ARLINGTON-RALE

RALEIGH- LOT 00.0099 BLAKE POND SF

(MODEL# 1630)

ELEVATION 3 - GR

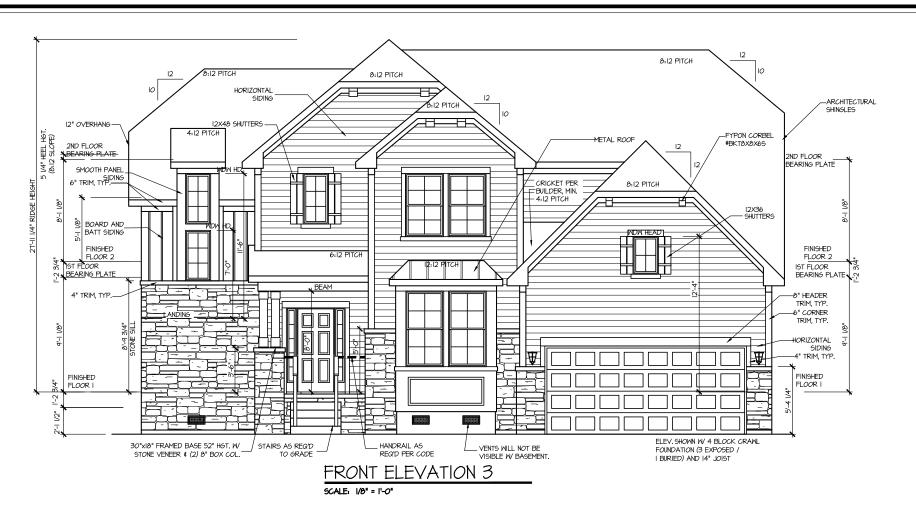
INDEX



AREA CALCULATIONS			COVERED /	
ELEVATION 3		HEATED	UNHEATED	UNCOVERED
FIRST FLOOR		1210 SF		
GARAGE			496 SF	
FRONT PORCH - ELEVATION 3			27 SF	
SECOND FLOOR		1284 SF		
OPTIONS				
MORNING ROOM		166 SF		
	TOTAL	2660 SF	523 SF	
			ı	

63 WHIMBREL COURT

LOT	SPECIFIC	
1		BLAKE POND SF
		ARLINGTON REV. RALE-1 ELEVATION 3
2	ADDRESS	63 WHIMBREL COURT LILLINGTON, NC 27546
<u> </u>		
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L		
		





REAR ELEVATION 3

SCALE: 1/8" = 1'-0"

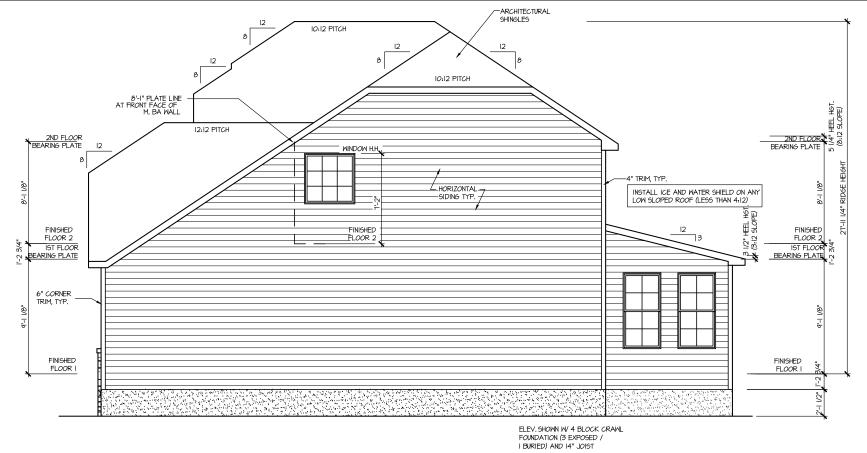
ELEV. SHOWN W/ 4 BLOCK CRAWL FOUNDATION (3 EXPOSED / I BURIED) AND I4" JOIST DRAWN BY:
ITS
DATE:
12/21/2023
PLAN NO.
1630



FRONT & REAR ELEVATIONS

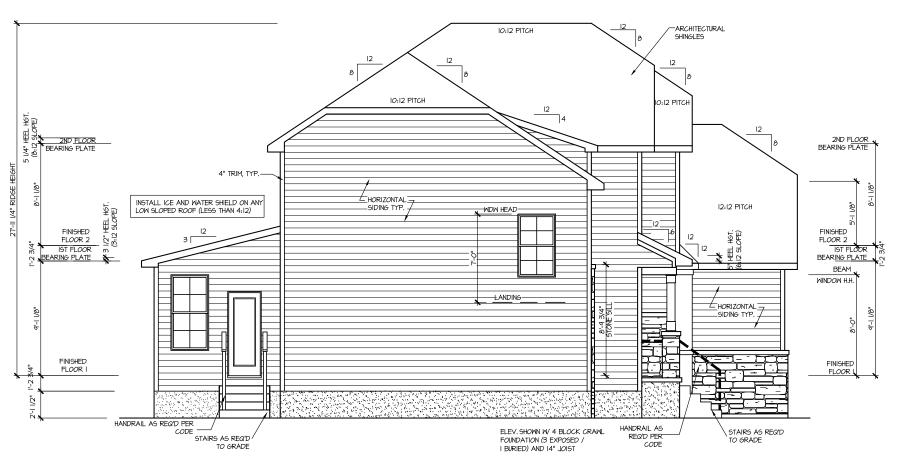
HOUSE NAME:
ARLINGTON
DRAWING TITLE

SHEET No.



RIGHT ELEVATION 3

SCALE: 1/8" = 1'-0"



LEFT ELEVATION 3

SCALE: 1/8" = 1'-0"



DATE: 12/21/2023 PLAN NO. 1630



RIGHT & LEFT ELEVATIONS HOUSE NAME:
ARLINGTON
DRAWING TITLE

SHEET No.



ROOF VENTILATION CALCULATIONS:
ROOF AREA! = 1660 50, FT.
OVERAL REGUIED VENTILATION.
110 80 = 1107 50, FT.
50% IN TOP THERD = 2,16 50, FT. (1 TO 300)

NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 18 SQ. IN/ LINEAR FT.

LOWER VENTINS. (BOTTOM 2/3 RDS)
40 LINEAR FIET OF SOFFIT X 5.1 50. IN. = 3.56 50. FT.
LIPPER VENTINS. (102 LIS RD)
24 LINEAR FIET OF RIDGE X ID 50. IN = 3 50. FT.
3 50. FT. AT 50%
(1 TO 300 ALLOWED)
TOTAL ROOF VENTILATION. 6.56 50. FT. > 5.53 50. FT. (Ra'D)

ROOF VENTILATION CALCULATIONS:

ROOF AREA 2 = 21 50. FT.

OVERAL LEGAURE VENTILATION.

OVERAL LEGAURE VENTILATION.

1 TO 300 = 0.045 50. FT.

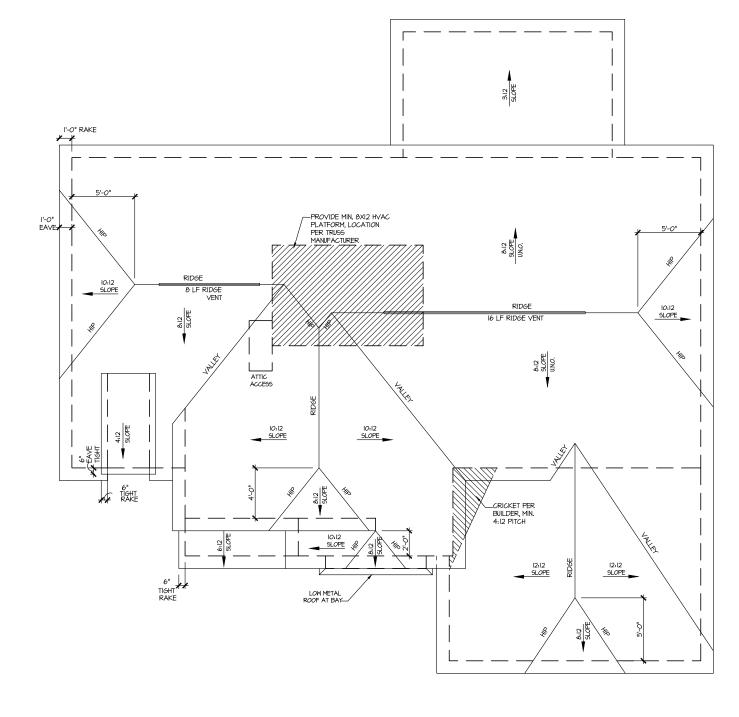
1 TO 300 = 0.045 50. FT.

50% IN TOP THIRD = 0.045 50. FT. (1 TO 300)

NET FREE AREA OF VENTED 50FFIT = 5.1 50. IN / LINEAR FT.

NET FREE AREA OF RIDGE VENT = 10 50. IN / LINEAR FT.

LOWER VENTING: (BOTTOM 2/3 RD5)
85 LINEAR FEET OF SOFFIT X 5.1 SQ. IN. = 0.34 SQ. FT.
TOTAL ROOF VENTILATION: 0.34 SQ. FT. > 0.04 SQ. FT. (RQD)



ROOF PLAN ELEV. 3

SCALE: 1/8" = 1'-0"

DRAWN BY: DATE: 12/21/2023

PLAN NO. 1630



HOUSE NAME:
ARLINGTON
DRAWING TITLE PLAN

SHEET No.

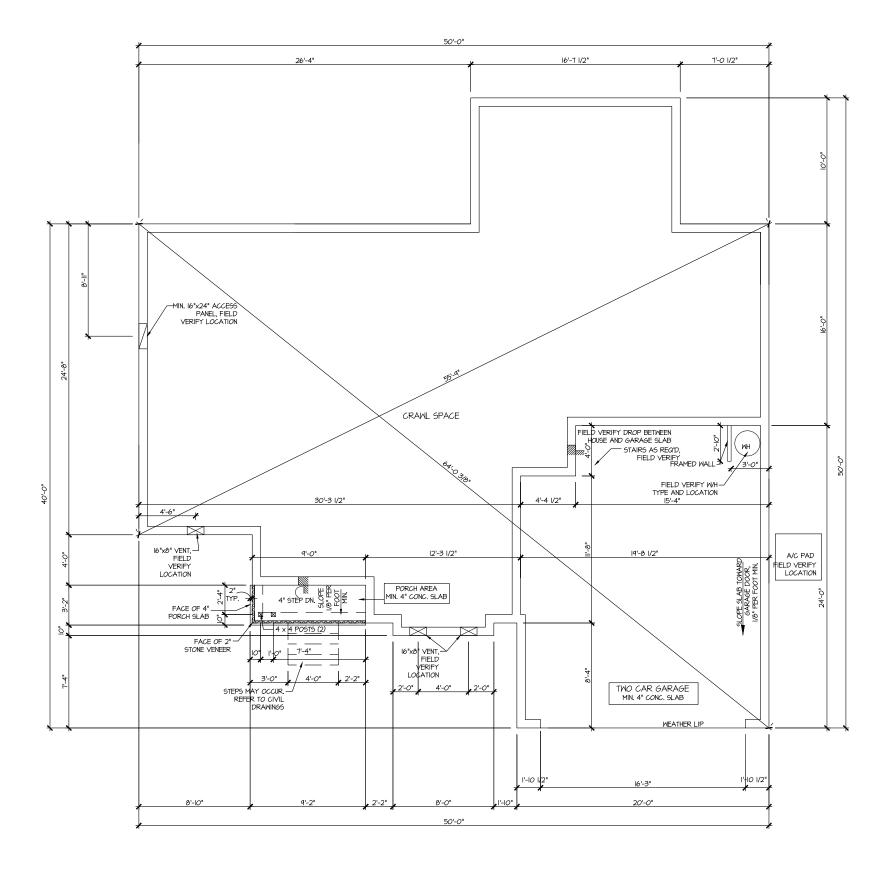
CRAWL SPACE VENT CALCULATIONS: ELEV 3
CRAWL AREA = 1210 SQ. FT.
OVERALL REQUIRED VENTILATION:
1 SQ. IN. PER 1 SQ. FT. = 1210 SQ. IN.

NET FREE AREA OF VENT = 72 SQ. IN. PER VENT WITTEN AUTOMATIC VENT OAL-I OR EQUAL

<u>VENTING REQUIREMENT:</u> 1210 SQ. IN. / 72 SQ. IN. = 16.8 VENTS = 17 VENTS

ONLY VENTS ON THE FRONT ELEVATION ARE SHOWN. ALL OTHERS TO BE FIELD LOCATED.

VENTS SHALL BE INSTALLED PER R322.2.2 - R322.2.2.1

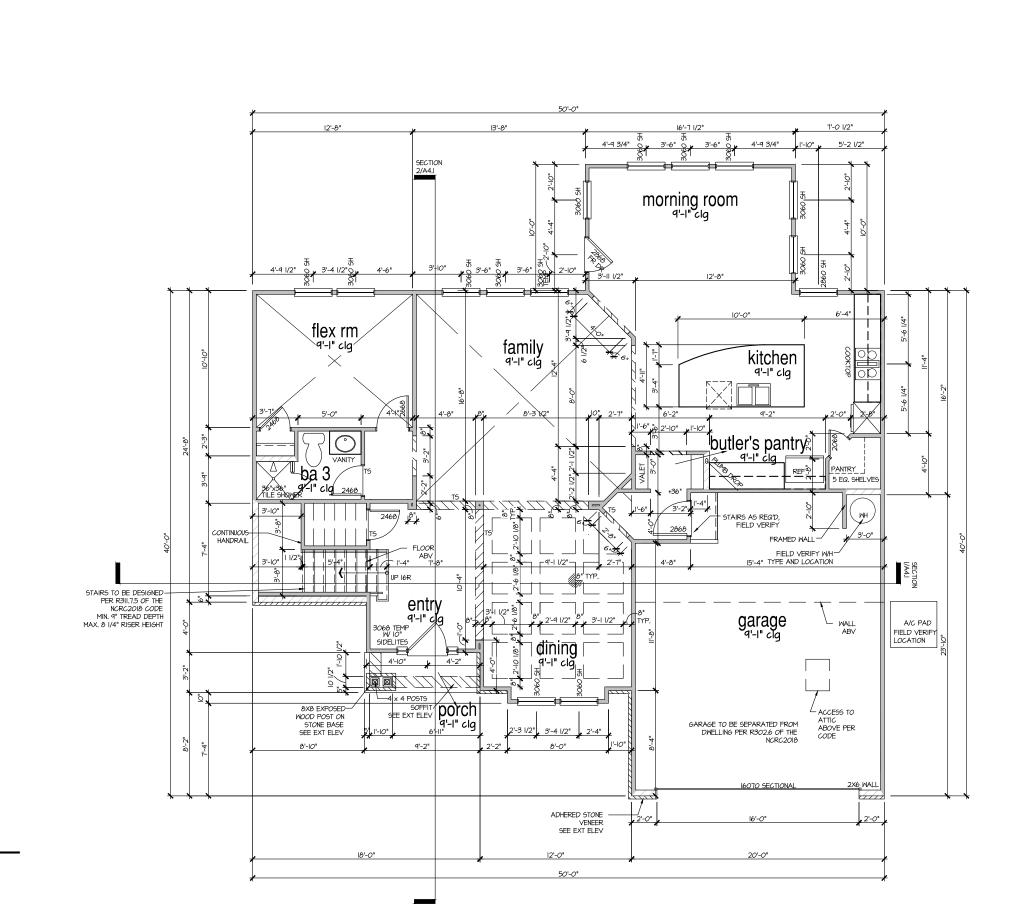


ELEVATION 3 CRAWL SPACE PLAN

DRAWN BY: DATE: 12/21/2023 PLAN NO. 1630

HOUSE NAME:
ARLINGTON
DRAWING TITLE

SHEET No. A2.I



ot 00.0099.dwg DATE: 12/21/2023 9:03 A

ELEVATION 3 FIRST FLOOR PLAN

SCALE: 1/8" = 1'-0"

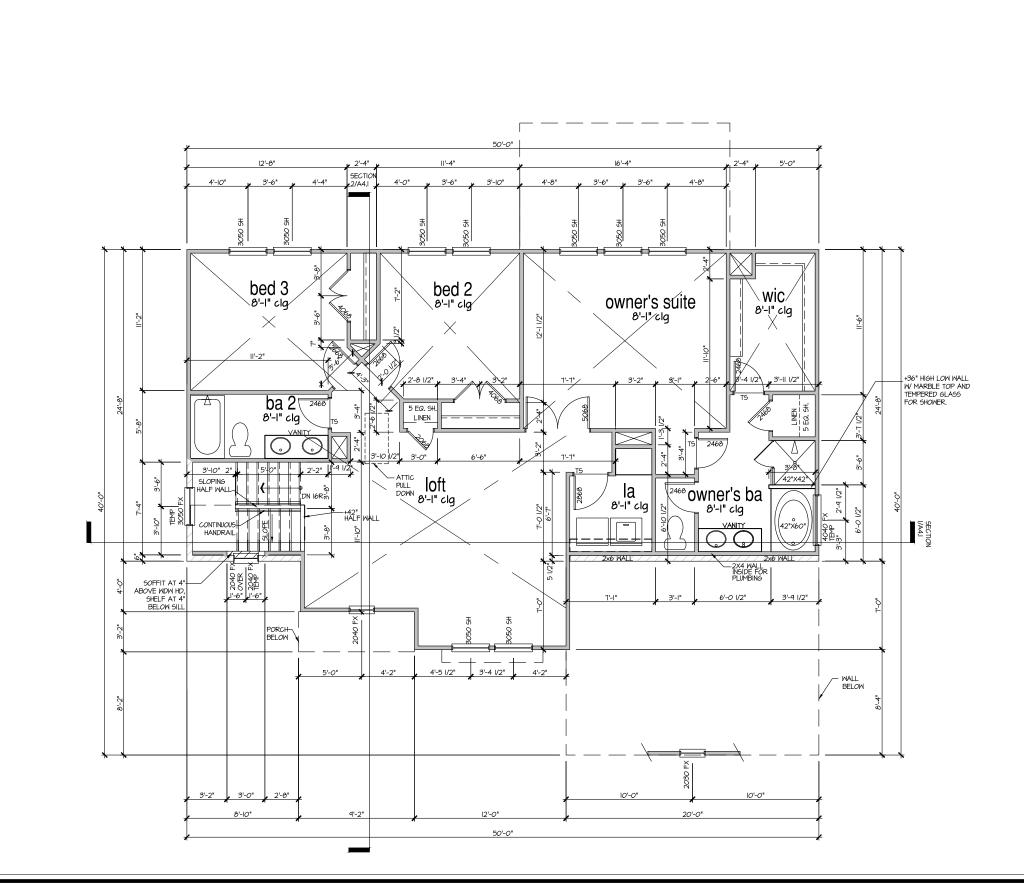
HOUSE NAME:
ARLINGTON
DRAWING TITLE
FIRST FLOOR PLAN

SHEET No.

DRAWN BY:

PLAN NO. 1630

DATE: 12/21/2023



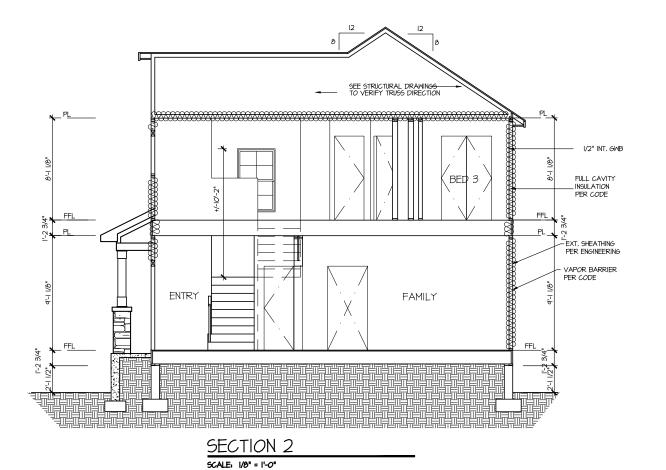
ELEVATION 3 SECOND FLOOR PLAN SCALE: 1/8" = 1'-0" DRAWN BY:
ITS

DATE:
12/21/2023

PLAN NO.
1630

HOUSE NAME:
ARLINGTON
DRAWING TITLE
SECOND FLOOR PLAN

SHEET No. A3.2



SHEET No.

HOUSE NAME:
ARLINGTON
DRAWING TITLE

BUILDING SECTION

DRAWN BY:

DATE: 12/21/2023 PLAN NO. 1630

A4.1

NOTES:

- BUILDING SECTIONS SHOWN HERE DEPICT VOLUMN SPACES WITHIN THE STRUCTURE, REFER TO STRUCTURAL DRAWINGS, TRUSS DRAWINGS, STRUCTURAL DETAILS AND CALCULATIONS BY OTHER FOR ALL STRUCTURAL INFO.

ROOFING: PITCHED SHINGLE ROOF, REFER TO ROOF PLAN FOR TYPICALS.

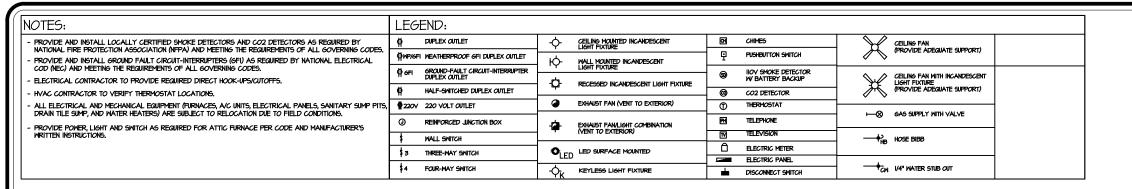
- WOOD FLOORS: FLOOR SHEATHING OVER FLOOR JOIST. REFER TO STRUCTURAL AND TRUSS DRAWINGS BY OTHERS.

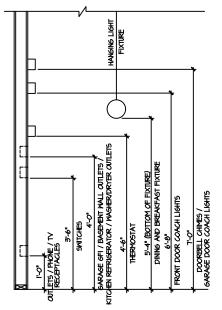
INSULATION: EXTERIOR WALLS:

R-13 BATTS MINIMUM. VERIFY CEILING WITH ATTIC ABOVE: R-38 BATTS MINIMUM, VERIFY

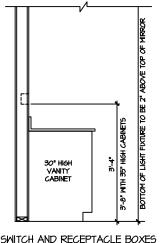
FLOOR OVER GARAGE: R-19 BATTS MINIMUM. VERIFY ATTIC KNEEWALL: R-19 BATTS MINIMUM, VERIFY

PER STATE RESIDENTIAL CODE COMPLIANCE METHOD TO BE DETERMINED BY BUILDER.

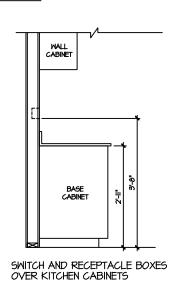




STANDARD ELECTRICAL BOX HEIGHTS

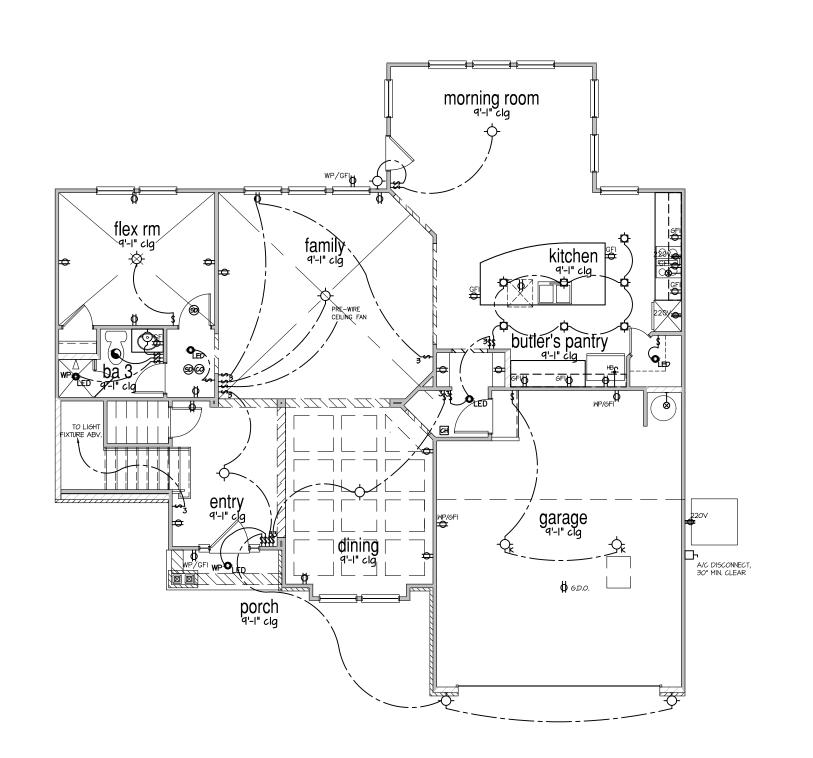


SWITCH AND RECEPTACLE BOXES OVER BATH CABINETS



ELECTRICAL PLAN FIRST FLOOR - ELEV. 3

SCALE: 1/8" = 1'-0'



: Lot UU:UU99.awg DAIE: 12/21/2023 9:US AM

SHEET No.

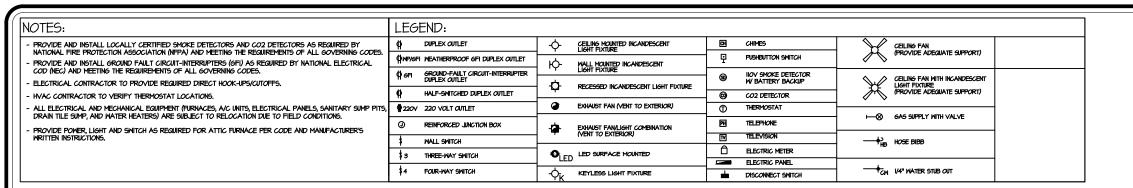
OUSE NAME: RLINGTON

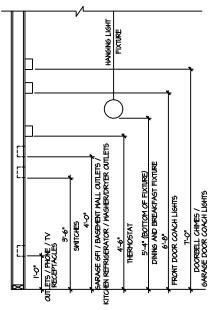
ARLIN DRAWING FLOOR ELECTRICAL

DRAWN BY:

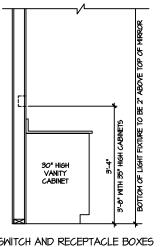
PLAN NO. 1630

DATE: 12/21/2023

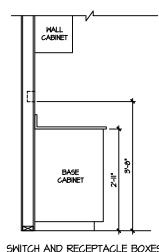


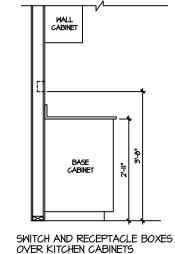


STANDARD ELECTRICAL BOX HEIGHTS

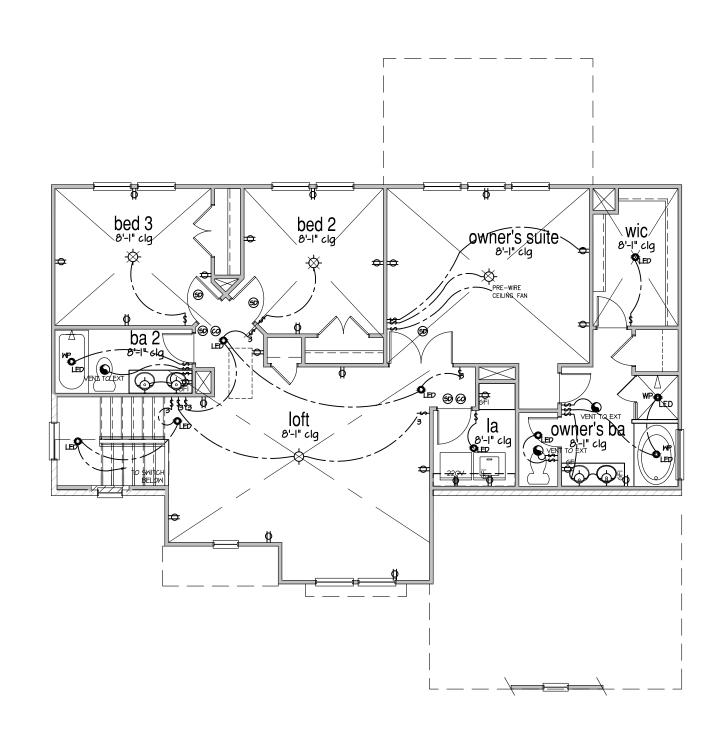


SWITCH AND RECEPTACLE BOXES OVER BATH CABINETS









SHEET No.

HOUSE NAME:
ARLINGTON
DRAWING TITLE

DRAWN BY:

DATE: 12/21/2023

PLAN NO. 1630

SECOND FLOOR ELECTRICAL

DESCRIPTION OF BLDG, ELEMENT	3"x0.131" NAILS	3"x0,120" NAILS
JOIST TO SOLE PLATE	(3) TOENAILS	(3) TOENAILS*
SOLE PLATE TO JOIST/BLK'G.	(3) NAILS • 4" O.C.	(3) NAILS ⊘ 4" O.C.
STUD TO SOLE PLATE	(2) TOENAILS	(3) TOENAILS*
TOP OR SOLE PLATE TO STUD	(2) NAILS	(3) NAILS
RIM TO TOP PLATE	TOENAILS • 8" O.C.	TOENAILS @ 6" O.C.*
BLK'G. BTWN. JOISTS TO TOP PL.	(3) TOENAILS	(3) TOENAILS*
DOUBLE STUD	NAILS @ 24" O.C.	NAILS @ 16" O.C.
DOUBLE TOP PLATE	NAILS @ 24" O.C.	NAILS @ 16" O.C.
DOUBLE TOP PLATE LAP SPLICE	(9) NAILS IN LAPPED AREA	(II) NAILS IN LAPPED AREA
TOP PLATE LAP @ CORNERS & INTERSECTING WALLS	(2) NAILS	(2) NAILS

2½"x0.113 is an acceptable alternative to a 3"x0.120", same spacing or number of nails.
 (ONLY ACCEPTABLE WHERE * ARE SHOWN)

MEANS & METHODS NOTES

THE STRUCTURE IS DESIGNED TO BE SELE FINISHED AND ALL PLAN, DETAIL, AND NOTE SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY DETERMINE THE ERECTION PROCEDURES AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF NECESSARY SHORING SHEETING TEMPORARY BRACING, GUYS, AND TIE-DOWNS. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED TO STABILIZE AND PROTECT EXISTING AND ADJACENT STRUCTURES AND SYSTEMS DURING COURSE OF DEMOLITION AND CONSTRUCTION OF THE PROJECT

STRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING FLEMEN IN CONTACT WITH FLOOR FRAMING ARE LEVEL, INCLUDING, BUT NOT LIMITED TO: FOUNDATIONS, SLABS ON GRADĖ, BEAMS, WALLS, AND NON-BEARING ELEMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LEVELNESS AND MAKE ADJUSTMENTS AS NECESSARY, INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTRY OR WARRANTY TOLERANCES.

ADDITIONAL NOTES FOR TRUSS & I-JOIST MANUFACTURER

ROOF TRUSS, FLOOR TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DIFFERENTIAL DEFLECTION CRITERIA BELOW, UNLESS NOTED OTHERWISE ON PLAN.

TRUSSES/JOISTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN ADJACENT PARALLEL TRUSSES/JOISTS OR GIRDER TRUSSES/FLUS BEAMS DO NOT EXCEED THE FOLLOWING:

- A ROOF TRUSSES.
- I/4" DEAD LOAD FLOOR TRUSSES, ATTIC TRUSSES, & I-JOISTS I/8" DEAD LOAD
- FLOOR TRUSSES & ATTIC TRUSSES ADJACENT TO FLOOR FRAMING BY OTHERS: LIMIT ABSOLUTE TRUSS DEFLECTION TO 3/16" DEAD LOAD. (NOT DIFFERENTIAL DEFLECTION)

GENERAL STRUCTURAL NOTES

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE:
- WOOD FRAME ENGINEERING IS BASED ON NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" - LATEST EDITION.

DESIGN LOADS:

ROOF DEAD = 7 PSF T.C., 10 PSF B.C.

LOAD DURATION FACTOR = 1.25

LIVE = 40 PSF (30 PSF @ SLEEPING AREAS) DEAD = 10 PSF (1-JOISTS & SOLID SAWN)

IO PSF T.C., 5 PSF B.C. (TRUSSES)

(ADD'L IO PSF @ TILE)

LATERAL 120 MPH, EXPOSURE B. SEISMIC A/B.

2,000 PSF ASSUMED ALLOWABLE BEARING PRESSURE (TO BE VERIFIED BY BUILDER)

ALL TYP, NAIL FASTENER REQUIREMENTS ARE NOTED IN STANDARD CONNECTIONS TABLE OR ON PLANS. ALL NAILS SPECIFIED ARE MIN DIAMETER AND LENGTH REQUIRED FOR CONNECTION. ALL HANGER NAILS SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS FOR MAX CHARTED CAPACITY. NOTE: HANGERS USE COMMON NAIL DIAMETERS NOT TYPICAL FRAMING GUN NAILS.

GENERAL FRAMING

- REFER TO FASTENING SCHEDULE TABLE R602.3(1) FOR ALL CONNECTIONS, TYP, U.N.O.
- EXT. & INT. BRG WALLS SHALL BE 2x4 OR 2x6 (AS SHOWN ON PLANS)
 IG* O.C. SPF OR SYP "STUD" GRADE LUMBER, OR BETTER, UN.O. . WALLS OVER 12' TALL SHALL BE PER PLAN.
- ALL HEADERS, BEAMS & OTHER STRUCTURAL MEMBERS SHALL BE SPRICE-PINE-FIR #2 (SPE) OR SOUTHERN PINE #2 (SYP) LUMBER, OR BETTER (KILN-DRIED). ALL HEADERS HAVE BEEN DESIGNED BASED ON CALCULATED LOADS & SIZED ACCORDINGLY. CODE TABLES HAVE NOT BEEN USED.
- ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED @ 16" O.C. (MAX. UN.O.) · HEADERS IN NON-LOAD BEARING WALLS SHALL BE:
- (1)2x4/6 FLAT @ OPENINGS UP TO 4'. (2)2x4/6 FLAT UP TO 8' ALL FRAMING LUMBER SHALL BE DRIED TO 15% MC (KD-15).
- ENGINEERED LUMBER BEAMS TO MEET OR EXCEED THE FOLLOWING
- 'LSL' Fb=2325 psi: Fv=3I0 psi: E=L55xI0^6 psi
- 'LVL' Fb=2600 psi; Fv=265 psi; E=2.0xl0^6 psi 'PSL' - FB=2400 PSI; FV=240 PSI; E=2.0XIO^6 PSI
- M+K SHALL BE FULLY INDEMNIFIED FOR ANY AND ALL ISSUES OWNER DOES NOT SUBMIT THE COMPONENT SHOP DRAWINGS TO M+K FOR STRUCTURAL REVIEW PRIOR TO FABRICATION, DELIVERY, OR INSTALLATION.
- FOR 2 & 3 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 3"X0.120" NAILS @ 8" O/C OR 2 ROWS 1/4"X31/5" SIMPSON SDS SCREMS (OR 3½" TRUSSLOK SCREMS) @ 16" O/C. USE A MINIMUM OF 3 ROMS FOR BEAM DEPTHS OF 14" OR GREATER. APPLY FASTENING AT BOTH FACES FOR 3-PLY CONDITION. LOCATE TOP & BOTTOM NAIL S/SCREWS 2" FROM EDGE SOLID 3 K" OR 5 K" BEAMS ARE ACCEPTABLE. USE 2 ROWS OF NAILS FOR 2x6 \$ 2x8 MEMBERS.
- FOR 4 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF $\frac{1}{4}$ "x6" SIMPSON SDS SCREWS (OR 6 $\frac{3}{4}$ " TRUSSLOK SCREWS) • 16" O/C. USE A MINIMUM OF 4 ROWS FOR BEAM DEPTHS OF 14" OR GREATER, APPLY FASTENING AT BOTH FACES (ONE SIDE ONLY FOR TRUSSLOK SCREWS). LOCATE TOP AND BOTTOM SCREWS 2" FROM EDGE, A SOLID 7" BEAM IS ACCEPTABLE
- ALL HEADERS SHALL BE SUPPORTED BY (1)2x JACK STUD & (1)2x KING STUD, MINIMUM,
- THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE NUMBER OF JACK STUDS REQUIRED, U.N.O.,
- ALL MULTI-PLY STUDS TO BE FASTENED TOGETHER w/ 3"X0.131" NAILS @ 24" O.C. (MIN.), EACH PLY.
- PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FND/BEARING. BLOCKING TO MATCH POST ABOVE.
- FASTEN 2x WOOD PLATES TO TOP FLANGE OF STEEL BEAMS WITH P.A.F.'s ('HILTI' X-CF PINS OR EQUAL) @ 16" O.C. STAGGERED, OR I/2" DIA. BOLTS @ 48" O.C. STAGGERED.
- ALL EXTERIOR 4x4 WOOD POSTS SHALL HAVE SIMPSON BC52-2/4 CAP & ABW44Z BASE, UN.O.

FLOOR FRAMING

- I-JOISTS/TRUSSES SHALL BE DESIGNED BY MANUF. TO MEET OR EXCEED L/480 LIVE LOAD DEFLECTION CRITERIA. (EXCLUDES MARBLE FLOORS - CONTACT MEK FOR MARBLE FLOOR DESIGNS)
- AT I-JOIST FLOORS, PROVIDE I I/8" MIN, OSB RIM BOARD.
- * METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.
- FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED 'STURD-I-FLOOR' 24" O.C. EXPOSURE I (OR APPROVED EQUAL) WITH TONGUE AND GROOVE EDGES. FASTEN TO FRAMING MEMBERS W GLUE AND
- 2 ½" x 0.131" NAILS @ 6"o.c. @ PANEL EDGES \$ @ 12"o.c. FIELD. - 2 2 X 0.120" NAILS @ 4" O.C. @ PANEL EDGES \$ @ 8" O.C. FIELD.
- 2 3" x 0.113" NAILS @ 3" O.C. @ PANEL EDGES & @ 6" O.C. IN FIELD.
- #6 x 2" MIN. SCREMS @ 6" O.C. @ PANEL EDGES & @ 12" O.C. FIELD.

ROOF FRAMING

- BAY WINDOWS & SHED ROOFS (UP TO 6' SPAN) CAN BE 2x4 OR 2x6 RAFTERS & CEILING JOISTS @ 16/24" O.C.
- FASTEN FACH ROOF TRUSS TO TOP PLATE W/ SIMPSON H2.5T CLIP (OR APPROVED EQUAL) • ALL BEARING POINTS. PROVIDE (2) H2.5T CLIPS AT 2-PLY GIRDER TRUSSES (3) H2 5T CLIPS AT 3-PLY GIRDER TRUSSES & ROOF BEAMS - AT ALL BEARING POINTS
- METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.O.
- ERECT AND INSTALL ROOF TRUSSES PER WTCA & TPI'S BCSI I-08 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- SUPPORT PORCH & SHORT SPAN ROOF TRUSSES (MAX 7' SPAN) w/ 2x4 LEDGER EASTENED TO:
 - RIM BOARD w/ (2) 3"x0.131" NAILS @ 16" O.C. MAX. (1-JOISTS) TRUSS VERTICALS W/ (3) 3"x0.131" NAILS @ 19.2" O.C. MAX. (FLOOR TRUSSES)
- ROOF SHEATHING SHALL BE 7/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE I (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS
- W/ 2 ½" x 0.131" NAILS @ 6"O.C. @ PANEL EDGES & @ 12" O.C. FIELD. - w/ 2 (× 0.120" NAILS • 4"o.c. • PANEL EDGES € • 8" O.C. FIELD.
- W/ 2 3 × 0.113" NAILS @ 3"O.C. @ PANEL EDGES \$ @ 6" O.C. FIELD.

VENEER LINTEL SCHEDULE

SPAN (MAX)	HEIGHT OF VENEER ABOVE LINTEL	Steel angle size
3'-0"	20 FT. MAX	L3"x3"x/4"
	3 FT. MAX	L3"x3"x/4"
6'-0"	I2 FT. MAX	L4"x3"x/4"
	20 FT. MAX	L5"x3½"x¾"
8'-0"	3 FT. MAX	L4"x4"x¼" "
0-0	I2 FT. MAX	L5"x3½"x¾;"
	I6 FT. MAX	L6"x3%"x%"
4'-6"	I2 FT. MAX	L6"x3½"x¾6"
16'-0"	2 FT. MAX	L7"x4"x½" ***
	3 FT. MAX	L8"x4"x½" **

- LINIELS. ALL SUPPORT 2 % 3 ½ YENEER W 40 ps MAXIMIM WEI 'SHALL HAVE 4" MIN BEARING 'SHALL NOT BE FASTENED BACK TO HEADER

- IF SHALL BUT BE FASTERED BACK TO FEADER IN WALL 6469 C. W. III'D DIA 25 JUL 10 SHALL BE FASTERED BACK TO MOOD FEADER IN WALL 6469 C. W. II'D DIA 25 JUL 10 SHALL BE FASTERED BACK TO MOOD FEADER IN COPE THE OFFERNMENT OF THE LIGHT EIGHT STRENGT TOE OF THE MORIZORTAL LES WHEN JUL 10 SHALL BE SHOULD BE SHOULD SHALL BE AND FEADER SHOULD SHALL BE SHOULD SHALL BE AND FEADER SHALL BE CONTINUED CONTINUED HIS SHALL BE SHOULD SHALL BE AND FEADER SHALL BE CONTINUED CONTINUED HIS SHALL BE SHOULD SHALL BE SHOUL
- R QUEEN VENEER USE LAXXXX'. OR 3½" VENEER ONLY SEE PLAN FOR VENEER SUPPORT IF VENEER < 3½" THICK.

SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

LATERAL BRACING & SHEAR NALL SHEATHING SPECIFICATIONS

THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM:) MPH WIND IN 2018 NCSBC:R(

(120 MPH WIND SPEED IN ASCE 7-10 WIND MAP, PER IRC R301,2,1,1) EXP. B, RISK CAT. 2 & SEISMIC CAT. A/B.

THE DESIGN WAS COMPLETED PER 2015 IBC (SECTION 1609) & ASCE 7-10, AS PERMITTED BY R30113 OF THE 2018 NGSBC-RC OR THE SIMPLIFIED PRESCRIPTIVE PROCEDURE IN ACCORDANCE WITH THE 2015 IRC IF THE PARAMETERS OF SECTION R602.12 COMPLY ACCORDINGLY, THIS MODEL, AS DOCUMENTED AND DETAILED HEREWITHIN, IS ADEQUATE TO RESIST THE CODE REQUIRED LATERAL FORCES.

DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 7-10 (ACCEPTED ENGINEERING PRACTICE) AS ALLOWED PER 2018 NCSBC:RC SECTION R802.II.I. THIS MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED TO RESIST THE WIND UPLIFT LOAD PATH PER SECTIONS R602 3 5& R802 II

EXT. WALL SHEATHING SPECIFICATION

- 7/16" OSB OR 15/32" PLYWOOD: FASTEN SHEATHING W 2 3/8"XOJI3" NAILS @ 6" O.C. AT EDGES \$ @ 12" O.C. IN THE PANEL FIELD. TYP, UN.C.
- HORIZONTAL BLOCKING OF EXT. WALL/SHEAR WALL PANEL EDGES IS NOT REQUIRED BY THIS DESIGN EXCEPT FOR THOSE AREAS SPECIFICALLY NOTED.
- ALL EXT. WALLS SHALL BE CONTINUOUSLY SHEATHED AND ARE CONSIDERED SHEAR WALLS.
- ALT. STAPLE CONNECTION SPEC: 1 名" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6" O.C IN FIELD.

BLOCKED PANEL EDGES

AT DESIGNATED AREAS - FASTEN SHEATHING w/ 2 %" x 0.113" NAILS @ 6" O.C. AT ALL PANEL EDGES AND 12" O.C. IN THE PANEL FIELD OR 1 34" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES \$ @ 6'
O.C. IN FIELD. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2X HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE FASTENING.

3" O.C. EDGE NAILING

AT DESIGNATED AREAS - FASTEN PANEL EDGES OF WOOD STRUCTURAL WALL SHEATHING TO FRAMING W 8d NAILS @ 3" O.C. NO STAPLE ALTERNATIVE AVAILABLE AT THIS SPEC. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING

NOTES

- SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING, IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN, IT WILL BE SPECIFICALLY NOTED ON PLAN.
- DESIGN ASSUMES 16" O.C. MAX. STUD SPACING, U.N.O.
- ALL STRUCTURAL PANELS ARE TO BE DIRECTLY APPLIED TO STUD FRAMING
- PRE-MANUFACTURED PANELIZED WALLS:
 FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD W/ 3" x 0.120" VAILS @ 4" O.C. (THRU ONE SIDE ONLY)
- INDICATES EXTENT OF INT. OSB SHEARWAI OR 3" O.C. OSB SHEARWALL.
- INDICATES HOLDOWN BELOW

GENERAL STRUCTURAL NOTES

FOUNDATION

- * DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE: RESIDENTIAL CODE.
- FOOTING DESIGN 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED. BUILDER/CONTRACTOR MUST VERIFY.
- FASTEN 2x4/6 SILL PLATES TO FND WITH A MINIMUM OF 2 ANCHORS PER PLATE, 12" MAX, FROM PLATE ENDS - UTILIZING
- 1/2" DIA. ANCHOR BOLTS 6'-0" O.C, 7" MIN. EMBEDMENT (CONC), 15" MIN, EMBEDMENT (CMU)
- SIMPSON MASA ANCHOR STRAPS @ 6'-0" O.C. (CONC)
- * SIMPSON MAB23 ANCHOR STRAPS 2'-8" O.C. (CMU) (REFER TO DETAILS FOR IO' TALL WALL ANCHOR REQUIREMENTS)
- * ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT W CONCRETE OR CMU SHALL BE PRESERVATIVE TREATED SOUTHERN PINE #2,
- BUILDER TO VERIFY CORROSION-RESISTANCE COMPATIBILITY OF HARDWARE & FASTENERS IN CONTACT W/ PRESERVATIVE-TREATED WOOD, CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD.
- BASEMENT INTERIOR BEARING WALLS & EXTERIOR WALK-OUT BASEMENT WALLS SHALL BE 2x6 € 16" O.C. SPF OR SYP, "STUD" GRADE OR BETTER.
- CONCRETE DESIGN BASED ON ACI 318, CONCRETE SHALL ATTAIN THE FOLLOWING MIN. COMPRESSIVE STRENGTHS IN 28 DAYS, UN.O.: 4,000 psi: FOUNDATION WALLS
 2,500 psi: FOOTINGS & INTERIOR SLABS ON GRADE 3,000 psi: GARAGE & EXTERIOR SLABS ON GRADE
- 60,000 psi BASEMENT FOUNDATION WALL DESIGN BASED ON:
- 9' OR 10' HEIGHT (AS NOTED ON PLANS)
 TALLER WALLS MUST BE ENGINEERED.
- NOMINAL WIDTH (91/3" FOR 10" THICK WALL).
- * BASEMENT WALL DESIGN IS BASED ON 60 PCF BACKFILL SOIL TYPE CLASSIFICATIONS (SC. ML-CL. OR CL).
- BASEMENT WALLS SHALL BE BRACED, PRIOR TO BACKFILLING, BY ADEQUATE TEMPORARY BRACING OR INSTALL ISE FLOOR DECK.
- PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN CONCRETE BSMT, FND, WALL WITH 2" CLEAR, REINFORCEMENT
- SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS.

 FOR OPENINGS UP TO 36", PROVIDE MINIMUM 10" CONCRETE DEPTH OVER OPENING OR (3)2x10 W (2)2x6 JACK STUDS, U.N.O.
- LARGER OPENINGS SHALL BE PER PLAN.
- * ALL CONCRETE EXPOSED TO THE WEATHER SHALL NOT HAVE LESS THAN 5% OR MORE THAN 7% AIR ENTRAINMENT
- * ALL FOOTINGS SHALL BEAR AT LEAST 12" BELOW FINISH GRADE.
- FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR 95% COMPACTED FILL.
- PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY TO DEVELOP
- . JOINTS SHALL BE LOCATED . 10'-0" O.C. (RECOMMENDED) OR
- 15'-0" O.C. (MAXIMUM) · JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS
- POSSIBLE (IN RATIO) WITH A MAXIMUM OF INS RATIO · CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL
- CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C90 WITH A MIN. COMPRESSIVE STRENGTH OF 1900 psi (Fm=1500 psi), MORTAR SHALL BE ASTM C270, TYPE S. CMU DESIGN PER ACI 530 € 530.I.
- · CMU FOUNDATION WALLS SHALL HAVE 'DUR-O-WALL' HORIZONTAL JOINT REINFORCEMENT (OR EQUAL) - 9 GA. MINIMUM @ 16" O.C.
- ◆ PROVIDE 2x8 x 16" LONG P.T. PLATE ON TOP OF ALL CRAWL SPACE PIERS. ALL PIERS SHALL BE GROUTED SOLID.
- PROVIDE 2x6 P.T. PLATE ON INTERIOR CRAWL SPACE WALLS. FASTENED PER ANCHORAGE SPECIFICATION NOTED ABOVE
- * BUILDER TO VERIFY THAT MODEL HAS BEEN ADEQUATELY TREATED BY A LICENSED AND BONDED PEST CONTROL COMPANY FOR SUBTERRANEAN TERMITES. METHOD AND TYPE OF TREATMENT TO BE DETERMINED BY PEST CONTROL COMPANY.

HOLD-DOWN SCHEDULE

* DIMENSIONS BY OTHERS, BUILDER TO VERIFY.

5YMB0L	SPECIFICATION
► HD-I	SIMPSON HTT4 HOLD-DOWN *
► HD-2	SIMPSON MSTC66 STRAP TIE (CENTER STRAP ON FLOOR SYSTEM UN.O.) (PRE-BENT MSTC66 ALT. WHEN SPECIFIED)
▶ нр-з	SIMPSON STHD14/14RJ HOLD-DOWN

ALTERNATIVE TO SSTB24 ANCHOR BOLT SPECIFICATION: UTILIZE SIMPSON "SET" EPOXY SYSTEM TO FASTEN 3/8 DIA. THREADED ROD INTO CONCRETE FOUNDATION, PROVIDE 12" MIN. EMBEDMENT INTO CONCRETE. INSTALL PER MANUF. RECOMMENDATIONS. DO NOT LOCATE ANCHORS WITHIN I 3/4" OF EDGE OF FOUNDATION

al: 1/10/24 CAR NOFESSIO, \circ ENGINE SEPH T. R

RN+KUL Ш#

I&K project numbe

126-2306 JTR rawn bv: ssue date: 01-10-2

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SEAL OF OFFICE OF THE RING NULP CARD OF THE

MULHERN+KUL RESIDENTIAL STRUCTURAL ENGINEERI SURMINISTA AND ADMINISTA PARSON

M&K project number:

M&K project number: 126-23061

project mgr: JTR drawn by: SJF issue date: O1-10-24

REVISIONS:

e: initial:

BLAKE POND COMMUNITY Lot 99 - arlington 3 raleigh, nc

S1.0

OUNDATION PL

INTERIOR BEARING WALL

• ==== BEARING WALL ABOVE

• ---- BEAM / HEADER
• ---- INDICATES SHEAR WALL & EXTENT

EXTENT OF OVERFRAMING

JL METAL HANGER

* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

LEGEND

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR
TYPICAL STRUCTURAL NOTES
\$ SCHEDULES

CRAWL SPACE FOUNDATION PLAN SCALE: 1/8"=1"-0"

ENGINEERED BEAM MATERIAL SCHEDULE					HA CAR	
BEAM NUMBER	LVL OPTION	PSL OPTION	LSL OPTION	FLITCH OPTION	STEEL OPTION	NO FESSIO
001	(3)13/4"×16" - FB	5 ¼"xl6" - FB	NA	(3)2xi2 + (2) ½"xil¼" STEEL FLITCH PLATES - F	WI2xI9 - F	S . 880
002	(2)134"×94" - D	3½"x9¼" - D	(2)134"x94;" - D	(2)2xi0 + (1) ¼"x4¼" STEEL FLITCH PLATE - D	₩8×10 - D	SEAL
003	(2)13/4"×14" - F	3½"x 4" - F	N/A	(2)2xl2 + (I) है"xll4" STEEL FLITCH PLATE - F	WI2xI4 - F	686612
004	(2)194"×16" - H	3 ½"x16" - H	(3)1%"×16" - H	(3)2xl2 + (2) xllk" STEEL FLITCH PLATES - F	NA 🍂	N Taraba
005	(2)1¾"x9¼" - D	3½"x9¼" - D	(2)134"×944" - D	(2)2x10 + (1) ¼"x4¼" STEEL FLITCH PLATE - D	M8xI0 − D	NGINEE PH T. R
006	(2)134"×1136" - D	3½"×11%" - D	(3)13/4"x11 🖟 - D	(2)2xl2 + (l)以"xll以" STEEL FLITCH PLATE - D	WIOx12 - D	WILL PHIR
001	(3)1¾"x14" - D	5 ¼"xl4" - D	(4)13/4"×14" - D	(3)2xl2 + (2) xll/4" STEEL FLITCH PLATE - D	MIOx19 - D	WINDEPH T. R
008	(2)1¾"x9¼" - D	3½"x9¼" - D	(2)134"x94" - D	(2)2x10 + (1) ¼"x4¼" STEEL FLITCH PLATE - D	W8x10 - D	
004	(2)1¾"x16" - H	3 ½"x16" - H	(3)134"×16" - H	(3)2xl2 + (2) \frac{1}{2}\k\rak* STEEL FLITCH PLATES - F	N/A	
010	(2)1¾"x11%" - H	3½"xI1%" - H	(3)13¼"×II 🖟 - H	(2)2xl2 + (l)从"xll以" STEEL FLITCH PLATE - H	MIOx12 - H	
OII	(2)13/4"×14" - F	3½"x 4" - F	(2)1 % "x14" - F	(2)2x12 + (1) है"xII¼" STEEL FLITCH PLATE - F	WI2xI4 - F	
012	¾"x 4" - F	3½"x 4" - F	i¾"xi4" - F	(2)2xl2 + (1) ‡"xll½" STEEL FLITCH PLATE - F	WI2xI4 - F	
013	(2)13/4"x16" - FB	3 ½"x16" - FB	(3)1¾"x16" - FB	(3)2xi2 + (2) 如水 STEEL FLITCH PLATES - F	WI2xI4 - F	
014	(2)13/4"×14" - F	3½"xl4" - F	(2)13/4"×14" - F	(2)2xl2 + (I) है"xll¼" STEEL FLITCH PLATE - F	WI2xI4 - F	
010	(2)1¾"x11%" - F	3½"×11%" - F	(3)1¾"×II }" - F	(2)2xl2 + (I) 从"xll以" STEEL FLITCH PLATE - F	₩0×12 - F	

JTR frawn by: issue date: 01-10-24

eal: 1/10/24

initial:

LEGEND

INTERIOR BEARING WALL

• = INDICATES SHEAR WALL & EXTENT

EXTENT OF OVERFRAMING

* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES \$ SCHEDULES

IST FLOOR FRAMING PLAN

MULHERN+KUL
RESIDENTIAL STRUCTURAL ENGINEERII BEAM NOTATION:

- "F" INDICATES FLUSH BEAM
- "IT" INDICATES FLUSH TOP BEAM
- "TO" INDICATES FLUSH BOTTOM BEAM
- "O" INDICATES FLUSH BOTTOM BEAM
- "O" INDICATES DROPPED BEAM
- "INDICATES DROPPED DEAM
- "IN INDICATES DROPPED OPENING HEADER
REFER TO DETAIL D'SP220 FOR TYPICAL FLITCH BEAM CONNECTIONS
REFER TO DETAIL D'SP220 FOR TYPICAL FLITCH BEAM CONNECTIONS
REFER TO DETAIL D'SP220 FOR TYPICAL SIEEL BEAM CONNECTIONS
FOR FLUSH TOP BEAM'S PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D. FASTEN
PLATES IN SUCCESSION W (2) 3"X0J20" NAILS 0 0" OC.

FOR FLUSH BOTTOM BEAM'S PROVIDE 2X STACKED PLATES ATOP BEAM AS REQ'D. FASTEN
PLATES IN SUCCESSION W (2) 3"X0J20" NAILS 0 0" OC. 126-2306 PLATES IN SUCCESSION w/ (2) 3"x0.120" NAILS @ 8" O.C. REVISIONS:

> SD2, REFERS TO SD2, IA FOR LVL/PSL/LSL BEAMS OR SD2.IB PLANS FOR FLITCH BEAMS OR SD2.IC BLAKE POND COMMUNITY Lot 99 - Arlington 3 Raleigh, ng FOR STEEL BEAMS

● □===□ BEARING WALL ABOVE

• BEAM / HEADER

JL METAL HANGER

OOR

ROOF TRUSSES @ 24" O.C.

2ND FLOOR FRAMING PL SCALE: 1/8"=1"-0"	.AN
SCALE: 1/8"=1'-0"	

ENGINEERED BEAM MATERIAL SCHEDULE					
BEAM NUMBER	LVL OPTION	PSL OPTION	LSL OPTION	FLITCH OPTION	STEEL OPTION
001	(3)13/4"x16" - FB	5 ‡"xl6" - FB	N/A	(3)2xi2 + (2) ½"xil¼" STEEL FLITCH PLATES - F	WI2xI9 - F
002	(2)1¾"x9¼" - D	3½"×94" - D	(2)134"×944" - D	(2)2x10 + (1) ¼"x4¼" STEEL FLITCH PLATE - D	W8x10 - D
003	(2)13/4"×14" - F	3½"x 4" - F	N/A	(2)2xl2 + (I) है"xll4" STEEL FLITCH PLATE - F	WI2xI4 - F
004	(2)194"×16" - H	3 ½"x16" - H	(3)194"×16" - H	(3)2xl2 + (2) إلالاً إِذْ (5) STEEL FLITCH PLATES - F	N/A
005	(2)134"×94" - D	3½"×94" - D	(2)1¾"xq4;" - D	(2)2xi0 + (i) ¼"x4¼" STEEL FLITCH PLATE - D	₩8x10 - D
006	(2)1¾"x11%" - D	3½"x11%" - D	(3)13¼"×II 🖟" - D	(2)2xl2 + (l)从"xll以" STEEL FLITCH PLATE - D	WI0x12 - D
001	(3)13/4"×14" - D	5 ‡"xl4" - D	(4)1¾"x14" - D	(3)2xl2 + (2)8"xll4" STEEL FLITCH PLATE - D	MIOXI9 - D
008	(2)134"×94" - D	3½"×94" - D	(2)134"×944" - D	(2)2xi0 + (1) ¼"x4¼" STEEL FLITCH PLATE - D	₩ 8 ×10 - D
P00	(2)13/4"x16" - H	3 ½"xl6" - H	(3)134"x16" - H	(3)2xl2 + (2) xilk STEEL FLITCH PLATES - F	N/A
010	(2)194"xII%" - H	3½"×II%" - H	(3)13¼"x 1 3	(2)2xl2 + (l)从"xll以" STEEL FLITCH PLATE - H	MIOxI2 - H
OII	(2)13/4"x14" - F	3½"x 4" - F	(2)134"×14" - F	(2)2x12 + (1) है"xII¼" STEEL FLITCH PLATE - F	WI2xI4 - F
012	1¾"x14" - F	3½"x14" - F	l¾"xl4" - F	(2)2xl2 + (1) ¼"xll¼" STEEL FLITCH PLATE - F	WI2xI4 - F
013	(2)134"x16" - FB	3 ½"xi6" - FB	(3)134"×16" - FB	(3)2xl2 + (2) ½"xll¼" STEEL FLITCH PLATES - F	WI2xI4 - F
014	(2)13/4"×14" - F	3½"xl4" - F	(2)15/4"×14" - F	(2)2xl2 + (I) है"xll¼" STEEL FLITCH PLATE - F	WI2xI4 - F
010	(2)1¾"x11%" - F	3½"xII%" - F	(3)1¾"x 1 ⅓" - F	(2)2xl2 + (l)以"xll以" STEEL FLITCH PLATE - F	WI0x12 - F

BEAM NOTATION:
- "F" INDICATES FLUSH BEAM
- "FT" INDICATES FLUSH TOP BEAM
- "FB" INDICATES FLUSH BOTTOM BEAM

- "D' INDICATES DROPPED BEAM
- "H" INDICATES DROPPED OPENING HEADER
REFER TO DETAIL D'8020 FOR TYPICAL FLITCH BEAM CONNECTIONS
REFER TO DETAIL E/8020 FOR TYPICAL STEEL BEAM CONNECTIONS

FOR FLUSH TOP DEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION W (2) 3"XO120" NAILS @ 8" O.C.
FOR FLUSH BOTTOM BEAMS PROVIDE 2X STACKED PLATES ATOP BEAM AS REQ'D. FASTEN

PLATES IN SUCCESSION w/ (2) 3"x0.120" NAILS @ 8" O.C.

eal: 1/10/24 TH CAR SEPH T. RIV

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RESIDENTIAL STRUCTURAL ENGINEERI



M&K project number: 126-2306

JTR frawn by: issue date: 01-10-24

REVISIONS:

initial:

LANS BLAKE POND COMMUNITY Lot 99 - arlington 3 raleigh, nc

● □===□ BEARING WALL ABOVE

• = INDICATES SHEAR WALL & EXTENT EXTENT OF OVERFRAMING

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

OOR

SD2.I REFERS TO SD2.IA FOR LYL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

LEGEND

INTERIOR BEARING WALL

• --- BEAM / HEADER

JL METAL HANGER

* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

MULHERN+KUL
RESIDENTIAL STRUCTURAL ENGINEERI



126-23061

JTR drawn by: issue date: 01-10-24

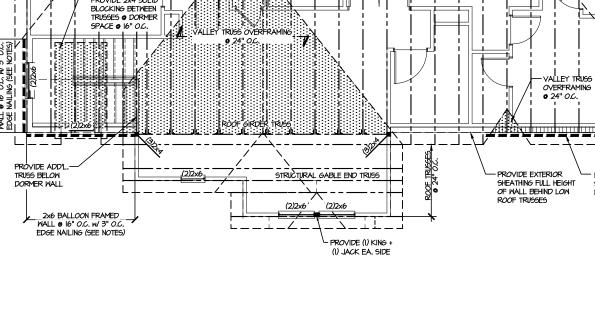
REVISIONS:

initial:

LEGEND

- IIIIIII INTERIOR BEARING WALL ● □===□ BEARING WALL ABOVE
- --- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- * INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- NDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES



ACCESS BETWEEN ROOF TRUSSES

— PROVIDE (2) KINGS + (1) JACK EA, SIDE

- PROVIDE (2) KINGS + (1) JACK EA. SIDE

- PROVIDE 1/6" OSB SHEATHING PER EXT. WALL SHEATHING SPEC.

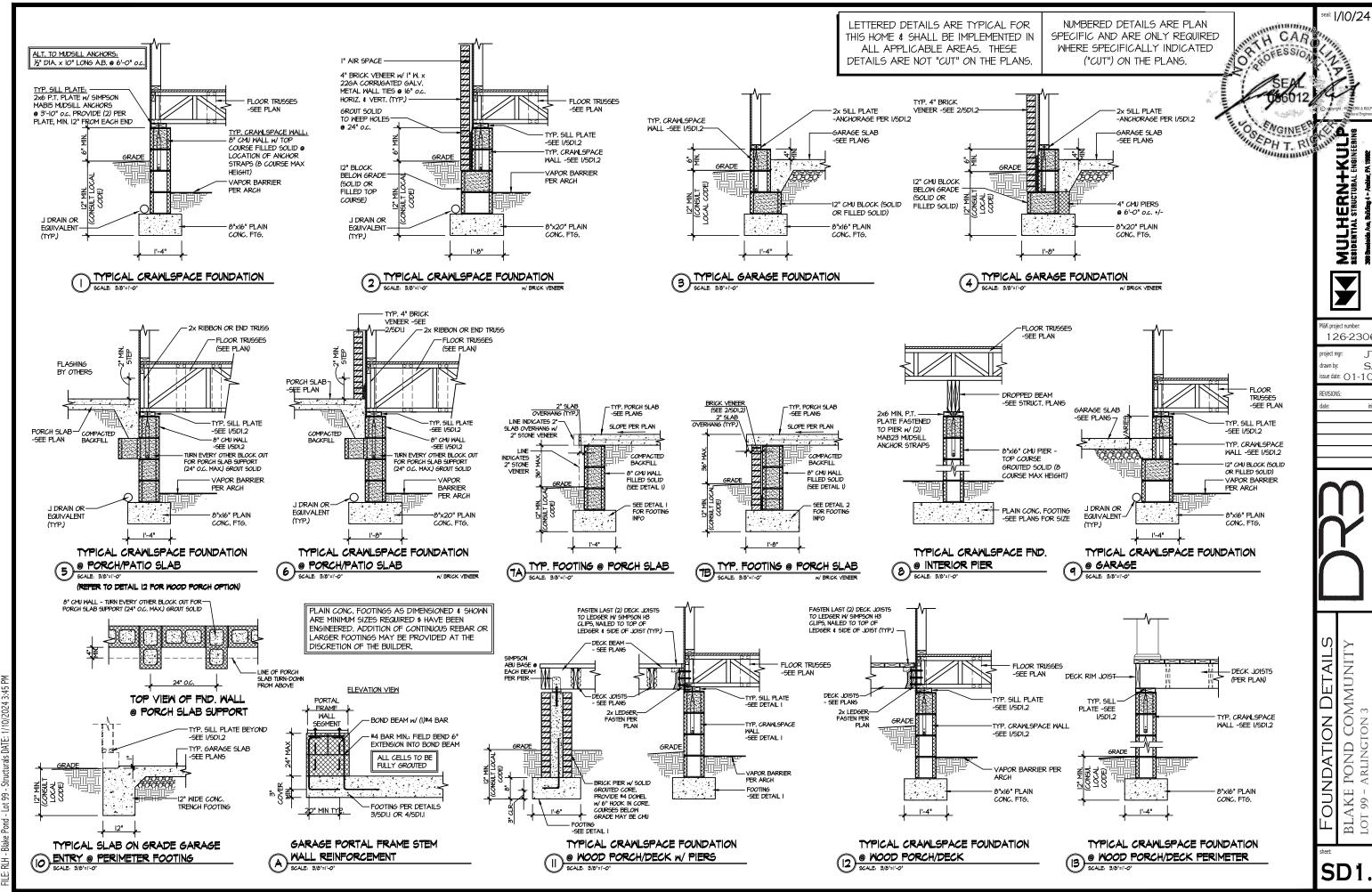
ROOF TRUSSES @ 24" O.C.

— PROVIDE (2) KINGS + (1) JACK EA, SIDE

PROVIDE 2x4 SOLID

ROOF FRAMING PLAN SCALE: 1/8"=1'-0"

BLAKE POND COMMUNITY Lot 99 - arlington 3 raleigh, nc OOF

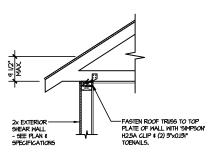


l&K project number 126-2306 JTR ssue date: 01-10-2

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POND ARLINGT , NC LOT 99 - A RALEIGH, AKE r 99 -

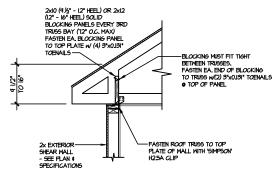


TYPICAL SHEAR

BETWEEN FLOORS @ EXTERIOR WALL
SCALE 500'-1-0' PARALLE FRAS

TRANSFER DETAIL @ ROOF

SCALE: 3/8'=1'-0' HEEL HEIGHT LESS THAN HEEL HEIGHT LESS THAN 9½" NO BLOCKING REQ'D

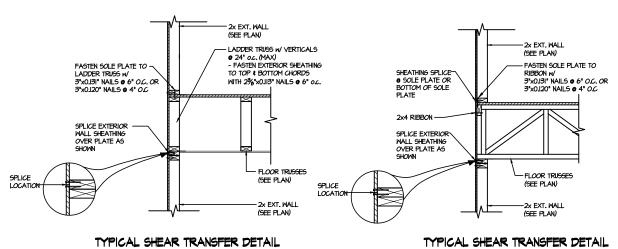


TYPICAL SHEAR

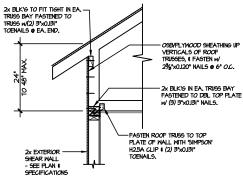
TRANSFER DETAIL @ ROOF

SCALE: 9/8'=1-0' HEEL HEIGHT BETWEEN 4)



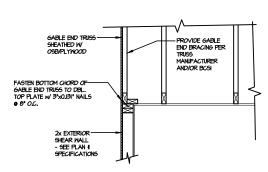


TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ EXTERIOR WALL
SCALE 200'-11-0' FERFEDICIAN FROMS



TYPICAL SHEAR TRANSFER

DETAIL @ RAISED HEEL TRUSS



TYPICAL GABLE END DETAIL SOLLE SIDE STATE

- STEEL FLITCH PLATE (SEE PLAN) FASTEN WOOD BEAMS W/ FLITCH -PLATE W/ (2) ROWS OF 36" DIA. THRU-BOLTS • 16" O.C. 2x WOOD BEAM -2x WOOD BEAM (SEE PLAN) 2-FLY WOOD SEAM W/ — STEEL FLITCH PLATE (SEE PLAN) FASTEN WOOD BEAMS W/ FLITCH-PLATES W (2) ROWS OF 5/8" DIA. THRU-BOLTS • 16" O.C.

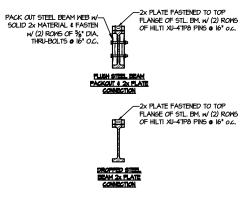
> (SEE PLAN) S-FLY OR MORE WOOD BEAM w/

-2x WOOD BEAM (SEE PLAN)

2Y WOOD BEAM

2x WOOD BEAM

TYPICAL FLITCH BEAM CONNECTION DETAIL SCALE 344-1-07



TYPICAL STEEL BEAM CONNECTION DETAIL SCALE 9/4"-1"-0"

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

G DETAILS RAMIN

SD2.0

BLAKE POND COMMUNIT LOT 99 - ARLINGTON 3 RALEIGH, NC

al: 1/10/24

MULHERN+KUL RESIDENTIAL STRUCTURAL ENGINEERIN

M&K project number:

frawn by:

REVISIONS

126-2306

issue date: 01-10-24

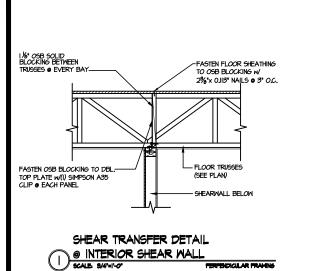
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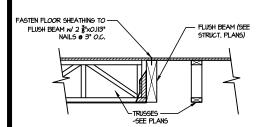
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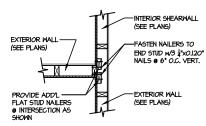
SEPH T. R

LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

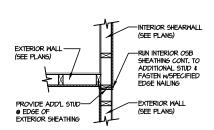




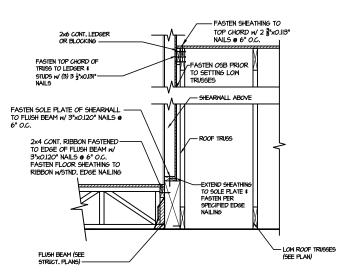
SHEAR TRANSFER DETAIL @ INTERIOR SHEARWALL ABOVE



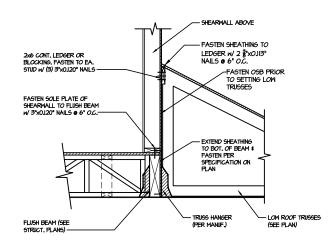
SHEAR TRANSFER DETAIL @ INTERSECTING INT. SHEARWALL SCALE. SIATH-O' SHITE.



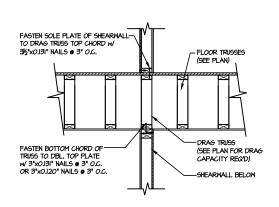
SHEAR TRANSFER DETAIL @ INTERSECTING INT. SHEARWALL



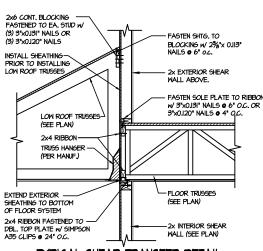
SHEAR TRANSFER DETAIL @ EXTERIOR SHEARWALL ABOVE



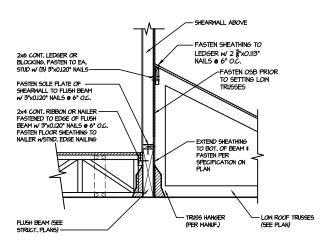
SHEAR TRANSFER DETAIL @ 6 EXTERIOR SHEARMALL ABOVE



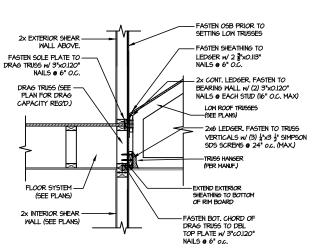
SHEAR TRANSFER DETAIL @ INT. SHEARWALL ABOVE & BELOW SCALE SATISTY OF PARTY.



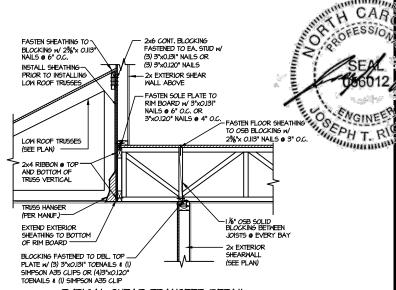
TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR MALL
SCALE 94'-1'-0" FENTEDICLAR FR



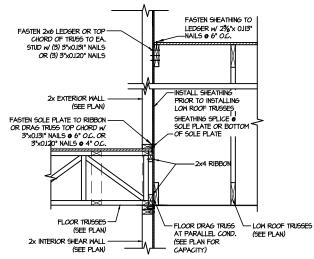
SHEAR TRANSFER DETAIL @ EXTERIOR SHEARWALL ABOVE



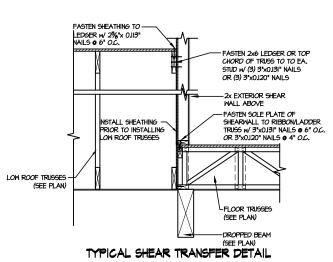
TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL



TYPICAL SHEAR TRANSFER DETAIL @ EXTERIOR WALL ABOVE SCALE SIATELO



TYPICAL SHEAR TRANSFER DETAIL 8 BETWEEN FLOORS @ INTERIOR WALL



BETWEEN FLOORS @ INTERIOR WALL
SCALE SAT-11-0"

SD2.1A

COMMUNIT

ARLINGI POND

BLAKE | LOT 99 - / RALEIGH,

AIL

al: 1/10/24

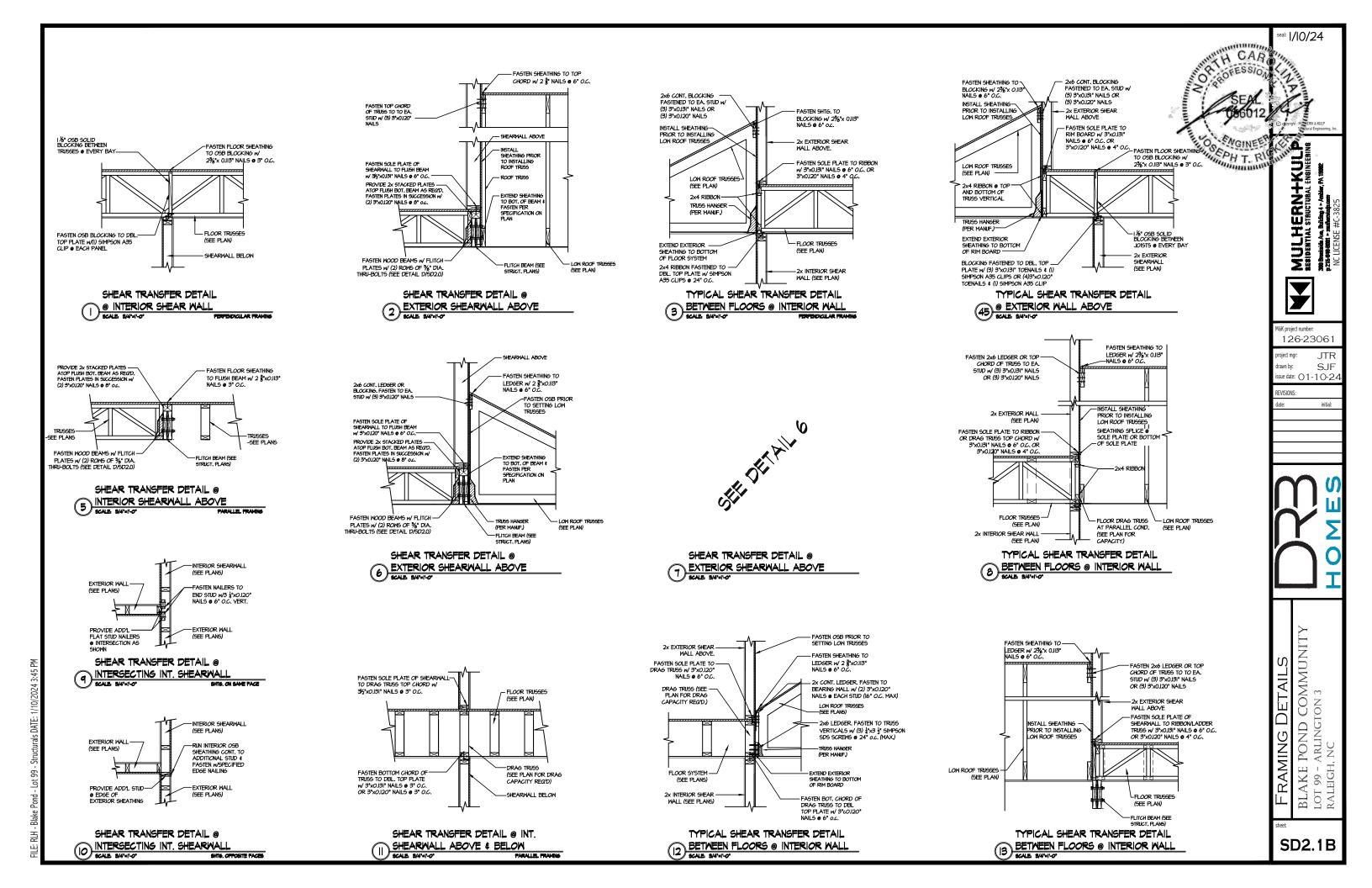
ERN+KUL STRUCTURAL ENSINEERIN

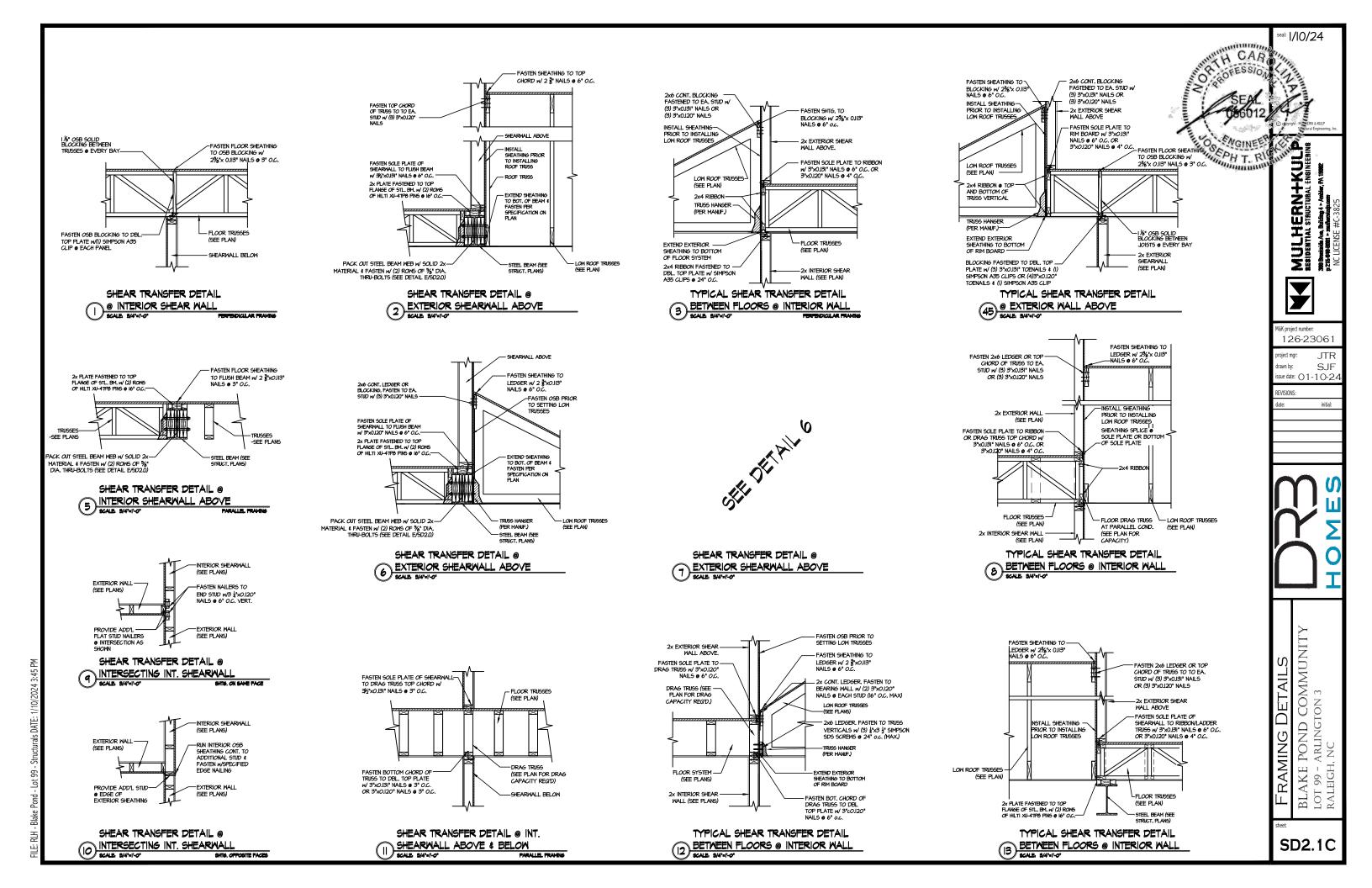
I&K project numbe 126-2306

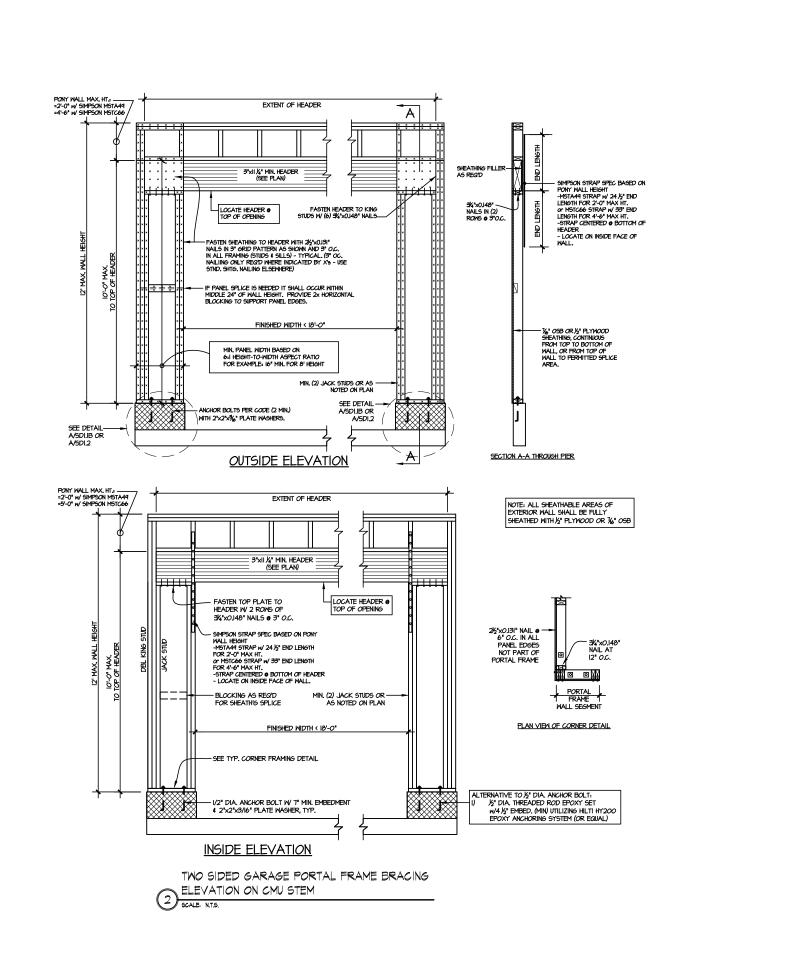
issue date: 01-10-2

JTR

initial:







eal: 1/10/24 TO PAOPES OSEPH T. R

MULHERN+KUL RESIDENTIAL STRUCTURAL ENSINEERIN



126-2306

roject mgr: **JTR** drawn by: SJF issue date: 01-10-24

REVISIONS:

initial:

BLAKE POND COMMUNITY Lot 99 - Arlington 3 Raleigh, nc RAMING DETAILS

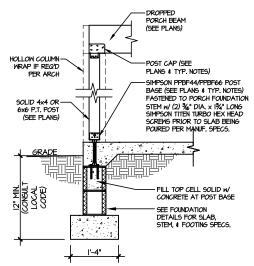
eal: 1/10/24 TOP SOFES O ENGINEE MULHERN+KULP
RESIDENTIAL STRUCTURAL ENSINEERINS

— DROPPED PORCH BEAM (SEE PLANS) HOLLOW COLUMN — WRAP IF REQ'D PER ARCH -POST CAP (SEE PLANS & TYP. NOTES) -- POST BASE (SEE PLANS & TYP, NOTES) W ½" DIA, ANCHOR BOLT OR SIMPSON TITEN HD W MIN, 6" EMBED. SOLID 4x4 OR -6x6 P.T. POST (SEE PLANS) GRADE I2" MIN. (CONSULT LOCAL CODE) - SEE FOUNDATION DETAILS FOR SLAB, STEM, & FOOTING SPECS. l'-4**"**

TYPICAL PORCH POST CONNECTION

3 DETAIL INSTALLED AFTER SLAB POUR CRANLSPACE FOUNDATION

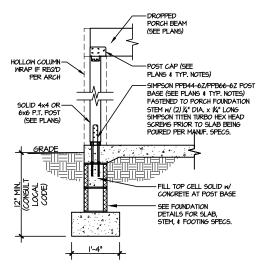
CRAWLSPACE FOUNDATION



TYPICAL PORCH POST CONNECTION TYPICAL PORCH POST CONNECTION DETAIL INSTALLED PRIOR TO SLAB POUR
SCALE. NONE

CRAIN GRAVE ECONOMICS B DETAIL INSTALLED PRIOR TO SLAB POUR

CRAW GRAVE EVANDATION



M&K project number: 126-2306

issue date: 01-10-24

JTR

initial:

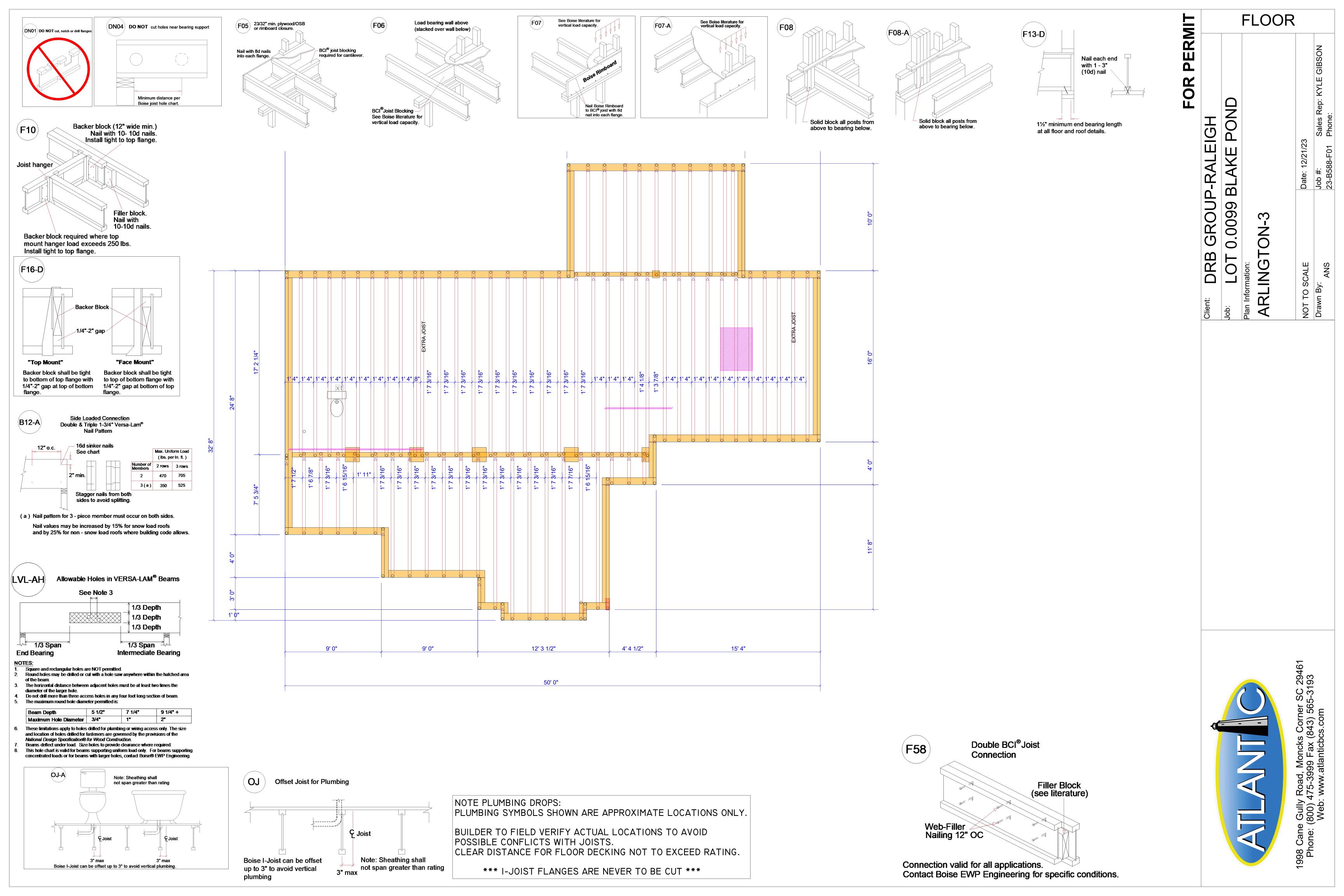
roject mgr:

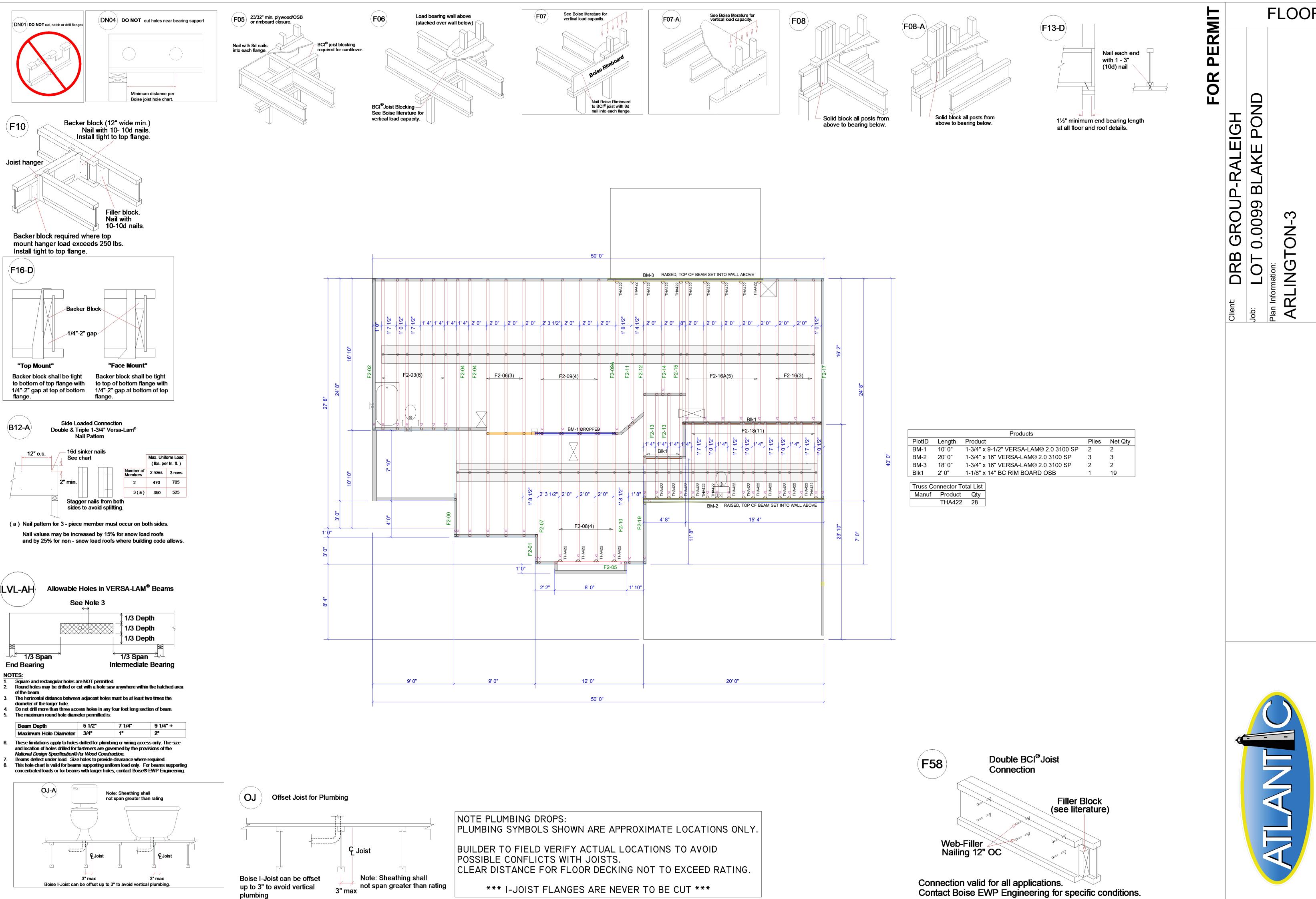
drawn by:

REVISIONS

BLAKE POND COMMUNITY Lot 99 - arlington 3 raleigh, nc FRAMING DETAILS

SD3.0



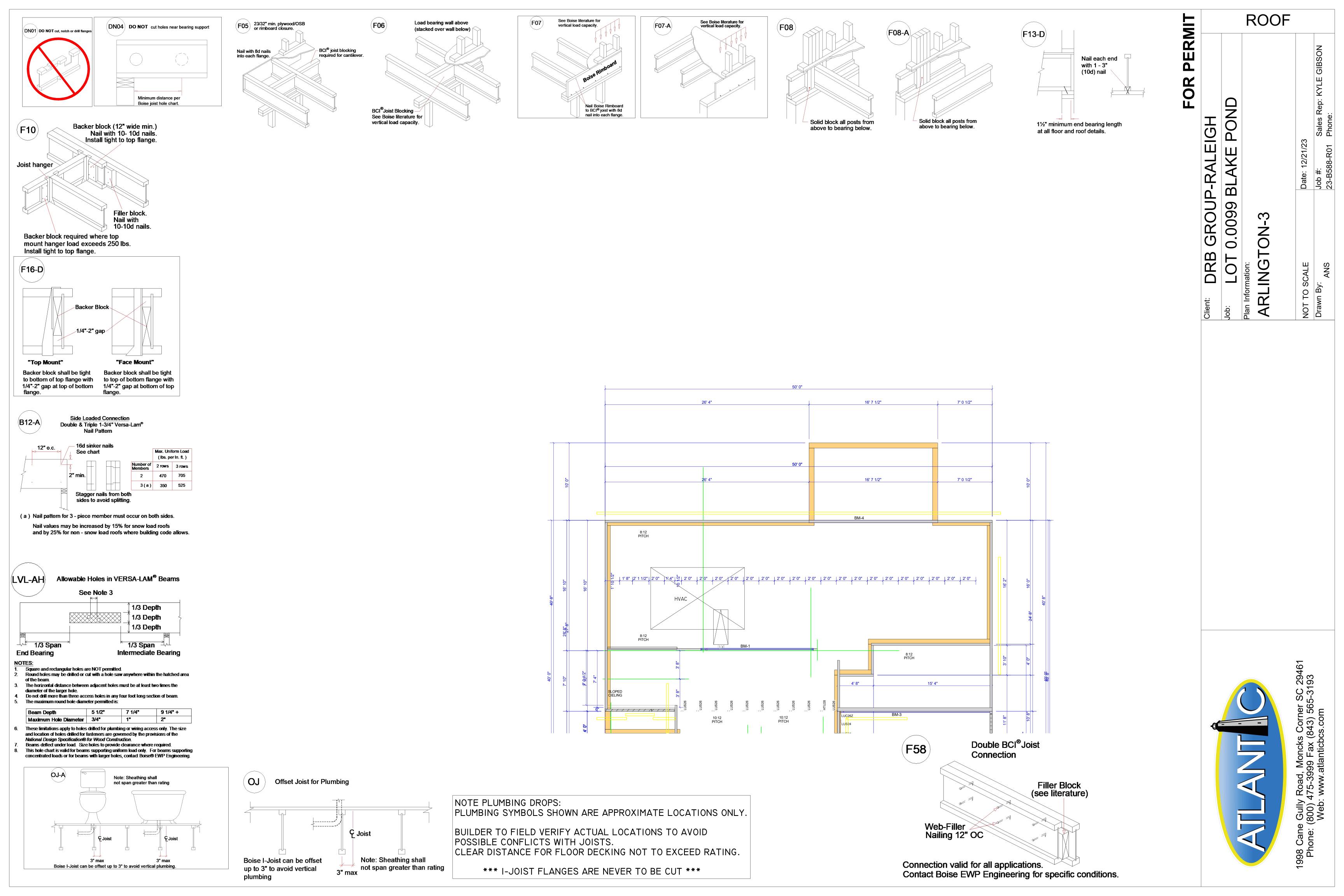


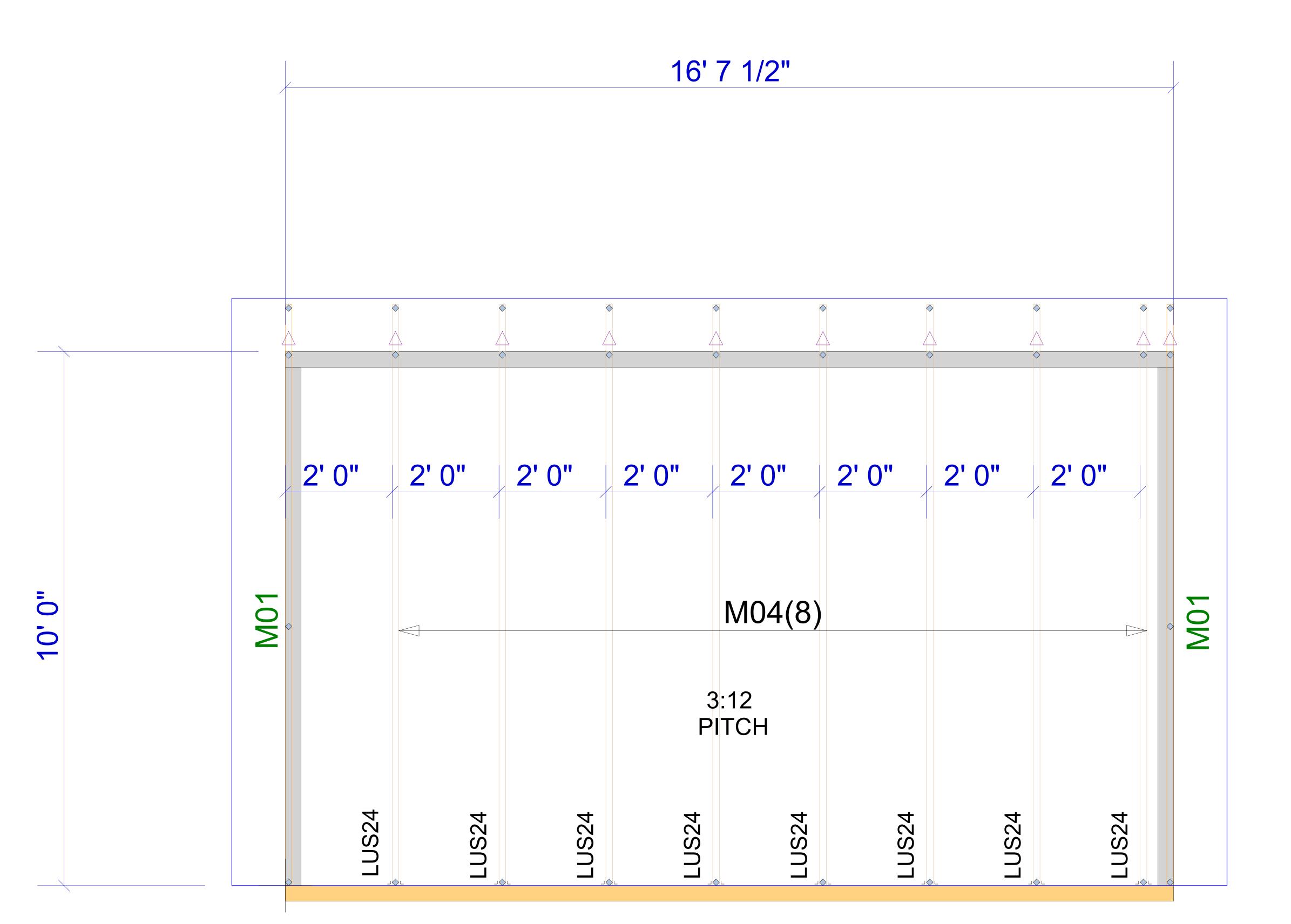
FLOOR

0

NOT Draw

1998 Cane Gully Road, Moncks Corner SC 29461 Phone: (800) 475-3999 Fax (843) 565-3193 Web: www.atlanticbcs.com





Truss Connector Total List				
Manuf Product Qty				
Simpson	LUS24	8		

