Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 43366 JOB: 23-B170-R01 JOB NAME: LOT 97 PROVIDENCE CREEK Wind Code: 37 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. *30 Truss Design(s)*

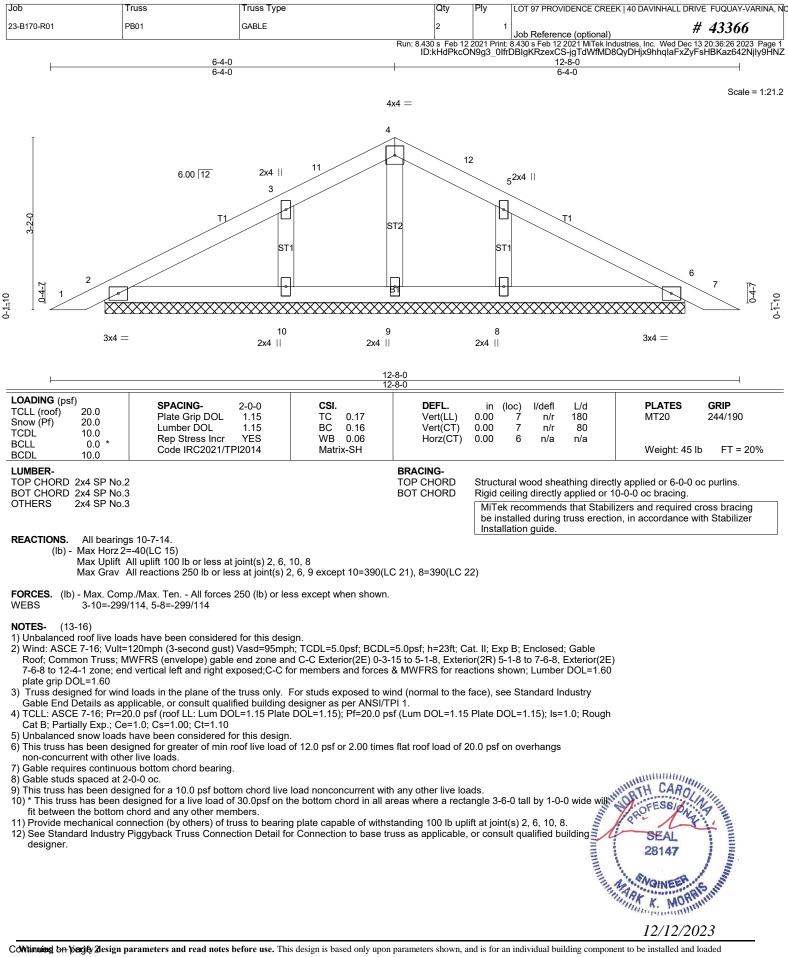
Trusses:

PB01, PB02, R01, R02, R03, R04, R05, R06, R07, R08, R09, R10, R11, R12, SP01, SP02, SPJ01, SPJ02, SPJ03, VS01, VS02, VS03, VT01, VT02, VT03, VT04, VT05, VT06, VT07, VT08



Warning !--- Verify design parameters and read notes before use.

This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for*



Job	Truss	Truss Type	Qty	Ply	LOT 97 PROVIDENCE CREEK 40 DAVINHALL DRIVE FUQ	UAY-VARINA, NC	
23-B170-R01	PB01	GABLE	2	1	Job Reference (optional) # 43.	366	
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Dec 13 20:36:26 2023 Page 2							

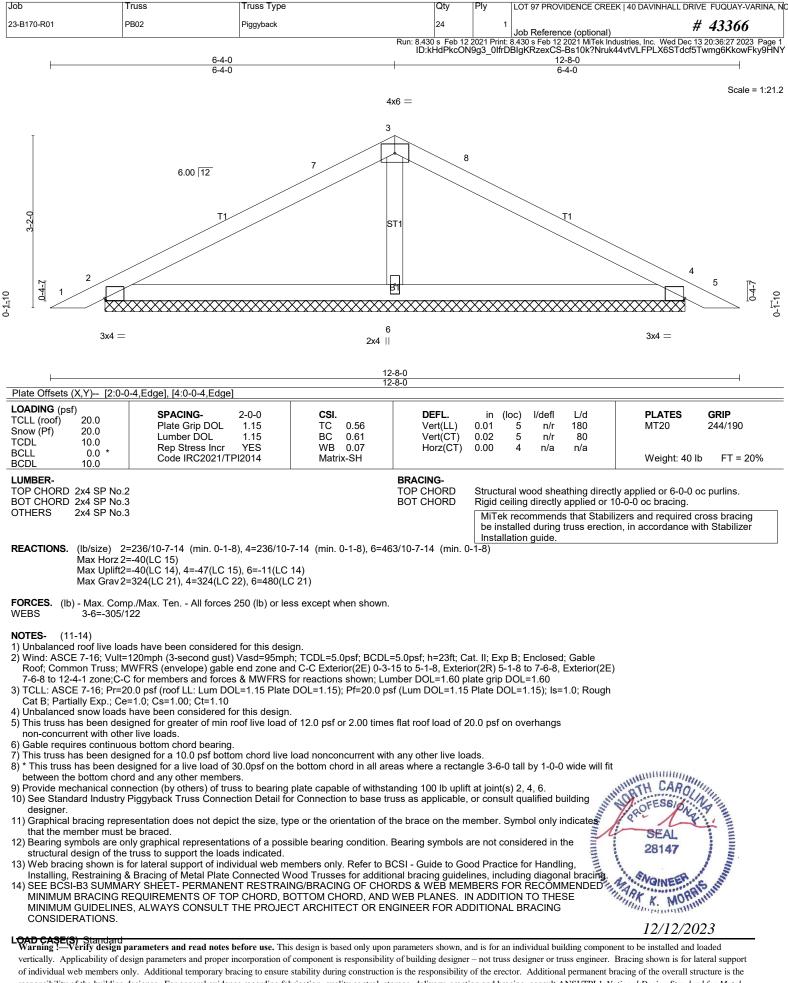
ID:kHdPkcON9g3_0lfrDBlgKRzexCS-jgTdWfMD8QyDHjx9hhqlaFxZyFsHBKaz642Njly9HNZ 13) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

15) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

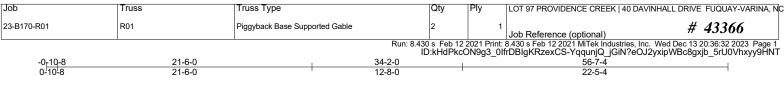
Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 16) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

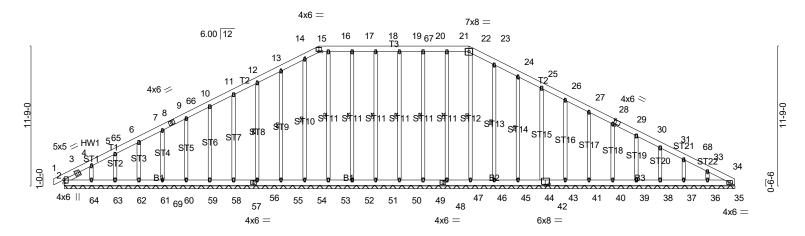




of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.







OADING (psf) COLL (psf) 20.0 20.0 (CLL (psf) SPACING- 20.0 (CLL (psf) 20.0 20.0 (CLL (psf) SPACING- 20.0 (CLL (psf) 20.0 20.0 (CLL (psf) SPACING- 20.0 (CLL (psf) 20.0 (CLL (psf) SPACING- 20.0 (CLL (psf) 20.0 (CLL (psf) SPACING- 20.0 (CLL (psf) PLATES GRIP (Psf) CLL (psf) 0.0 ClL (psf) 0.0 1 n/r 18.0 CLL (psf) 0.0 ClL (psf) 0.0 1 n/r 18.0 CLL (psf) 0.0 ClL (psf) 0.0 1 n/r 18.0 CLL (psf) 0.0 1 n/r 18.0 1.15 BC (DSF) OF CHORD 2x6 SP No.2 TDP CHORD 2x6 SP No.3.2 1.5 TDP CHORD DOT CHORD Structural wood sheathing directly applied or 6-0-0 cp putins. UIDER Left 2x4 SP No.3.3 1.5.4 1.5.5 5.6.58,59,60,61,62,63,54,49,47,45,44,45 12-56,20-49,21-47,22-46,23-45,24-44 EACTONS All bearings 56-74. (lb) Max Horz2-150(LC 15) 1.62,63,64,46,38,37,26,53,34 except 50-232(LC 45),58-222(LC 45), 55-229(LC 45),43-232(LC 45),43-232(LC 45),44-234(LC 45),43-232(LC 45),44-234(LC 45),43-232(LC 45),44-234(LC 45),43-232(LC 45),44-234(LC 45),43-232(LC 45),44	Parte Offsets (X,Y)- [29:0-2-11.6ge]. [43:0-4-0.0-1-4]. [48:0-2-10.0-2-0]. CDL (rop) 0.0 SPACING- 2-0.0 CSL DEFL in (loc) ///defl //defl //defl CDL (rop) 0.0 Lumber DOL 1.15 BC 0.03 Vert(C1) 0.00 1 n/r 180 COL 10.0 Code (RC2021/TPI2014 Matrix-SH Weight 559 lb FT = 20% MOP CHORD 2x6 SP No.2 STOCHORD 2x6 SP No.2 STOCHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.3 TOP CHORD Mittain gdirectly applied or 60-00 co parting. STO CHORD 2x6 SP No.2 THER S 2x4 SP No.3 11-6-4 10-0-00 co tracing. 10-0-00 co tracing. VIDER Lizz X4 SP No.3 3.7. 36, 3.5 51.52, 55, 55, 55, 59, 60, 61, 62, 63, 64, 49, 47, 45, 44, 43, 41, 40, 39, 38, 73, 83, 39, 73, 83, 39, 73, 83, 39, 73, 83, 39, 73, 83, 39, 73, 83, 39, 73, 83, 39, 73, 93, 39, 73, 93, 39, 73, 93, 39, 73, 93, 39, 73, 93, 39, 73, 93, 39, 73, 93, 39, 73, 93, 39, 73, 93, 39, 73, 93, 39, 73, 93, 39, 73, 93, 21, 62, 63, 64, 49, 47, 45, 44, 44, 44, 44, 44, 44, 44, 44, 44	I			6-7-4				
OADING (psf) COLL (psf) 20.0 20.0 (CLL (psf) SPACING- 20.0 (CLL (psf) 20.0 20.0 (CLL (psf) SPACING- 20.0 (CLL (psf) 20.0 20.0 (CLL (psf) SPACING- 20.0 (CLL (psf) 20.0 (CLL (psf) SPACING- 20.0 (CLL (psf) 20.0 (CLL (psf) SPACING- 20.0 (CLL (psf) PLATES GRIP (Psf) CLL (psf) 0.0 ClL (psf) 0.0 1 n/r 18.0 CLL (psf) 0.0 ClL (psf) 0.0 1 n/r 18.0 CLL (psf) 0.0 ClL (psf) 0.0 1 n/r 18.0 CLL (psf) 0.0 1 n/r 18.0 1.15 BC (DSF) OF CHORD 2x6 SP No.2 TDP CHORD 2x6 SP No.3.2 1.5 TDP CHORD DOT CHORD Structural wood sheathing directly applied or 6-0-0 cp putins. UIDER Left 2x4 SP No.3.3 1.5.4 1.5.5 5.6.58,59,60,61,62,63,54,49,47,45,44,45 12-56,20-49,21-47,22-46,23-45,24-44 EACTONS All bearings 56-74. (lb) Max Horz2-150(LC 15) 1.62,63,64,46,38,37,26,53,34 except 50-232(LC 45),58-222(LC 45), 55-229(LC 45),43-232(LC 45),43-232(LC 45),44-234(LC 45),43-232(LC 45),44-234(LC 45),43-232(LC 45),44-234(LC 45),43-232(LC 45),44-234(LC 45),43-232(LC 45),44	CADING (pdf) SPACING- 20.0 SPACING- Piala Girp DOL 2-0- 1.15 CSI. DEFL TC in (in) (in) (in) PLATES GRIP MT20 Since (P) 20.0 Plate Girp DOL 1.15 TC 0.06 Ver(TL) -0.00 in n/r 80 SiGL 0.0 Rep Stress Incr YES WB 0.25 Hor2(CT) 0.00 1 n/r 80 SiGL 0.0 Rep Stress Incr YES WB 0.25 Hor2(CT) 0.00 1 n/r 80 SiGL 0.0 Rep Stress Incr YES WB 0.25 Hor2(CT) 0.00 1 n/r 80 SiGL 0.0 Rep Stress Incr YES WES Structural wood sheathing directly applied or 60-0 oc praims. Weight 559 lb FT = 20% JUMBER top C + IORD XeS P No.3 T-6.4 WES TOP C + IORD Mark and pit 1950. It 55. Stress 51. 45. 59. 58. 59. 60. 61. 62. 63. 64. 49. 47. 75. 44. Mark and pit 1950. It 55. 1756. 55. 58. 59. 60. 61. 62. 63. 64. 49. 47. 35. 34. 40. 4	Plate Offsets (X,Y) [29:0)-2-1,Edge], [43:0-4-0,0-1-4], [48:0-2-1						
UMBER- OP CHORD 2x6 SP No.2 BRACING- TOP CHORD 2x6 SP No.3 Structural wood sheathing directly applied or 6-0-0 oc purlins. Rgid celling directly applied or 10-0-0 oc bracing. THERS 2x4 SP No.3 Structural wood sheathing directly applied or 10-0-0 oc purlins. Rgid celling directly applied or 10-0-0 oc bracing. 11cm at midpt 19-50, 18-51, 17-52, 16-53, 14-54, 13-55, 12-56, 20-49, 21-47, 22-46, 23-45, 24-44 EACTIONS. Max Hor 2z-150(LC 15) Max Hor 2z-150(LC 15) Inaw at midpt 19-50, 18-54, 17-52, 16-53, 14-54, 13-55, 12-56, 20-49, 21-47, 22-46, 23-45, 24-44 EACTIONS. Max Grav All reactions 250 lo bor less at joint(s) 2, 50, 51, 52, 55, 56, 58, 59, 60, 61, 62, 63, 64, 49, 47, 45, 44, 43, 41, 40, 39, 33, 37, 36, 35 Sar 230(LC 44), 58-298(LC 45), 58-298(LC 44), 47, 45, 44, 45, 12-298(LC 45), 62-293(LC 45), 58-298(LC 45), 68-298(LC 45), 43-291(LC 45), 41-293(LC 45), 40-293(LC 45), 39-275(LC 45) ORCES. (Ib) - Max. Comp /Max. Ten All forces 250 (Ib) or less except when shown. OP CHORD 14-1242(26), 19-67-1242(26), 20-27-1-1242(26), 21-22-1-1242(26), 22-23-1392(ZT 45) Wind: ASCE - Ti6; Vult=120mph (3-second just) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; BCDL=5.0psf; BCDL=5.0psf - 150; 10-15-10-1, 0Vind: ASCE - Ti6; Vult=120mph (3-second just) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; BCDL=5	UNDER- IOP CHORD 2x6 SP No.2 BRACING- TOP CHORD 2x6 SP No.2 DOT CHORD 2x6 SP No.2 TOP CHORD 2x6 SP No.2 DT CHORD 2x6 SP No.3 TOP CHORD 2x6 SP No.3 SLIDER Left 2x4 SP No.3 SLIDER Left 2x4 SP No.3 SLIDER Left 2x4 SP No.3 (I)- Max Horz 2=: fold(C15) Max Upiff 1x41 upiff 100 bor less at joint(s) 2, 50, 51, 52, 55, 56, 58, 59, 60, 61, 62, 63, 64, 49, 47, 45, 44, 43, 41, 40, 39, 38, 37, 36, 35, 34 Figure 2x10C 4x1, 52=295(IC 4x1, 53=295(IC 4x1, 53=292(IC 4x1, 53=	LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.06 BC 0.03 WB 0.25	DEFL. Vert(LL) Vert(CT)	-0.00 1 n/r 0.00 1 n/r	180 80	MT20	244/190
 (b) - Max Horz 2²-150(LC 15) Max Upift A01 upift 100 hor less at joint(s) 2, 50, 51, 52, 55, 56, 58, 59, 60, 61, 62, 63, 64, 49, 47, 45, 44, 43, 41, 40, 39, 38, 37, 36, 35 Max Grav All reactions 250 hor less at joint(s) 2, 61, 62, 63, 64, 46, 38, 37, 36, 35, 34 except 50=292(LC 44), 51=293(LC 45), 55=293(LC 45), 55=293(LC 45), 55=293(LC 45), 55=293(LC 45), 65=293(LC 45), 65=293(LC 45), 65=293(LC 45), 49=293(LC 45), 49=293(LC 45), 49=293(LC 44), 47=287(LC 44), 45=302(LC 49), 44=294(LC 45), 43=291(LC 45), 41=293(LC 45), 49=293(LC 45), 49=294(LC 45), 49=40, 49=40, 49=40, 49=40, 49=40, 49=40, 49=40	 (b) - Max Horz 2²-150(LC 15) Max Upitit 710(b or less at joint(s) 2, 50, 51, 52, 55, 56, 58, 59, 60, 61, 62, 63, 64, 49, 47, 45, 44, 43, 41, 40, 39, 38, 37, 36, 35 Max Grav All reactions 250(b) or less at joint(s) 2, 61, 62, 63, 64, 46, 38, 37, 36, 35, 34 except 50=292(LC 44), 51=293(LC 44), 53=294(LC 45), 53=294(LC 55), 54=294(LC 53), 55=293(LC 45), 58=292(LC 45), 41=293(LC 45), 40=293(LC 45), 39=275(LC 45) SORCES. (b) - Max. Comp. Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 13-14a-124/265, 19-67=-124/265, 20-21=-124/265, 17-18=-124/265, 12-23=-132/271 SORCES. (c) - Max. Comp. Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 13-14a-124/265, 19-67=-124/265, 20-21=-124/265, 21-22=-124/265, 22-23=-132/271 SORCES. (c) - Max. Comp. Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 13-14a-124/265, 19-67=-124/265, 20-21=-124/265, 21-22=-124/265, 22-23=-132/271 SORCES. (c) - Max. Comp. J. Seecond gust) Vasd=96mph; TCDL=5.0psf; BCDL=5.0psf; h=23f; Cat. II; Exp B; Enclosed; Gable Rod; Common Truss; MWFRS (envelope) gable end zone and C-C Cormer(3E) -0-10-8 to 4-9-7, Exterior(2N) 49-7 to 15-10-1, Comer(3F) 15-10-1 1, Comer(3F) 15-10-1, 10, Comer(3F) 28-3-10 to 39-9-15 to 50-11-5, Comer(3F) 5-11-5, 16, Common Truss; MWFRS (or reactions shown; Lumber DOL=1.60) paff cord LL: Lum DOL=1.160, Date or pDOL=1.60 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Datals as applicable; or consult qualified building designer as per ANSI/TP1 1. This truss has been designed for a 10.0 psf to 11.0 the 124.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with there ive loads. This truss has been designed for a 10.0 psf bothom chord inte load	LUMBER- TOP CHORD 2x6 SP No. BOT CHORD 2x6 SP No. OTHERS 2x4 SP No. SLIDER Left 2x4 SP	2 3 ? No.3 -° 1-6-4		TOP CHORD BOT CHORD	Rigid ceiling directly a	pplied or 10-0-0 19-50, 18-5	oc bracing. I, 17-52, 16-53,	14-54, 13-55,
 OTES- (14-17) Uhbalanced roof live loads have been considered for this design. Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Cormer(3E) -0-10-8 to 4-9-7, Exterior(2N) 4-9-7 to 15-10-1, Cormer(3R) 15-10-1 to 271-15, Exterior(2V) 271-15 to 28-3-10, Cormer(3R) 28-3-10 to 39-9-15, Exterior(2N) 39-9-15 to 50-11-5, Cormer(3E) 50-11-5 to 56-7-4 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rourg Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 Uhbalanced snow loads have been considered for this design. This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. Provide adequate drainage to prevent water ponding. All plates are 2x4 MT20 unless otherwise indicated. Gable requires continuous bottom chord bearing. Gable studs spaced at 2-0-0 oc. This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will 	 NOTES- (14-17) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-1-0-8 to 4-9-7, Exterior(2N) 39-9-15 to 50-11-5, Corner(3R) 15-10-1 to 27-1-15, Exterior(2N) 27-1-15 to 28-3-10, Corner(3R) 28-3-10 to 39-9-15, Exterior(2N) 39-9-15 to 50-11-5, Corner(3R) 15-10-1 to 27-1-15, Exterior(2N) 27-1-15 to 28-3-10, Corner(3R) 28-3-10 to 39-9-15, Exterior(2N) 39-9-15 to 50-11-5, Corner(3R) 15-10-1 to 27-1-5, Exterior(2N) 27-1-15 to 28-3-10, Corner(3R) 28-3-10 to 39-9-15, Exterior(2N) 39-9-15 to 50-11-5, Corner(3R) 15-10-1 to 27-1-15, Exterior(2N) 29-9-15, Exterior(2N) 39-9-15 to 50-11-5, Corner(3R) 26-50-11-5, Corner(3R) 26-50-11-5, Corner(3R) 26-50-50, Corner (3R) 15-10-1 to 27-1-15, Exterior(2N) 27-1-15 to 28-3-10, Corner(3R) 28-3-10 to 39-9-15, Exterior(2N) 39-9-15 to 50-11-5, Corner(3R) 15-10-1 to 27-1-15, Exterior(2N) 29-9-15, Exterior(2N) 39-9-15 to 50-11-5, Corner(3R) 26-50-12, Corner (3R) 27-1-15 to 28-3-10, Corner(3R) 27-1-50, Corner(3R) 27-1-50, Corner (3R) 27-1-50, Corner (3R) 27-1-50, Corner (3R) 27-1-50, Corner (3R) 27-10, Corner (3R) 20-0, Corne	(Ib) - Max Horz 2 Max Uplift Max Grav 5 FORCES. (Ib) - Max. Con TOP CHORD 13-1412 18-19=-12	2=-150(LC 15) All uplift 100 lb or less at joint(s) 2, 50 i3, 41, 40, 39, 38, 37, 36, 35 All reactions 250 lb or less at joint(s) j1=293(LC 44), 52=295(LC 44), 53=28 j9=294(LC 45), 60=287(LC 45), 49=29 i1=293(LC 45), 40=293(LC 45), 39=27 np./Max. Ten All forces 250 (lb) or le 24/261, 14-15=-129/268, 15-16=-124/2 24/265, 19-67=-124/265, 20-67=-124/2	2, 61, 62, 63, 64, 46, 38 34(LC 52), 54=284(LC 5 35(LC 44), 47=287(LC 4 5(LC 45) ss except when shown. 265, 16-17=-124/265, 17	, 37, 36, 35, 34 ex 3), 55=296(LC 45 4), 45=302(LC 49 7-18=-124/265,	ccept 50=292(LC 44),), 56=293(LC 45), 58=29			
	- 58, 59, 50, 61, 62, 63, 64, 49, 47, 45, 44, 43, 41, 40, 39, 38, 37, 36, 35.	 Wind: ASCE 7-16; Vult: Roof; Common Truss; I Corner(3R) 15-10-1 to 2 Corner(3E) 50-11-5 to 5 Lumber DOL=1.60 plate Truss designed for win Gable End Details as a (1) TCLL: ASCE 7-16; Pr=2 Cat B; Partially Exp.; Cd Unbalanced snow loads This truss has been des non-concurrent with oth 7) Provide adequate drain All plates are 2x4 MT2C Gable studs spaced ai This truss has been des non-concursent with oth 7) Provide adequate drain All plates are 2x4 MT2C Gable studs spaced ai This truss has been des 	=120mph (3-second gust) Vasd=95mp WWFRS (envelope) gable end zone an 27-1-15, Exterior(2N) 27-1-15 to 28-3- 36-7-4 zone; end vertical left and right a grip DOL=1.60 d loads in the plane of the truss only. pplicable, or consult qualified building 20.0 psf (roof LL: Lum DOL=1.15 Plate s=1.0; Cs=1.00; Ct=1.10 s have been considered for this design signed for greater of min roof live load er live loads. age to prevent water ponding.) unless otherwise indicated. bus bottom chord bearing. t 2-0-0 oc. signed for a live load of 30.0psf on chord and any other members, with E sonnection (by others) of truss to bearing.	in; TCDL=5.0psf; BCDL ad C-C Corner(3E) -0-10 10, Corner(3R) 28-3-10 exposed;C-C for member For studs exposed to w designer as per ANSI/T e DOL=1.15); Pf=20.0 ps n. of 12.0 psf or 2.00 time the bottom chord in all a CDL = 10.0psf. g plate capable of withs	D-8 to 4-9-7, Exteri to 39-9-15, Exteri ers and forces & N ind (normal to the PI 1. sf (Lum DOL=1.15 s flat roof load of 2 ith any other live I reas where a rect	ior(2N) 4-9-7 to 15-10-1, or(2N) 39-9-15 to 50-11- /WFRS for reactions sh face), see Standard Ind 5 Plate DOL=1.15); Is=1. 20.0 psf on overhangs 0ads. angle 3-6-0 tall by 1-0-0	5, own; ustry .0; Roughtung wide will	H CARO OFESSION SEAL 28147 VOINEER K. MORAN	IIII A TANAN AND AND AND AND AND AND AND AND AND

of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 97 PROVIDENCE CREEK 40 DAVIN	HALL DRIVE FUQUAY-VARINA, NC
23-B170-R01	R01	Piggyback Base Supported Gable	2	1	Job Reference (optional)	# 43366
		Run: 8.4	30 s Feb 12	2021 Print:	8.430 s Feb 12 2021 MiTek Industries. Inc.	Wed Dec 13 20:36:33 2023 Page 2

ID:kHdPkcON9g3_0lfrDBlgKRzexCS-00OH_2RcUaqDdozVbfSxMjjnu4HyKRL?jfFETOy9HNS

14) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

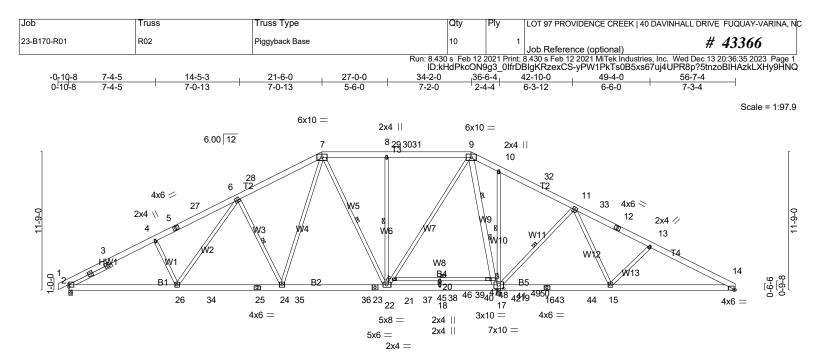
16) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trustees for additional bracing guidelines, including diagonal bracing. 17) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



12/12/2023



—	<u>9-2-5</u> 9-2-5	18-1-3	27-0-0	31-6-0 36-6-4		56-7-4				
Plate Offsets (8-10-13 1-4,Edge], [17:0-5-0,0-4-0]	8-10-13	4-6-0 5-0-4	9-5-12	10-7-4				
LOADING (psf TCLL (roof) Snow (Pf) TCDL BCLL BCDL		SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.53 BC 0.83 WB 0.86 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) I/defl L/d -0.25 19-20 >999 240 -0.37 19-20 >999 180 0.05 14 n/a n/a	PLATES GRIP MT20 244/190 Weight: 471 lb FT = 20%				
LUMBER- TOP CHORD 2x6 SP No.2 BRACING- TOP CHORD 2x6 SP No.2 *Except* B4: 2x4 SP No.2 TOP CHORD B4: 2x4 SP No.2 *Except* WEBS Structural wood sheathing directly applied or 4-2-0 oc purlins. B4: 2x4 SP No.3 *Except* W7: 2x4 SP No.2, W9: 2x6 SP No.1 WEBS 2x4 SP No.2, W9: 2x6 SP No.1 WEBS 1 Row at midpt 9-19 6-24, 7-22, 8-22, 10-17, 11-17 2 Rows at 1/3 pts 9-19										
SLIDER Left 2x4 SP No.3 - ° 4-0-11 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS. (Ib/size) 2=1377/0-3-8 (min. 0-1-14), 14=511/Mechanical, 17=2860/0-3-8 (min. 0-2-9) Max Horz 2=-150(LC 19) Max Uplift2=-126(LC 14), 14=-76(LC 15), 17=-34(LC 15) Max Grav 2=1614(LC 92), 14=576(LC 41), 17=3755(LC 43) MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.										
FORCES. (Ib) TOP CHORD	2-3=-2669/ 6-28=-1805 30-31=-103	o./Max. Ten All forces 250 (lb) or le 178, 3-4=-2571/209, 4-5=-2477/215, 5/243, 7-28=-1666/272, 7-29=-1038/2 37/216, 9-31=-1038/216, 9-10=0/753, 8/106, 12-13=-458/94, 13-14=-756/14	5-27=-2410/226, 6-2 16, 8-29=-1037/216 10-32=0/938, 11-32	27=-2378/244, , 8-30=-1037/216,	120,					
BOT CHORD	2-26=-237/ 35-36=-31/ 38-39=-131 17-42=-131 4-26=-294/ 8-22=-744/	2243, 26-34=-121/1878, 25-34=-121/ 1249, 23-36=-31/1249, 22-23=-31/12 1/358, 18-39=-131/358, 18-40=-131/3 1/358, 14-15=-60/620 171, 6-26=-78/541, 6-24=-1002/235, 151, 21-22=-134/1727, 9-21=-114/18	1878, 24-25=-121/1 49, 22-37=-131/358 58, 40-41=-131/358 7-24=-135/1228, 7-2 92, 9-19=-2031/135	, 37-38=-131/358, , 41-42=-131/358, 22=-867/149, , 17-19=-2103/104,	,					
 8-22=-744/151, 21-22=-134/1727, 9-21=-114/1892, 9-19=-2031/135, 17-19=-2103/104, 10-17=-628/146, 11-17=-976/198, 11-15=-29/652, 13-15=-455/178, 18-20=-263/0 NOTES- (17-20) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Hip Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 4-9-7, Interior(1) 4-9-7 to 15-10-1, Exterior(2R) 15-10-1 to 27-0-0, Interior(1) 27-0-0 to 28-6-1, Exterior(2R) 28-6-1 to 39-9-15, Interior(1) 39-9-15 to 50-10-9, Exterior(2E) 50-10-9 to 56-6-8 zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Reugh Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Unbalanced snow loads have been considered for this design. 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. 6) Provide adequate drainage to prevent water ponding. 7) All plates are 5x5 MT20 unless otherwise indicated. 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 7) All plates are 5x5 MT20 unless otherwise indicated. 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 7) All plates are 5x6 MT20 unless otherwise indicated. 7) All plates are 5x6 MT20 unless otherwise indicated. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. <l< td=""></l<>										
,		5 I		,	ads.	12/12/2023				
0 1	0.	arameters and read notes before use. This sign parameters and proper incorporation of		• • ·	ind is for an individual building compon	chi to be instance and loaded				

Containing Br paggy Zesign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 97 PROVIDENCE CREEK 40 DA	VINHALL DRIVE FUQUAY-VARIN
3-B170-R01	R02	Piggyback Base	10		1 Job Reference (optional)	# 43366
			Run: 8.430 s Feb	12 2021 Prin	t: 8.430 s Feb 12 2021 MiTek Industries, In DBIgKRzexCS-yPW1PkTs0B5xs67uj	nc. Wed Dec 13 20:36:35 2023 Page
NOTES- (17-20)			Maga_onn	DBIGNNZEXCO-yr W IFKI SOBDXS07 uj	
		ve load of 30.0psf on the bottom chord in all	areas where a rect	angle 3-6-	0 tall by 1-0-0 wide will fit between	n the bottom chord and any
	, with BCDL = 10.0psf.					
	r(s) for truss to truss co					
		el to grain value using ANSI/TPI 1 angle to				ng surface.
		thers) of truss to bearing plate capable of wi				r must review leads to verify
	correct for the intended	92, 93, 94, 95, 96, 97, 98, 99, 100, 101, 102,	, 103, 104, 105, 106	nas/nave	been modified. Building designer	must review loads to verify
		ase of this trass. gn is the composite result of multiple load ca	2505			
		w the load cases for details.	ases.			
		applied to the face of the truss are noted as	s front (E) or back (E	3)		
		s not depict the size, type or the orientation			Symbol only indicates that the me	ember must be braced
		presentations of a possible bearing condition				
loads indicate						
		port of individual web members only. Refer to	to BCSI - Guide to G	Good Prac	tice for Handling, Installing, Restra	aining & Bracing of Metal PI
Connected Wo	ood Trusses for addition	nal bracing guidelines, including diagonal br	racing.			0 0
20) SEE BCSI-B3	SUMMARY SHEET- P	ERMANENT RESTRAING/BRACING OF CI	HORDS & WEB ME	EMBERS I	FOR RECOMMENDED MINIMUM	I BRACING REQUIREMEN
OF TOP CHO	RD, BOTTOM CHORD	, AND WEB PLANES. IN ADDITION TO TH	HESE MINIMUM GL	JIDELINE	S, ALWAYS CONSULT THE PRO	JECT ARCHITECT OR
ENGINEER F	OR ADDITIONAL BRAC	CING CONSIDERATIONS.				
	tendend Excent					
LOAD CASE(S) S		d + Snow (balanced): Lumber Increase=1.15	E Diata Ingragoa-1	15		
Uniform Loads		1 + Show (balanced). Lumber increase=1.13	5, Flate Increase-1.	15		
		9-14=-60(F), 2-14=-20(F), 19-21=-20(F)				
Concentrated		5 14 - 00(1), 2 14 - 20(1), 10 21 - 20(1)				
	22=-150 37=-150					
		d + Snow (balanced): Lumber Increase=1.1	5, Plate Increase=1	.15		
Uniform Loads	s (plf)	, , , , , , , , , , , , , , , , , , ,	,			
Vert: 1	I-7=-60(F), 7-9=-60(F),	9-14=-60(F), 2-14=-20(F), 19-21=-20(F)				
Concentrated	Loads (lb)					
	37=-150 39=-150					
		d + Snow (balanced): Lumber Increase=1.15	5, Plate Increase=1	.15		
Uniform Loads						
		9-14=-60(F), 2-14=-20(F), 19-21=-20(F)				
Concentrated						
	39=-150 40=-150	d + Snow (balanced): Lumber Increase-1 45	5 Plata Ingraaca-1	15		
Uniform Loads		d + Snow (balanced): Lumber Increase=1.15	J, FIALE INCIEASE-1.	.15		
		9-14=-60(F), 2-14=-20(F), 19-21=-20(F)				
Concentrated		5 · · · · · · · · · · · · · · · · · · ·				
	10=-150 42=-150					
		d + Snow (balanced): Lumber Increase=1.15	5. Plate Increase=1.	.15		
Uniform Loads		(-,	-		
		9-14=-60(F), 2-14=-20(F), 19-21=-20(F)				
Concentrated						
Vert: 1	17=-150 41=-150					
91) 7th Unbal 1st	User Defined Moving L	oad - Dead + Snow (balanced)-Parallel [.] I un	mber Increase=1 15	Plate Inc	rease=1 15	

91) 7th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-7=-32(F=-20), 7-9=-101(F=-20), 9-14=-32(F=-20), 2-14=-20(F), 19-21=-20(F)

Concentrated Loads (lb) Vert: 22=-150 37=-150

92) 8th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-5=-60(F=-20), 5-7=-101(F=-20), 7-9=-32(F=-20), 9-12=-101(F=-20), 12-14=-60(F=-20), 2-14=-20(F), 19-21=-20(F) Concentrated Loads (lb)

Vert: 22=-150 37/ 37 -- 150

93) 7th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-7=-32(F=-20), 7-9=-101(F=-20), 9-14=-32(F=-20), 2-14=-20(F), 19-21=-20(F)

Concentrated Loads (lb)

Vert: 22=-150 37=-150

94) 8th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-5=-60(F=-20), 5-7=-101(F=-20), 7-9=-32(F=-20), 9-12=-101(F=-20), 12-14=-60(F=-20), 2-14=-20(F), 19-21=-20(F) Concentrated Loads (lb) Vert: 22=-150 37=-150

95) 7th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-7=-32(F=-20), 7-9=-101(F=-20), 9-14=-32(F=-20), 2-14=-20(F), 19-21=-20(F)

Concentrated Loads (lb)

Vert: 22=-150 37=-150

96) 8th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-5=-60(F=-20), 5-7=-101(F=-20), 7-9=-32(F=-20), 9-12=-101(F=-20), 12-14=-60(F=-20), 2-14=-20(F), 19-21=-20(F) Concentrated Loads (lb)

Vert: 22=-150 37=-150

97) 7th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-7=-32(F=-20), 7-9=-101(F=-20), 9-14=-32(F=-20), 2-14=-20(F), 19-21=-20(F)

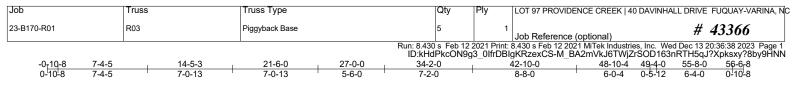
(F) NAVIN AND MARINE CONTRACTOR WORRIG 12/12/2023

Job	Truss	Truss Type	Qty	Ply	LOT 97 PROVIDENCE CREEK 40 DAVINHALL DRIVE FUQUAY-VARINA, N
23-B170-R01	R02	Piggyback Base	10	1	Job Reference (optional) # 43366

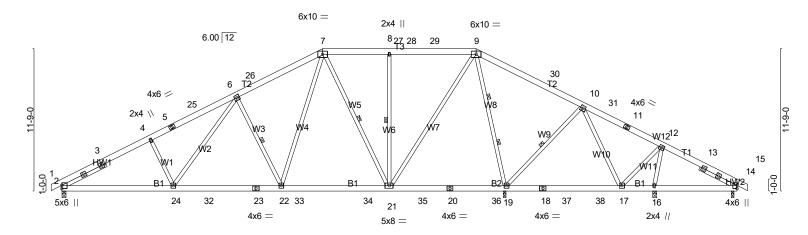
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Dec 13 20:36:35 2023 Page 3 ID:kHdPkcON9g3_0lfrDBlgKRzexCS-yPW1PkTs0B5xs67uj4UPR8p?5tnzoBlHAzkLXHy9HNQ

	ID:KHUPKCON9g5_UIIDBIgKR2exC5-yPW1Pk1S0B5x567uj40PR6p?5th20BIHA2kLXHy
LOAD CASE(S)	
Concentrated Loads (lb)	
Vert: 22=-150 37=-150	
98) 8th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber I Uniform Loads (plf)	ncrease=1.15, Plate Increase=1.15
Vert: 1-5=-60(F=-20), 5-7=-101(F=-20), 7-9=-32(F=-20), 9-12=-101(F=-20), 12-	.14=-60(F=-20), 2-14=-20(F), 19-21=-20(F)
Concentrated Loads (Ib)	
Vert: 22=-150 37=-150 99) 7th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber I	norogon-1.15. Plata Ingragon-1.15
Uniform Loads (plf)	1000000 = 1.10, Flate III0100000 = 1.10
Vert: 1-7=-32(F=-20), 7-9=-101(F=-20), 9-14=-32(F=-20), 2-14=-20(F), 19-21=-	20(F)
Concentrated Loads (lb)	
Vert: 22=-150 37=-150	
100) 8th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber	Increase=1.15, Plate Increase=1.15
Uniform Loads (plf) Vert: 1-5=-60(F=-20), 5-7=-101(F=-20), 7-9=-32(F=-20), 9-12=-101(F=-20), 12	2-14=-60/F=-20) 2-14=-20(F) 10-21=-20(F)
Concentrated Loads (Ib)	
Vert: 22=-150 37=-150	
101) 7th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber	Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)	
Vert: 1-7=-32(F=-20), 7-9=-101(F=-20), 9-14=-32(F=-20), 2-14=-20(F), 19-21= Concentrated Loads (Ib)	20(F)
Vert: 22=-150 37=-150	
102) 8th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber	Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)	
Vert: 1-5=-60(F=-20), 5-7=-101(F=-20), 7-9=-32(F=-20), 9-12=-101(F=-20), 12	2-14=-60(F=-20), 2-14=-20(F), 19-21=-20(F)
Concentrated Loads (lb) Vert: 22=-150 37=-150	
103) 7th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber	Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)	
Vert: 1-7=-32(F=-20), 7-9=-101(F=-20), 9-14=-32(F=-20), 2-14=-20(F), 19-21=	20(F)
Concentrated Loads (Ib) Vert: 22=-150 37=-150	
104) 8th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber	Increase=1.15. Plate Increase=1.15
Uniform Loads (plf)	
Vert: 1-5=-60(F=-20), 5-7=-101(F=-20), 7-9=-32(F=-20), 9-12=-101(F=-20), 12	2-14=-60(F=-20), 2-14=-20(F), 19-21=-20(F)
Concentrated Loads (Ib) Vert: 22=-150 37=-150	
105) 7th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber	Increase=1.15. Plate Increase=1.15
Uniform Loads (plf)	
Vert: 1-7=-32(F=-20), 7-9=-101(F=-20), 9-14=-32(F=-20), 2-14=-20(F), 19-21=	20(F)
Concentrated Loads (lb)	
Vert: 22=-150 37=-150 106) 8th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber	Increase=1 15 Plate Increase=1 15
Uniform Loads (plf)	
Vert: 1-5=-60(F=-20), 5-7=-101(F=-20), 7-9=-32(F=-20), 9-12=-101(F=-20), 12	2-14=-60(F=-20), 2-14=-20(F), 19-21=-20(F)
Concentrated Loads (Ib)	
Vert: 22=-150 37=-150	





Scale = 1:95.0



	<u> </u>	27-0-0 8-10-13	<u>36-6-4</u> 9-6-4		46-0-0 9-5-12	48-10-4	55-5-8 6-7-4	55 _⊺ 8-0 0-2-8
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.72 BC 0.66 WB 0.94 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.17 22-24 -0.26 22-24 0.05 19	l/defl L/d >999 240 >999 180 n/a n/a	PLAT MT20 Weig	0	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP No. BOT CHORD 2x6 SP No. B2: 2x6 SP WEBS 2x4 SP No. W8: 2x4 SP SLIDER Left 2x4 SP	2 *Except* DSS 3 *Except*		BRACING- TOP CHORD BOT CHORD WEBS	Rigid ceiling 1 Row at mi 2 Rows at 1 MiTek reco	/3 pts 9-19 ommends that Stat d during truss erec	6-0-0 oc brac , 7-21, 8-21, 1 pilizers and rec	oing. 0-19 quired cro	ss bracing
(lb) - Max Horz 2 Max Uplift Max Grav		14, 16 except 2=-136(L0		14=364(LC 43), 16=576(LC			
TOP CHORD 2-3=-2672 6-26=-182	np./Max. Ten All forces 250 (lb) or 2/222, 3-4=-2578/253, 4-5=-2500/264 24/290, 7-26=-1676/318, 7-27=-916/2 15/271, 9-29=-916/271, 9-30=0/790,	4, 5-25=-2415/270, 6-25= 271, 8-27=-915/271, 8-28	=-2386/288, 3=-915/271,					
BOT CHORD 2-24=-256 33-34=-35 WEBS 4-24=-307	5/2251, 24-32=-137/1864, 23-32=-13 5/1169, 21-34=-35/1169 7/172, 6-24=-81/537, 6-22=-999/236, 3/150, 9-21=-133/1632, 9-19=-2115/2	7/1864, 22-23=-137/186 7-22=-130/1269, 7-21=-	4, 22-33=-35/1169 917/132,					

Job	Truss	Truss Type	Qty	Ply	LOT 97 PROVIDENCE CREEK 40 DAVINI	HALL DRIVE FUQUAY-VARINA, NC	
23-B170-R01	R03	Piggyback Base	5	1	Job Reference (optional)	# 43366	
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Dec 13 20:36:38 2023 Page 2							

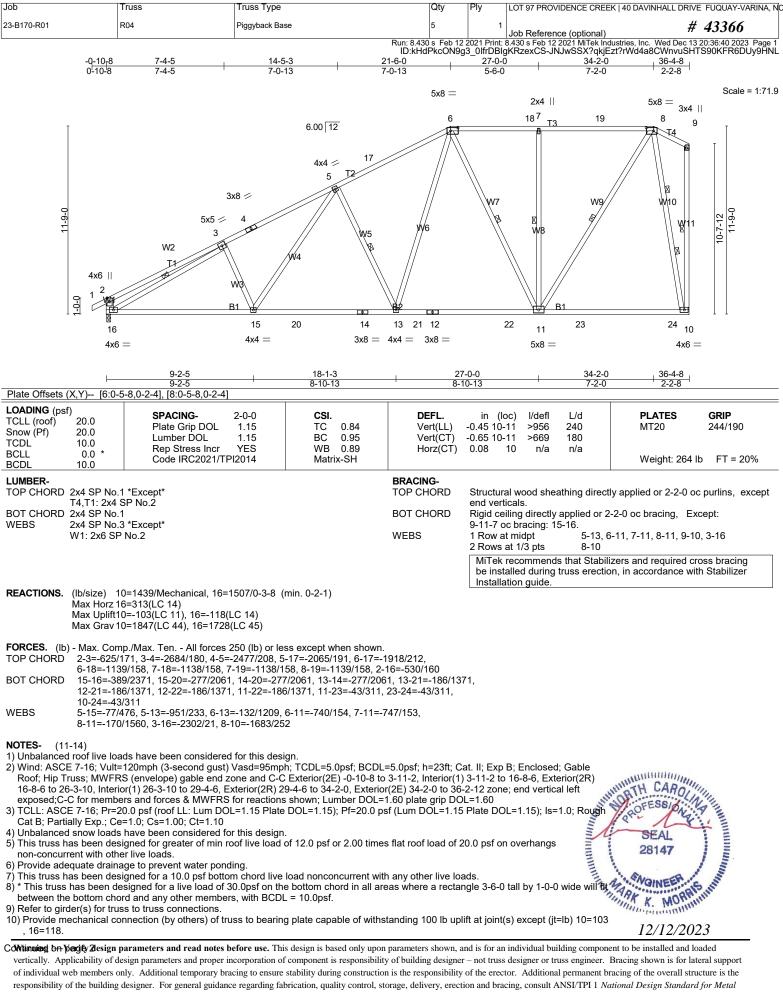
ID:kHdPkcON9g3_0lfrDBlgKRzexCS-M_BA2mVkJ6TWjZrSOD163nRTH5qJ?Xpksxy?8by9HNN 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

Connected Wood Trustees for additional bracing guidelines, including diagonal bracing. 14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Met Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 97 PROVIDENCE CREEK 40 D	AVINHALL DRIVE FUQUAY-VARINA, NC			
23-B170-R01	R04	Piggyback Base	5	1	Job Reference (optional)	# 43366			
	Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 Mittek Industries, Inc. Wed Dec 13 20:36:41 2023 Page 2 ID:kHdPkcON9g3 0lfrDBlgKRzexCS-nZtlgnXdb1r5a1a13LbphP2yfloWCvPAYvBglwy9HNK								

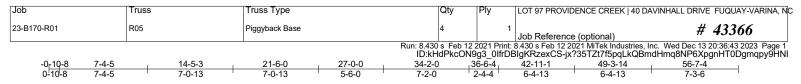
11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

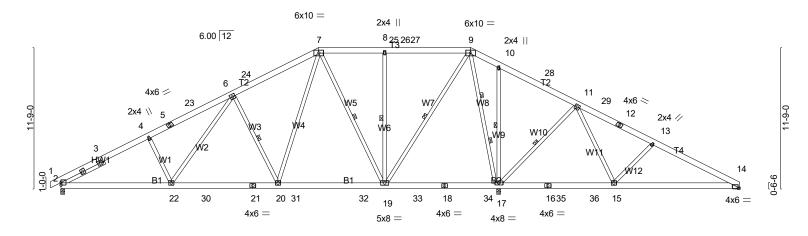
Connected Wood Trustees for additional bracing guidelines, including diagonal bracing. 14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Scale: 1/8"=1'



9-2-5 late Offsets (X,Y) [14:0 OADING (psf)	8-10-13	27-0-0	<u>36-6-4</u> 9-6-4	46-1-8	<u>56-7-4</u> 10-5-12
DADING (pof)		0-10-10	3-0-4	5-1-+	10-0-12
CLL (roof) 20.0 now (Pf) 20.0 CDL 10.0 CLL 0.0 * CDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.51 BC 0.64 WB 0.99 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d -0.16 20-22 >999 240 -0.25 20-22 >999 180 0.05 14 n/a n/a	PLATES GRIP MT20 244/190 Weight: 449 lb FT = 20%
UMBER- OP CHORD 2x6 SP No. OT CHORD 2x6 SP No. B2: 2x6 SP /EBS 2x4 SP No.	2 *Except* DSS		BRACING- TOP CHORD BOT CHORD WEBS	2 Rows at 1/3 pts 9-17 MiTek recommends that Stabil	
Max Horz 2 Max Uplift2	2=1335/0-3-8 (min. 0-1-12), 17=276 2=-150(LC 15) 2=-136(LC 14), 17=-108(LC 15), 14= 2=1502(LC 39), 17=3481(LC 45), 14	=-77(LC 15)	475/Mechanical		
OP CHORD 2-3=-2523 6-24=-166 26-27=-82 12-13=-41 OT CHORD 2-22=-255 31-32=-52 17-34=-40 /EBS 4-22=-310 8-19=-742	np./Max. Ten All forces 250 (lb) or 3/198, 3-4=-2430/227, 4-5=-2352/23 36/260, 7-24=-1529/289, 7-25=-825, 55/241, 9-27=-826/241, 9-10=0/842, 11/99, 13-14=-698/145 5/2121, 22-30=-140/1753, 21-30=-1- 2/1114, 19-32=-52/1114, 19-33=-40 06/229, 14-15=-60/568 0/170, 6-22=-79/542, 6-20=-1004/23 2/151, 9-19=-161/1721, 9-17=-1966, 3/635, 13-15=-444/175	38, 5-23=-2269/243, 6-23= /241, 8-25=-824/241, 8-26 , 10-28=0/1014, 11-28=0/4 40/1753, 20-21=-140/1753 6/229, 18-33=-406/229, 14 35, 7-20=-128/1268, 7-19=	=-2237/261,)=-825/241, 863, 12-29=-323/1 3, 20-31=-52/1114 8-34=-406/229, =-1001/131,		
OTES- (12-15)) Unbalanced roof live loa) Wind: ASCE 7-16; Vult= Roof; Hip Truss; MWFR 15-10-1 to 27-0-0, Interi 56-6-8 zone; end vertica grip DOL=1.60) TCLL: ASCE 7-16; Pr=2 Cat B; Partially Exp.; Ce) Unbalanced snow loads) This truss has been des	ads have been considered for this d =120mph (3-second gust) Vasd=95i RS (envelope) gable end zone and (ior(1) 27-0-0 to 28-6-1, Exterior(2R) al left and right exposed;C-C for me 20.0 psf (roof LL: Lum DOL=1.15 Pl ==1.0; Cs=1.00; Ct=1.10 s have been considered for this desi signed for greater of min roof live loa er live loads. age to prevent water ponding. 0 unless otherwise indicated. signed for a 10.0 psf bottom chord li esigned for a live load of 30.0psf on	lesign. mph; TCDL=5.0psf; BCDL 2-C Exterior(2E) -0-10-8 to 28-6-1 to 39-9-15, Interior mbers and forces & MWF ate DOL=1.15); Pf=20.0 p ign. ad of 12.0 psf or 2.00 time	L=5.0psf; h=23ft; C o 4-9-7, Interior(1) r(1) 39-9-15 to 50- RS for reactions s osf (Lum DOL=1.15 es flat roof load of 2	at. II; Exp B; Enclosed; Gable 4-9-7 to 15-10-1, Exterior(2R) 10-9, Exterior(2E) 50-10-9 to hown; Lumber DOL=1.60 plate 5 Plate DOL=1.15); Is=1.0; Rough 20.0 psf on overhangs	SEAL 28147

responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583

D'Onofrio Drive, Madison, WI 53719.

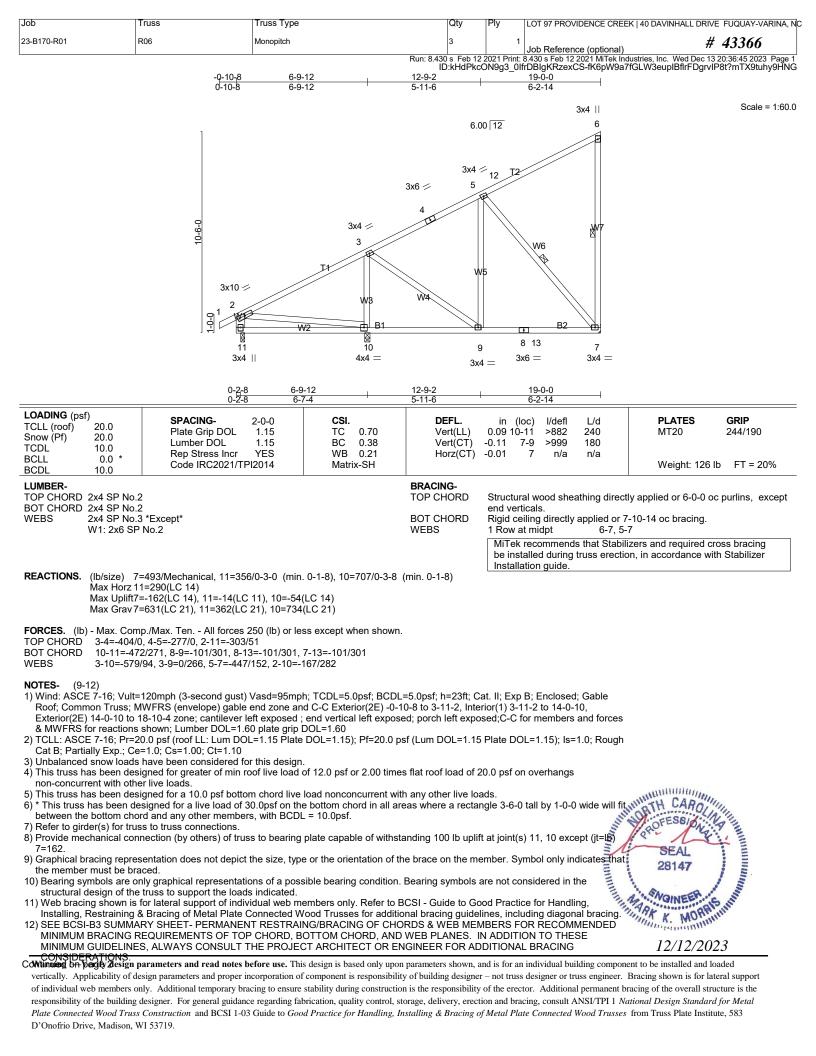
F	Job	Truss	Truss Type	Qty	Ply	LOT 97 PROVIDENCE CREEK 40 DAVINHALL DI	RIVE FUQUAY-VARINA, NC		
	23-B170-R01	R05	Piggyback Base	4	1	Job Reference (optional)	# 43366		
-	Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Dec 13 20:36:44 2023 Page 2 ID:kHdPkcON9g3 0lfrDBlgKRzexCS-B8YRIpaVuyDgRUJcIT8WJ2gY9Vt2PEXcFtPKMFy9HNH								

NOTES- (12-15)

- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14 except (jt=lb) 2=136, 17=108.
- 12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 13) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 15) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENCINEER FOR ADDITIONAL DEACING CONCEPTENTIONS. ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





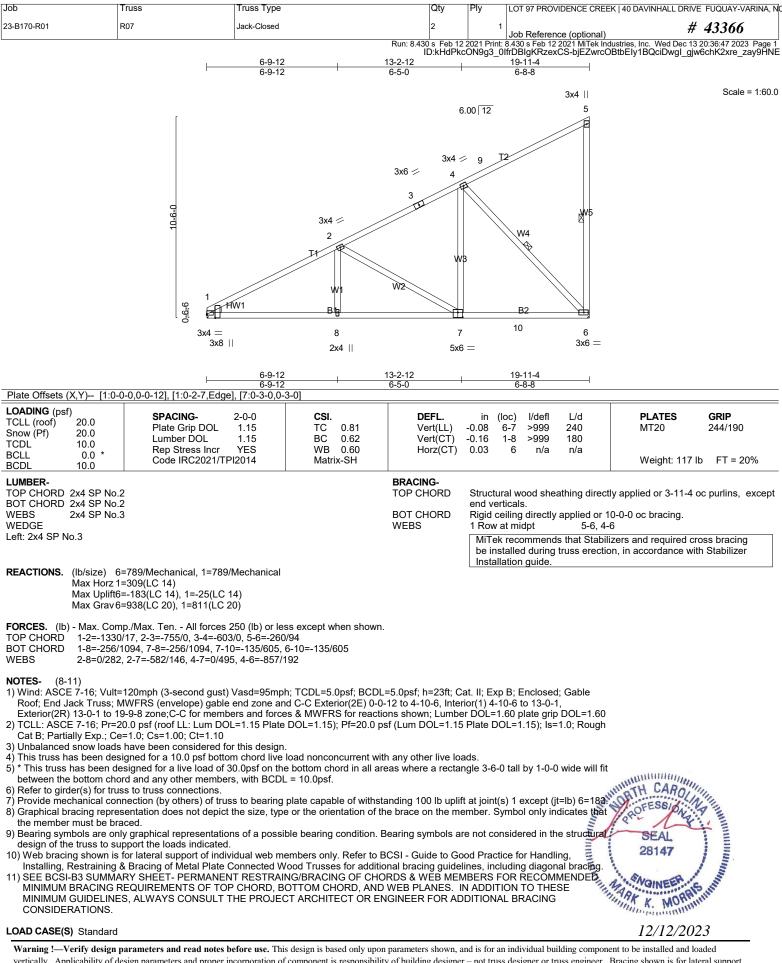
Job	Truss	Truss Type	Qty	Ply	LOT 97 PROVIDENCE CREEK 40 D/	AVINHALL DRIVE FUQUAY-VARINA, NC
23-B170-R01	R06	Monopitch	3	1	Job Reference (optional)	# 43366
			Rup: 8/130 s Eeb 12.3	021 Print	8 430 s Eeb 12 2021 MiTek Industries	nc Wed Dec 13 20:36:45 2023 Page 2

ID:kHdPkcON9g3_0lfrDBlgKRzexCS-fK6pW9a7fGLW3euplBflrFDgrvIP8t?mTX9tuhy9HNG

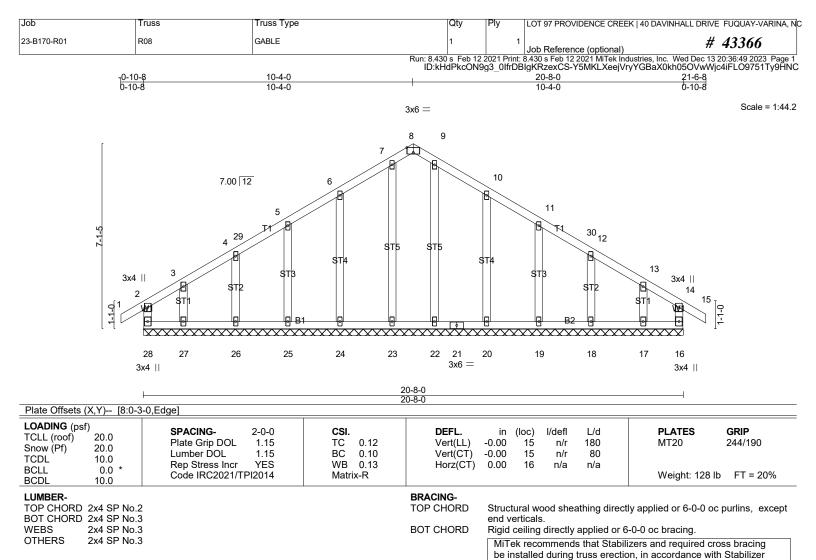
LOAD CASE(S) Standard



12/12/2023



vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Installation guide.

REACTIONS.

All bearings 20-8-0. (lb) - Max Horz 28=152(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 28, 16, 24, 25, 26, 27, 20, 19, 18, 17

Max Grav All reactions 250 lb or less at joint(s) 28, 16, 23, 25, 26, 27, 22, 19, 18, 17 except 24=302(LC 5),

20=302(LC 6)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(14-17)

- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 5-6-4, Corner(3R) 5-6-4 to 15-1-12, Exterior(2N) 15-1-12 to 16-8-14, Corner(3E) 16-8-14 to 21-6-8 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough
- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads. 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 10) Gable studs spaced at 2-0-0 co.
 11) This truss has been designed for a 10.0 psf bottom chord live load from the bottom chord in all areas where a requirement of the bottom chord and any other members, with BCDL = 10.0psf.
 12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a requirement of the bottom chord and any other members, with BCDL = 10.0psf.
 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 28, 16, 24, 25, 26, 27, 20, 19, 18, 17.

Job	Truss	Truss Type	Qty	Ply	LOT 97 PROVIDENCE CREEK 40 DAVINHAL	L DRIVE FUQUAY-VARINA, NC
23-B170-R01	R08	GABLE	1	1	Job Reference (optional)	# 43366
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Dec 13 20:36:50 2023				Dec 13 20:36:50 2023 Page 2		

12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. (15) Bearing symbols are not considered in the structural design of the truss to support the

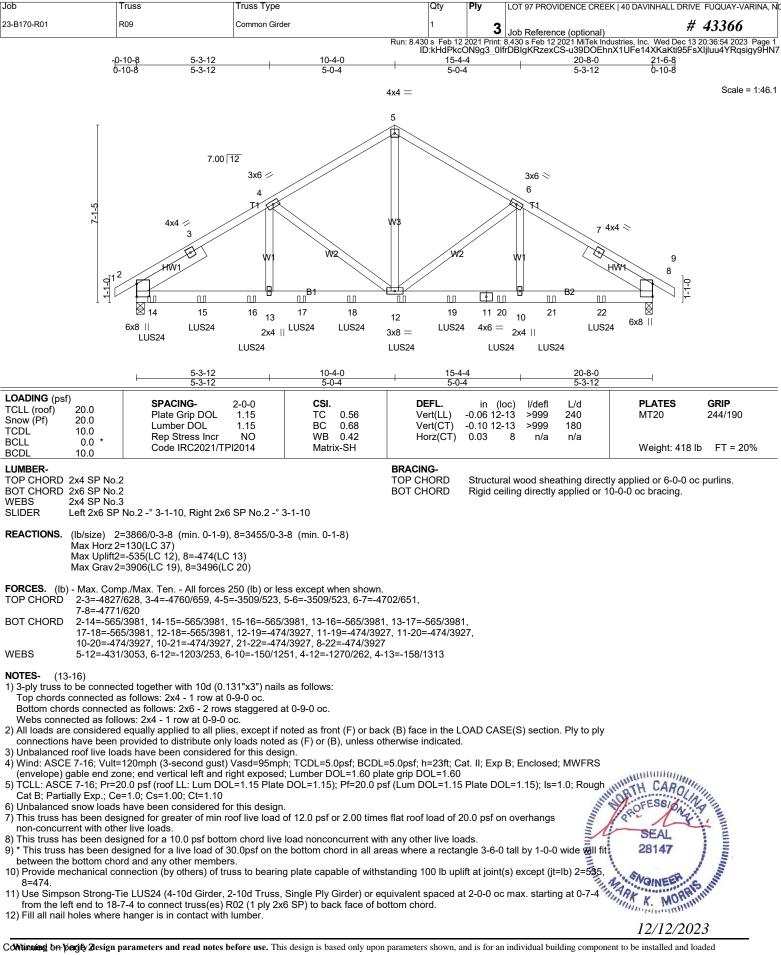
 bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

10) Web blacing shown is to rate a support of individual web members only. Refer to BCS1 - Guide to Good Practice for Particle for Part

LOAD CASE(S) Standard

ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.





Job	Truss	Truss Type	Qty	Ply	LOT 97 PROVIDENCE CREEK 40 DAVINH	ALL DRIVE FUQUAY-VARINA, NC
23-B170-R01	R09	Common Girder	1	3	Job Reference (optional)	# 43366
		Run: 8.43	0 s Feb 12	2021 Print:	8.430 s Feb 12 2021 MiTek Industries, Inc. W	ed Dec 13 20:36:54 2023 Page 2

ID:kHdPkcON9g3_0lfrDBlgKRzexCS-u39DOEhnX1UFe14XKaKti95FsXljluu4YRqsigy9HN7 13) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

15) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 16) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

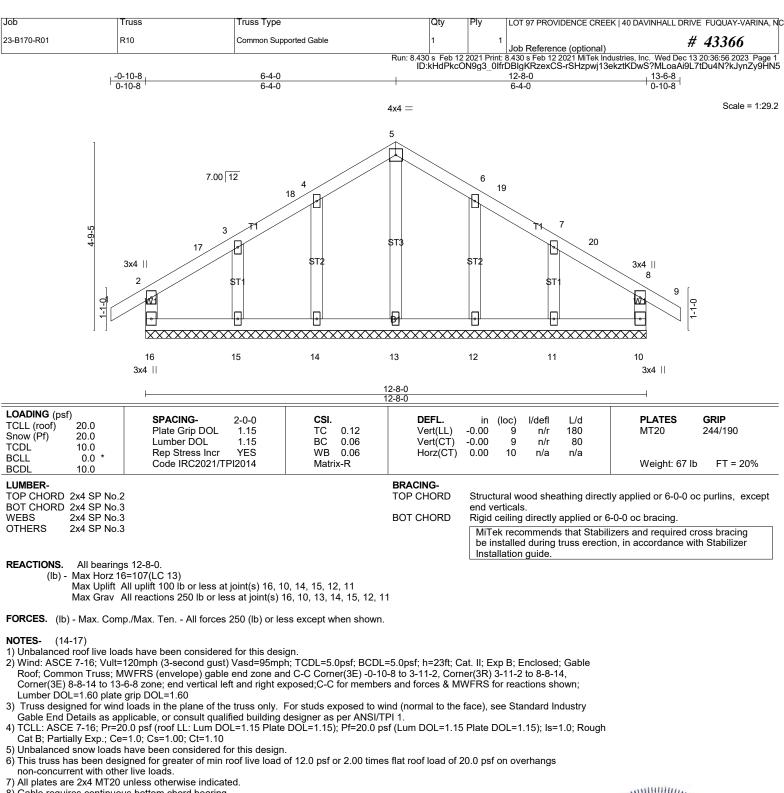
Uniform Loads (plf) Vert: 1-5=-60, 5-9=-60, 2-8=-20

Concentrated Loads (lb)

Vert: 12=-556(B) 14=-560(B) 15=-556(B) 16=-556(B) 17=-556(B) 18=-556(B) 19=-556(B) 20=-556(B) 21=-556(B) 22=-556(B)



12/12/2023



- Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 11.

SEAL 28147 MONEER C. MORRES 12/20

Job	Truss	Truss Type	Qty	Ply	LOT 97 PROVIDENCE CREEK 40 DAVINHALL DRIVE FUQUAY-VARINA, I	NC
23-B170-R01	R10	Common Supported Gable	1	1	Job Reference (optional) # 43366	
					8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Dec 13 20:36:56 2023 Page : DBIgKRzexCS-rSHzpwj13ekztKDwS?MLoaAi9L7tDu4N?kJynZy9HN	

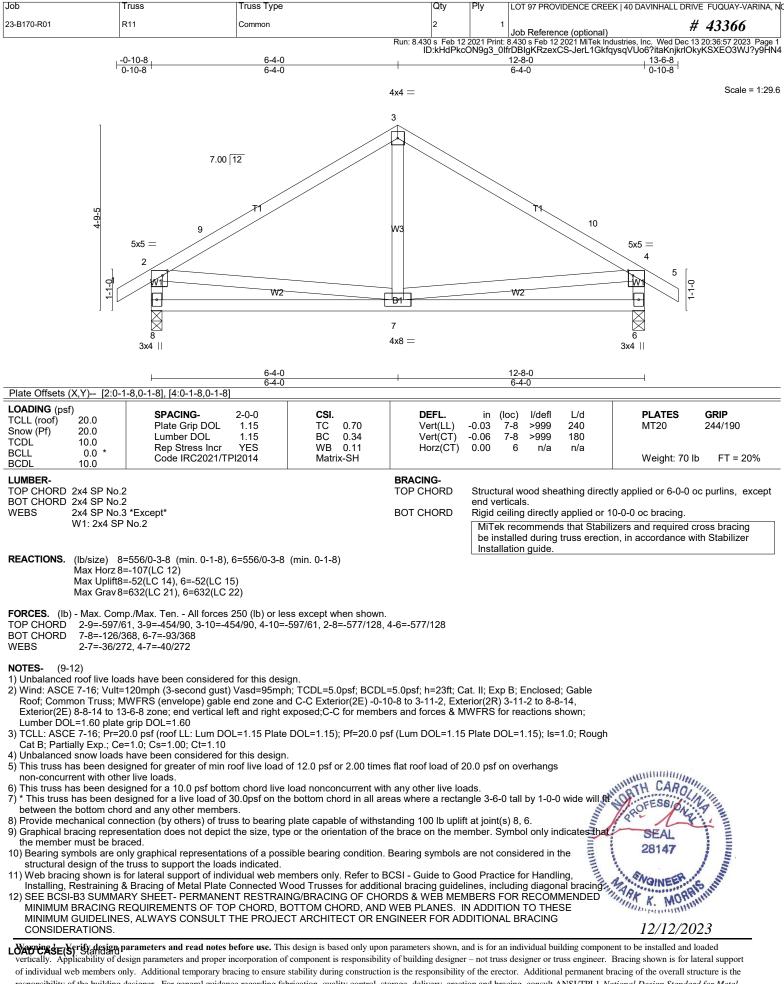
14) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

16) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

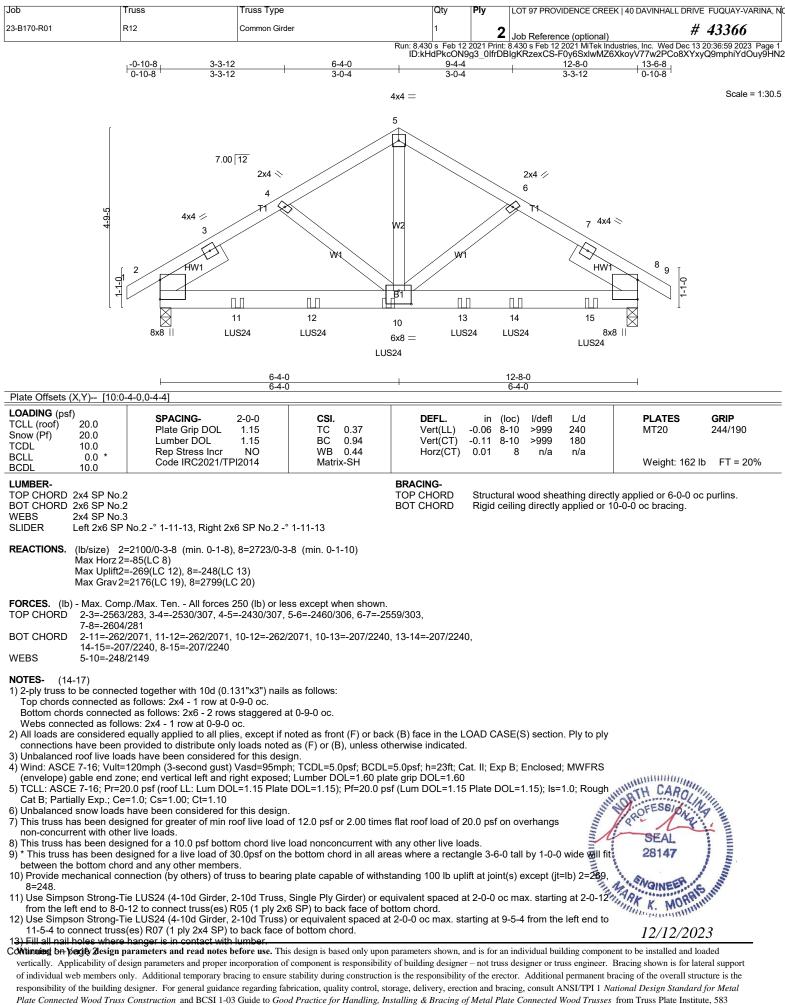
Connected Wood Trustees for additional bracing guidelines, including diagonal bracing. 17) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 97 PROVIDENCE CREEK 40 DAVINHA	ALL DRIVE FUQUAY-VARINA, NC
23-B170-R01	R12	Common Girder	1	2	Job Reference (optional)	# 43366
	Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Dec 13 20:37:00 2023 Pa				ed Dec 13 20:37:00 2023 Page 2	

ID:kHdPkcON9g3_0lfrDBlgKRzexCS-jDWUfHmY7tEOMyXhhqRHyQLJHyHB9c0zwMHAwKy9HN1

- 14) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 16) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 17) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS
- OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

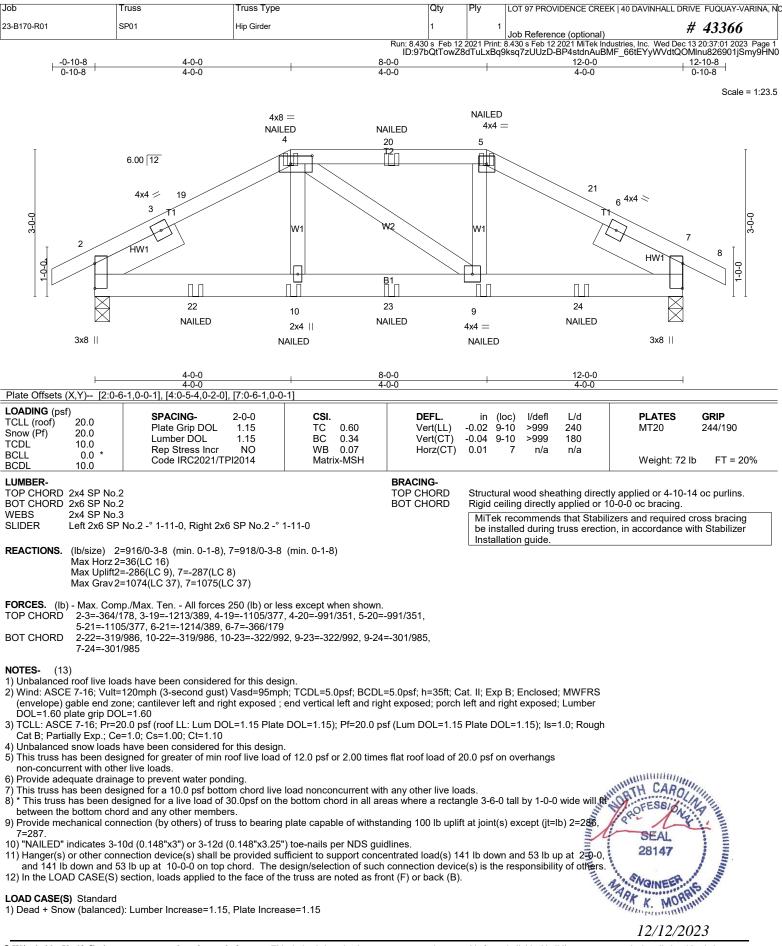
Uniform Loads (plf) Vert: 1-5=-60, 5-9=-60, 2-8=-20

Concentrated Loads (lb)

Vert: 10=-530(B) 11=-530(B) 12=-530(B) 13=-530(B) 14=-791(B) 15=-791(B)



12/12/2023



Job	Truss	Truss Type	Qty	Ply	LOT 97 PROVIDENCE CREEK 40 DAVINHA	ALL DRIVE FUQUAY-VARINA, NC
23-B170-R01	SP01	Hip Girder	1	1	Job Reference (optional)	# 43366
	Dun: 9,420 a. Eab 12,2021 Drint: 9,420 a. Eab 12,2021 MiTak Industrias, Inc., Mod Dec 12,2027;01,2022, Dage 2,					

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 Mi Tek Industries, Inc. Wed Dec 13 20:37:01 2023 Page 2 ID:97bQtTowZ8dTuLxBq9ksq7zUUzD-BP4stdnAuBMF_66tEYyWVdtQOMInu826901jSmy9HN0

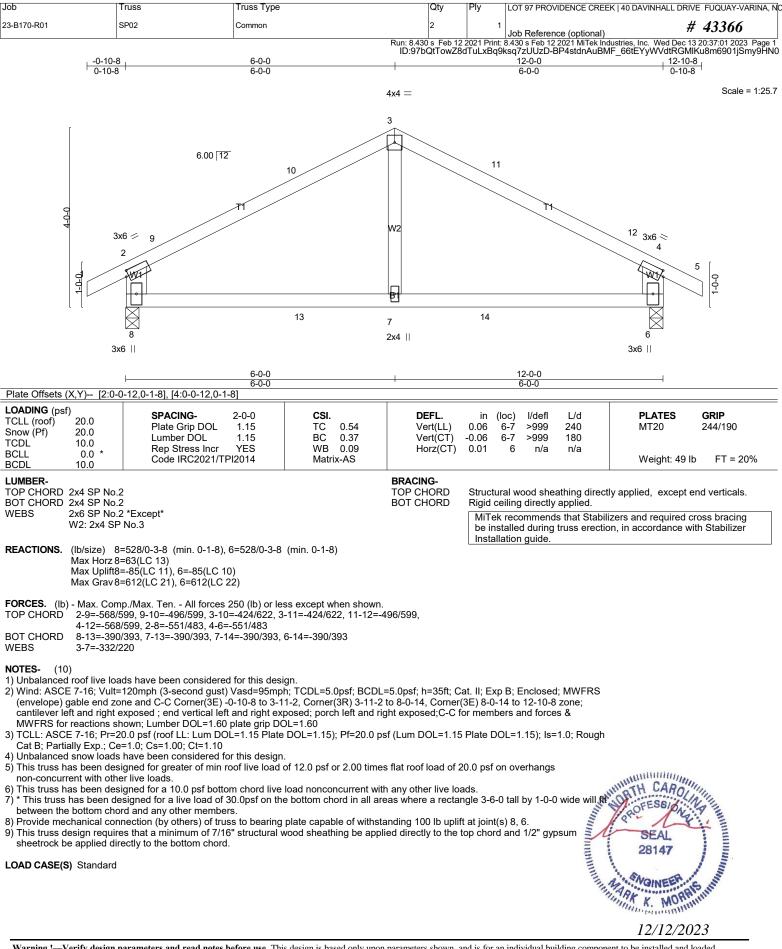
LOAD CASE(S) Standard

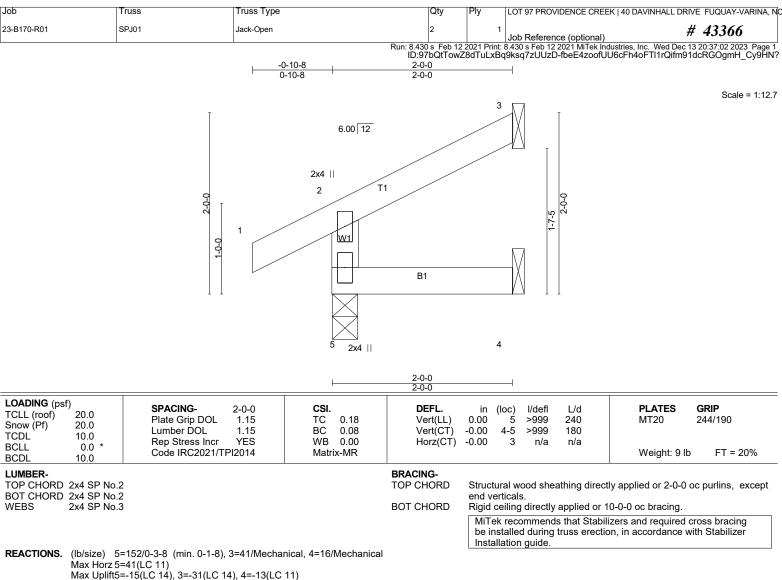
Uniform Loads (plf) Vert: 1-4=-60, 4-5=-60, 5-8=-60, 11-15=-20

Concentrated Loads (lb) Vert: 4=-94(B) 5=-94(B) 10=-27(B) 9=-27(B) 19=-139(F) 20=-94(B) 21=-139(F) 22=-64(B) 23=-27(B) 24=-64(B)



12/12/2023





Max Grav 5=208(LC 21), 3=57(LC 21), 4=34(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone; cantilever left and right exposed ; end vertical left and right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

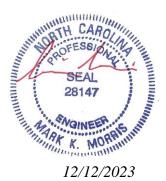
6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

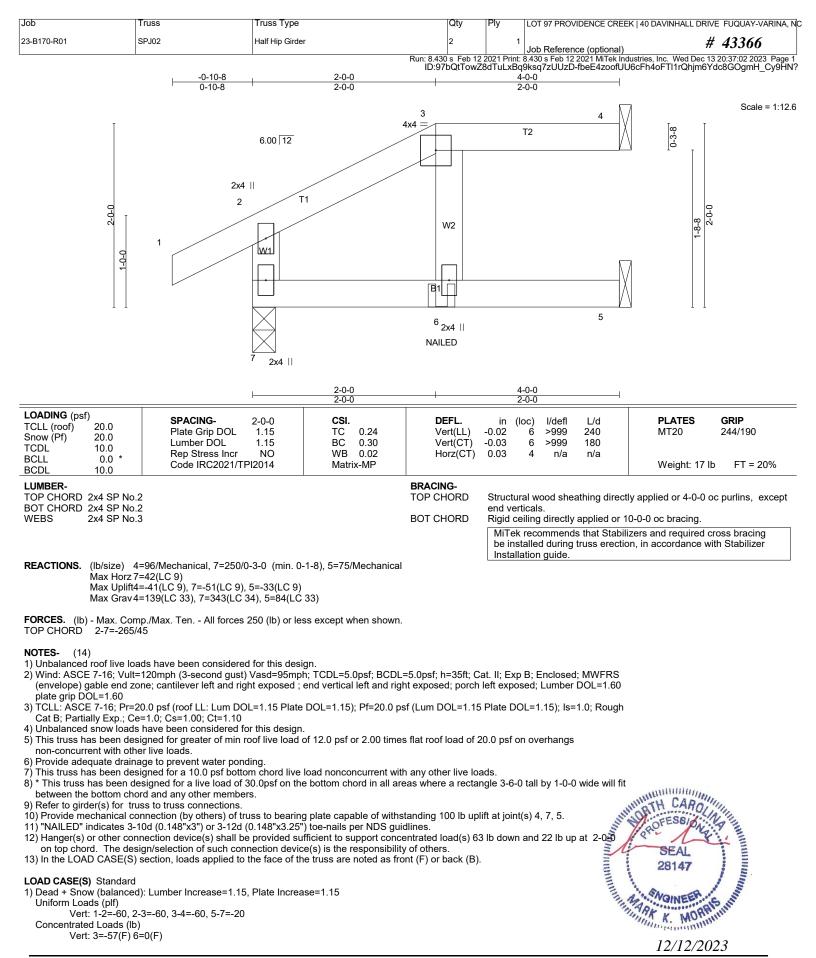
between the bottom chord and any other members.

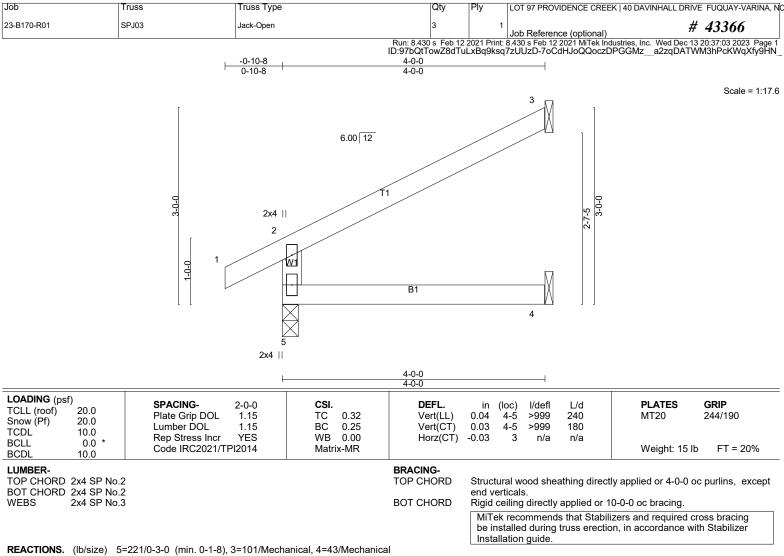
7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3, 4.

LOAD CASE(S) Standard







Max Horz 5=71(LC 14)

Max Uplift5=-20(LC 11), 3=-60(LC 14), 4=-19(LC 11)

Max Grav 5=322(LC 21), 3=154(LC 21), 4=72(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-5=-293/119

NOTES- (9)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members. 7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3, 4.

LOAD CASE(S) Standard

