

Trenco 818 Soundside Rd Edenton, NC 27932

Re: FNC92-R Chesapeake-6260A:Lot92 FarmNeillsCreek

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource-Apex,NC.

Pages or sheets covered by this seal: I62330698 thru I62330730

My license renewal date for the state of North Carolina is December 31, 2023.

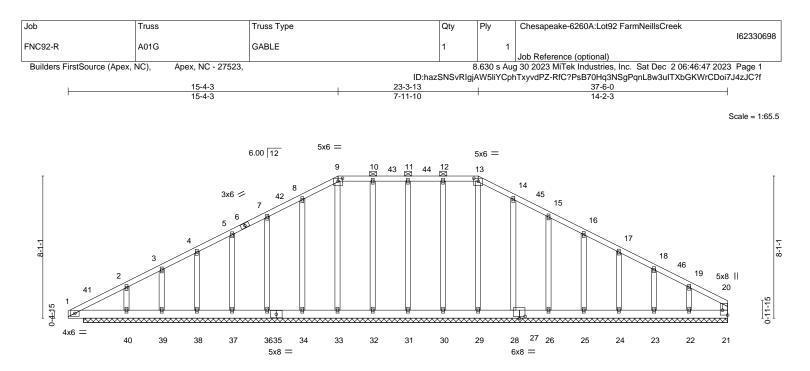
North Carolina COA: C-0844



December 5,2023

# Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



0-10-8 late Offsets (X,Y) [	9:0-3-0,0-2-0], [13:0-3-0,0-2-0], [20:Ed	ge,0-3-8], [28:0-3-12,0-1-4	36-7-8 4]					
OADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.23	Vert(LL) n/	a -	n/a	999	MT20	244/190
CDL 10.0	Lumber DOL 1.15	BC 0.17	Vert(CT) n/	a -	n/a	999		
CLL 0.0 *	Rep Stress Incr YES	WB 0.18	Horz(CT) -0.0	0 21	n/a	n/a		
CDL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 273 lb	FT = 20%

TOF CHORD	2X4 3F N0.2	TOF CHORD	Structural wood sheatining directly applied of 10-0-0 oc putilits,
BOT CHORD	2x6 SP No.2		except end verticals, and 2-0-0 oc purlins (10-0-0 max.): 9-13.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
OTHERS	2x4 SP No.3		

### REACTIONS. All bearings 36-7-8.

(lb) - Max Horz 40=113(LC 12)

Max Uplift All uplift 100 b or less at joint(s) 30, 31, 32, 34, 36, 37, 38, 39, 40, 28, 26, 25, 24, 23, 22, 21 Max Grav All reactions 250 lb or less at joint(s) 29, 30, 31, 32, 33, 34, 36, 37, 38, 39, 28, 26, 25, 24, 23, 22, 21 except 40=424(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 15-4-3, Exterior(2) 15-4-3 to 18-4-3, Interior(1) 18-4-3 to 23-3-13, Exterior(2) 23-3-13 to 26-3-13, Interior(1) 26-3-13 to 37-4-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Provide adequate drainage to prevent water ponding.

All plates are 2x4 MT20 unless otherwise indicated.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 30, 31, 32, 34, 36, 37, 38, 39, 40, 28, 26, 25, 24, 23, 22, 21.

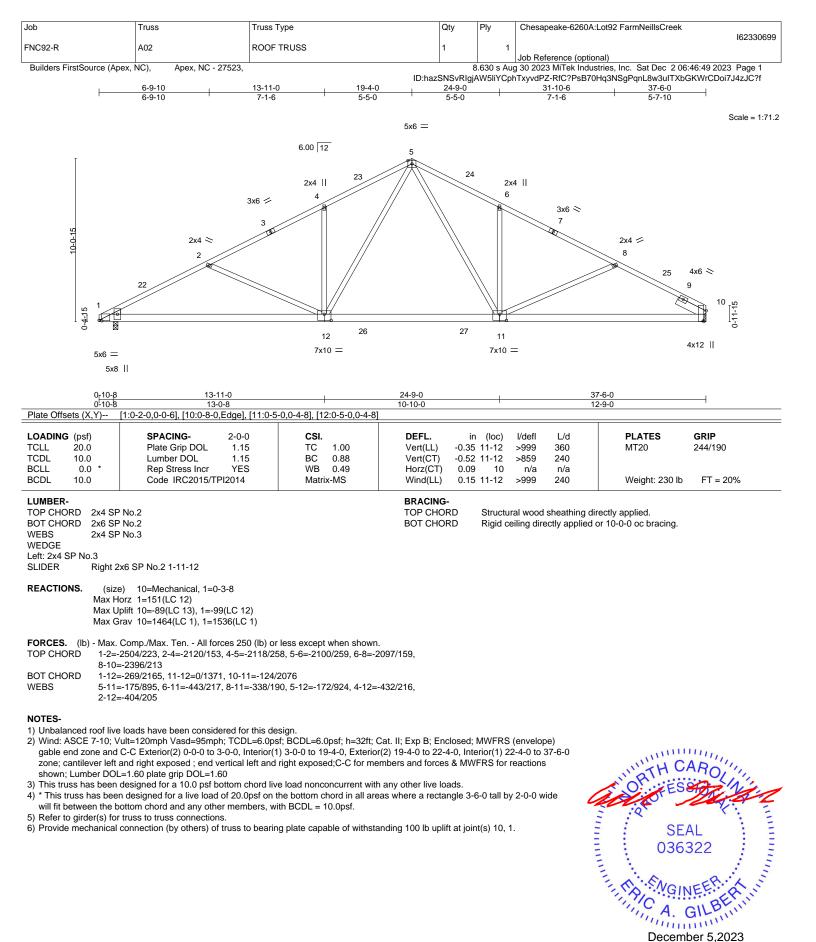
10) Non Standard bearing condition. Review required.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

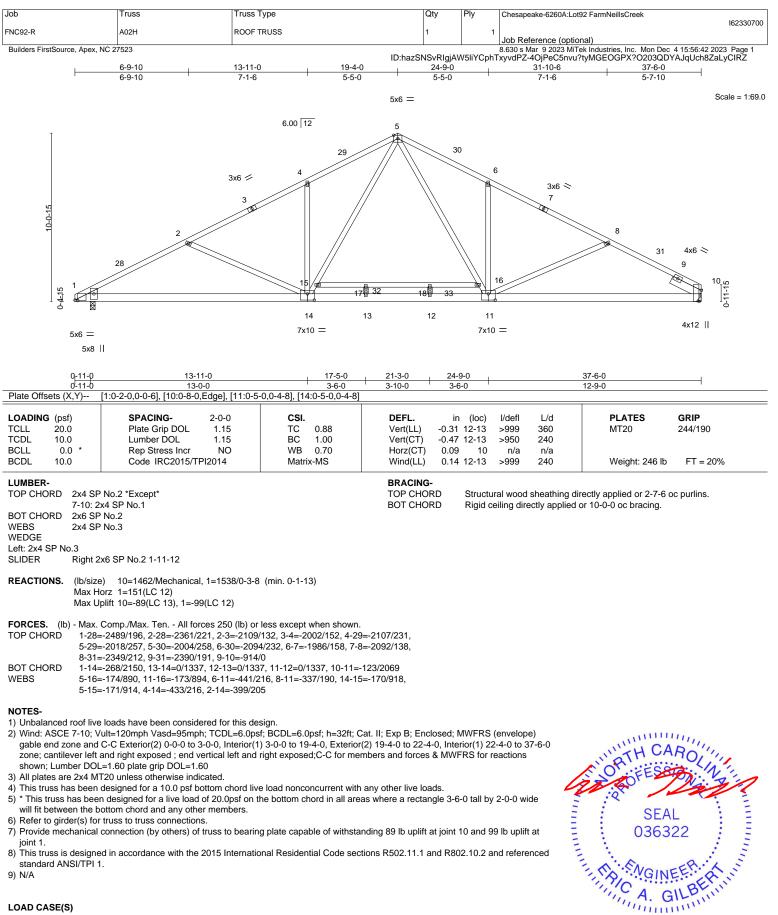


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

A MiTek / 818 Soundside Road



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## LOAD CASE(S)

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818 Soundside Road Edenton, NC 27932

December 5,2023

[	Job	Truss	Truss Type	Qty	Ply	Chesapeake-6260A:Lot92 FarmNeillsCreek
	FNC92-R	A02H	ROOF TRUSS	1	1	162330700
						Job Reference (optional)
-	Builders FirstSource, Apex, NC 27	7523	ID:hazS	NSvRIgjAV		8.630 s Mar 9 2023 MiTek Industries, Inc. Mon Dec 4 15:56:42 2023 Page 2 xyvdPZ-4OjPeC5nvu?tyMGEOGPX?O203QDYAJqUch8ZaLyCIRZ

Builders FirstSource, Apex, NC 27523	8.630 s Mar 9 2023 MiTek Industries, Inc. Mon Dec 4 15:56:42
	ID:hazSNSvRIgjAW5liYCphTxyvdPZ-40jPeC5nvu?tyMGEOGPX?0203QDYAJqUch8Z
LOAD CASE(S)	
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15	
Uniform Loads (plf)	
Vert: 1-5=-60, 5-10=-60, 19-23=-20	
<ol><li>Dead + 0.75 Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15</li></ol>	
Uniform Loads (plf)	
Vert: 1-5=-50, 5-10=-50, 19-23=-20, 32-33=-30	
3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.2	25
Uniform Loads (plf)	
Vert: 1-5=-20, 5-10=-20, 19-23=-40, 32-33=-40	
4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.	.60
Uniform Loads (plf)	
Vert: 1-28=25, 5-28=14, 5-30=25, 10-30=14, 19-23=-12	
Horz: 1-28=-37, 5-28=-26, 5-30=37, 10-30=26	
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.	.60
Uniform Loads (plf)	
Vert: 1-29=14, 5-29=25, 5-31=14, 10-31=25, 19-23=-12	
Horz: 1-29=-26, 5-29=-37, 5-31=26, 10-31=37	
6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1	.60
Uniform Loads (plf)	
Vert: 1-5=-33, 5-10=-33, 19-23=-20	
Horz: 1-5=13, 5-10=-13	
7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1	.60
Uniform Loads (plf)	
Vert: 1-5=-33, 5-10=-33, 19-23=-20	
Horz: 1-5=13, 5-10=-13	
8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase="	1.60
Uniform Loads (plf)	
Vert: 1-5=-2, 5-10=9, 19-23=-12	
Horz: 1-5=-10, 5-10=21	
9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=	=1.60
Uniform Loads (plf)	
Vert: 1-5=9, 5-10=-2, 19-23=-12	
Horz: 1-5=-21, 5-10=10	
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase	=1.60
Uniform Loads (plf)	
Vert: 1-5=-20, 5-10=-9, 19-23=-20	
Horz: 1-5=-0, 5-10=11	
11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increas	e=1.60
Uniform Loads (plf)	
Vert: 1-5=-9, 5-10=-20, 19-23=-20	
Horz: 1-5=-11, 5-10=0	
12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate In	icrease=1.60
Uniform Loads (plf)	
Vert: 1-5=22, 5-10=7, 19-23=-12	
Horz: 1-5=-34, 5-10=19	
<ol> <li>Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase</li> </ol>	ncrease=1.60
Uniform Loads (plf)	
Vert: 1-5=7, 5-10=22, 19-23=-12	
Horz: 1-5=-19, 5-10=34	
14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Ir	crease=1.60
Uniform Loads (plf)	
Vert: 1-5=11, 5-10=3, 19-23=-12	
Horz: 1-5=-23, 5-10=15	
15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate In	icrease=1.60
Uniform Loads (plf)	
Vert: 1-5=3, 5-10=11, 19-23=-12	
Horz: 1-5=-15, 5-10=23	

Horz: 1-5=-15, 5-10=23

16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-5=4. 5-10=-11. 19-23=-20

Horz: 1-5=-24, 5-10=9

17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-5=-11, 5-10=4, 19-23=-20

Horz: 1-5=-9, 5-10=24

18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90

Uniform Loads (plf)

Vert: 1-5=-20, 5-10=-20, 19-23=-20, 32-33=-40

19) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

tinued on page 3

Vert: 1-5=-50, 5-10=-42, 19-23=-20, 32-33=-30

Horz: 1-5=-0, 5-10=8

20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60

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Job	Truss	Truss Type	Qty	Ply	Chesapeake-6260A:Lot92 FarmNeillsCreek
FNC92-R	A02H	ROOF TRUSS	1	1	162330700
FINC 92-N	AUZH	ROOF IR033	1	'	Job Reference (optional)

Builders FirstSource, Apex, NC 27523

8.630 s Mar 9 2023 MiTek Industries, Inc. Mon Dec 4 15:56:42 2023 Page 3 ID:hazSNSvRIgjAW5liYCphTxyvdPZ-40jPeC5nvu?tyMGEOGPX?0203QDYAJqUch8ZaLyCIRZ

### LOAD CASE(S)

#### Uniform Loads (plf)

Vert: 1-5=-42, 5-10=-50, 19-23=-20, 32-33=-30

Horz: 1-5=-8, 5-10=0

21) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-5=-32, 5-10=-43, 19-23=-20, 32-33=-30 Horz: 1-5=-18, 5-10=7

22) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

- Vert: 1-5=-43, 5-10=-32, 19-23=-20, 32-33=-30 Horz: 1-5=-7, 5-10=18
- 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
  - Vert: 1-5=-60, 5-10=-20, 19-23=-20
- 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

- Vert: 1-5=-20, 5-10=-60, 19-23=-20
- 25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
- Vert: 1-5=-50, 5-10=-20, 19-23=-20, 32-33=-30

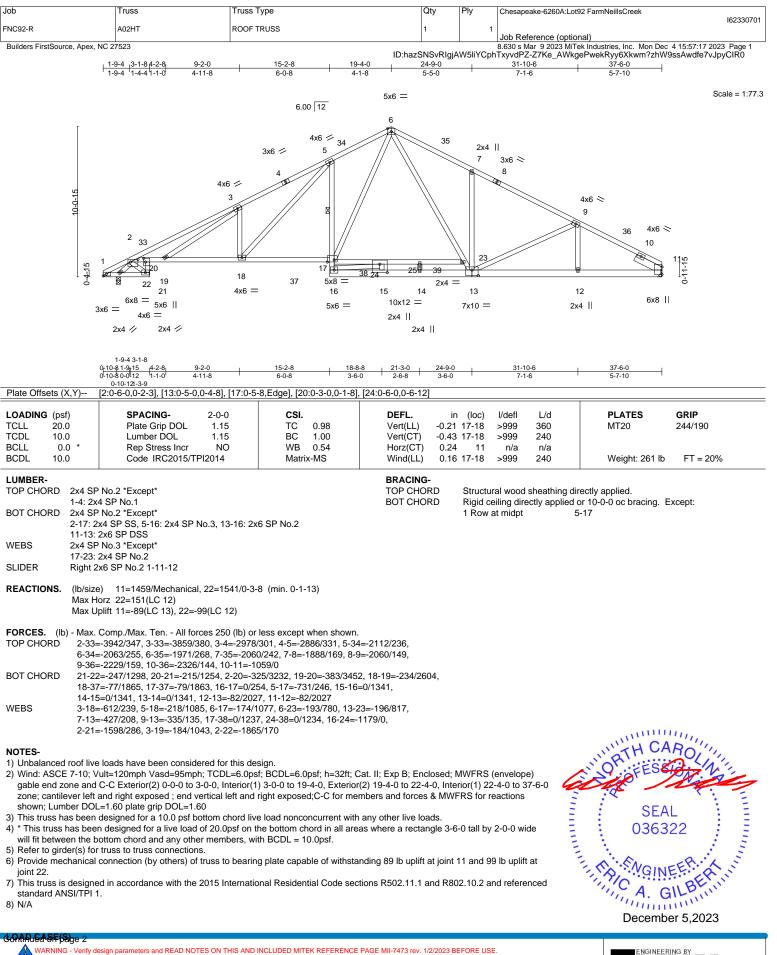
26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-20, 5-10=-50, 19-23=-20, 32-33=-30

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JOD	Truss	Truss Type	Qty	Ply	Chesapeake-6260A:Lot92 FarmNeillsCreek	162330701
FNC92-R	A02HT	ROOF TRUSS	1	1	Job Reference (optional)	102000701
Builders FirstSource, Apex, NC 2	7523	lD-bc			8.630 s Mar 9 2023 MiTek Industries, Inc. Mon Dec 4 15:57:17 20; TxyvdPZ-Z7Ke_AWkgePwekRyy6Xkwm?zhW9ssAwdfe7vJ	
		ID.Ha	ZONOVRIYJA	wonrepn		JYCIKU
LOAD CASE(S)		Dista la successión de da				
<ol> <li>Dead + Roof Live (balan Uniform Loads (plf)</li> </ol>	ced): Lumber Increase=1.15	Plate Increase=1.15				
	-11=-60, 21-26=-20, 17-20=-	20, 16-29=-20				
<ol> <li>Dead + 0.75 Roof Live ( Uniform Loads (plf)</li> </ol>	balanced) + 0.75 Uninhab. At	tic Storage: Lumber Increase=1.15, Plate Incr	ease=1.15			
	-11=-50, 21-26=-20, 20-37=-	20, 17-37=-50, 16-29=-20, 38-39=-30				
	ttic Without Storage: Lumber	Increase=1.25, Plate Increase=1.25				
Uniform Loads (plf) Vert: 1-6=-20_6	-11=-20, 21-26=-40, 17-20=-	40 16-29=-40 38-39=-40				
		Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	3-33-14 6-35-25 11-35-14	22-26=18, 21-22=-12, 17-20=-12, 16-29=-12				
	6-33=-26, 6-35=37, 11-35=2					
,	os. Internal) Case 2: Lumber	Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf) Vert: 1-34=14, 6	6-34=25, 6-36=14, 11-36=25,	22-26=18, 21-22=-12, 17-20=-12, 16-29=-12				
	6-34=-37, 6-36=26, 11-36=3					
<ol> <li>Dead + 0.6 C-C Wind (N Uniform Loads (plf)</li> </ol>	leg. Internal) Case 1: Lumber	Increase=1.60, Plate Increase=1.60				
	-11=-33, 22-26=-15, 21-22=-	20, 17-20=-20, 16-29=-20				
Horz: 1-6=13, 6		Increase 1.60 Plate Increase 1.60				
Uniform Loads (plf)	leg. Internal) Case 2. Lumber	Increase=1.60, Plate Increase=1.60				
	-11=-33, 22-26=-15, 21-22=-	20, 17-20=-20, 16-29=-20				
Horz: 1-6=13, 6 8) Dead + 0.6 MWFRS Wir		r Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	, , , , , , , , , , , , , , , , , , ,					
Vert: 1-6=-2, 6- Horz: 1-6=-10, 6	11=9, 22-26=4, 21-22=-12, 1 <sup>-</sup> 5-11=21	7-20=-12, 16-29=-12				
9) Dead + 0.6 MWFRS Wir		per Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf) Vert: 1-6=9, 6-1	1=-2, 21-26=-12, 17-20=-12,	16-29=-12				
Horz: 1-6=-21, 6	6-11=10					
10) Dead + 0.6 MWFRS W Uniform Loads (plf)	(ind (Neg. Internal) Left: Lum	per Increase=1.60, Plate Increase=1.60				
Vert: 1-6=-20,	6-11=-9, 22-26=-4, 21-22=-2	0, 17-20=-20, 16-29=-20				
Horz: 1-6=-0, ( 11) Dead + 0.6 MWERS W		nber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)						
Vert: 1-6=-9, 6 Horz: 1-6=-11,	6-11=-20, 21-26=-20, 17-20=- 6-11=0	20, 16-29=-20				
12) Dead + 0.6 MWFRS W		el: Lumber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	6-11=7, 21-26=-12, 17-20=-1	2 16 20- 12				
Horz: 1-6=-34		2, 10-2312				
,	ind (Pos. Internal) 2nd Parall	el: Lumber Increase=1.60, Plate Increase=1.6	C			
Uniform Loads (plf) Vert: 1-6=7, 6-	-11=22, 21-26=-12, 17-20=-1	2, 16-29=-12				
Horz: 1-6=-19		al Lumber Increase 4.00 Dista Increase 4.00	<b>`</b>			
Uniform Loads (plf)	inu (FOS. Internal) siu Paralie	el: Lumber Increase=1.60, Plate Increase=1.60	)			
	6-11=3, 21-26=-12, 17-20=-1	2, 16-29=-12				
Horz: 1-6=-23 15) Dead + 0.6 MWFRS W		el: Lumber Increase=1.60, Plate Increase=1.60	)			
Uniform Loads (plf)	44 44 04 00 40 47 00 4	2 40 00 40				
Vert: 1-6=3, 6- Horz: 1-6=-15	·11=11, 21-26=-12, 17-20=-1 , 6-11=23	2, 16-29=-12				
,	ind (Neg. Internal) 1st Paralle	el: Lumber Increase=1.60, Plate Increase=1.60	)			
Uniform Loads (plf) Vert: 1-6=4, 6-	-11=-11, 21-26=-20, 17-20=-2	20, 16-29=-20				
Horz: 1-6=-24	, 6-11=9		•			
17) Dead + 0.6 MWFRS W Uniform Loads (plf)	ind (Neg. Internal) 2nd Paral	el: Lumber Increase=1.60, Plate Increase=1.6	0			
Vert: 1-6=-11,	6-11=4, 21-26=-20, 17-20=-2	20, 16-29=-20				
Horz: 1-6=-9, ( 18) Dead + Uninhabitable		se=1.25, Plate Increase=1.25				
Uniform Loads (plf)	Ū					
		20, 17-37=-60, 16-29=-20, 38-39=-40 Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Le	t): Lumber	Increase=	1.60. Plate	
Increase=1.60						
Uniform Loads (plf) Vert: 1-6=-50.	6-11=-42, 22-26=-8 21-22	20, 20-37=-20, 17-37=-50, 16-29=-20, 38-39=	-30			
Horz: 1-6=-0,	6-11=8					
20) Dead + 0.75 Roof Live	(bal.) + 0.75 Uninhab. Attic S	storage + 0.75(0.6 MWFRS Wind (Neg. Int) Rig	ght): Lumbe	r Increase	e=1.60, Plate	

Qty

Ply

Chesapeake-6260A:Lot92 FarmNeillsCreek

20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60

Job

Truss

Truss Type





Job	Truss	Truss Type	Qty	Ply	Chesapeake-6260A:Lot92 FarmNeillsCreek
					162330701
FNC92-R	A02HT	ROOF TRUSS	1	1	
					Job Reference (optional)
Builders FirstSource, Apex, NC	27523				8,630 s Mar 9 2023 MiTek Industries, Inc. Mon Dec 4 15:57:17 2023 Page 3

Builders FirstSource, Apex, NC 27523

ID:hazSNSvRIgjAW5liYCphTxyvdPZ-Z7Ke\_AWkgePwekRyy6Xkwm?zhW9ssAwdfe7vJpyCIR0

### LOAD CASE(S)

#### Uniform Loads (plf)

Vert: 1-6=-42, 6-11=-50, 21-26=-20, 20-37=-20, 17-37=-50, 16-29=-20, 38-39=-30

Horz: 1-6=-8, 6-11=0

21) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-6=-32, 6-11=-43, 21-26=-20, 20-37=-20, 17-37=-50, 16-29=-20, 38-39=-30

Horz: 1-6=-18. 6-11=7

22) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-6=-43, 6-11=-32, 21-26=-20, 20-37=-20, 17-37=-50, 16-29=-20, 38-39=-30

Horz: 1-6=-7, 6-11=18 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-60, 6-11=-20, 21-26=-20, 17-20=-20, 16-29=-20

24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-6=-20, 6-11=-60, 21-26=-20, 17-20=-20, 16-29=-20

25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-50, 6-11=-20, 21-26=-20, 20-37=-20, 17-37=-50, 16-29=-20, 38-39=-30

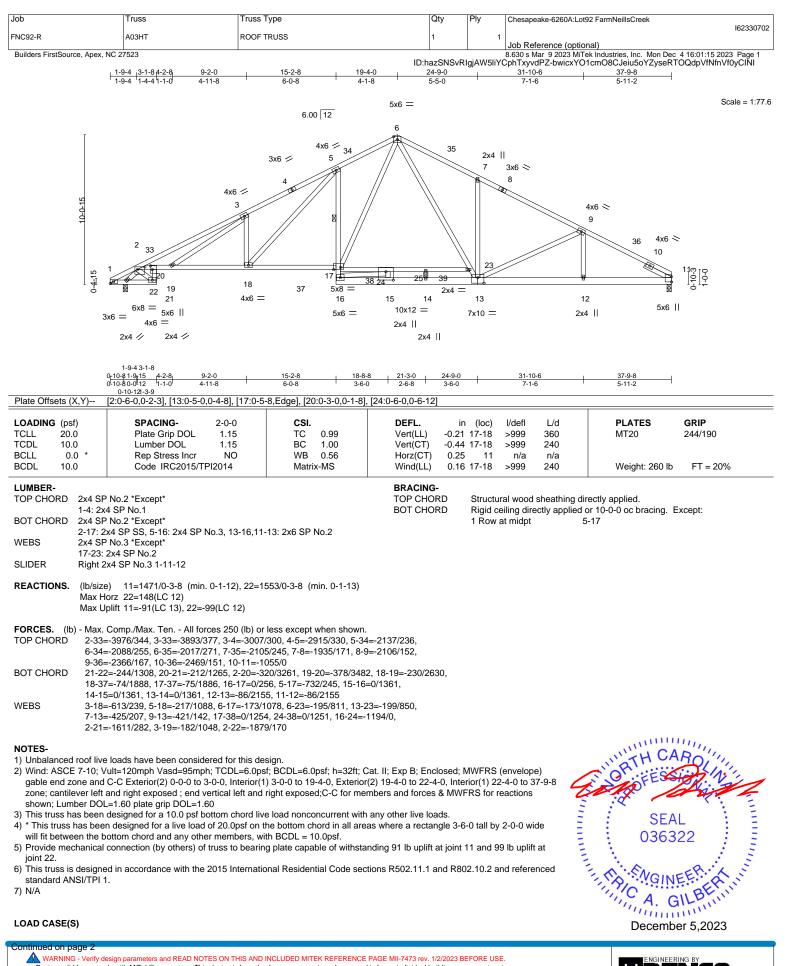
26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-6=-20, 6-11=-50, 21-26=-20, 20-37=-20, 17-37=-50, 16-29=-20, 38-39=-30

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancing Component Advancing Component Advancing and PCB and Component Advancing Component Compone and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





Design valid for use only with MTeVe® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlages with possible personal injury and property damage. For general guidance regarding the tabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Chesapeake-6260A:Lot92 FarmNeillsCreek	62330702
FNC92-R	A03HT	ROOF TRUSS	1	1		12000102
Builders FirstSource, Apex, NC 2	7523				Job Reference (optional) 8.630 s Mar 9 2023 MiTek Industries, Inc. Mon Dec 4 16:01:15 2023 F	
		ID:h	azSNSvRI		CphTxyvdPZ-bwicxYO1cmO8CJeiu5oYZyseRTOQdpVfNfnVf0y	
LOAD CASE(S)						
	ced): Lumber Increase=1.15,	Plate Increase=1.15				
Uniform Loads (plf)						
	-11=-60, 21-26=-20, 17-20=-2 balanced) + 0 75 Uninhab At	20, 16-29=-20 tic Storage: Lumber Increase=1.15, Plate Increa	se-1 15			
Uniform Loads (plf)		tic Storage. Lumber increase=1.13, 1 late increa	36-1.15			
		20, 17-37=-50, 16-29=-20, 38-39=-30				
,	ttic Without Storage: Lumber	Increase=1.25, Plate Increase=1.25				
Uniform Loads (plf) Vert <sup>.</sup> 1-6=-20_6	-11=-20, 21-26=-40, 17-20=-4	40 16-29=-40 38-39=-40				
		Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)		~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~				
	6-33=14, 6-35=25, 11-35=14, 6-33=-26, 6-35=37, 11-35=2	22-26=18, 21-22=-12, 17-20=-12, 16-29=-12 6				
		Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	, 	·				
	6-34=25, 6-36=14, 11-36=25, 6-34=-37, 6-36=26, 11-36=3	22-26=18, 21-22=-12, 17-20=-12, 16-29=-12				
		Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	· · · · · · · · · · · · · · · · · · ·					
	-11=-33, 22-26=-15, 21-22=-2	20, 17-20=-20, 16-29=-20				
Horz: 1-6=13, 6		Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	leg. memai) base 2. Lumber					
· · · · · · · · · · · · · · · · · · ·	-11=-33, 22-26=-15, 21-22=-2	20, 17-20=-20, 16-29=-20				
Horz: 1-6=13, 6		r Ingrada 1.60. Plata Ingrada 1.60				
Uniform Loads (plf)	iu (Pos. Internal) Leit. Lumbe	r Increase=1.60, Plate Increase=1.60				
(i )	11=9, 22-26=4, 21-22=-12, 17	7-20=-12, 16-29=-12				
Horz: 1-6=-10, 6						
<li>9) Dead + 0.6 MWFRS Wir Uniform Loads (plf)</li>	nd (Pos. Internal) Right: Lumb	per Increase=1.60, Plate Increase=1.60				
	1=-2, 21-26=-12, 17-20=-12,	16-29=-12				
Horz: 1-6=-21, 6						
	(ind (Neg. Internal) Left: Lumb	per Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf) Vert: 1-6=-20.	6-11=-9, 22-26=-4, 21-22=-2	0. 17-20=-20. 16-29=-20				
Horz: 1-6=-0, 6		o, 20 20, .0 20 20				
	ind (Neg. Internal) Right: Lun	nber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	-11=-20, 21-26=-20, 17-20=-	20 16-2920				
Horz: 1-6=-11,		20, 10-2320				
,	ind (Pos. Internal) 1st Paralle	I: Lumber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)		2 16 20 12				
Horz: 1-6=-34,	6-11=7, 21-26=-12, 17-20=-12 . 6-11=19	2, 16-29=-12				
		el: Lumber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)						
Vert: 1-6=7, 6- Horz: 1-6=-19,	·11=22, 21-26=-12, 17-20=-12 6-11=34	2, 16-29=-12				
		el: Lumber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)						
	6-11=3, 21-26=-12, 17-20=-12	2, 16-29=-12				
Horz: 1-6=-23, 15) Dead + 0.6 MWFRS W		el: Lumber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)						
	11=11, 21-26=-12, 17-20=-12	2, 16-29=-12				
Horz: 1-6=-15, 16) Dead + 0.6 MWERS W		el: Lumber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	ind (Neg. internal) 13(1 aran					
	11=-11, 21-26=-20, 17-20=-2	20, 16-29=-20				
Horz: 1-6=-24,		al: Lumber Increase-1.60. Plate Increase-1.60				
Uniform Loads (plf)	inu (Neg. Internal) zhu Palali	el: Lumber Increase=1.60, Plate Increase=1.60				
	6-11=4, 21-26=-20, 17-20=-2	20, 16-29=-20				
Horz: 1-6=-9, 6						
<ol> <li>Dead + Uninhabitable / Uniform Loads (plf)</li> </ol>	Attic Storage: Lumber Increas	se=1.25, Plate Increase=1.25				
u /	6-11=-20, 21-26=-20, 20-37=	-20, 17-37=-60, 16-29=-20, 38-39=-40				
19) Dead + 0.75 Roof Live		storage + 0.75(0.6 MWFRS Wind (Neg. Int) Left)	: Lumber I	ncrease=	1.60, Plate	
Increase=1.60						
Uniform Loads (plf) Vert: 1-6=-50.	6-11=-42, 22-26=-8 21-22=-	20, 20-37=-20, 17-37=-50, 16-29=-20, 38-39=-3	C			
Horz: 1-6=-0, 6			-			
20) Dead + 0.75 Roof Live	(bal.) + 0.75 Uninhab. Attic S	torage + 0.75(0.6 MWFRS Wind (Neg. Int) Righ	t): Lumbe	r Increase	=1.60, Plate	

Qty

Ply

20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60

Truss Type

Job





Job	Truss	Truss Type	Qty	Ply	Chesapeake-6260A:Lot92 FarmNeillsCreek
FNC92-R	A03HT	ROOF TRUSS	1	1	162330702
					Job Reference (optional)
Builders FirstSource, Apex, N	Builders FirstSource, Apex, NC 27523				8.630 s Mar 9 2023 MiTek Industries, Inc. Mon Dec 4 16:01:15 2023 Page 3
		ID:	hazSNSvR	IgjAW5IiY0	CphTxyvdPZ-bwicxYO1cmO8CJeiu5oYZyseRTOQdpVfNfnVf0yCINI

# LOAD CASE(S)

#### Uniform Loads (plf)

Vert: 1-6=-42, 6-11=-50, 21-26=-20, 20-37=-20, 17-37=-50, 16-29=-20, 38-39=-30

Horz: 1-6=-8, 6-11=0

21) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-6=-32, 6-11=-43, 21-26=-20, 20-37=-20, 17-37=-50, 16-29=-20, 38-39=-30

Horz: 1-6=-18. 6-11=7 22) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-6=-43, 6-11=-32, 21-26=-20, 20-37=-20, 17-37=-50, 16-29=-20, 38-39=-30

Horz: 1-6=-7, 6-11=18 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-6=-60, 6-11=-20, 21-26=-20, 17-20=-20, 16-29=-20

24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-6=-20, 6-11=-60, 21-26=-20, 17-20=-20, 16-29=-20

25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-50, 6-11=-20, 21-26=-20, 20-37=-20, 17-37=-50, 16-29=-20, 38-39=-30

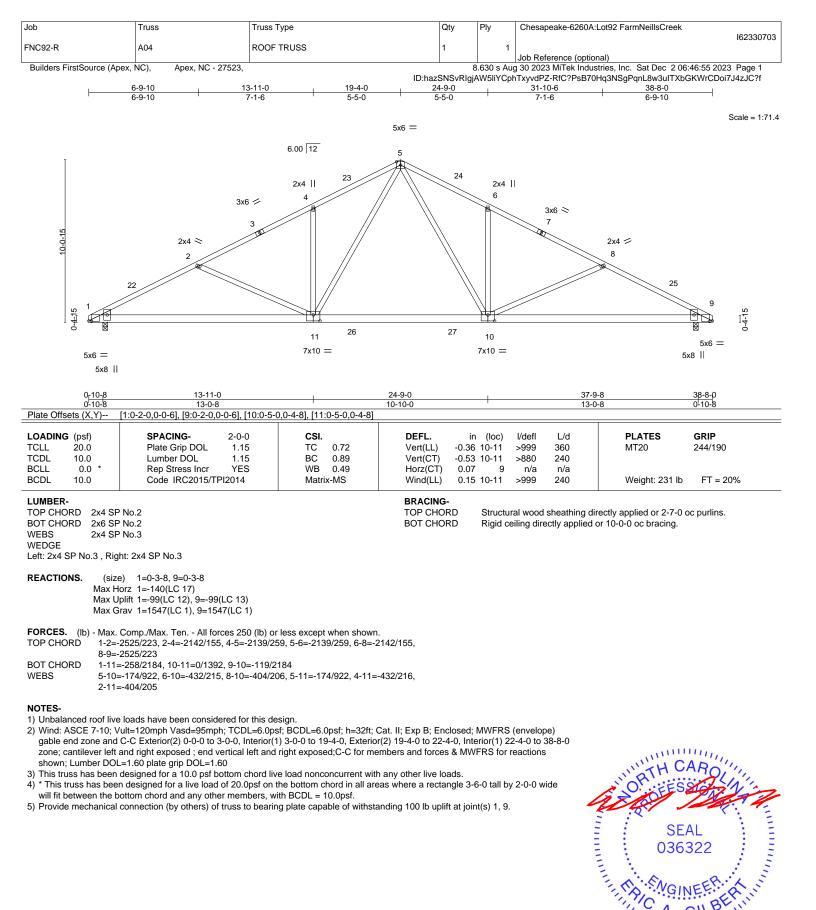
26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-6=-20, 6-11=-50, 21-26=-20, 20-37=-20, 17-37=-50, 16-29=-20, 38-39=-30

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December 5,2023

NC92-R	A04HT	ROOF TRU	lss		1		erence (optiona	al)		16233070
Builders FirstSource, Apex,	, NC 27523				ISvRlgjAW5liYC	8.630 s M phTxyvdP2	ar 9 2023 MiTek	Industries, Inc. xvU4?9TLbsL	Mon Dec Eh9LCPk	4 16:01:59 2023 Page 1 xjX8VVRqCTyCIMc
	1-9-4 3-1-84-2-8 9-2-1 1-9-4 1-4-41-1-0 4-11-		5-2-8 19-4-0 6-0-8 4-1-8	24-9-0 5-5-0		<u>31-10-6</u> 7-1-6		<u>38-8-0</u> 6-9-10		
			6.00 12	5x6 =						Scale = 1:78
10-0-15 0-4-15 1 3x	$ \begin{array}{c} 2 \\ 33 \\ 1 \\ 33 \\ 19 \\ 21 \\ 18 \\ 20 \\ 66 = 6x8 = 5x6 \\ 4x6 = \frac{5x6 \\ 2x4 \\$	3x6 - 4 4x6 = 3 17 4x6 =	4x6 = 34 5 4x6 = 34 16 37 5x8 = 15 5x6 =		5 2x4    7 3) 8 8 22 4 = 12 7x10 =	x6 <	4x6 × 9 9 11 2x4	36	5x6 =	1949 100- 00-
0	1-9-4 3-1-8 <u>}-10-8 1-9-15 4-2-8 9-2-</u> )-10-8 0-0112 1-1-0 4-11- 0-10-121-3-9	3 6	5-2-8 18-8-8 6-0-8 3-6-0	<u>21-3-0</u> 24- 2-6-8 3-6	3-0 <sup> </sup>	<u>31-10-6</u> 7-1-6		37-9-8 5-11-2	<u>38-8-0</u> 0-10-8	
Plate Offsets (X,Y)	[2:0-6-0,0-2-3], [10:0-2-4					-				
OADING (psf)	SPACING-	2-0-0	<b>CSI.</b> TC 0.99	DEFL. Vert(LL)	in (loc) -0.21 16-17 -0.43 16-17	l/defl >999 >999	L/d 360 240	PLATE MT20	S	<b>GRIP</b> 244/190
CDL 10.0 CLL 0.0 *	Plate Grip DOL Lumber DOL Rep Stress Incr	1.15 1.15 NO	BC 1.00 WB 0.56	Vert(CT) Horz(CT)	0.24 10	n/a	n/a	Weight:	262 lb	FT - 20%
TCDL         10.0           3CLL         0.0           3CDL         10.0           JUMBER-         100           TOP CHORD         2x4 SI           1-4,8-         1-4,8-           3OT CHORD         2x4 SI	Lumber DOL Rep Stress Incr Code IRC2015/TI P No.2 *Except* 10: 2x4 SP No.1 P No.2 *Except*	1.15 NO Pl2014	BC 1.00 WB 0.56 Matrix-MS	Vert(CT)	0.24 10 0.16 16-17 D Structu D Rigid co	n/a >999 ral wood s		10-0-0 oc bra		FT = 20%
CDL 10.0 CLL 0.0 * CDL 10.0 UMBER- OP CHORD 2x4 SI 1-4,8- OT CHORD 2x4 SI 2-16: 2 10-12: VEBS 2x4 SI 16-22: VEDGE tight: 2x4 SP No.3 EACTIONS. (lb/siz Max H	Lumber DOL Rep Stress Incr Code IRC2015/TI P No.2 *Except* 10: 2x4 SP No.1 P No.2 *Except* 2x4 SP SS, 5-15: 2x4 SP I : 2x6 SP DSS P No.3 *Except* : 2x4 SP No.2 ze) 21=1552/0-3-8 (min Horz 21=-140(LC 13)	1.15 NO PI2014 No.3, 12-15: 2x6 \$ 0-1-13), 10=1542	BC 1.00 WB 0.56 Matrix-MS	Vert(CT) Horz(CT) Wind(LL) BRACING- TOP CHOR	0.24 10 0.16 16-17 D Structu D Rigid co	n/a >999 ral wood s eiling diree	n/a 240 heathing directly applied or	ctly applied. 10-0-0 oc bra		
CDL         10.0           3CLL         0.0           3CDL         10.0           3CDL         10.0           JUMBER- TOP CHORD           COP CHORD         2x4 SI 1-4,8-           SOT CHORD         2x4 SI 2-16:1 10-12:           VEBS         2x4 SI 16-22:           VEBS         2x4 SI 16-22:           VEDGE         (Ib/siz Max H Max L           CORCES. (Ib) - Max OP CHORD           GORCES. (Ib) - Max 9-36           GORCES. (Ib) - Max 10-34           GORCES. (Ib) - Max 10-34           TOP CHORD           2-22 17-3 13-1           VEBS           3-17           VEBS           3-17           VEBS           3-17           YEBS           3-17           3-17           7-12	Lumber DOL Rep Stress Incr Code IRC2015/TI P No.2 *Except* 10: 2x4 SP No.1 P No.2 *Except* 2x4 SP SS, 5-15: 2x4 SP I : 2x6 SP DSS P No.3 *Except* : 2x4 SP No.2 2t=1552/0-3-8 (min	1.15 NO Pl2014 No.3, 12-15: 2x6 \$ 0-1-13), 10=154: 29(LC 13) ces 250 (lb) or les 70, 3-4=-3005/29 70, 7-35=-2095/2 145 1264, 2-19=-305/ 84, 15-16=0/255, 11-12=-71/2140, 8, 6-16=-172/107 , 16-38=0/1253, 2	BC 1.00 WB 0.56 Matrix-MS SP No.2 2/0-3-8 (min. 0-1-13) ss except when shown. 7, 4-5=-2913/328, 5-34 44, 7-8=-1928/167, 8-5 /3259, 18-19=-363/348 5-16=-731/245, 14-15 10-11=-71/2140 9, 6-22=-196/801, 12-2 23-38=0/1251, 15-23=-	Vert(CT) Horz(CT) Wind(LL) BRACING- TOP CHOR BOT CHOR BOT CHOR BOT CHOR -2100/148, 0, 17-18=-220/2624 =0/1360, 22=-198/838,	0.24 10 0.16 16-17	n/a >999 ral wood s eiling diree	n/a 240 heathing directly applied or	ctly applied. 10-0-0 oc bra		

building design. Bracing indicated is to prevent buckling of individual russ web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Chesapeake-6260A:Lot92 FarmNeillsCreek	330704
FNC92-R	A04HT	ROOF TRUSS	1	1		330704
Builders FirstSource, Apex, NC	27523	1	1		Job Reference (optional) 8.630 s Mar 9 2023 MiTek Industries, Inc. Mon Dec 4 16:01:59 2023 Pa	age 2
		ID:ha	zSNSvRlgj	AW5liYCp	hTxyvdPZ-IONEAbwdFIxvU4?9TLbsLEh9LCPkxjX8VVRqCTyCI	vic
LOAD CASE(S)						
1) Dead + Roof Live (bala	nced): Lumber Increase=1.15	, Plate Increase=1.15				
Uniform Loads (plf)	0.40, 00, 00, 05, 00, 40, 15	00.45.00.00				
	6-10=-60, 20-25=-20, 16-19=-	20, 15-28=-20 ttic Storage: Lumber Increase=1.15, Plate Increa	200-1 15			
Uniform Loads (plf)	(balanceu) + 0.75 Ommab. A	lic Storage. Lumber increase=1.13, Flate increa	456=1.15			
(i )	6-10=-50, 20-25=-20, 19-37=-	20, 16-37=-50, 15-28=-20, 38-39=-30				
	Attic Without Storage: Lumber	Increase=1.25, Plate Increase=1.25				
Uniform Loads (plf)	6-10=-20, 20-25=-40, 16-19=-	40 15-2840 38-3940				
		Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	·					
	, 6-33=14, 6-35=25, 10-35=14 7, 6-33=-26, 6-35=37, 10-35=2	21-25=18, 20-21=-12, 16-19=-12, 15-28=-12				
		Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	,					
		21-25=18, 20-21=-12, 16-19=-12, 15-28=-12				
	6, 6-34=-37, 6-36=26, 10-36=3 Neg. Internal) Case 1: Lumber	r Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)						
	6-10=-33, 21-25=-15, 20-21=-	20, 16-19=-20, 15-28=-20				
Horz: 1-6=13,		r Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	Neg. Internal) Case 2. Lumber	niclease=1.00, Flate niclease=1.00				
	6-10=-33, 21-25=-15, 20-21=-	20, 16-19=-20, 15-28=-20				
Horz: 1-6=13,						
8) Dead + 0.6 MWFRS W Uniform Loads (plf)	ind (Pos. Internal) Left: Lumbe	er Increase=1.60, Plate Increase=1.60				
	-10=9, 21-25=4, 20-21=-12, 1	6-19=-12, 15-28=-12				
Horz: 1-6=-10,						
<li>9) Dead + 0.6 MWFRS W Uniform Loads (plf)</li>	ind (Pos. Internal) Right: Lum	per Increase=1.60, Plate Increase=1.60				
	10=-2, 20-25=-12, 16-19=-12,	15-28=-12				
Horz: 1-6=-21,						
	Nind (Neg. Internal) Left: Lum	per Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf) Vert: 1-6=-20	), 6-10=-9, 21-25=-4, 20-21=-2	0 16-19=-20 15-28=-20				
Horz: 1-6=-0		0, 10 10 20, 10 20 20				
,	Nind (Neg. Internal) Right: Lur	nber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	6-10=-20, 20-25=-20, 16-19=-	20 15 28- 20				
Horz: 1-6=-1		20, 13-28=-20				
12) Dead + 0.6 MWFRS \	Vind (Pos. Internal) 1st Paralle	el: Lumber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	C 40 7 00 05 40 4C 40 4	2 45 20 42				
Vert: 1-6=22, Horz: 1-6=-34	6-10=7, 20-25=-12, 16-19=-1 6-10=19	2, 15-28=-12				
		el: Lumber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)						
Vert: 1-6=7, 6 Horz: 1-6=-1	6-10=22, 20-25=-12, 16-19=-1	2, 15-28=-12				
		el: Lumber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)						
	6-10=3, 20-25=-12, 16-19=-1	2, 15-28=-12				
Horz: 1-6=-23 15) Dead + 0.6 MWFRS \		el: Lumber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)						
	6-10=11, 20-25=-12, 16-19=-1	2, 15-28=-12				
Horz: 1-6=-1	-,	el: Lumber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	wind (rieg. internal) ist i arain					
	6-10=-11, 20-25=-20, 16-19=-2	20, 15-28=-20				
Horz: 1-6=-24	,	lel: Lumber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	Minu (Neg. Internal) zhu Parai	lei. Lumber merease=1.00, Plate merease=1.00				
. ,	, 6-10=4, 20-25=-20, 16-19=-2	20, 15-28=-20				
Horz: 1-6=-9						
	Attic Storage: Lumber Increas	se=1.25, Plate Increase=1.25				
Uniform Loads (plf) Vert: 1-6=-20	), 6-10=-20, 20-25=-20. 19-37=	20, 16-37=-60, 15-28=-20, 38-39=-40				
		Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Left	: Lumber I	Increase=	1.60, Plate	
Increase=1.60		,				
Uniform Loads (plf)	6-1042 21-258 20 21-	20, 19-37=-20, 16-37=-50, 15-28=-20, 38-39=-3	0			
Horz: 1-6=-0		20, 10 07 - 20, 10 07 - 30, 13 20 - 20, 30 39 - 3				
		Storage + 0 75(0 6 MWERS Wind (Neg. Int) Right	nt): Lumbe	r Increase	e=1.60 Plate	

Qty

Ply

20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60

Truss Type

Job

ARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUFTPI Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	Chesapeake-6260A:Lot92 FarmNeillsCreek	
FNC92-R	A04HT	ROOF TRUSS	1	1	16	2330704
FNC92-R	A04111		1	'	Job Reference (optional)	

Builders FirstSource, Apex, NC 27523

8.630 s Mar 9 2023 MiTek Industries, Inc. Mon Dec 4 16:01:59 2023 Page 3 ID:hazSNSvRIgjAW5IiYCphTxyvdPZ-IONEAbwdFIxvU4?9TLbsLEh9LCPkxjX8VVRqCTyCIMc

### LOAD CASE(S)

#### Uniform Loads (plf)

Vert: 1-6=-42, 6-10=-50, 20-25=-20, 19-37=-20, 16-37=-50, 15-28=-20, 38-39=-30

Horz: 1-6=-8, 6-10=0

21) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-6=-32, 6-10=-43, 20-25=-20, 19-37=-20, 16-37=-50, 15-28=-20, 38-39=-30

Horz: 1-6=-18, 6-10=7

22) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-6=-43, 6-10=-32, 20-25=-20, 19-37=-20, 16-37=-50, 15-28=-20, 38-39=-30

Horz: 1-6=-7, 6-10=18 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-6=-60, 6-10=-20, 20-25=-20, 16-19=-20, 15-28=-20

24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-20, 6-10=-60, 20-25=-20, 16-19=-20, 15-28=-20

25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

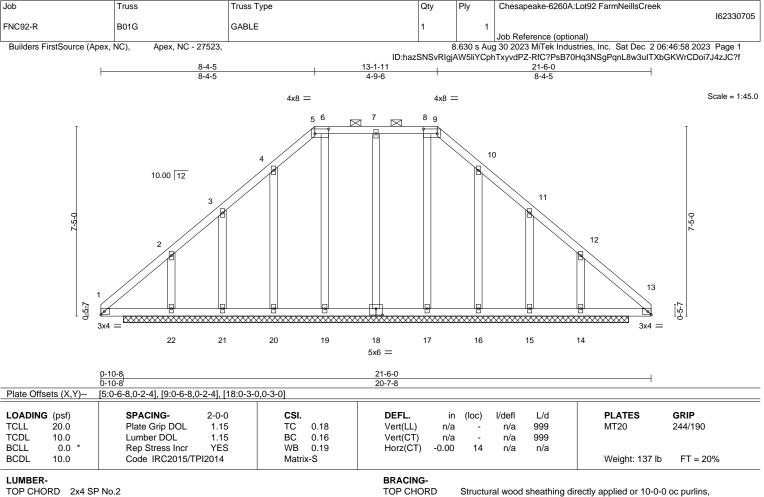
Vert: 1-6=-50, 6-10=-20, 20-25=-20, 19-37=-20, 16-37=-50, 15-28=-20, 38-39=-30

26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-6=-20, 6-10=-50, 20-25=-20, 19-37=-20, 16-37=-50, 15-28=-20, 38-39=-30

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BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

BOT CHORD

except 2-0-0 oc purlins (10-0-0 max.): 5-9.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 19-9-0. (lb) -

Max Horz 22=-153(LC 8) Max Uplift All uplift 100 lb or less at joint(s) 18, 20, 22, 16, 14 except 21=-138(LC 12), 15=-136(LC 13) Max Grav All reactions 250 lb or less at joint(s) 18, 19, 20, 21, 17, 16, 15 except 22=306(LC 23), 14=306(LC

24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-0-0 to 2-9-0, Exterior(2) 2-9-0 to 8-4-5, Corner(3) 8-4-5 to 11-4-5, Exterior(2) 11-4-5 to 13-1-11, Corner(3) 13-1-11 to 16-1-11, Exterior(2) 16-1-11 to 21-6-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 8) will fit between the bottom chord and any other members.

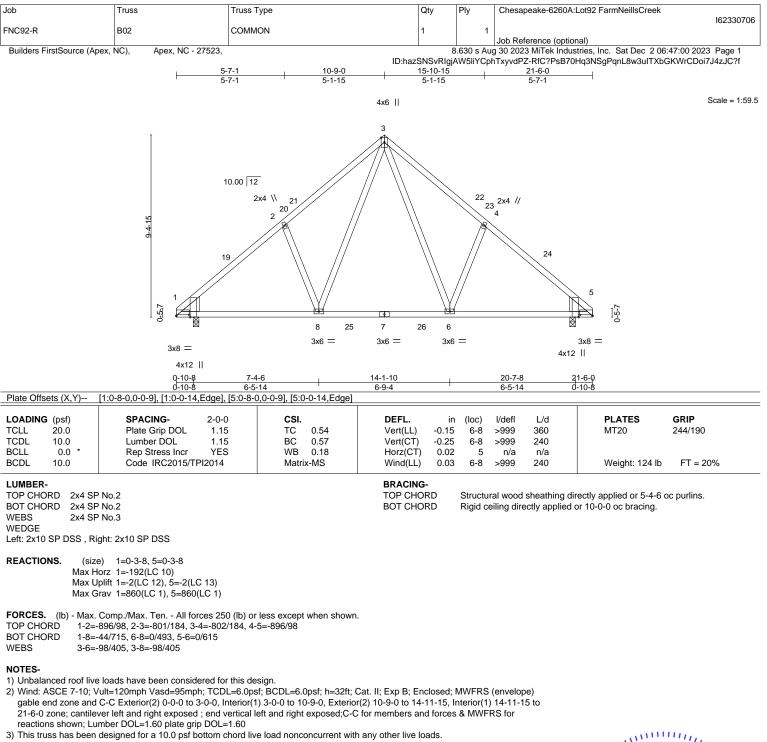
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 20, 22, 16, 14 except (jt=lb) 21=138, 15=136.

10) Non Standard bearing condition. Review required.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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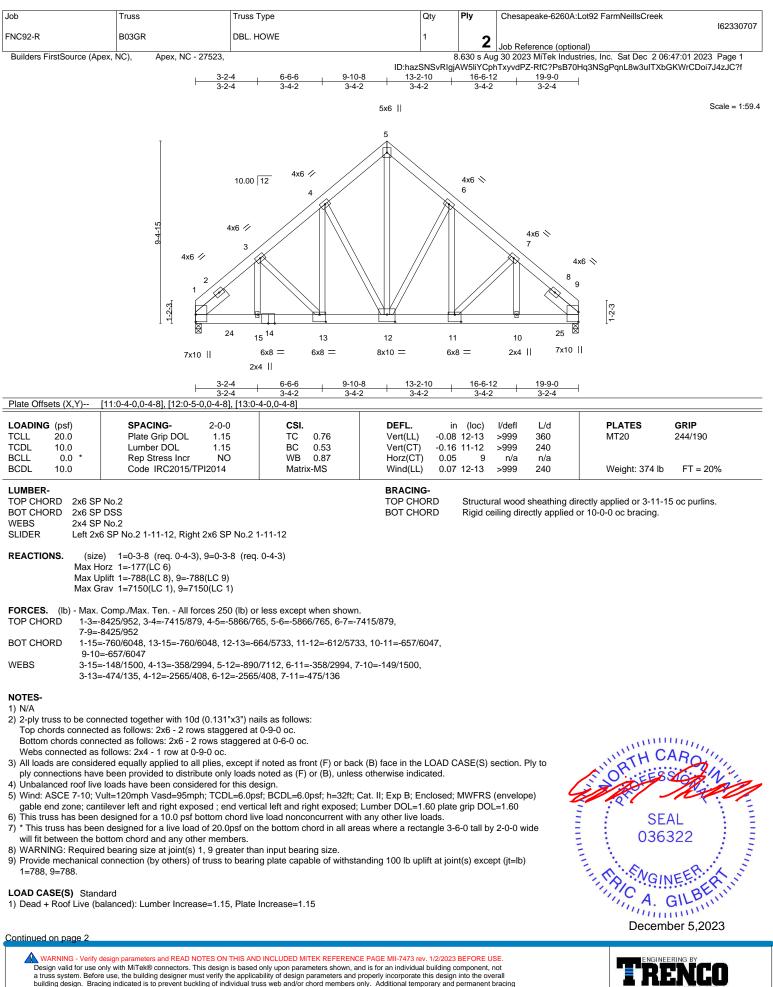
4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.



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A MiTek A 818 Soundside Road Edenton, NC 27932



building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria and DSR-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Chesapeake-6260A:Lot92 FarmNeillsCreek
					162330707
FNC92-R	B03GR	DBL. HOWE	1	2	
				<b></b>	Job Reference (optional)
Builders FirstSource (Apex, N	NC), Apex, NC - 27523,			3.630 s Au	g 30 2023 MiTek Industries, Inc. Sat Dec 2 06:47:01 2023 Page 2

ID:hazSNSvRIgjAW5liYCphTxyvdPZ-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

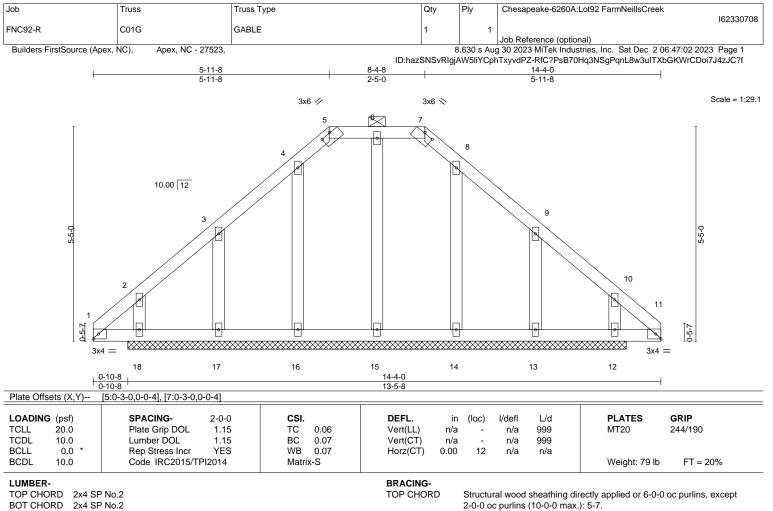
## LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 16-24=-20, 24-25=-751(F=-731), 20-25=-20, 1-3=-20, 3-5=-60, 5-7=-60, 7-9=-20

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BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

OTHERS 2x4 SP No.3

REACTIONS. All bearings 12-7-0.

Max Horz 18=110(LC 11) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 18, 12 except 17=-125(LC 12), 13=-123(LC 13) Max Grav All reactions 250 lb or less at joint(s) 15, 16, 17, 18, 14, 13, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-0-0 to 3-2-0, Exterior(2) 3-2-0 to 5-11-8, Corner(3) 5-11-8 to 11-2-0, Exterior(2) 11-2-0 to 14-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Provide adequate drainage to prevent water ponding. 5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

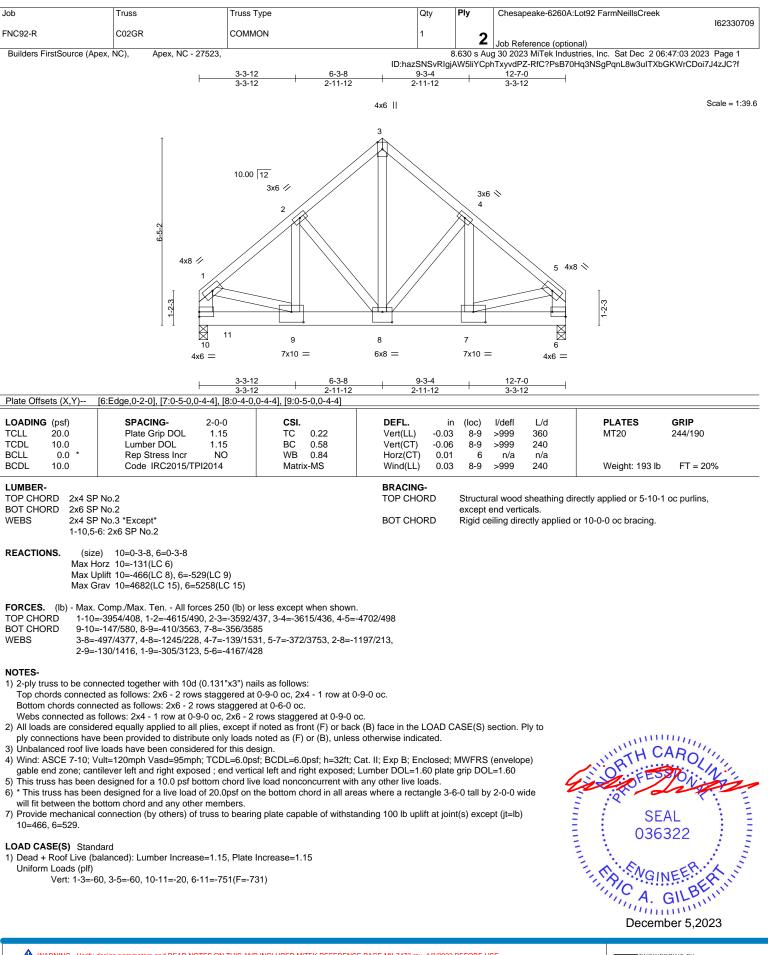
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 12 except (jt=lb) 17=125, 13=123.

10) Non Standard bearing condition. Review required.

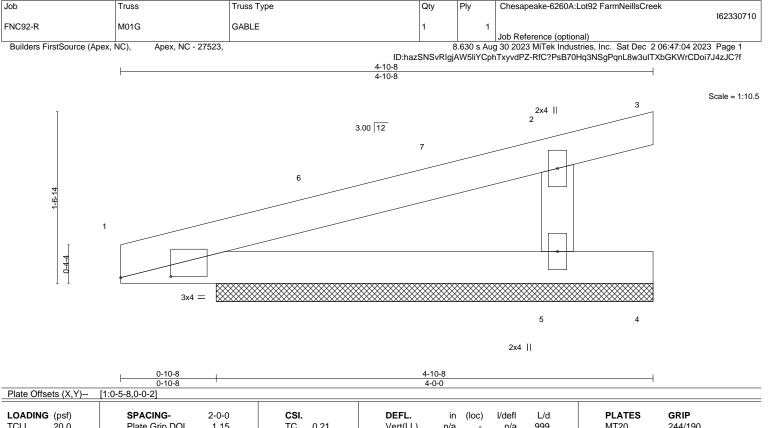
11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.21 BC 0.13 WB 0.08 Matrix-P	DEFL. ii Vert(LL) n/a Vert(CT) n/a Horz(CT) -0.00	a - n/a	L/d 999 999 n/a	PLATES MT20 Weight: 16 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF			BRACING- TOP CHORD BOT CHORD			rectly applied or 4-10- or 10-0-0 oc bracing.	-8 oc purlins.

BOT CHORD2x4 SP No.2OTHERS2x4 SP No.3

REACTIONS. (size) 1=4-0-0, 3=4-0-0, 4=4-0-0, 5=4-0-0 Max Horz 1=43(LC 8) Max Uplift 1=-8(LC 8), 3=-85(LC 1), 4=-58(LC 3), 5=-55(LC 8)

Max Grav 1=127(LC 1), 3=34(LC 8), 5=377(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-5=-282/285

NOTES-1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-0 to 3-0-0, Exterior(2) 3-0-0 to 4-10-8 zone; cantilever left and right exposed ; end vertical left

and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

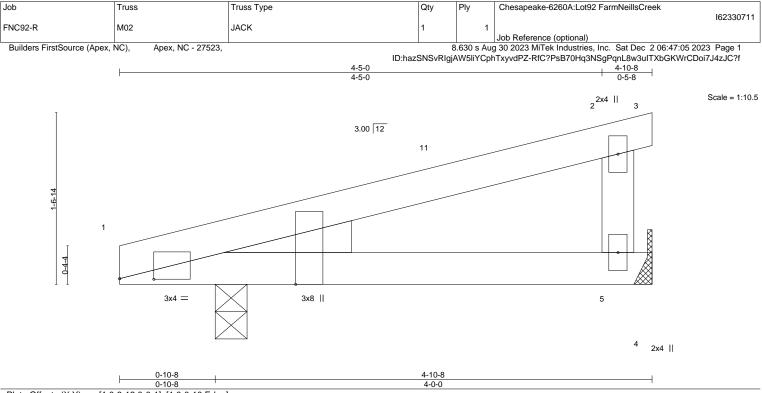
5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4, 5.

7) Non Standard bearing condition. Review required.



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LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.15	Vert(LL) -0.01 5-10 >999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.15	Vert(CT) -0.02 5-10 >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.02	Horz(CT) 0.00 1 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MP	Wind(LL) 0.01 5-10 >999 240	Weight: 18 lb FT = 20%

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3

REACTIONS. (size) 1=0-3-8, 5=Mechanical

Max Horz 1=41(LC 8) Max Uplift 1=-20(LC 8), 5=-25(LC 8) Max Grav 1=225(LC 1), 5=165(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 4-10-8 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.



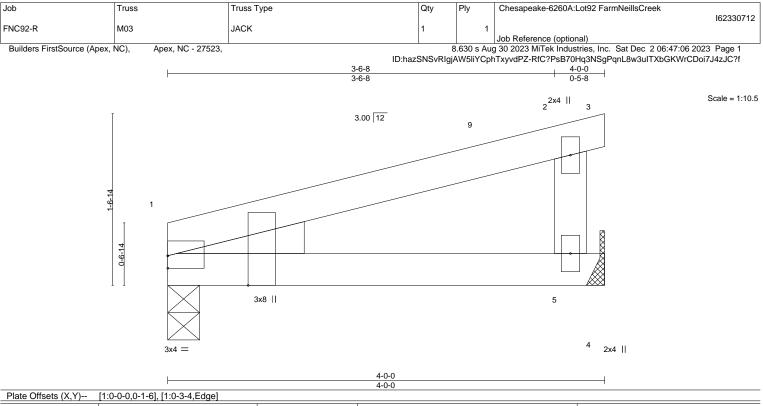
Structural wood sheathing directly applied or 4-10-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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818 Soundside Road



	COADING         (psf)           "CLL         20.0           "CDL         10.0           SCLL         0.0           "SCDL         10.0	Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	TC 0.16 BC 0.18 WB 0.02 Matrix-MP	Vert(LL) Vert(CT) Horz(CT) Wind(LL)	-0.01 -0.02 0.00 0.01	5-8 5-8 1 5-8	>999 >999 n/a >999	360 240 n/a 240	MT20 Weight: 15 lb	244/190 FT = 20%
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BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 WEDGE
 Left: 2x4 SP No.3

REACTIONS. (size) 1=0-3-8, 5=Mechanical

Max Horz 1=33(LC 8) Max Uplift 1=-11(LC 8), 5=-26(LC 8) Max Grav 1=146(LC 1), 5=174(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-10-8 to 3-10-8, Interior(1) 3-10-8 to 4-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

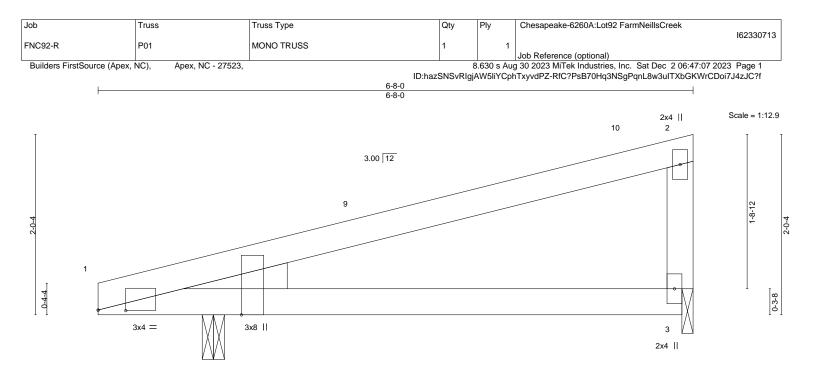
3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



<b>⊢</b>	<u>1-2-0</u> 1-2-0			
Plate Offsets (X,Y)	[1:0-3-11,0-0-1], [1:0-0-10,Edge]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.39 BC 0.31 WB 0.00 Matrix-MP	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.04         3-8         >999         360           Vert(CT)         -0.08         3-8         >991         240           Horz(CT)         0.00         1         n/a         n/a           Wind(LL)         0.04         3-8         >999         240	PLATES         GRIP           MT20         244/190           Weight: 24 lb         FT = 20%

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 WEDGE
 Left: 2x4 SP No.3

Left: 2x4 SP No.3 REACTIONS. (size) 1=0-3-0, 3=0-1-8

Max Horz 1=59(LC 11) Max Uplift 1=-34(LC 8), 3=-29(LC 8) Max Grav 1=318(LC 1), 3=204(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 6-6-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Bearing at joint(s) 3 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 3.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

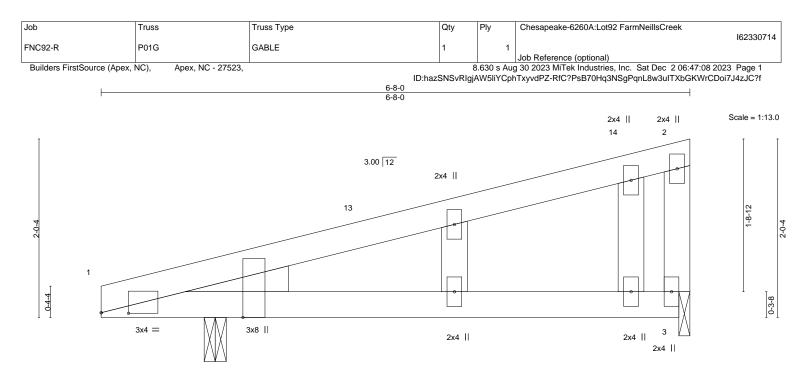
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road



ł	<u>1-2-0</u> 1-2-0										
Plate Offsets (X,Y)	[1:0-3-11,0-0-1], [1:0-0-1	0,Edge]			1						
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.39	Vert(LL)	-0.04	3-12	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.31	Vert(CT)	-0.08	3-12	>991	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	1	n/a	n/a		
BCDL 10.0	Code IRC2015/TI	PI2014	Matrix	:-MP	Wind(LL)	0.04	3-12	>999	240	Weight: 27 lb	FT = 20%
LUMBER-					BRACING-						

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3 WEDGE

Left: 2x4 SP No.3

REACTIONS. (size) 1=0-3-0. 3=0-1-8 Max Horz 1=59(LC 11) Max Uplift 1=-34(LC 8), 3=-29(LC 8) Max Grav 1=318(LC 1), 3=204(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 6-6-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

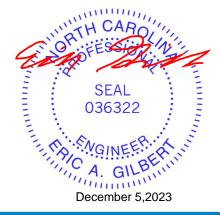
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 3 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 3.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

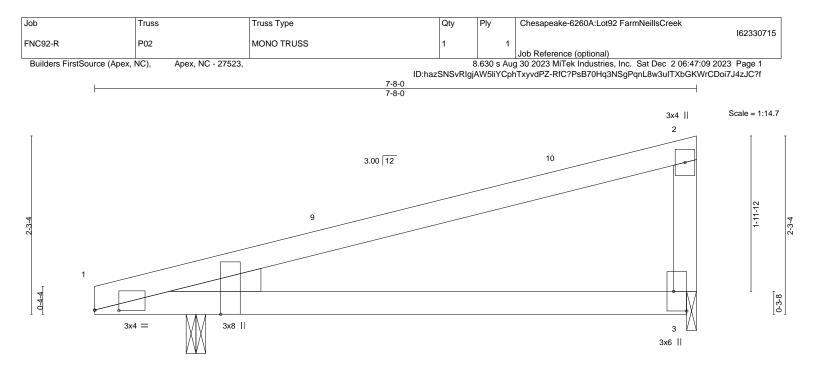


Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design that the operating of the second se and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



		<u>1-2-0</u> 1-2-0										
Plate Offs	sets (X,Y)	[1:0-3-11,0-0-1], [1:0-0-1	0,Edge], [3:Ed	ge,0-2-0]								
	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL	20.0	Plate Grip DOL	1.15	тс	0.44	Vert(LL)	-0.04	3-8	>999	360	MT20	244/190
DL	10.0	Lumber DOL	1.15	BC	0.36	Vert(CT)	-0.10	3-8	>907	240		
CLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	1	n/a	n/a		
CDL	10.0	Code IRC2015/T	PI2014	Matri	x-MS	Wind(LL)	0.04	3-8	>999	240	Weight: 27 lb	FT = 20%
UMBER	ORD 2x4 SP					BRACING- TOP CHOR	RD		ıral wood end verti	0	rectly applied or 6-0-0	oc purlins,

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3

**REACTIONS.** (size) 1=0-3-0, 3=0-1-8

Max Horz 1=68(LC 11) Max Uplift 1=-38(LC 8), 3=-34(LC 8) Max Grav 1=356(LC 1), 3=246(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 7-6-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Bearing at joint(s) 3 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 3.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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A MiTek Affilia 818 Soundside Road

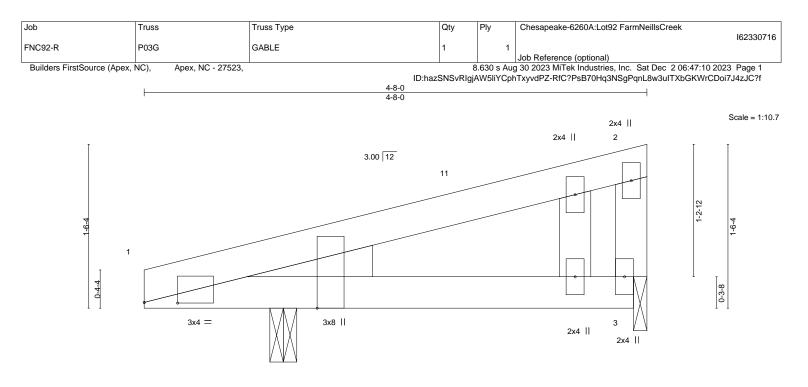


Plate Offs	ets (X,Y)	<u>1-2-0</u> [1:0-3-11,0-0-1], [1:0-0-1	0,Edge]									
OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL	20.0	Plate Grip DOL	1.15	тс	0.11	Vert(LL)	-0.00	3-1Ó	>999	360	MT20	244/190
CDL	10.0	Lumber DOL	1.15	BC	0.10	Vert(CT)	-0.01	3-10	>999	240		
CLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	1	n/a	n/a		
SCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-MP	Wind(LL)	0.01	3-10	>999	240	Weight: 18 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

 WEDGE
 2x4 SP No.3

Left: 2x4 SP No.3

REACTIONS. (size) 1=0-3-0, 3=0-1-8 Max Horz 1=41(LC 11) Max Uplift 1=-26(LC 8), 3=-18(LC 12) Max Grav 1=244(LC 1), 3=118(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

1.2.0

#### NOTES-

 Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 4-6-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 3 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 3.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

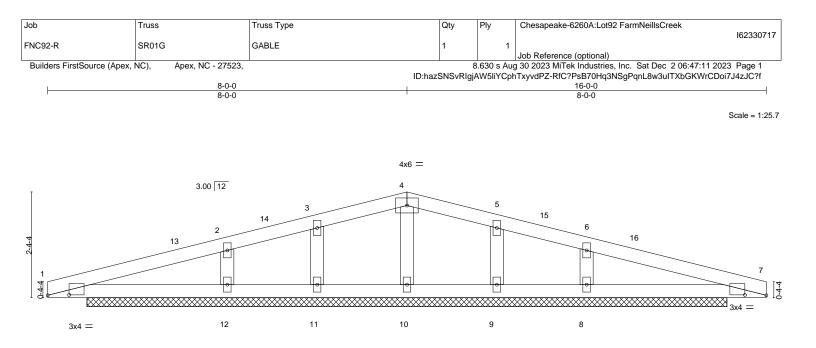


Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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0-10-8			<u>16-0-0</u> 15-1-8						
	1:0-5-11,0-0-2], [7:0-5-11,0-0-2]							T	
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.19	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.12	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.05	Horz(CT)	0.00	7	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S						Weight: 57 lb	FT = 20%
LUMBER-			BRACING-						

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

REACTIONS. All bearings 14-3-0.

Max Horz 1=-30(LC 13) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 11, 12, 9, 8

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 11, 9 except 12=327(LC 1), 8=327(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 8-0-0, Exterior(2) 8-0-0 to 11-0-0, Interior(1) 11-0-0 to 16-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 11, 12, 9, 8. 9) Non Standard bearing condition. Review required.

 $\cap$ WITTELL COMPANY SEAL 036322 G (1111111) December 5,2023

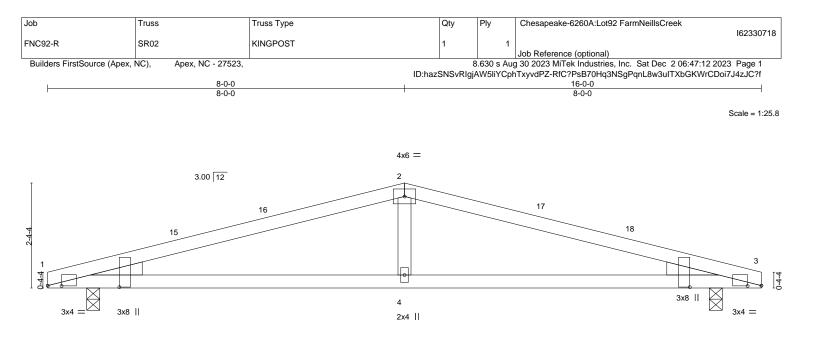
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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818 Soundside Road



+ 0-10-8 0-10-8 Plate Offsets (X,Y)	8-0-0 7-1-8 [1:0-3-11,0-0-1], [1:0-0-6,1-7-4], [3:0-3-1	1,0-0-1], [3:0-0-6,1-7-4]			15-1-8 7-1-8		<u> </u>
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.55 BC 0.53 WB 0.11 Matrix-MS	DEFL.         in           Vert(LL)         -0.08           Vert(CT)         -0.14           Horz(CT)         0.02           Wind(LL)         0.06	4-9 4-9 3	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 55 lb	<b>GRIP</b> 244/190 FT = 20%
Max H Max U	P No.2 P No.3		BRACING- TOP CHORD BOT CHORD			irectly applied or 4-7-3 or 10-0-0 oc bracing.	oc purlins.
TOP CHORD 1-2=- BOT CHORD 1-4=-	Comp./Max. Ten All forces 250 (lb) or -1168/114, 2-3=-1168/114 -60/1093, 3-4=-60/1093 0/280	less except when shown.					
/	e loads have been considered for this de /ult=120mph Vasd=95mph; TCDL=6.0ps	5	. II; Exp B; Enclosed; I	MWFRS (	(envelope)		

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 8-0-0, Exterior(2) 8-0-0 to 11-0-0, Interior(1) 11-0-0 to 16-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

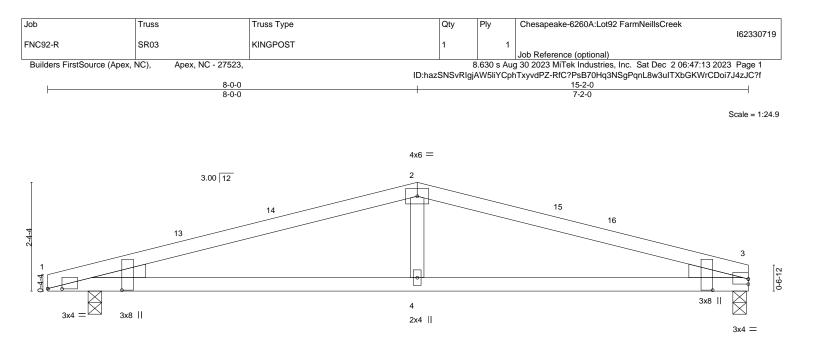
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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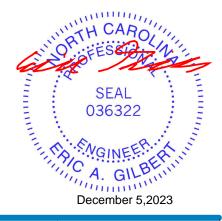
0-10-8 0-10-8 Plate Offsets (X,Y)	<u>8-0-0</u> 7-1-8 [1:0-3-11,0-0-1], [1:0-0-6,1-7-4], [3:0-0-0	0,0-1-5], [3:0-2-14,0-9-4]			15-1- 7-1-8		<u>15-</u> 2-0 0-0-8
LOADING         (psf)           TCLL         20.0           TCDL         10.0           3CLL         0.0           3CDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.58 BC 0.57 WB 0.11 Matrix-MS	DEFL.         ir           Vert(LL)         -0.09           Vert(CT)         -0.15           Horz(CT)         0.02           Wind(LL)         0.06	4-12 3	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 52 lb	<b>GRIP</b> 244/190 FT = 20%
Max H Max L	P No.2 P No.3		BRACING- TOP CHORD BOT CHORD			directly applied or 4-5-1 d or 10-0-0 oc bracing.	11 oc purlins.
TOP CHORD 1-2= BOT CHORD 1-4=	Comp./Max. Ten All forces 250 (lb) or -1186/124, 2-3=-1185/129 -81/1110, 3-4=-81/1110 0/284	less except when shown.					
/	e loads have been considered for this de /ult=120mph Vasd=95mph: TCDL=6.0ps	5	II: Eyn B: Enclosed:				

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 8-0-0, Exterior(2) 8-0-0 to 11-0-0, Interior(1) 11-0-0 to 15-2-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

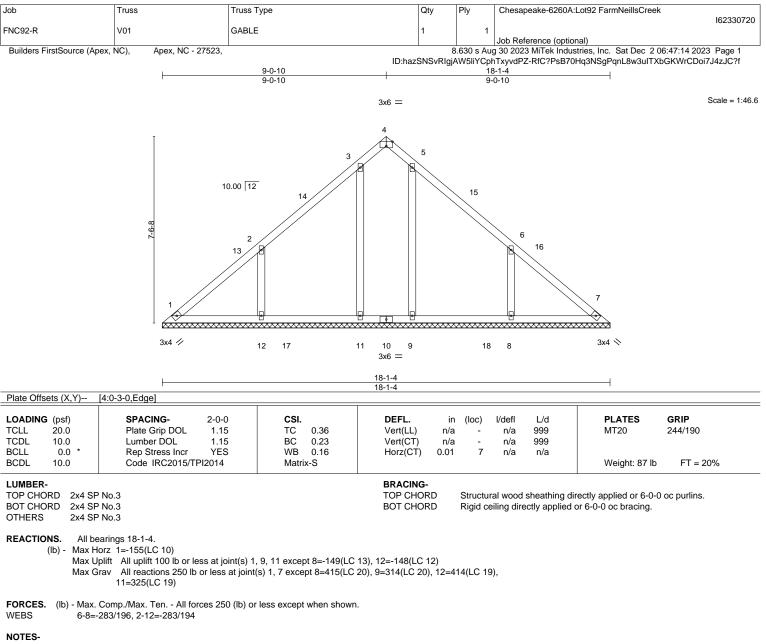
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



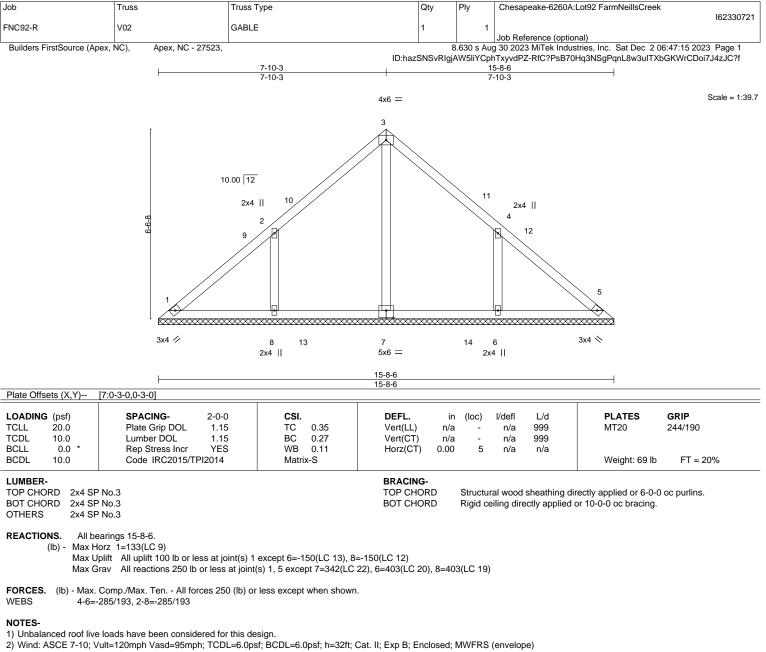
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 9-0-10, Exterior(2) 9-0-10 to 12-0-10, Interior(1) 12-0-10 to 17-8-7 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 11 except (jt=lb) 8=149, 12=148.



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2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 7-10-3, Exterior(2) 7-10-3 to 10-10-3, Interior(1) 10-10-3 to 15-3-9 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

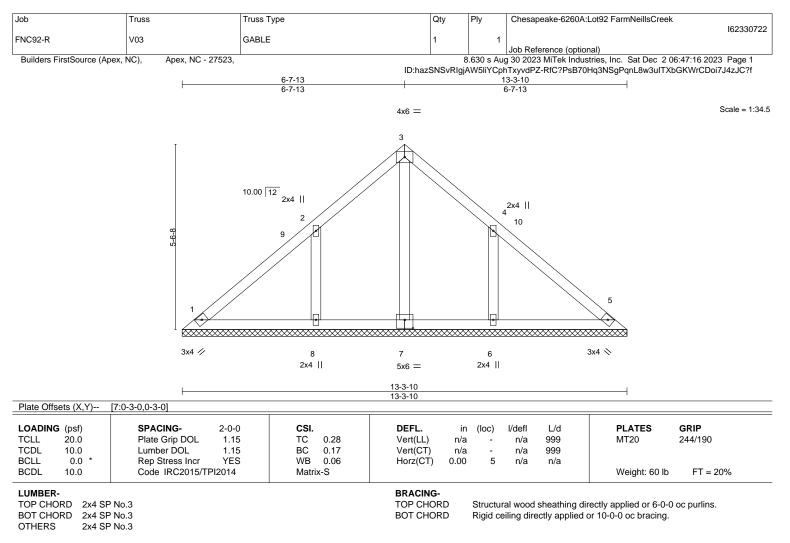
5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 6=150, 8=150.



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**REACTIONS.** All bearings 13-3-10.

(lb) - Max Horz 1=112(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) except 6=-129(LC 13), 8=-129(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 6=333(LC 20), 8=333(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

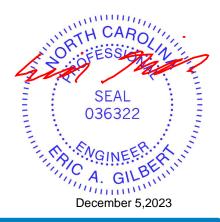
2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 6-7-13, Exterior(2) 6-7-13 to 9-7-13, Interior(1) 9-7-13 to 12-10-13 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

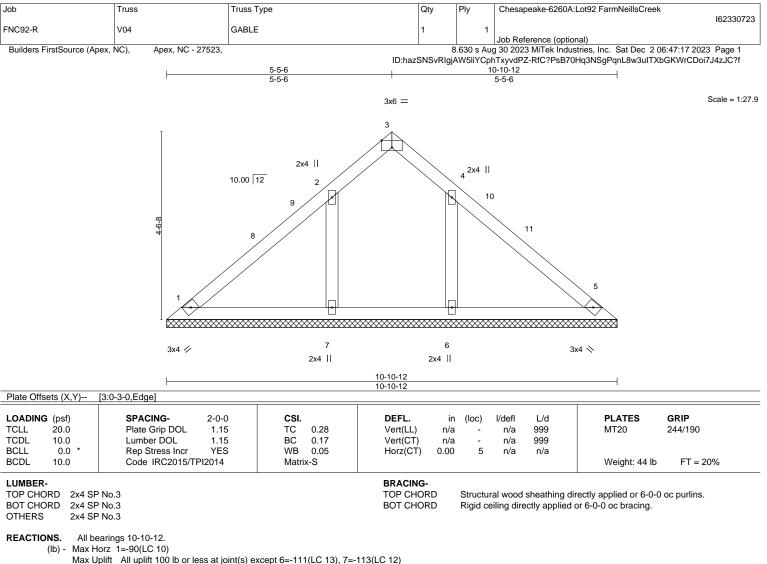
5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint 6 and 129 lb uplift at joint 8.



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Max opint All quint too ib of less at joint(s) except 6=111(LC 10), 7=113(LC 12)Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 6=311(LC 20), 7=313(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

5)

1) Unbalanced roof live loads have been considered for this design.

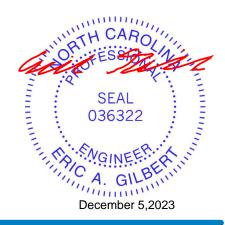
2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 5-5-6, Exterior(2) 5-5-6 to 8-5-6, Interior(1) 8-5-6 to 10-5-15 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

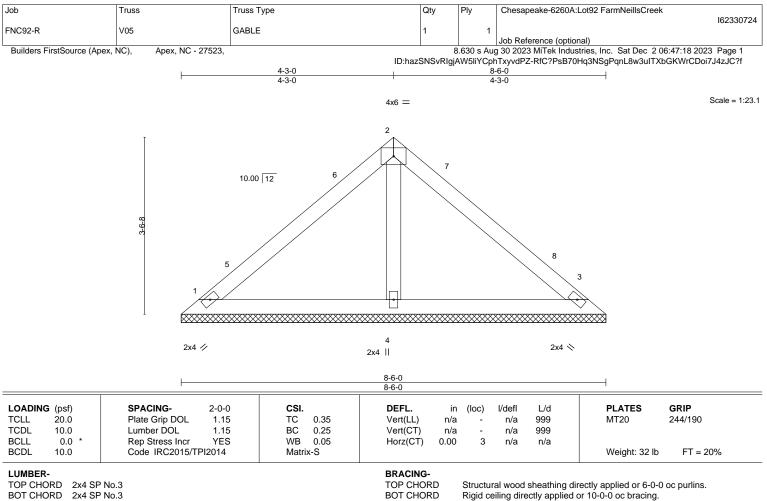
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 6 and 113 lb uplift at joint 7.



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BOT CHORD 2x4 SP No.3 OTHERS 2x4 SP No.3

REACTIONS. (size) 1=8-6-0, 3=8-6-0, 4=8-6-0 Max Horz 1=69(LC 9) Max Uplift 1=-17(LC 13), 3=-26(LC 13) Max Grav 1=161(LC 1), 3=161(LC 1), 4=293(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 4-3-0, Exterior(2) 4-3-0 to 7-3-0, Interior(1) 7-3-0 to 8-1-3 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

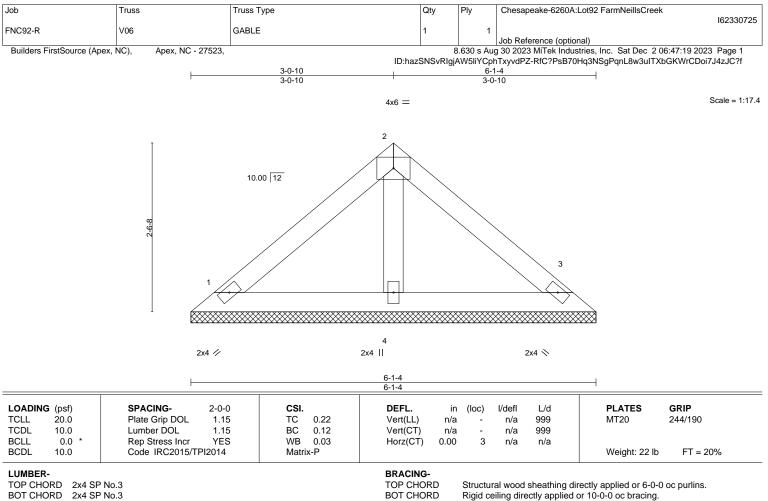
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 1 and 26 lb uplift at joint 3.



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BOT CHORD 2x4 SP No.3 OTHERS 2x4 SP No.3

REACTIONS. 1=6-1-4, 3=6-1-4, 4=6-1-4 (size) Max Horz 1=-47(LC 8) Max Uplift 1=-18(LC 13), 3=-24(LC 13) Max Grav 1=120(LC 1), 3=120(LC 1), 4=183(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

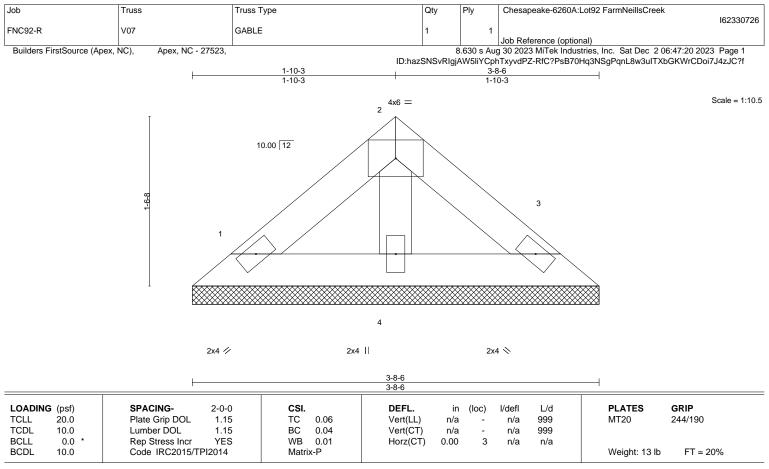
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 5) will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 1 and 24 lb uplift at joint 3.



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LUMBER-

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3OTHERS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-8-6 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=3-8-6, 3=3-8-6, 4=3-8-6 Max Horz 1=-26(LC 8) Max Uplift 1=-10(LC 13), 3=-13(LC 13) Max Grav 1=66(LC 1), 3=66(LC 1), 4=100(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

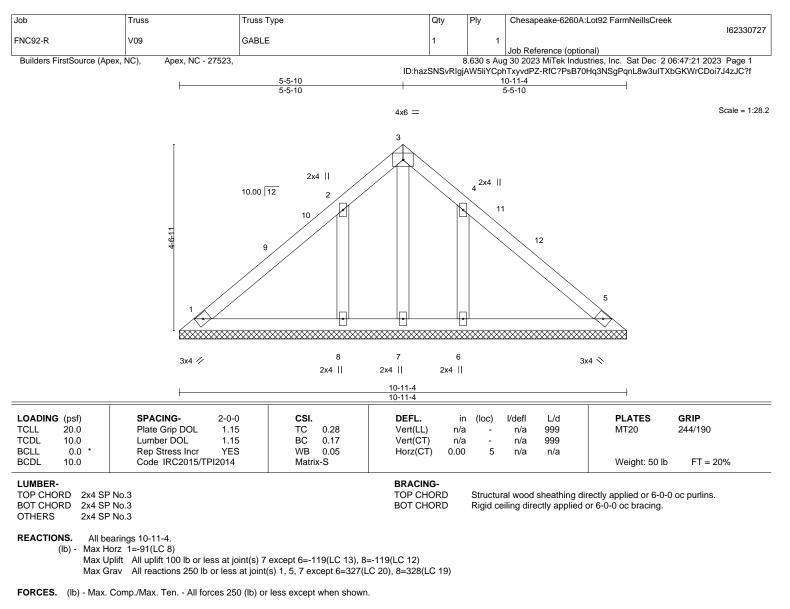
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 1 and 13 lb uplift at joint 3.



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#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 5-5-10, Exterior(2) 5-5-10 to 8-5-10, Interior(1) 8-5-10 to 10-6-7 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

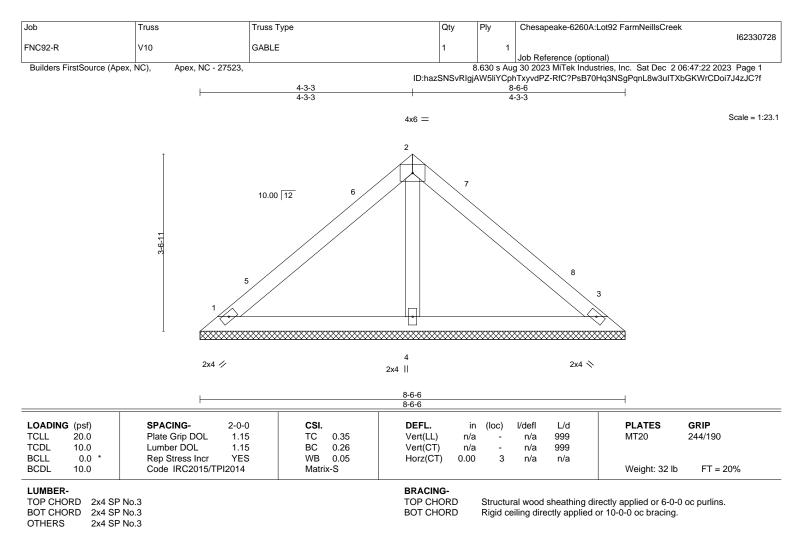
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7 except (jt=lb) 6=119, 8=119.



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REACTIONS. (size) 1=8-6-6, 3=8-6-6, 4=8-6-6 Max Horz 1=-69(LC 8) Max Uplift 1=-17(LC 13), 3=-26(LC 13)

Max Grav 1=162(LC 1), 3=162(LC 1), 4=294(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 4-3-3, Exterior(2) 4-3-3 to 7-3-3, Interior(1) 7-3-3 to 8-1-9 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

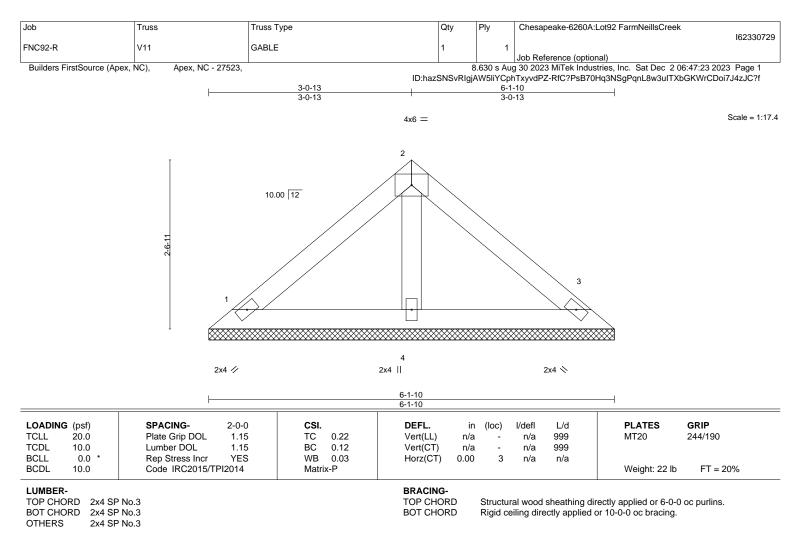
5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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REACTIONS. (size) 1=6-1-10, 3=6-1-10, 4=6-1-10 Max Horz 1=-48(LC 10) Max Uplift 1=-18(LC 13), 3=-24(LC 13)

Max Grav 1=121(LC 1), 3=121(LC 1), 4=184(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

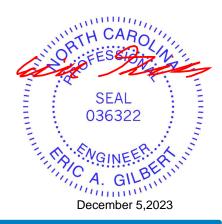
3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

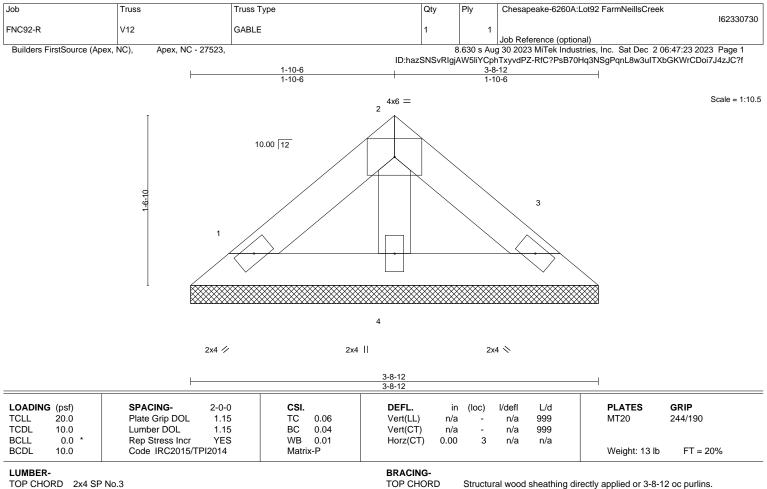
5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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BOT CHORD 2x4 SP No.3 OTHERS 2x4 SP No.3 BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 1=3-8-12, 3=3-8-12, 4=3-8-12 (size) Max Horz 1=-26(LC 8) Max Uplift 1=-10(LC 13), 3=-13(LC 13) Max Grav 1=66(LC 1), 3=66(LC 1), 4=101(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 5)

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

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