GENERAL NOTES

1. NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) SHALL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, ENGINEER, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THOSE SET FORTH IN THE CONTRACT DOCUMENTS. NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE STRUCTURAL ENGINEER OF RECORD OR ANY OF THE STRUCTURAL ENGINEER OF RECORD'S CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.
2. CONTRACT DOCUMENTS INCLUDE, BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS AND SPECIFICATIONS), BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR.
3. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.
4. THE MORE STRINGENT CONDITION WILL GOVERN IN THE EVENT OF A CONFLICT BETWEEN CONTRACT DOCUMENTS AND THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AISC, SJI, SDI, OR OTHER STANDARDS. WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT SHALL GOVERN.
5. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.
6. CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND CIVIL DOCUMENTS. THE ARCHITECT/ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS, SEE THE ARCHITECTURAL DRAWINGS.
7. CONTRACTOR SHALL OBTAIN AND COORDINATE EDGE OF SLAB DIMENSIONS, OPENING LOCATIONS AND DIMENSIONS, DEPRESSED SLAB LOCATIONS AND EXTENTS, SLAB SLOPES, CURB LOCATIONS, AND CMU WALL LOCATIONS. THE ARCHITECT/ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION.
8. CONTRACTOR SHALL VERIFY EXISTING DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK. THE ARCHITECT/ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY.
9. CONTRACTOR SHALL VERIFY THE STRUCTURALLY SUPPORTED MECHANICAL EQUIPMENT WEIGHTS, OPENINGS AND LOCATIONS IDENTIFIED ON THE STRUCTURAL DRAWINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
10. CONTRACTOR SHALL VERIFY THAT MISCELLANEOUS FRAMING SHOWN ON THE STRUCTURAL DRAWINGS FOR MECHANICAL EQUIPMENT, OWNER-FURNISHED ITEMS, PARTITIONS, ETC. IS CONSISTENT WITH THE REQUIREMENTS OF SUCH ITEMS.
11. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, SAFETY, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
12. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR. CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTABILITY ANALYSIS AND ERECTION PROCEDURES, INCLUDING DESIGN AND ERECTION OF FALSEWORK, TEMPORARY BRACING, ETC.
13. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.
14. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED. ELECTRONIC DRAWING FILES WILL NOT BE PROVIDED TO THE CONTRACTOR.
15. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN IN THE CONTRACT DOCUMENTS. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE ARCHITECT/ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE ARCHITECT/ENGINEER. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE ARCHITECT/ ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.
16. WHERE A SECTION OR DETAIL IS SHOWN OR DETAILED FOR ONE CONDITION, IT SHALL APPLY TO ALL SIMILAR AND LIKE CONDITIONS. DETAILS LABELED "TYPICAL" ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR. THE CONTRACTOR SHALL CONSIDER ALL OF THE CONTRACT DOCUMENTS IN DETERMINING SIMILAR AND LIKE CONDITIONS.
17. THE STRUCTURAL ENGINEER OF RECORD IS NOT RESPONSIBLE FOR THE DESIGN OF STEEL STAIRS, HANDRAILS, CURTAIN WALL/WINDOW WALL SYSTEMS, COLD-FORMED METAL FRAMING, OR OTHER SYSTEMS NOT SHOWN IN THE STRUCTURAL DOCUMENTS. SUCH SYSTEMS SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY OTHERS AS REQUIRED BY OTHER PORTIONS OF THE CONTRACT DOCUMENTS.
18. NO STRUCTURAL MEMBER OR COMPONENT SHALL BE CUT, NOTCHED OR OTHERWISE ALTERED UNLESS APPROVED IN WRITING BY THE ENGINEER OF RECORD. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL COSTS INCURRED BY THE ENGINEER OF RECORD FOR REVIEW OF ANY SUCH DEVIATIONS.
19. DO NOT SCALE DRAWINGS.
20. FINISH FLOOR SLAB ELEVATION (FIRST FLOOR) OF 0'-0" IS USED AS A REFERENCE ELEVATION. SEE CIVIL DRAWINGS FOR ACTUAL FINISH FLOOR SLAB ELEVATION.

LEGEND

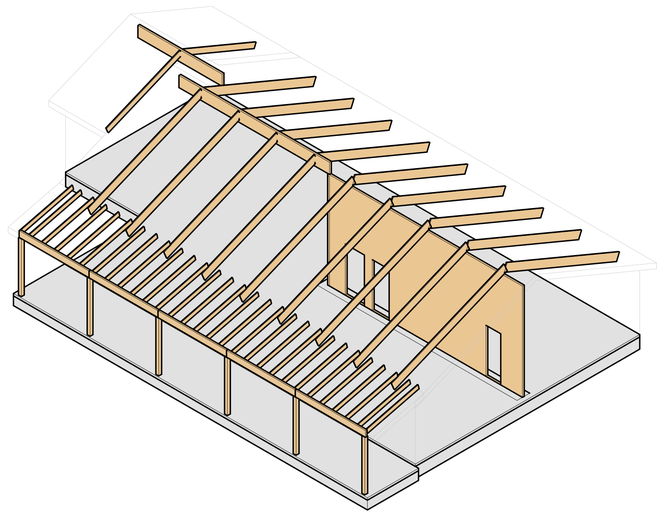
- # — Wall Type (see wall type schedule)
- C# — Column Type (see column schedule)
- WF# — Wall Footing Type (see wall footing schedule)
- F# — Column Footing Type (see column footing schedule)
- P# — Pedestal Type (see pedestal schedule)
- 1 / S# — Building Section
- 1 / S# — Wall Section
- 1 / S# — Detail Number
- 1 / S# — Building Elevation
- 1 / S# — Bracing Elevation
- +2'-0" — Elevation Mark (T.O. Member unless noted)
- A — Column Line
- ℄ — Centerline
- 3 — Revision Tag

ABBREVIATIONS

A.C.I.	AMERICAN CONCRETE INSTITUTE
ARCH.	ARCHITECTURAL
BM.	BEAM
BLDG.	BUILDING
BRG.	BEARING
CONC.	CONCRETE
C.J.	CONTROL JOINT
COL.	COLUMN
DET.	DETAIL
DIA.	DIAMETER
DN.	DOWN
EL/ELEV.	ELEVATION
ELEC.	ELECTRICAL
EQ.	EQUAL
ENGR.	ENGINEER
FIN.	FINISH
FLR.	FLOOR
FTG.	FOOTING
GALV.	GALVANIZED
GEOTECH.	GEOTECHNICAL
JT.	JOINT
MFR.	MANUFACTURER
MECH.	MECHANICAL
MIN.	MINIMUM
N.T.S.	NOT TO SCALE
NO.	NUMBER
O.C.	ON CENTER
OPG.	OPENING
REF.	REFERENCE
SIM.	SIMILAR
S/STL.	STAINLESS STEEL
STRUCT.	STRUCTURAL
SPEC.	SPECIFICATIONS
T.O.	TOP OF (...)
T.O.CONC.	TOP OF CONCRETE
T.O.FTG.	TOP OF FOOTING
T.O.F.	TOP OF FRAMING
T.O.STL.	TOP OF STEEL
T.O.W.	TOP OF WALL
TYP.	TYPICAL
@	AT
+	AND
A.F.F.	ABOVE FINISHED FLOOR
U.N.O.	UNLESS OTHERWISE NOTED
V.I.F.	VERIFY IN FIELD

SHEET LIST

STRUCTURE



nash engineers

Structural Engineer

[615] 880.8461 ___ main
[615] 880.8459 ___ fax

info@nashengineers.com
http://nashengineers.com

PROJECT:
FITZPATRICK RESIDENCE

769 Manor Hills Rd
Lillington, NC 27546

DATE: 11-18-2024
PROJECT NO 24S202

REVISION DATE

NOTES:

NORTH CAROLINA PROFESSIONAL SEAL 045631 ENGINEER VINCE LERAY NASH

11-18-2024

GENERAL NOTES & LEGEND

SCALE: 1/4" = 1'-0"

S001

QUALITY ASSURANCE

NOTES

DEFINITIONS:

A. SPECIAL INSPECTOR:
   a. A QUALIFIED PERSON EMPLOYED OR RETAINED BY AN APPROVED AGENCY AND APPROVED BY THE BUILDING OFFICIAL AS HAVING THE COMPETENCE NECESSARY TO INSPECT A PARTICULAR TYPE OF CONSTRUCTION REQUIRING SPECIAL INSPECTION.

B. SPECIAL INSPECTION:
   a. INSPECTION OF CONSTRUCTION REQUIRING THE EXPERTISE OF AN APPROVED SPECIAL INSPECTOR IN ORDER TO ENSURE COMPLIANCE WITH THE CODE AND APPROVED CONSTRUCTION DOCUMENTS.

C. APPROVED AGENCY:
   a. DETERMINED BY THE BUILDING OFFICIAL. AGENCY NEEDS TO SUBMIT INFO TO SATISFY BUILDING OFFICIAL.
      • INDEPENDENCE: AGENCY MUST BE OBJECTIVE, COMPETENT AND INDEPENDENT FROM CONTRACTOR AND MUST DISCLOSE ANY POSSIBLE CONFLICTS OF INTEREST.

D. INSPECT CONTINUOUSLY:
   a. SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS PRESENT WHEN AND WHERE THE WORK TO BE INSPECTED IS BEING PERFORMED.

E. INSPECT PERIODICALLY:
   a. SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS INTERMITTENLY PRESENT WHERE THE WORK TO BE INSPECTED HAS BEEN OR IS BEING PERFORMED.

SPECIAL INSPECTOR:

1. REVIEW APPROVED PLANS AND SPECIFICATIONS FOR SPECIAL INSPECTION REQUIREMENTS: SPECIAL INSPECTORS WILL COMPLY WITH THE SPECIAL INSPECTION REQUIREMENTS OF THE ENFORCING JURISDICTION.
2. SIGNIFY PRESENCE AT JOBSITE: SPECIAL INSPECTORS SHALL NOTIFY CONTRACTOR PERSONNEL OF THEIR PRESENCE AND RESPONSIBILITIES AT THE JOBSITE. IF REQUIRED BY THE BUILDING OFFICIAL, THEY SHALL SIGN IN ON THE APPROPRIATE FORM POSTED WITH THE BUILDING PERMIT.
3. OBSERVE ASSIGNED WORK: SPECIAL INSPECTORS SHALL INSPECT ALL WORK FOR WHICH THEY ARE RESONSIBLE FOR CONFORMANCE WITH THE BUILDING DEPARTMENT APPROVED (STAMPED) PLANS AND SPECIFICATIONS AND APPLICABLE PROVISIONS OF THE IBC CODE CHAPTER 17.
4. REPORT NONCONFORMING ITEMS: SPECIAL INSPECTORS SHALL BRING ALL NONCONFORMING ITEMS TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR. IF ANY SUCH ITEM IS NOT RESOLVED IN A TIMELY MANNER OR IS ABOUT TO BE INCORPORATED INTO THE WORK, THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE AND THE BUILDING OFFICIAL SHOULD BE NOTIFIED IMMEDIATELY AND THE ITEM NOTED IN THE SPECIAL INSPECTOR'S WRITTEN REPORT (SEE IBC CHAPTER 17).
5. THE SPECIAL INSPECTOR SHALL WRITE A SEPARATE REPORT TO BE POSTED AT THE JOBSITE REGARDING NOTED DISCREPANCIES, WHICH SHOULD CONTAIN, AS A MINIMUM, THE FOLLOWING INFORMATION ABOUT EACH NONCONFORMING ITEM:
   a. DESCRIPTION AND EXACT LOCATION.
   b. REFERENCE TO APPLICABLE DETAIL OF APPROVED PLANS/SPECIFICATIONS.
   c. NAME AND TITLE OF EACH INDIVIDUAL NOTIFIED AND METHOD OF NOTIFICATION.
   d. RESOLUTION OR CORRECTIVE ACTION TAKEN.
6. PROVIDE TIMELY REPORTS: THE SPECIAL INSPECTOR SHALL COMPLETE WRITTEN INSPECTION REPORTS FOR EACH INSPECTION VISIT AND PROVIDE THE REPORTS ON A TIMELY BASIS AS DETERMINED BY THE BUILDING OFFICIAL, REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE AND OTHERS. THESE REPORTS SHOULD BE ORGANIZED ON A DAILY FORMAT AND MAY BE SUBMITTED WEEKLY AT THE OPTION OF THE BUILDING OFFICIAL. IN THESE REPORTS, SPECIAL INSPECTORS SHOULD:
   1. DESCRIBE INSPECTIONS AND TESTS MADE WITH APPLICABLE LOCATIONS
   2. INDICATE HOW NONCONFORMING ITEMS WERE RESOLVED.
   3. LIST UNRESOLVED ITEMS, PARTIES NOTIFIED, AND TIME AND METHOD OF NOTIFICATION.
   4. ITEMIZE CHANGES AUTHORIZED BY REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE IF NOT INCLUDED IN NONCONFORMING ITEMS.
7. SUBMIT FINAL REPORT: SPECIAL INSPECTORS OR INSPECTION AGENCIES SHALL SUBMIT A FINAL SIGNED REPORT TO THE BUILDING DEPARTMENT STATING THAT ALL ITEMS REQUIRING SPECIAL INSPECTION AND TESTING WERE FULFILLED AND REPORTED AND, TO THE BEST OF THEIR KNOWLEDGE IN CONFORMANCE WITH
   THE APPROVED PLANS, SPECIFICATIONS AND THE APPLICABLE PROVISIONS OF THE IBC. ITEMS NOT IN CONFORMANCE, UNRESOLVED ITEMS OR ANY DISCREPANCIES IN INSPECTION COVERAGE SHOULD BE SPECIFICALLY ITEMIZED IN THIS REPORT.

CONTRACTOR:

1. NOTIFY THE SPECIAL INSPECTOR.
2. PROVIDE ACCESS TO APPROVED PLANS.
3. RETAIN SPECIAL INSPECTION RECORDS..
4. WHEN RESPONSIBLE FOR CONSTRUCTION OF MAIN WIND FORCE RESISTING SYSTEM OR SEISMIC LATERAL FORCE RESISTING SYSTEM:
   A. MUST WRITE A LETTER TO BUILDING OFFICIAL & OWNER PRIOR TO COMMENCEMENT OF WORK STATING THAT THEY ARE AWARE OF SPECIAL INSPECTION REQUIREMENTS.

BUILDING OFFICIAL:

1. REVIEW SUBMITTAL DOCUMENTS FOR COMPLIANCE WITH SPECIAL INSPECTION REQUIREMENTS.
2. APPROVE SPECIAL INSPECTION PROGRAM.
3. APPROVE SPECIAL INSPECTORS/INSPECTION AGENCIES.
4. MONITOR SPECIAL INSPECTION ACTIVITIES.
5. REVIEW INSPECTION REPORTS.

INSPECTION FOR SOIL

INSPECT CONTINUOUSLY	INSPECT PERIODICALLY	TASK
	X	1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.
	X	2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.
	X	3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.
X		4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.
	X	5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.

INSPECTION FOR MASONRY CONSTRUCTION NOT USED

INSPECT CONTINUOUSLY	INSPECT PERIODICALLY	TASK
-	-	1. MASONRY CONSTRUCTION SHALL BE INSPECTED AND VERIFIED IN ACCORDANCE WITH TMS 402/ACI 530/ASCE 5 AND TMS 602/ACI 530.1/ASCE 6.

INSPECTION FOR STRUCTURAL WOOD CONSTRUCTION

INSPECT CONTINUOUSLY	INSPECT PERIODICALLY	TASK
X		1. FIELD GLUING OPERATIONS OF ELEMENTS OF THE MAIN WIND FORCE RESISTING SYSTEM.
	X	2. NAILING, BOLTING, ANCHORING, AND OTHER FASTENING COMPONENTS WITHIN THE MAIN WIND FORCE RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES AND HOLD-DOWNS, EXCEPT WHERE THE FASTENER SPACING OF THE SHEATHING IS MORE THAN 4 INCHES ON CENTER.

INSPECTION FOR WOOD CONSTRUCTION

INSPECT CONTINUOUSLY	INSPECT PERIODICALLY	TASK
-	-	1. UNLESS WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION, SPECIAL INSPECTIONS OF THE FABRICATION PROCESS OF PREFABRICATED WOOD STRUCTURAL ELEMENTS AND ASSEMBLIES SHALL BE VERIFIED BY THE SPECIAL INSPECTOR THAT THE FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS FOR INSPECTION CONTROL OF THE WORKMANSHIP AND THE FABRICATOR'S ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS. THE SPECIAL INSPECTOR SHALL REVIEW THE PROCEDURES FOR COMPLETENESS AND ADEQUACY RELATIVE TO THE CODE REQUIREMENTS FOR THE FABRICATOR'S SCOPE OF WORK.

INSPECTION FOR STEEL CONSTRUCTION NOT USED

INSPECT CONTINUOUSLY	INSPECT PERIODICALLY	TASK
-	-	1. SPECIAL INSPECTION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360.

INSPECTION FOR CONCRETE CONSTRUCTION

INSPECT CONTINUOUSLY	INSPECT PERIODICALLY	TASK
	X	1. INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.
	X	2. INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH IBC TABLE 1705.2.2, ITEM 2b.
	X	3. INSPECTION OF ANCHORS CAST IN CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED.
	X	4. INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.
	X	5. VERIFYING USE OF REQUIRED DESIGN MIX.
X		6. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.
X		7. INPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.
X		8. INSPECTION OF MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.
	X	9. INSPECTION OF PRESTRESSED CONCRETE: A. APPLICATION PRESTRESSING FORCES. B. GROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC FORCE-RESISTING SYSTEM.
	X	10. ERECTION OF PRECAST CONCRETE MEMBERS.
	X	11. VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.
	X	12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED. A. EXCEPTIONS: SPECIAL INSPECTIONS SHALL NOT BE REQUIRED FOR: ISOLATED SPREAD CONCRETE FOOTINGS OF BUILDINGS THREE STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK. B. CONTINUOUS CONCRETE FOOTINGS SUPPORTING WALLS OF BUILDINGS THREE STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK WHERE: a. THE FOOTINGS SUPPORT WALLS OF LIGHT-FRAME CONSTRUCTION b. THE FOOTINGS ARE DESIGNED IN ACCORDANCE WITH TABLE 1809.7; OR c. THE STRUCTURAL DESIGN OF THE FOOTING IS BASED ON A SPECIFIED COMPRESSIVE STRENGTH, f'c, NO GREATER THAN 2,500 PSI, REGARDLESS OF THE COMPRESSIVE STRENGTH SPECIFIED IN THE CONSTRUCTION DOCUMENTS OR USED IN THE FOOTING CONSTRUCTION. C. NONSTRUCTURAL CONCRETE SLABS SUPPORTED DIRECTLY ON THE GROUND, INCLUDING PRESTRESSED SLABS ON GRADE, WHERE THE EFFECTIVE PRESTRESSED IN THE CONCRETE IS LESS THAN 150 PSI. D. CONCRETE FOUNDATION WALLS CONSTRUCTED IN ACCORDANCE WITH TABLE 1807.1.6.2. E. CONCRETE PATIOS, DRIVEWAYS AND SIDEWALKS, ON GRADE.

INSPECTION FOR CONCRETE CONSTRUCTION

INSPECT CONTINUOUSLY	INSPECT PERIODICALLY	TASK
	X	1. MATERIAL VERIFICATION OF COLD-FORMED STEEL DECK: A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS. B. MANUFACTURER'S CERTIFIED TEST REPORTS
	X	2. INSPECTION OF WELDING: A. COLD-FORMED STEEL DECK: B. FLOOR AND ROOF DECK WELDS: C. REINFORCING STEEL: D. VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706.
X		3. REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS OF CONCRETE AND SHEAR REINFORCEMENT.
X		4. SHEAR REINFORCEMENT.
	X	5. OTHER REINFORCING STEEL.

nash engineers
Structural Engineer
[615] 880.8461 ___ main
[615] 880.8459 ___ fax
info@nashengineers.com
http://nashengineers.com

PROJECT:
FITZPATRICK RESIDENCE

769 Manor Hills Rd
Lillington, NC 27546

DATE: 11-18-2024
PROJECT NO 24S202

REVISION DATE

NOTES:

NORTH CAROLINA PROFESSIONAL SEAL 045631 ENGINEER LANCE LERAY NASH
11-18-2024

QUALITY ASSURANCE

SCALE: 1/4" = 1'-0"

S002

SPECIFICATIONS

DESIGN LOADS

A.	THIS STRUCTURE IS DESIGNED TO MEET OR EXCEED THE REQUIREMENTS OF:		
	A. NORTH CAROLINA RESIDENTIAL CODE	2018	
	B. MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES	ASCE 7-16	
	C. NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION	NDS	
	D. AMERICAN CONCRETE INSTITUTE	ACI 318	
B.	SNOW LOADS		
	A. GROUND SNOW LOAD (Pg)	10 PSF	
	B. FLAT-ROOF SNOW LOAD (Pf)	10 PSF	
	C. SNOW EXPOSURE FACTOR (Ce)	1.0	
	D. SNOW LOAD IMPORTANCE FACTOR (Is)	1.0	
	E. THERMAL FACTOR (Ct)	1.0	
C.	WIND LOADS		
	A. ULTIMATE DESIGN WIND SPEED (Vult)	150 MPH	
	B. RISK CATEGORY	II	
	C. EXPOSURE CATEGORY	B	
	D. INTERNAL PRESSURE COEFFICIENT (Gcpi)	±0.18	
D.	EARTHQUAKE LOADS		
	A. MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD (Ss)	0.1341	
	B. MAPPED SPECTRAL RESPONSE ACCELERATION AT 1 SEC PERIOD (S1)	0.0657	
	C. DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD (Sds)	0.1430	
	D. DESIGN SPECTRAL RESPONSE ACCELERATION AT 1 SEC PERIOD (Sd1)	0.1050	
	E. SOIL SITE CLASS	D	
	F. IMPORTANCE FACTOR (Ie)	1.0	
	G. SEISMIC DESIGN CATEGORY	B	
	H. SEISMIC FORCE RESISTING SYSTEM	SHEATHED WOOD PANELS	
	I. RESPONSE MODIFICATION COEFFICIENT (R)	6.5	
	J. SYSTEM OVERSTRENGTH FACTOR (O)	3.0	
	K. DEFLECTION AMPLIFICATION FACTOR (Cd)	4.0	
	L. SEISMIC RESPONSE COEFFICIENT (Cs)	0.0220	
	M. ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE	

E. ESTIMATED DEFLECTIONS (IN INCHES) ARE AS FOLLOWS

	L or Lr	S or W	D+L
A. ROOF MEMBERS			
a. SUPPORTS PLASTER OR STUCCO FINISH	L/360	L/360	L/240
b. SUPPORTS NONPLASTER CEILING	L/240	L/240	L/180
c. NOT SUPPORTING CEILING	L/180	L/180	L/120
B. FLOOR MEMBERS	L/360		L/240
C. EXTERIOR WALLS			
a. PLASTER OR STUCCO FINISH		L/360	
b. BRITTLE FINISH		L/240	
c. FLEXIBLE FINISH		L/120	
D. INTERIOR PARTITIONS			
a. PLASTER OR STUCCO FINISH	L/360		
b. BRITTLE FINISH	L/240		
c. FLEXIBLE FINISH	L/120		
E. FARM BUILDINGS			L/180
F. GREENHOUSES			L/120

WOOD

1.	INTERIOR AND EXTERIOR LOADBEARING WALLS	SOUTHERN PINE NO. 2
2.	LINTELS, FLOOR JOISTS, AND BEAMS	SOUTHERN PINE NO. 2
3.	WOOD EXPOSED TO WEATHER OR IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE-TREATED. USE HOT-DIPPED GALVANIZED OR STAINLESS STEEL CONNECTORS AND NAILS IN ALL PRESSURE-TREATED WOOD	
4.	STRUCTURAL WALL AND ROOF PANELS	APA RATED
5.	ALL WOOD SHALL HAVE A MOISTURE CONTENT < 19%	

SHOP DRAWING REVIEW

1. SHOP DRAWINGS SHALL ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN ON THE CONTRACT DOCUMENTS. SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC. REVIEW OF SUBMITTALS AND SHOP DRAWINGS DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF THE SHOP DRAWINGS.
2. SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR AND MARKED APPROVED PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. NON-CONFORMING DRAWING SUBMITTALS WILL BE RETURNED WITHOUT REVIEW.
3. THE CONTRACTOR IS RESPONSIBLE FOR ALL NECESSARY COPYING AND DISTRIBUTION TO REQUIRED SUB-CONTRACTORS AND SUPPLIERS. SHOP DRAWING SUBMITTALS MAY BE MADE ELECTRONICALLY VIA PDF. REVIEW AND COMMENT WILL BE MADE VIA PDF.
4. THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN WRITING BY THE ENGINEER OF RECORD.
5. CHANGES AND ADDITIONS MADE ON RE-SUBMITTLAS SHALL BE CLEARLY FLAGGED AND NOTED. THE PURPOSE OF RE-SUBMITTALS SHALL BE CLEARLY NOTED ON THE LETTER OF TRANSMITTAL. THE ARCHITECT/ENGINEER OF RECORD REVIEW WILL BE LIMITED TO THOSE ITEMS CAUSING THE RE-SUBMITTAL. CONTRACTOR IS RESPONSIBLE FOR COSTS INCURRED BY MULTIPLE RE-SUBMITTALS AT ARCHITECT/ENGINEER'S CURRENT HOURLY RATE.

FOUNDATIONS

1.	THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD WHICH VARIES FROM THOSE CONDITIONS ASSUMED FOR DESIGN BASED ON THE GEOTECHNICAL REPORT.	FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS IN THE IRC.
	A. SPREAD FOOTINGS ALLOWABLE BEARING CAPACITY	2,000 PSF
	B. STRIP FOOTINGS ALLOWABLE BEARING CAPACITY	2,000 PSF

nash engineers

Structural Engineer

[615] 880.8461 ___ main
[615] 880.8459 ___ fax

info@nashengineers.com
http://nashengineers.com

PROJECT:
FITZPATRICK RESIDENCE

769 Manor Hills Rd
Lillington, NC 27546

DATE: 11-18-2024
PROJECT NO 24S202

REVISION	DATE

NOTES:

NORTH CAROLINA PROFESSIONAL SEAL 045631 ENGINEER VINCE LERAY NASH

11-18-2024

SPECIFICATIONS

SCALE: 1/4" = 1'-0"

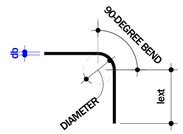
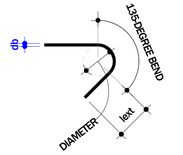
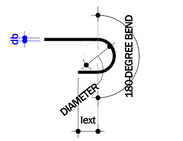
S003

REBAR & CONCRETE DETAILING

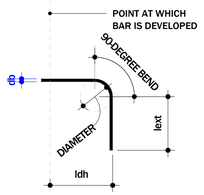
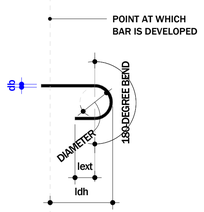
CONCRETE CLASSIFICATION

CONCRETE USAGE	MINIMUM CONCRETE COMPRESSIVE STRENGTH (f'c), PSI		CONCRETE TYPE	MAXIMUM W/C RATIO
	28 DAYS	56 DAYS		
DEEP FOUNDATIONS				
CONCRETE FILLED STEEL SHELL PILES				
SHALLOW FOUNDATIONS				
GRADE BEAMS				
PILE CAPS				
PILASTERS				
SPREAD FOOTINGS	3,000		NW	0.45
FOUNDATION WALLS				
BASEMENT WALLS				
RETAINING WALLS				
ALL OTHER FOUNDATION WALLS	4,000		NW	0.45
SLABS-ON-GRADE				
LOADING DOCK AND ICE SHEET				
INTERIOR	3,000		NW	0.45
EXTERIOR				
FLOOR/ROOF FRAMING				
PRECAST SEATING UNITS				
EXTERIOR PRECAST SOLID SLABS				
INTERIOR STEEL DECK SLABS				
EXTERIOR STEEL DECK SLABS				
INTERIOR TOPPING SLABS				
EXTERIOR TOPPING SLABS				
WALLS				
INTERIOR PRECAST WALLS				
EXTERIOR PRECAST WALLS				

MINIMUM INSIDE BEND DIAMETERS AND HOOK GEOMETRY

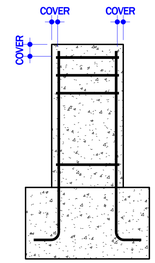
STIRRUPS, TIES, AND HOOPS				
	BAR SIZE	MINIMUM INSIDE BEND DIAMETER, in.	STRAIGHT EXTENSION, lext, in.	TYPE OF STANDARD HOOKS
90-DEGREE HOOK	#3-#5	4db	GREATER OF 6db AND 3 in.	
	#6-#8	6db	12db	
135-DEGREE HOOK	#3-#5	4db	GREATER OF 6db AND 3 in.	
	#6-#8	6db		
180-DEGREE HOOK	#3-#5	4db	GREATER OF 4db AND 2.5 in.	
	#6-#8	6db		

DEVELOPMENT LENGTH OF DEFORMED BARS IN TENSION, ldh

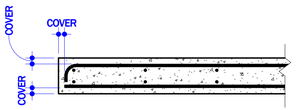
TYPE OF STANDARD HOOKS	BAR SIZE	MINIMUM INSIDE BEND DIAMETER, in.	DEVELOPMENT LENGTH, ldh, in.	STRAIGHT EXTENSION, lext, in.	TYPE OF STANDARD HOOKS
90-DEGREE HOOK	#3-#8	6db	GREATER OF 19db 8db OR 6 in.	12db	
	#9-#11	8db			
	#14-#18	10db			
180-DEGREE HOOK	#3-#8	6db	GREATER OF 19db 8db OR 6 in.	GREATER OF 4db AND 2.5 in.	
	#9-#11	8db			
	#14-#18	10db			

SPECIFIED CONCRETE COVER FOR CAST-IN-PLACE NONPRESTRESSED CONCRETE MEMBERS

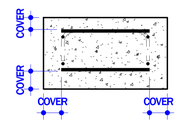
CONCRETE EXPOSURE	MEMBER	REINFORCEMENT	COVER, in.
CAST AGAINST AND PERMANENTLY IN CONTACT WITH GROUND	ALL	ALL	3
EXPOSED TO WEATHER OR IN CONTACT WITH GROUND	ALL	#6 - #18	2
		#5, W31 OR D31 WIRE, AND SMALLER	1-1/2
NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND	SLABS, JOISTS, AND WALLS	#14 AND #18	1-1/2
		#11 AND SMALLER	3/4
	BEAMS, COLUMNS, PEDESTALS, AND TENSION TIES	PRIMARY REINFORCEMENT, STIRRUPS, TIES, SPIRALS, AND HOOPS	1-1/2



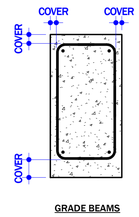
PEDESTALS



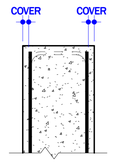
SLAB-ON-GRADE



SPREAD FOOTINGS AND MATS



GRADE BEAMS



FOUNDATION WALLS

ALL CONCRETE PROFILES AND REINFORCING STEEL SHOWN IS FOR THE TYPICAL CONCRETE COVERS ONLY. REFER TO DETAILS FOR ADDITIONAL INFORMATION. ALL COVERS SHOWN ARE CLEAR FROM THE OUTERMOST SURFACE OF THE TRANSVERSE AND LONGITUDINAL REINFORCING STEEL TO THE CLOSEST OUTER SURFACE OF THE CONCRETE, INCLUDING REVEALS, DRIP GROOVES, OR RUSTICATIONS.

nash engineers

Structural Engineer

[615] 880.8461 ___ main
[615] 880.8459 ___ fax

info@nashengineers.com
http://nashengineers.com

PROJECT:
FITZPATRICK RESIDENCE

769 Manor Hills Rd
Lillington, NC 27546

DATE: 11-18-2024
PROJECT NO 24S202

REVISION DATE

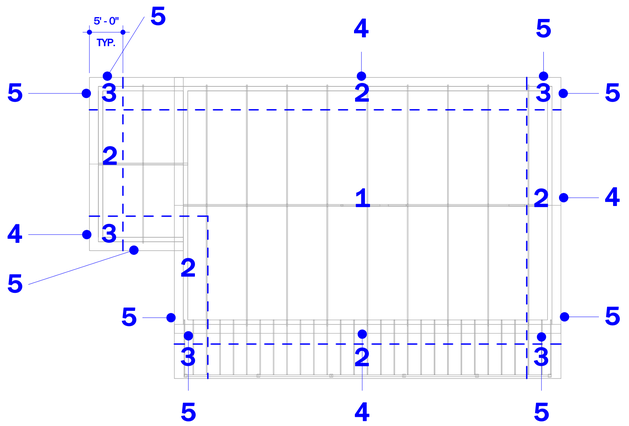
NOTES:

11-18-2024

REBAR & CONCRETE DETAILING

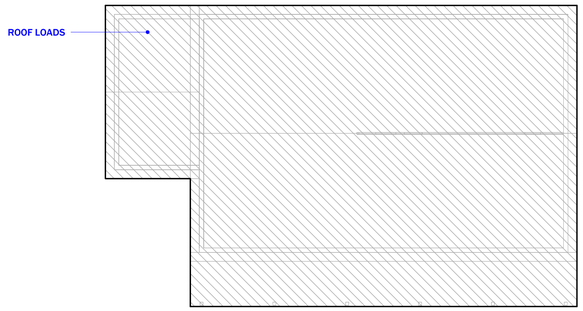
SCALE: As indicated

S004



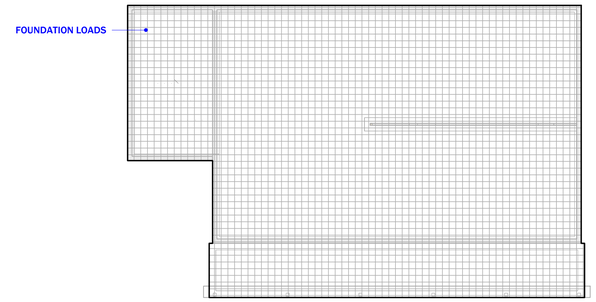
3 ROOF WIND LOADING PLAN

SCALE: 3/32" = 1'-0"



2 ROOF LOADING PLAN

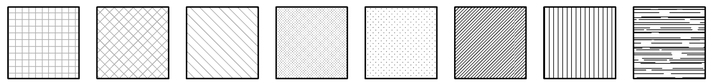
SCALE: 3/32" = 1'-0"



1 FOUNDATION LOADING PLAN

SCALE: 3/32" = 1'-0"

LOAD KEY



FOUNDATION LOADS	FLOOR LOADS	ROOF LOADS	SNOW DRIFT LOAD	SNOW BALANCED LOAD	STORAGE LOAD	MECHANICAL LOAD	SITE LOAD
DL 50 PSF LL 40 PSF	NA	DL 20 PSF LL 20 PSF	NA	SL 10 PSF	NA	NA	NA

LOAD KEY NOTES:
1. FOUNDATION LOADS DL INCLUDE SELF WEIGHT AND SUPERIMPOSED LOADS.
2. FOUNDATION LOADS LL (REFERENCE IBC CHAPTER 16).
3. FLOOR LOADS DL INCLUDE SELF WEIGHT (MEMBERS+ROOFING+MEP+CEILING).
4. FLOOR LOADS LL (REFERENCE IBC CHAPTER 16).
5. ROOF LOADS DL INCLUDE SELF WEIGHT (MEMBERS+ROOFING+MEP+CEILING)
6. ROOF LOADS LL (REFERENCE IBC CHAPTER 16).
5. SNOW DRIFT LOAD SL (REFERENCE ASCE 7 CHAPTER 7).
6. SNOW BALANCED LOAD SL (REFERENCE ASCE 7 CHAPTER 7).
7. STORAGE LOAD LL (REFERENCE IBC CHAPTER 16).
8. MECHANICAL LOAD DL INCLUDE SELF WEIGHT. SEE MECHANICAL FOR EXACT LOCATIONS.
9. SITE LOADING LL SEE PLAN. SEE CIVIL AND LANDSCAPING PLAN FOR FINAL GRADE AND ADDITIONAL LOADING CRITERIA.
10. SEE GENERAL CONTRACTOR FOR ANY TEMPORARY CONSTRUCTION LOADS GREATER THAN THOSE LISTED IN THE LOAD KEY PLAN.
11. SNOW DRIFT IN PSF AS INDICATED ON PLAN IS THE PEAK OF THE TRIANGULAR DISTRIBUTION LOAD.
12. SEE CIVIL AND LANDSCAPING PLAN FOR FINAL GRADE AND ADDITIONAL LOADING CRITERIA.
13. SEE GENERAL CONTRACTOR FOR ANY TEMPORARY CONSTRUCTION LOADS GREATER THAN THOSE LISTED IN THE LOAD KEY.
14. NA NOT APPLICABLE

COMPONENT AND CLADDING ROOF WIND PRESSURES

ROOF ZONE	EFFECTIVE WIND AREA (SF)	POSITIVE (PSF)	NEGATIVE (PSF)
1	10	16.0	-48.0
	20	16.0	-43.0
	50	16.0	-39.0
	100	16.0	-36.0
2	10	16.0	-61.0
	20	16.0	-57.0
	50	16.0	-52.0
	100	16.0	-48.0
3	10	16.0	-83.0
	20	16.0	-75.0
	50	16.0	-65.0
	100	16.0	-57.0

NOTES
A. WIND PRESSURES ACT NORMAL TO THE SURFACE. POSITIVE PRESSURES ACT TOWARDS THE SURFACEAND NEGETIVE PRESSURES ACT AWAY FROM THE SURFACE.
B. THE EFFECTIVE WIND AREA IS THE SPAN LENGTH MULTIPLIED BY AN EFFECTIVE WIDTH THAT NEED NOT BE LESS THAN ONE-THIRD THE SPAN LENGTH. FOR CLADDING FASTENERS, THE EFFECTIVE WIND AREA SHALL NOT BE GREATER THAN THE AREA THAT IS TRIBUTARY TO AN INDIVIDUAL FASTENER.

COMPONENT AND CLADDING WALL WIND PRESSURES

WALL ZONE	EFFECTIVE WIND AREA (SF)	POSITIVE (PSF)	NEGATIVE (PSF)
4	10	26.7	-33.0
	20	25.5	-32.0
	50	23.9	-30.0
	100	22.7	-28.0
5	10	26.7	-33.0
	20	25.5	-32.0
	50	23.9	-30.0
	100	22.7	-28.0

NOTES
A. WIND PRESSURES ACT NORMAL TO THE SURFACE. POSITIVE PRESSURES ACT TOWARDS THE SURFACEAND NEGETIVE PRESSURES ACT AWAY FROM THE SURFACE.
B. THE EFFECTIVE WIND AREA IS THE SPAN LENGTH MULTIPLIED BY AN EFFECTIVE WIDTH THAT NEED NOT BE LESS THAN ONE-THIRD THE SPAN LENGTH. FOR CLADDING FASTENERS, THE EFFECTIVE WIND AREA SHALL NOT BE GREATER THAN THE AREA THAT IS TRIBUTARY TO AN INDIVIDUAL FASTENER.
C. WIDTH OF PRESSURE COEFFICIENT ZONE: 2a = 10'-0"

nash engineers

Structural Engineer

[615] 880.8461 ___ main
[615] 880.8459 ___ fax

info@nashengineers.com
http://nashengineers.com

PROJECT:
FITZPATRICK RESIDENCE

769 Manor Hills Rd
Lillington, NC 27546

DATE: 11-18-2024
PROJECT NO 24S202

REVISION DATE

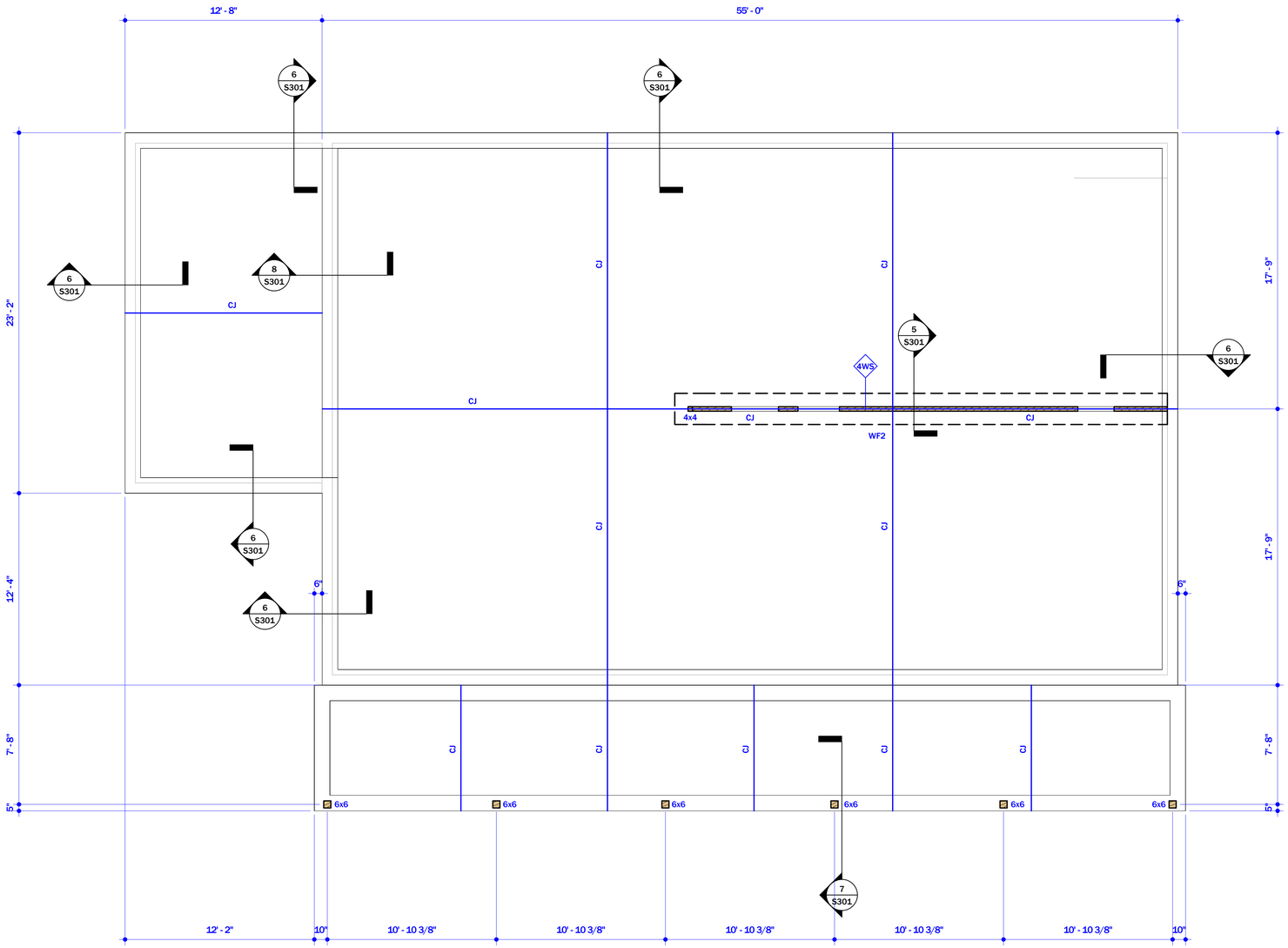
NOTES:

11-18-2024

LOADING PLANS

SCALE: As indicated

S005



WOOD STUD WALL SCHEDULE

MARK	SIZE (WIDTH x DEPTH)	SPACING
4WS	2 x 4	16" O.C.

NOTES
A. SEE 2/S401 FOR WALL CONSTRUCTION

WALL FOOTING SCHEDULE

MARK	SIZE (WIDTH x THICKNESS)	TRANSVERSE REBAR	LONGITUDINAL REBAR
WF2	2'-0" x 1'-0"		(2) #5

GENERAL NOTES

It is the intent that all work shown is constructed as shown on plan. If field conditions arise that make such work impossible, consult the Structural Engineer for guidance on final construction. If additional work is required to accomodate this layout, the Contractor shall consult the Owner before the work is started.

A. FIRST FLOOR ELEVATION = 0'-0" (0'-0")
B. FOUNDATION FLOOR 4" CONCRETE SLAB ON GRADE WITH W.W.F. 6x6-W1.4xW1.4 ON VAPOR BARRIER ON SPECIFIED FILL.
C. MAIN ROOF DECK SIP (STRUCTURAL INSULATED PANELS) BY OTHERS
D. PORCH ROOF DECK 5/8" PERFORMANCE CATEGORY APA STRUCTURAL 1 SHEATHING, 24" O.C., EXPOSURE 1. (SEE 1/S401)
E. OPENING CONSTRUCTION (SEE 8/S401)
F. ALL INTERIOR WALLS AND DOORS NOT SHOWN. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.

SYMBOLS

CONTROL JOINT (SEE 1/S301) — CJ

SIP OR SHEATHING SPAN (ONE-WAY)

1 FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

nash engineers

Structural Engineer

[615] 880.8461 ___ main
[615] 880.8459 ___ fax

info@nashengineers.com
http://nashengineers.com

PROJECT:
FITZPATRICK RESIDENCE

769 Manor Hills Rd
Lillington, NC 27546

DATE: 11-18-2024
PROJECT NO 24S202

REVISION	DATE

NOTES:

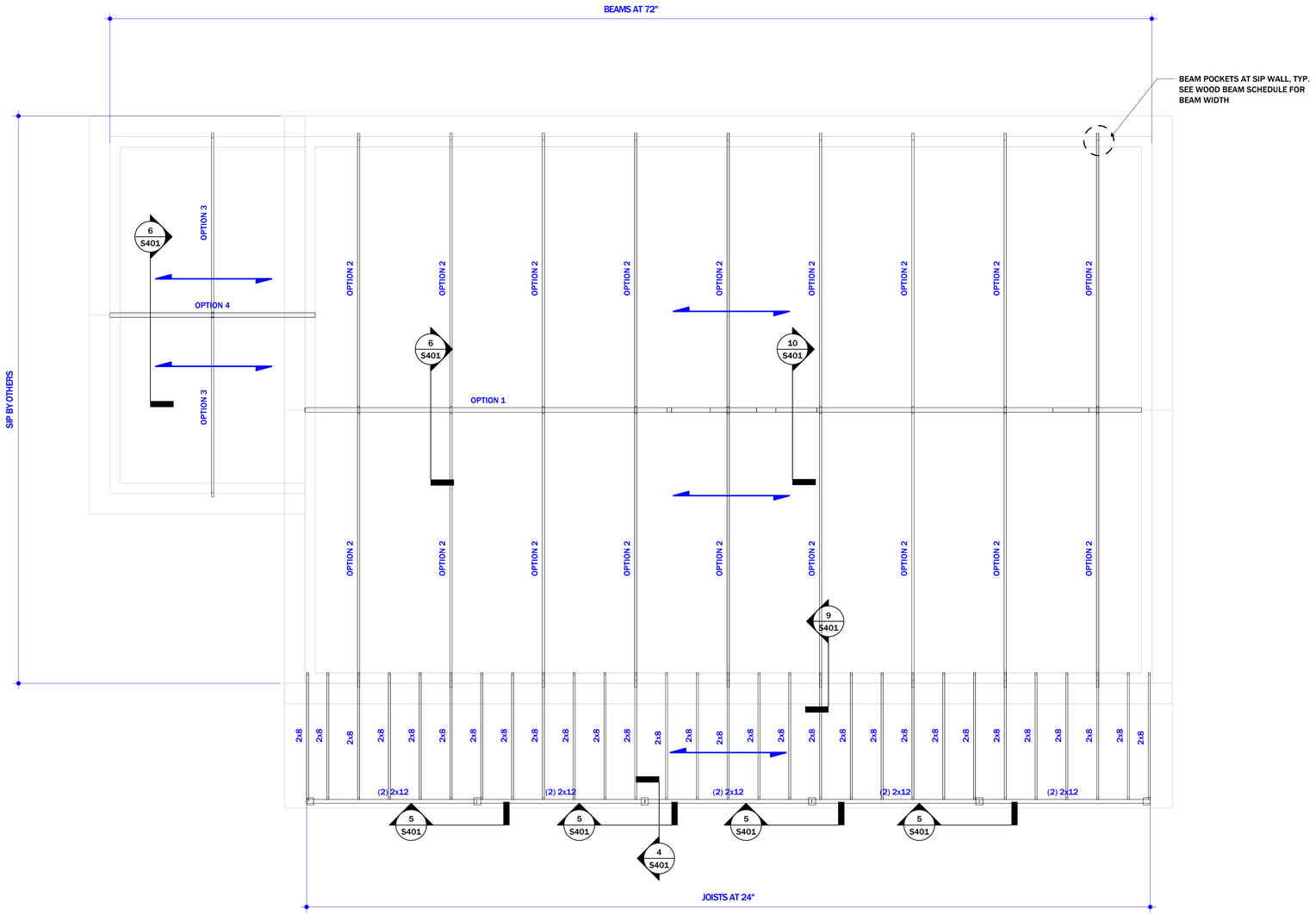
NORTH CAROLINA PROFESSIONAL SEAL 045631 ENGINEER VINCE LERAY NASH

11-18-2024

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

S101



1 ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

WOOD BEAM SCHEDULE

MARK	LVL (WIDTH x DEPTH)	GLULAM (WIDTH x DEPTH)
OPTION 1	(2) 1-3/4" x 20"	3-1/2" x 24"
OPTION 2	1-3/4" x 14"	2-1/2" x 13-3/4"
OPTION 3	1-3/4" x 10"	2-1/2" x 9-5/8"
OPTION 4	(2) 1-3/4" x 14"	2-1/2" x 13-3/4"

NOTES
A. SEE 3/S401 FOR BEAM CONSTRUCTION
B. BEAMS OPTIONS SHOWN ARE MINIMUM SIZES.

GENERAL NOTES

It is the intent that all work shown is constructed as shown on plan. If field conditions arise that make such work impossible, consult the Structural Engineer for guidance on final construction. If additional work is required to accomodate this layout, the Contractor shall consult the Owner before the work is started.

A. FIRST FLOOR ELEVATION = 0'-0" (0'-0")
B. FOUNDATION FLOOR 4" CONCRETE SLAB ON GRADE WITH W.W.F. 6x6-W1.4xW1.4 ON VAPOR BARRIER ON SPECIFIED FILL.
C. MAIN ROOF DECK SIP (STRUCTURAL INSULATED PANELS) BY OTHERS
D. PORCH ROOF DECK 5/8" PERFORMANCE CATEGORY APA STRUCTURAL 1 SHEATHING, 24" O.C., EXPOSURE 1. (SEE 1/S401)
E. OPENING CONSTRUCTION (SEE 8/S401)
F. ALL INTERIOR WALLS AND DOORS NOT SHOWN. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.

SYMBOLS

CONTROL JOINT (SEE 1/S301) — CJ

SIP OR SHEATHING SPAN (ONE-WAY)

nash engineers

Structural Engineer

[615] 880.8461 ___ main
[615] 880.8459 ___ fax

info@nashengineers.com
http://nashengineers.com

PROJECT:
FITZPATRICK RESIDENCE

769 Manor Hills Rd
Lillington, NC 27546

DATE: 11-18-2024
PROJECT NO 24S202

REVISION DATE

NOTES:

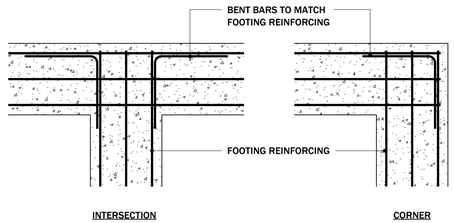
NORTH CAROLINA PROFESSIONAL SEAL 045631 ENGINEER LANCE LEROY NASH

11-18-2024

ROOF FRAMING PLAN

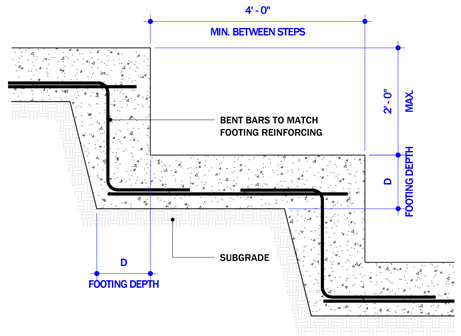
SCALE: 1/4" = 1'-0"

S102



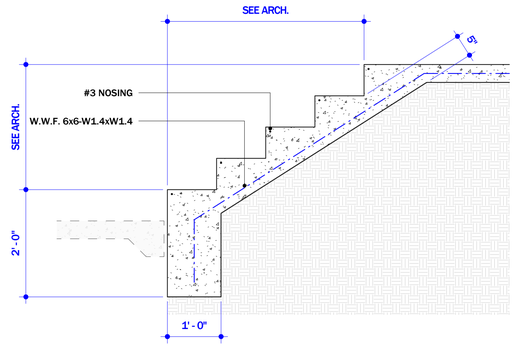
4 TYPICAL FOOTING PLAN

SCALE: 1/2" = 1'-0"



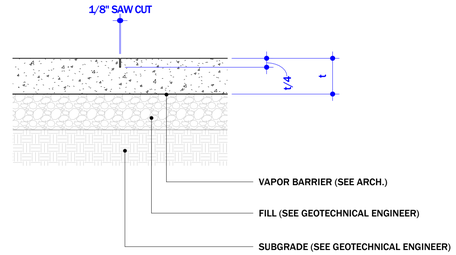
3 TYPICAL STEPPED FOOTING

SCALE: 3/4" = 1'-0"



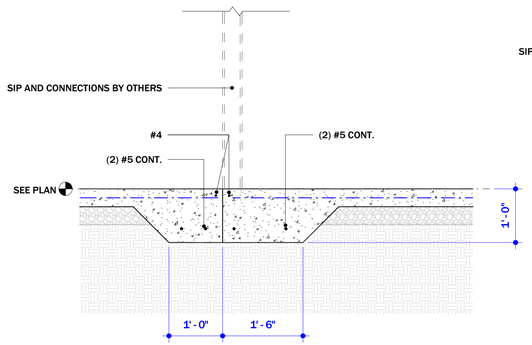
2 TYPICAL STAIR ON GRADE

SCALE: 3/4" = 1'-0"



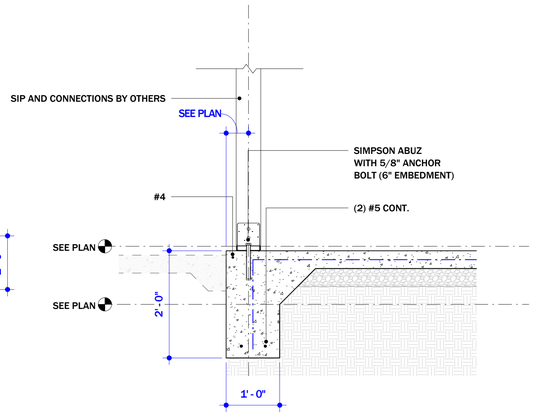
1 TYPICAL CONTROL JOINT

SCALE: 1 1/2" = 1'-0"



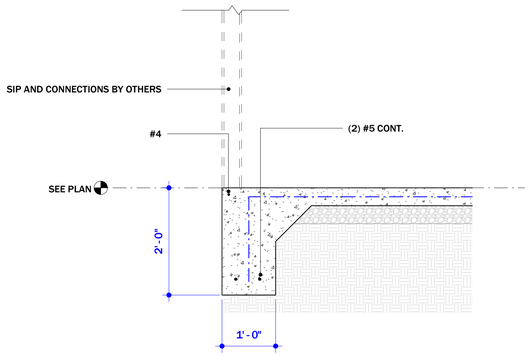
8 AT INTERIOR

SCALE: 3/4" = 1'-0"



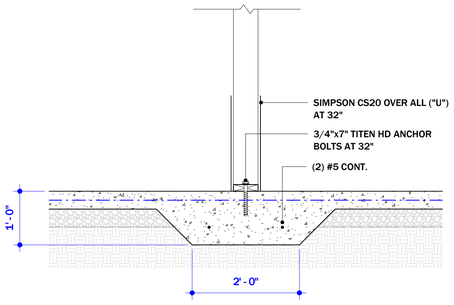
7 AT PORCH

SCALE: 3/4" = 1'-0"



6 AT EXTERIOR

SCALE: 3/4" = 1'-0"



5 AT INTERIOR WALL

SCALE: 3/4" = 1'-0"

nash engineers

Structural Engineer

[615] 880.8461 ___ main
[615] 880.8459 ___ fax

info@nashengineers.com
http://nashengineers.com

PROJECT:
FITZPATRICK RESIDENCE

769 Manor Hills Rd
Lillington, NC 27546

DATE: 11-18-2024
PROJECT NO 24S202

REVISION	DATE

NOTES:

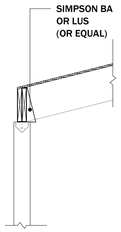
NORTH CAROLINA PROFESSIONAL SEAL 045631 ENGINEER VINCE LERAY NASH

11-18-2024

FOUNDATION DETAILS

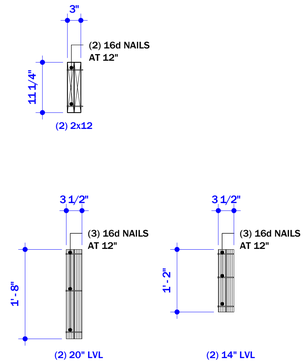
SCALE: As indicated

S301



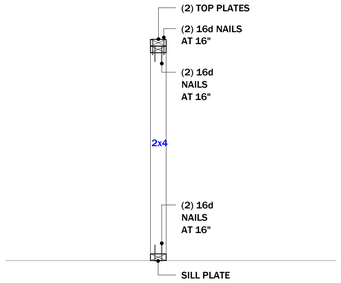
4 AT PORCH BEAMS

SCALE: 1/2" = 1'-0"



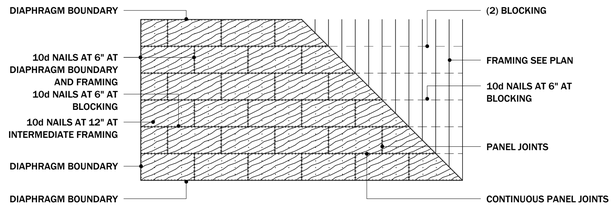
3 WOOD - COMPOSITE DETAILS

SCALE: 3/4" = 1'-0"



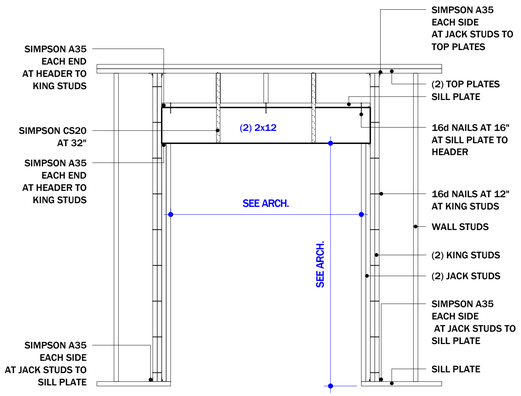
2 WOOD - WALL DETAIL

SCALE: 3/4" = 1'-0"



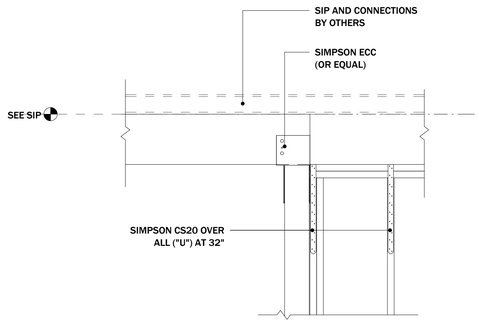
1 WOOD - SHEATHING DETAIL

SCALE: 3/32" = 1'-0"



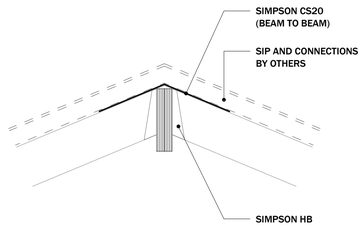
8 AT INTERIOR OPENING

SCALE: 1/2" = 1'-0"



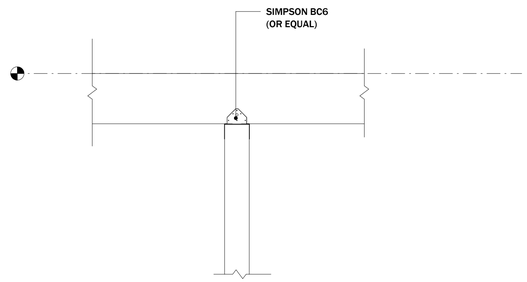
7 AT END GIRDER

SCALE: 3/4" = 1'-0"



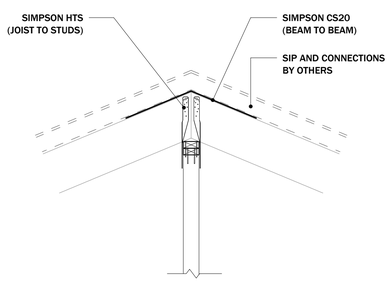
6 AT SMALL PEAK

SCALE: 3/4" = 1'-0"



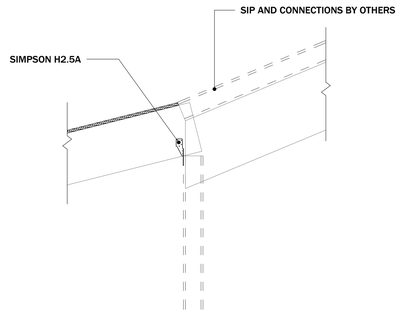
5 AT INTERIOR BEAM

SCALE: 3/4" = 1'-0"



10 AT WALL

SCALE: 3/4" = 1'-0"



9 AT PORCH AND SIP

SCALE: 3/4" = 1'-0"

nash engineers

Structural Engineer

[615] 880.8461 ___ main
[615] 880.8459 ___ fax

info@nashengineers.com
http://nashengineers.com

PROJECT:
FITZPATRICK RESIDENCE

769 Manor Hills Rd
Lillington, NC 27546

DATE: 11-18-2024
PROJECT NO 24S202

REVISION DATE

NOTES:

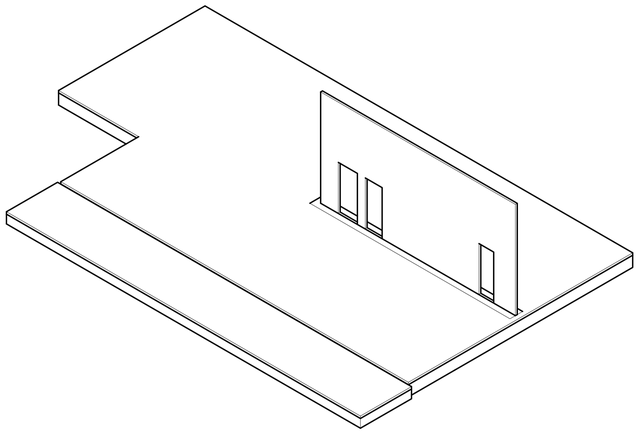
NORTH CAROLINA PROFESSIONAL SEAL 045631 ENGINEER LANCE LERAY NASH

11-18-2024

FRAMING DETAILS

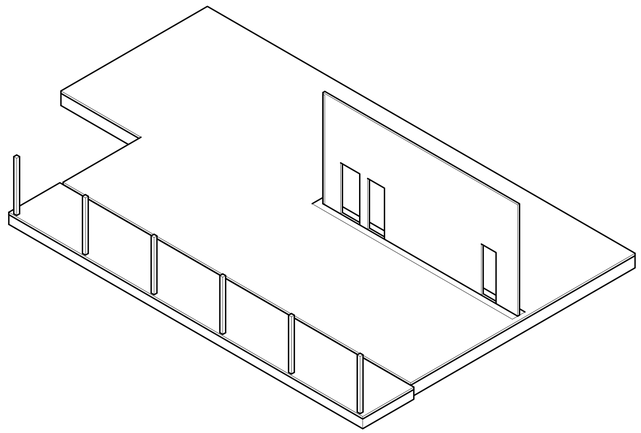
SCALE: As indicated

S401



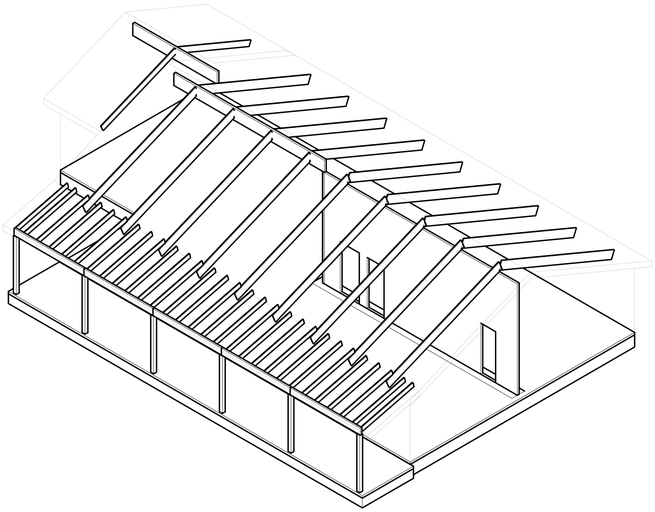
1 **FOUNDATION AXON**

SCALE:



2 **COLUMN AXON**

SCALE:



3 **BUILDING AXON**

SCALE:

nash engineers

Structural Engineer

[615] 880.8461 ___ main
[615] 880.8459 ___ fax

info@nashengineers.com
http://nashengineers.com

PROJECT:
FITZPATRICK RESIDENCE

769 Manor Hills Rd
Lillington, NC 27546

DATE: 11-18-2024
PROJECT NO 24S202

REVISION DATE

NOTES:

NORTH CAROLINA PROFESSIONAL SEAL 045631 ENGINEER VANCE LERAY NASH

11-18-2024

AXONOMETRICS

SCALE:

S501