-0-0 oc.
 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 11) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 1.

13) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 10. This connection is for uplift only and does not consider lateral forces.
 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
LOAD CASE(S) Standard



April 12, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

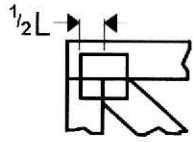
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbccomponents.com)



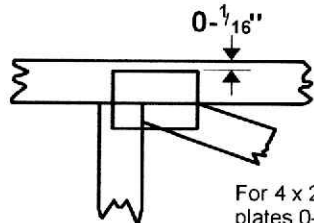
818 Soundside Road
 Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

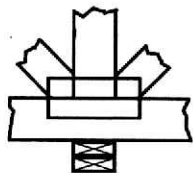
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

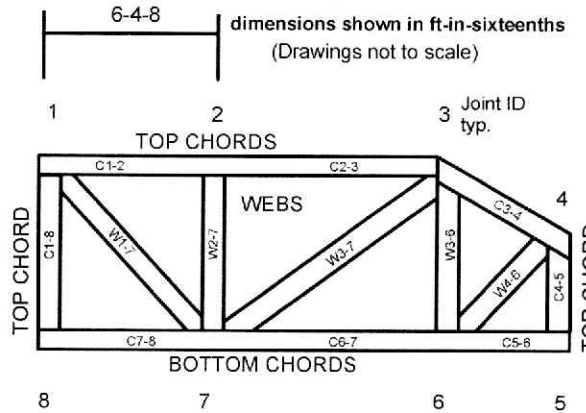


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:

- ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
 DSB-22: Design Standard for Bracing.
 BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282
 ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek

ENGINEERING BY
TRENCO
 A MiTek Affiliate

MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never slack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- The design does not take into account any dynamic or other loads other than those expressly stated.