

RE: J0723-3722 Precision/59 Liberty Meadows/Harnett **Trenco** 818 Soundside Rd Edenton, NC 27932

Truss Name

VA2

VA3

Date

3/6/2023

3/6/2023

Site Information:

Customer: Project Name: J0723-3722 Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 22 individual, dated Truss Design Drawings and 0 Additional Drawings.

| No. | Seal# | Truss Name | Date | No. | Seal# |
|-----|-----------|------------|----------|-----|-----------|
| 1 | 157003248 | A1-SG | 3/6/2023 | 21 | 157003268 |
| 2 | 157003249 | A2 | 3/6/2023 | 22 | 157003269 |
| 3 | 157003250 | A3 | 3/6/2023 | | |
| 4 | 157003251 | A4 | 3/6/2023 | | |
| 5 | 157003252 | B1-GE | 3/6/2023 | | |
| 6 | 157003253 | B2 | 3/6/2023 | | |
| 7 | 157003254 | B3 | 3/6/2023 | | |
| 8 | 157003255 | B4 | 3/6/2023 | | |
| 9 | 157003256 | B5-GE | 3/6/2023 | | |
| 10 | 157003257 | C1-GE | 3/6/2023 | | |
| 11 | 157003258 | C2 | 3/6/2023 | | |
| 12 | 157003259 | C3 | 3/6/2023 | | |
| 13 | 157003260 | C4 | 3/6/2023 | | |
| 14 | 157003261 | D1-GE | 3/6/2023 | | |
| 15 | 157003262 | D2 | 3/6/2023 | | |
| 16 | 157003263 | M1-GE | 3/6/2023 | | |
| 17 | 157003264 | M2 | 3/6/2023 | | |
| 18 | 157003265 | M3 | 3/6/2023 | | |
| 19 | 157003266 | P1 | 3/6/2023 | | |
| 20 | 157003267 | VA1 | 3/6/2023 | | |
| | | | | | |

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

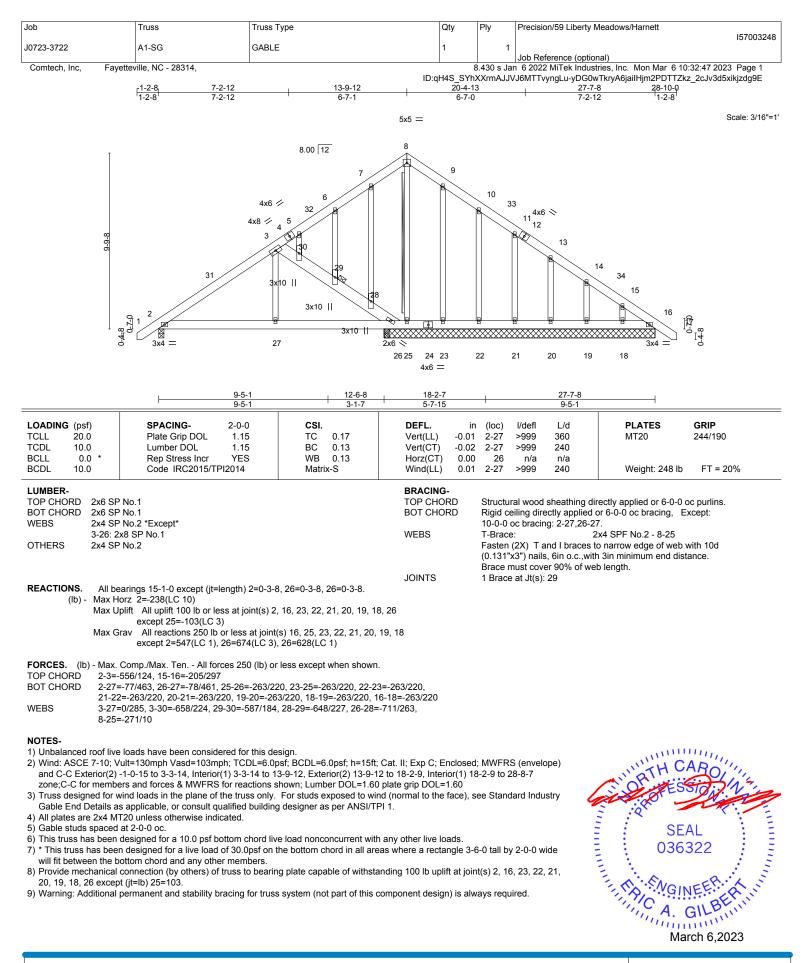
Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2023

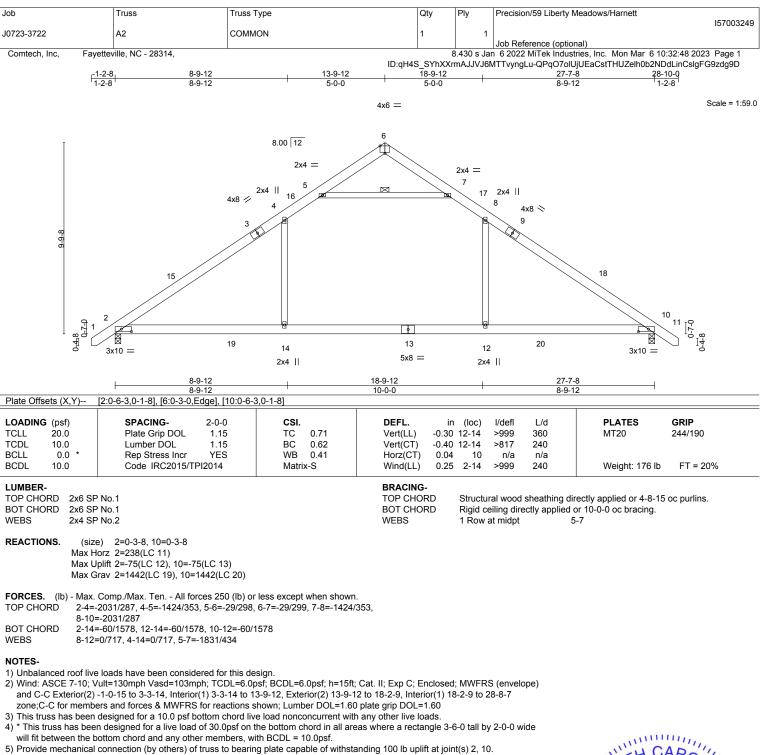
North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





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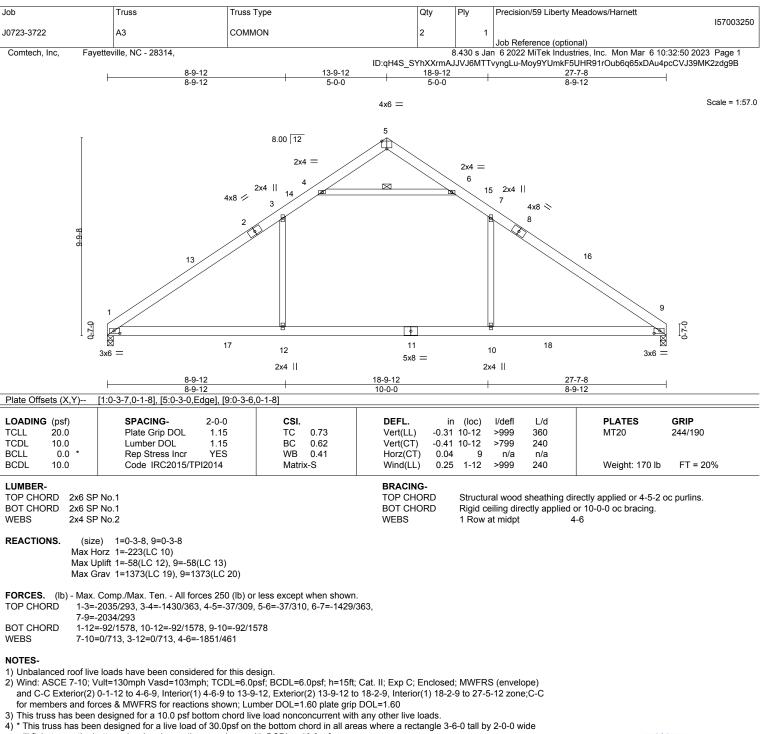




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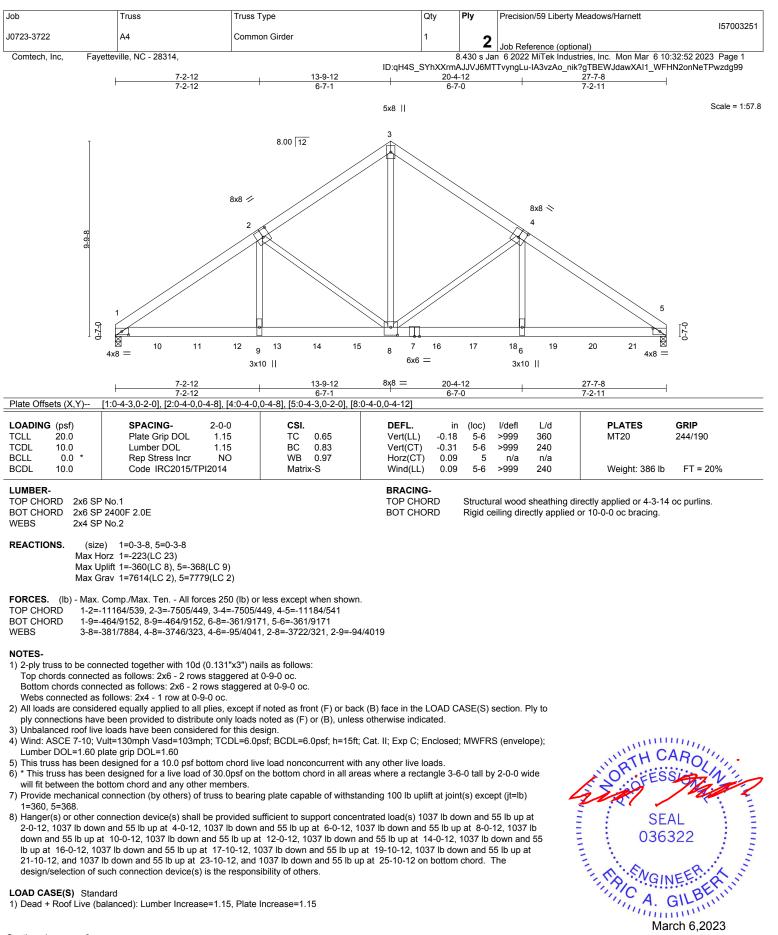
will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.



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A MITEK A 818 Soundside Road Edenton, NC 27932



Continued on page 2

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| Job | Truss | Truss Type | Qty | Ply | Precision/59 Liberty Meadows/Harnett |
|---------------|---------------------------|---------------|-----------------|------------|--|
| 10700 0700 | | | | | 157003251 |
| J0723-3722 | A4 | Common Girder | 1 | 2 | lab Deference (artismal) |
| | | | | - | Job Reference (optional) |
| Comtech, Inc, | Fayetteville, NC - 28314, | | | 8.430 s Ja | n 6 2022 MiTek Industries, Inc. Mon Mar 6 10:32:52 2023 Page 2 |
| | | | ID:qH4S_SYhXXrn | nAJJVJ6M1 | TvyngLu-IA3vzAo_nik?gTBEWJdawXAI1_WFHN2onNeTPwzdg99 |

LOAD CASE(S) Standard

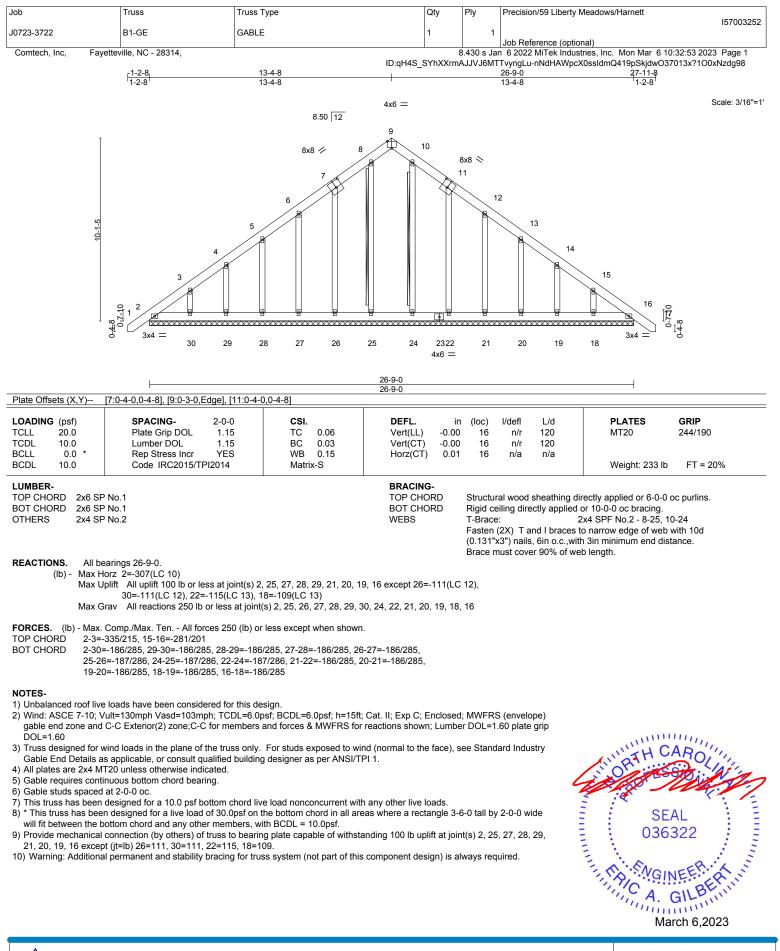
Uniform Loads (plf)

Vert: 1-3=-60, 3-5=-60, 1-5=-20 Concentrated Loads (lb)

Vert: 8=-868(B) 10=-868(B) 11=-868(B) 12=-868(B) 13=-868(B) 14=-868(B) 15=-868(B) 16=-868(B) 17=-868(B) 18=-868(B) 19=-868(B) 20=-868(B) 21=-868(B) 20=-868(B) 20=-86

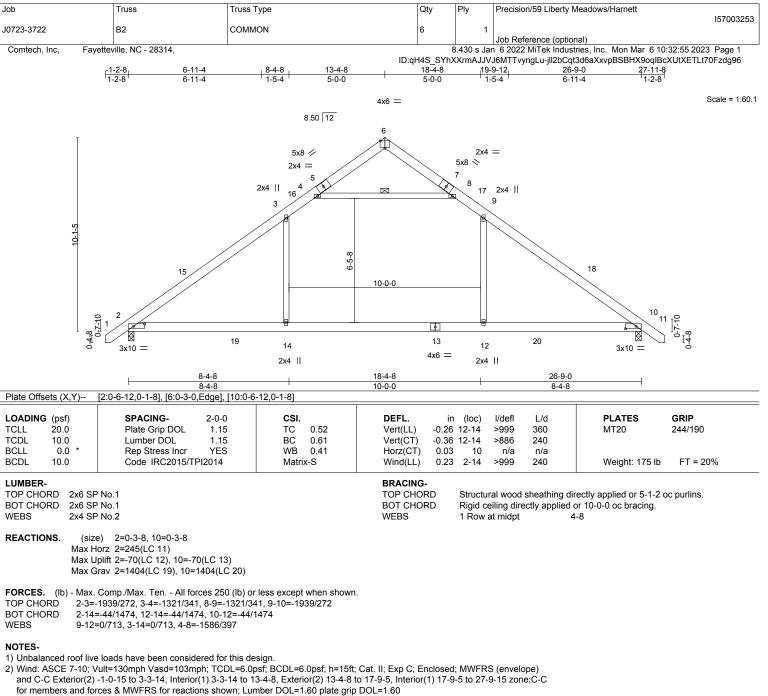
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A MiTek Affil 818 Soundside Road



This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

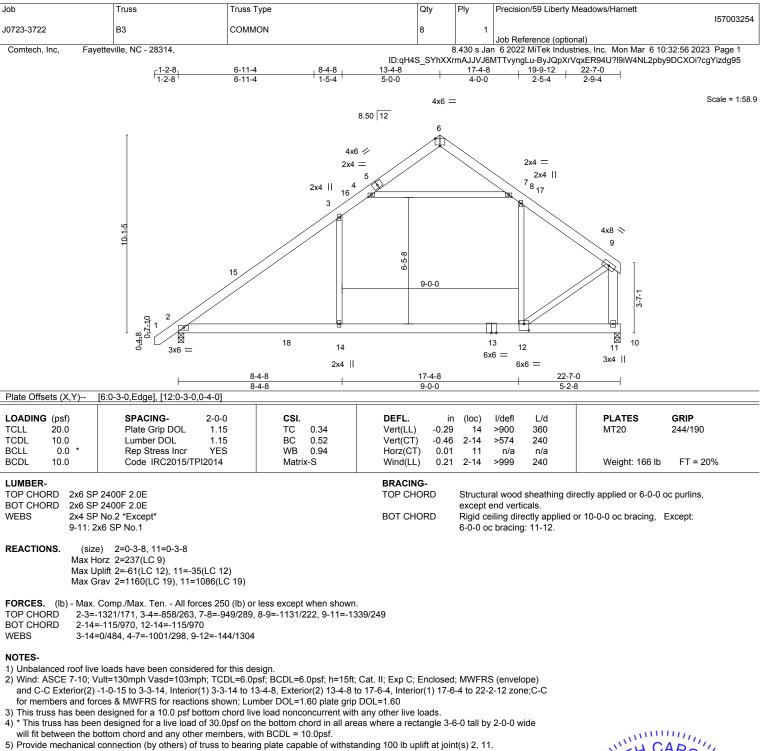
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.



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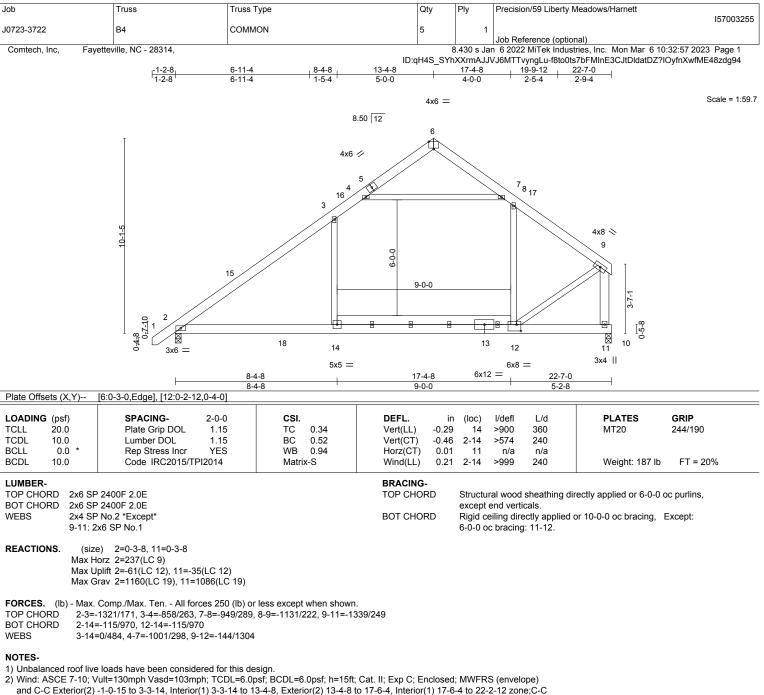




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for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11.



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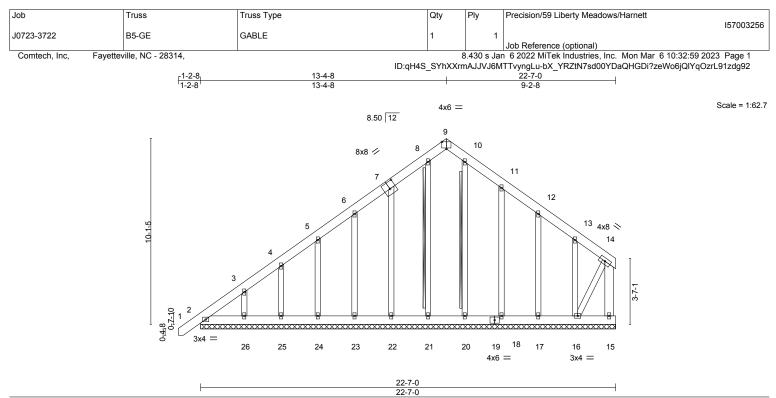


Plate Offsets (X,Y)-- [7:0-4-0,0-4-8], [9:0-3-0,Edge]

| LOADING (psf) TCLL 20.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 | CSI. TC 0.05 | DEFL. Vert(LL) -0. | in (loc) 00 1 | l/defl n/r | L/d 120 | PLATES MT20 | GRIP 244/190 |
|----------------------------|---------------------------------------|------------------------|-----------------------|------------------|---------------|------------|----------------|------------------------|
| TCDL 10.0 | Lumber DOL 1.15 | BC 0.01 | Vert(CT) -0. | 00 1 | n/r | 120 | | |
| BCLL 0.0 * | Rep Stress Incr YES | WB 0.15 | Horz(CT) 0. | 00 15 | n/a | n/a | | |
| BCDL 10.0 | Code IRC2015/TPI2014 | Matrix-S | | | | | Weight: 216 lb | FT = 20% |
| LUMBER- | | | BRACING- | | | | | |

TOP CHORD

BOT CHORD

WEBS

| LUMBER- |
|---------|
|---------|

| LOWIDER. | |
|-----------|----------------------|
| TOP CHORD | 2x6 SP No.1 |
| BOT CHORD | 2x6 SP No.1 |
| WEBS | 2x6 SP No.1 *Except* |
| | 14-16: 2x4 SP No.2 |
| OTHERS | 2x4 SP No.2 |

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

2x4 SPF No.2 - 8-21, 10-20 T-Brace: Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 22-7-0. Max Horz 2=308(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 15, 21, 23, 24, 25, 17 except 22=-106(LC 12), 26=-116(LC 12), 18=-109(LC 13), 16=-290(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 15, 21, 22, 23, 24, 25, 26, 20, 18.17.16

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-343/221

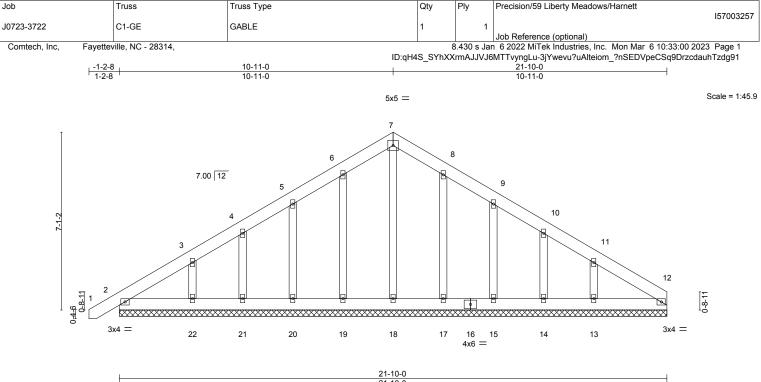
NOTES-

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOI = 1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 15, 21, 23, 24, 25, 17 except (jt=lb) 22=106, 26=116, 18=109, 16=290.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.





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| | | | 21-10-0 | | | | | 1 |
|--------------|-----------------------|----------|----------------|-----------|--------|-----|----------------|----------|
| OADING (psf) | SPACING- 2-0-0 | CSI. | DEFL. in | (loc) | l/defl | L/d | PLATES | GRIP |
| TCLL 20.0 | Plate Grip DOL 1.15 | TC 0.03 | Vert(LL) -0.00 | <u></u> 1 | n/r | 120 | MT20 | 244/190 |
| CDL 10.0 | Lumber DOL 1.15 | BC 0.02 | Vert(CT) 0.00 | 1 | n/r | 120 | | |
| BCLL 0.0 * | Rep Stress Incr YES | WB 0.08 | Horz(CT) 0.00 | 12 | n/a | n/a | | |
| BCDL 10.0 | Code IRC2015/TPI2014 | Matrix-S | . , | | | | Weight: 162 lb | FT = 20% |

LUMBER-

2x6 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 OTHERS

2x4 SP No.2

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 21-10-0. Max Horz 2=206(LC 9) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 19, 20, 21, 17, 15, 14 except 22=-119(LC 12), 13=-127(LC 13) Max Grav All reactions 250 lb or less at joint(s) 12, 2, 18, 19, 20, 21, 22, 17, 15, 14 except 13=262(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

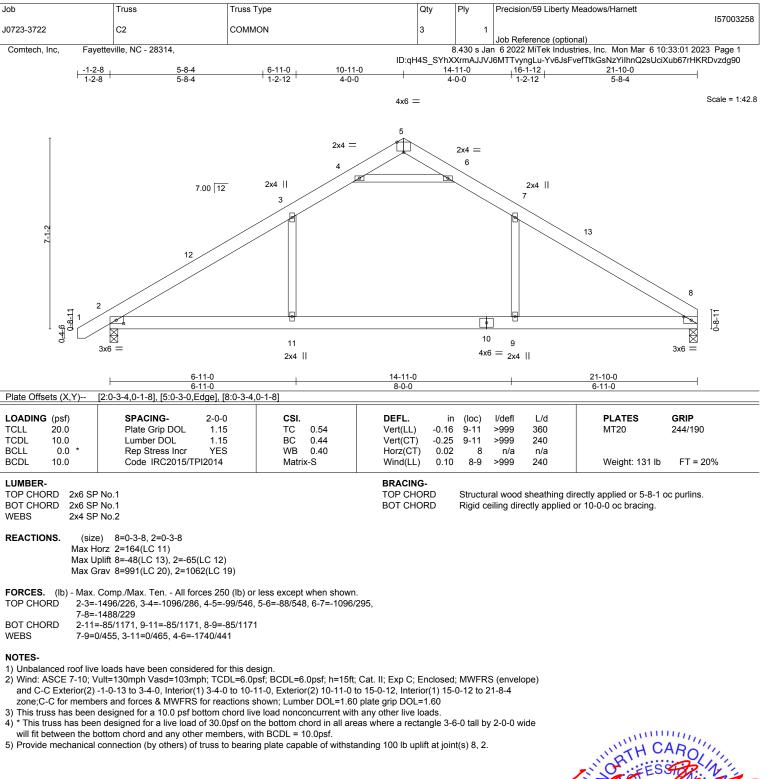
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- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 8) will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 19, 20, 21, 17, 15, 14 except (jt=lb) 22=119, 13=127.



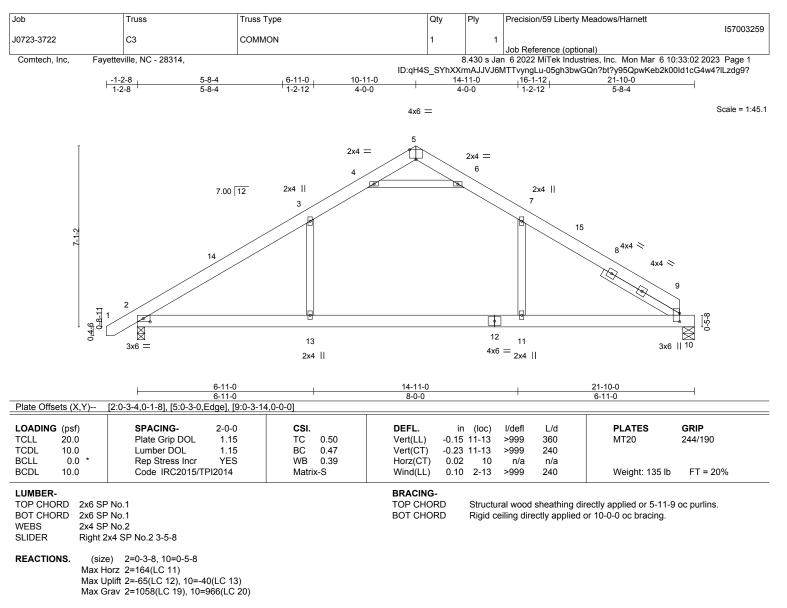
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 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-1478/222, 3-4=-1089/286, 4-5=-84/493, 5-6=-77/502, 6-7=-1079/292, 7-9=-1480/239

 BOT CHORD
 2-13=-82/1157, 11-13=-82/1157, 9-11=-82/1157

WEBS 7-11=0/492, 3-13=0/449, 4-6=-1664/425

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-13 to 3-4-0, Interior(1) 3-4-0 to 10-11-0, Exterior(2) 10-11-0 to 15-0-12, Interior(1) 15-0-12 to 21-3-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

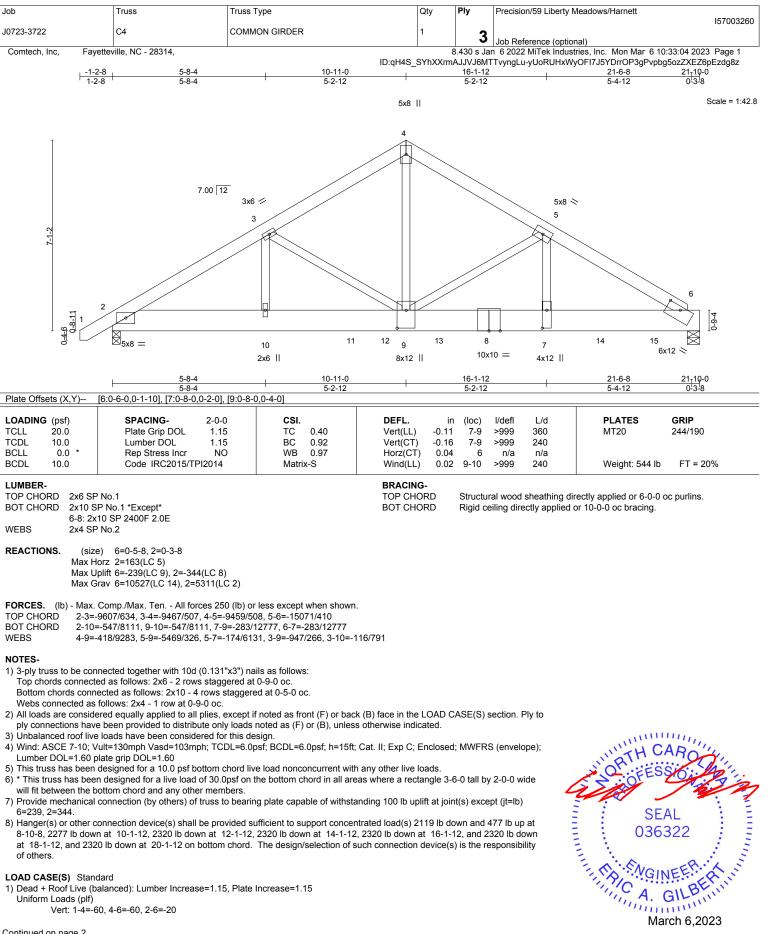
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5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.



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818 Soundside Road Edenton, NC 27932

Continued on page 2

ᄊ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

| Job | Truss | Truss Type | Qty | Ply | Precision/59 Liberty Meadows/Harnett |
|------------------------|-------------------|---------------|-----|-------------|--|
| | | | | | 157003260 |
| J0723-3722 | C4 | COMMON GIRDER | 1 | 2 | |
| | | | | 3 | Job Reference (optional) |
| Comtech, Inc, Fayettev | ille, NC - 28314, | | 8 | 3.430 s Jar | 1 6 2022 MiTek Industries, Inc. Mon Mar 6 10:33:04 2023 Page 2 |

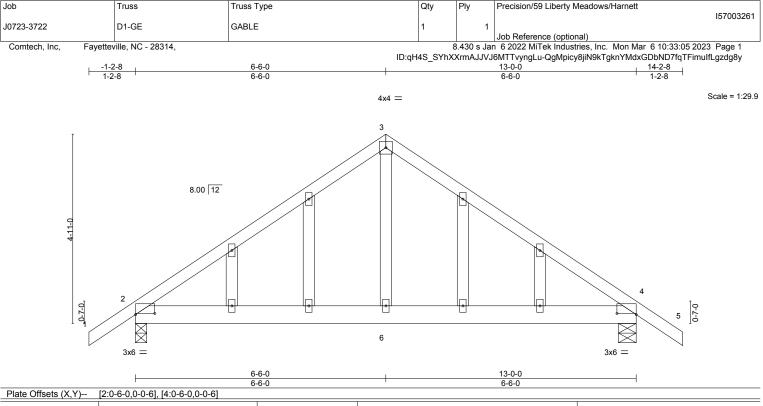
8.430 s Jan 6 2022 Mi lek Industries, Inc. Mon Mar 6 10:33:04 2023 Page 2 ID:qH4S_SYhXXrmAJJVJ6MTTvyngLu-yUoRUHxWyOFI7J5YDrrOP3gPvpbg5ozZXEZ6pEzdg8z

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 8=-598(F) 7=-598(F) 11=-2119(F) 12=-606(F) 13=-598(F) 14=-598(F) 15=-598(F)

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| _OADING (| psf) | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|-----------|-------|-----------------|--------|--------|------|----------|-------|-------|--------|-----|---------------|----------|
| | 20.0 | Plate Grip DOL | 1.15 | TC | 0.35 | Vert(LL) | -0.01 | 2-6 | >999 | 360 | MT20 | 244/190 |
| TCDL 1 | 10.0 | Lumber DOL | 1.15 | BC | 0.17 | Vert(CT) | -0.03 | 2-6 | >999 | 240 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.07 | Horz(CT) | 0.00 | 4 | n/a | n/a | | |
| BCDL 1 | 10.0 | Code IRC2015/TI | PI2014 | Matrix | k-S | Wind(LL) | 0.02 | 2-6 | >999 | 240 | Weight: 78 lb | FT = 20% |

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2 OTHERS 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 4=0-5-8 Max Horz 2=-154(LC 10)

Max Uplift 2=-136(LC 12), 4=-138(LC 13) Max Grav 2=586(LC 1), 4=592(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

2-3=-605/168, 3-4=-606/168 TOP CHORD

BOT CHORD 2-6=-20/416. 4-6=-20/416 WEBS 3-6=0/323

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 2x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

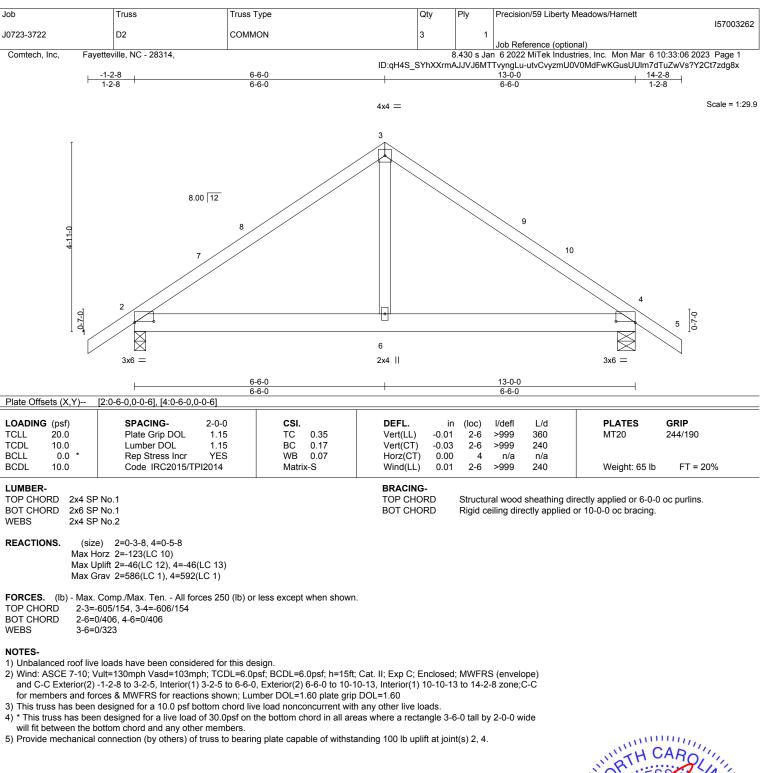
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7) will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=136, 4=138.



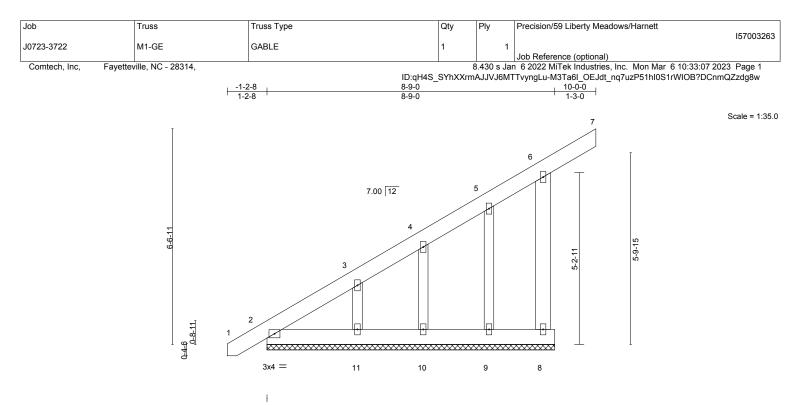
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| OADING (psf) | SPACING- 2-0-0 | CSI. | DEFL. in | (loc) | l/defl L/d | PLATES GRIP |
|--------------|-----------------------|----------|---------------|-------|------------|------------------------|
| CLL 20.0 | Plate Grip DOL 1.15 | TC 0.06 | Vert(LL) 0.00 | 7 | n/r 120 | MT20 244/190 |
| CDL 10.0 | Lumber DOL 1.15 | BC 0.02 | Vert(CT) 0.00 | 6 | n/r 120 | |
| BCLL 0.0 * | Rep Stress Incr YES | WB 0.04 | Horz(CT) 0.00 | | n/a n/a | |
| BCDL 10.0 | Code IRC2015/TPI2014 | Matrix-P | | | | Weight: 74 lb FT = 20% |

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS OTHERS 2x4 SP No.2 TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 8-9-0.

(lb) -Max Horz 2=293(LC 12)

2-3=-333/250

Max Uplift All uplift 100 lb or less at joint(s) 9, 10 except 8=-130(LC 12), 11=-121(LC 12) Max Grav All reactions 250 lb or less at joint(s) 8, 2, 9, 10, 11

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

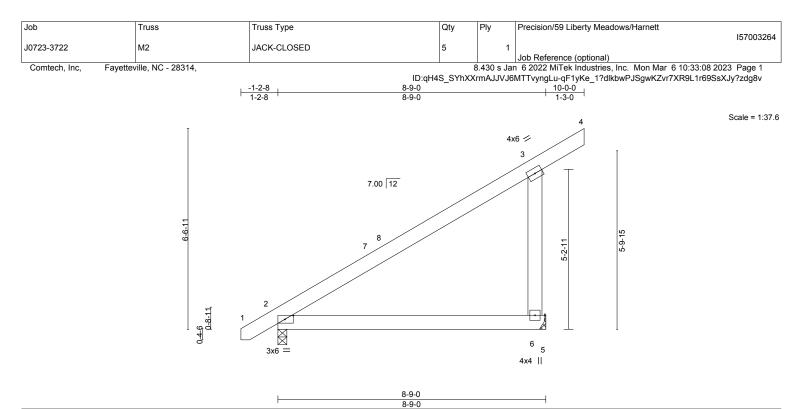
7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 10 except (jt=lb) 8=130, 11=121.



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| LOADING (psf) | SPACING- 2-0-0 | CSI. | DEFL. ir | (loc) | l/defl | L/d | PLATES GR | IP |
|---------------|----------------------|----------|----------------|-------|--------|-----|---------------|----------|
| TCLL 20.0 | Plate Grip DOL 1.15 | TC 0.29 | Vert(LL) -0.03 | 2-6 | >999 | 360 | MT20 244 | /190 |
| TCDL 10.0 | Lumber DOL 1.15 | BC 0.17 | Vert(CT) -0.06 | 2-6 | >999 | 240 | | |
| BCLL 0.0 * | Rep Stress Incr YES | WB 0.00 | Horz(CT) 0.00 | 6 | n/a | n/a | | |
| BCDL 10.0 | Code IRC2015/TPI2014 | Matrix-P | Wind(LL) 0.02 | 2-6 | >999 | 240 | Weight: 62 lb | FT = 20% |

TOP CHORD

BOT CHORD

LUMBER-

2x6 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 WEBS 2x6 SP No.1

REACTIONS. (size) 6=Mechanical, 2=0-3-8 Max Horz 2=201(LC 12)

Max Uplift 6=-122(LC 12)

Max Grav 6=456(LC 19), 2=399(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 3-6=-363/312

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-13 to 3-4-0, Interior(1) 3-4-0 to 10-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 3) will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=122.

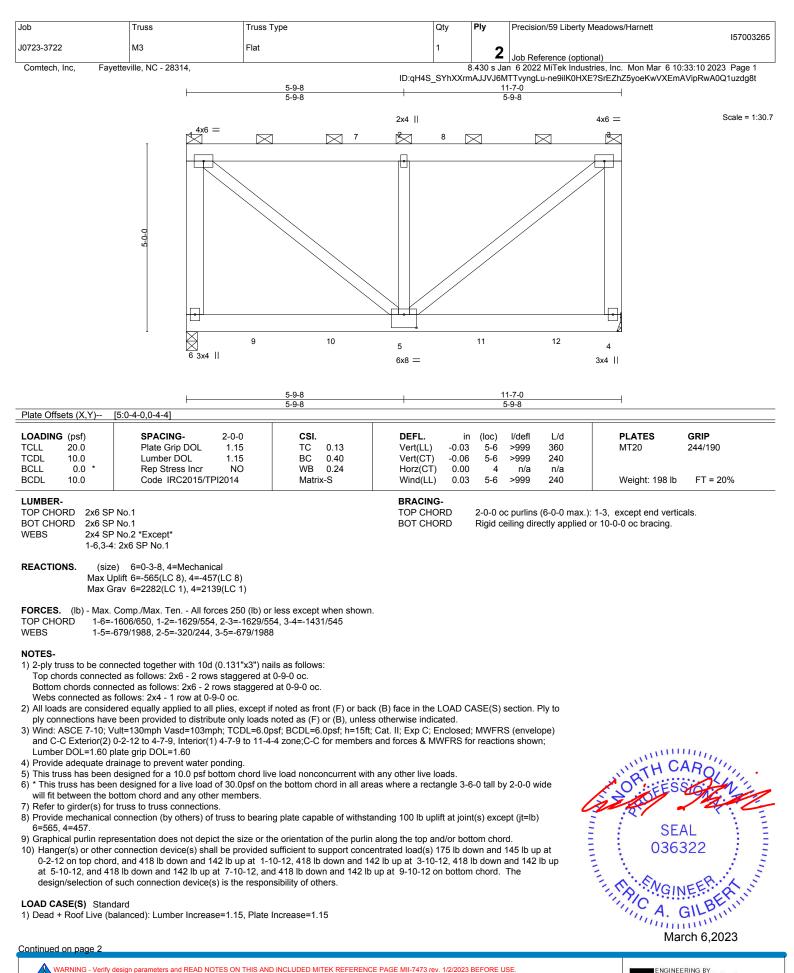


Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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| Job | Truss | Truss Type | Qty | Ply | Precision/59 Liberty Meadows/Harnett |
|------------------------|-------------------|------------|-----|-------------|--|
| | | | | | 157003265 |
| J0723-3722 | M3 | Flat | 1 | 2 | |
| | | | | 2 | Job Reference (optional) |
| Comtech, Inc, Fayettev | ille, NC - 28314, | | 8 | 3.430 s Jar | 6 2022 MiTek Industries, Inc. Mon Mar 6 10:33:10 2023 Page 2 |

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 6 10:33:10 2023 Page 2 ID:qH4S_SYhXXrmAJJVJ6MTTvyngLu-ne9ilK0HXE?SrEZhZ5yoeKwVXEmAVipRwA0Q1uzdg8t

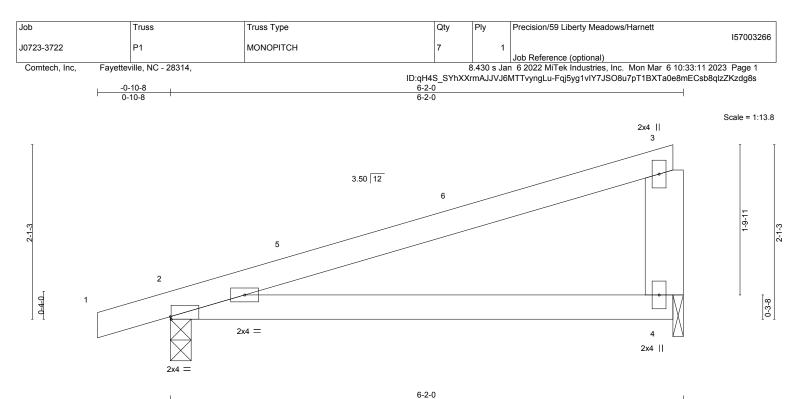
LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-3=-60, 4-6=-134(F=-114) Concentrated Loads (lb)

Vert: 1=-175 5=-418(B) 9=-418(B) 10=-418(B) 11=-418(B) 12=-418(B)

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| | | | | | | 6-2-0 | | | | | | 7 |
|-----------|------------|-----------------|--------|-------|------|----------|-------|-------|--------|-----|---------------|----------|
| Plate Off | sets (X,Y) | [2:0-0-2,Edge] | | | | | | | | | | |
| LOADIN | G (psf) | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
| TCLL | 20.0 | Plate Grip DOL | 1.15 | TC | 0.47 | Vert(LL) | -0.06 | 2-4 | >999 | 360 | MT20 | 244/190 |
| TCDL | 10.0 | Lumber DOL | 1.15 | BC | 0.32 | Vert(CT) | -0.12 | 2-4 | >582 | 240 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.00 | Horz(CT) | 0.00 | | n/a | n/a | | |
| BCDL | 10.0 | Code IRC2015/TE | PI2014 | Matri | x-P | Wind(LL) | 0.13 | 2-4 | >526 | 240 | Weight: 23 lb | FT = 20% |
| | | | | | | | | | | | | |

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1WEBS2x6 SP No.1

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=68(LC 8)

Max Uplift 2=-120(LC 8), 4=-95(LC 8) Max Grav 2=298(LC 1), 4=227(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 5-11-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=120.



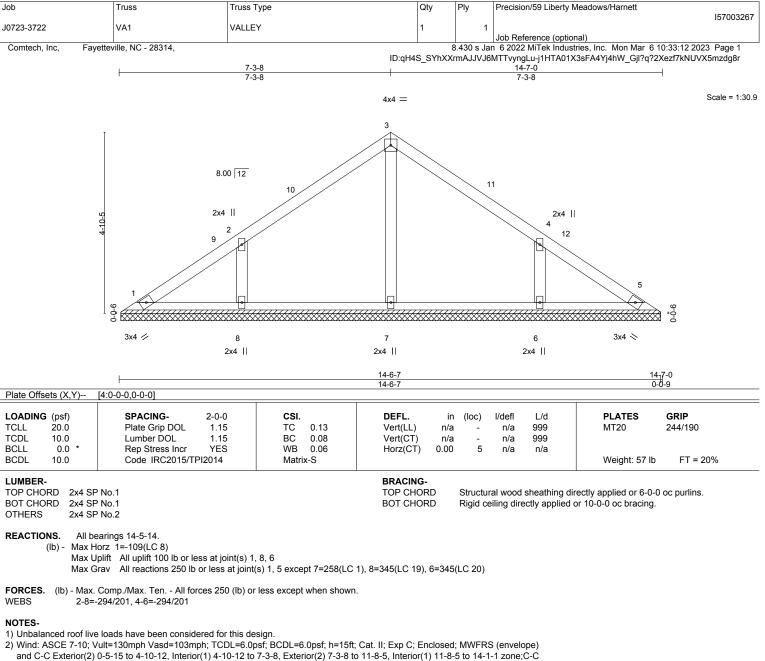
Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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A MiTek Affili 818 Soundside Road Edenton, NC 27932



for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

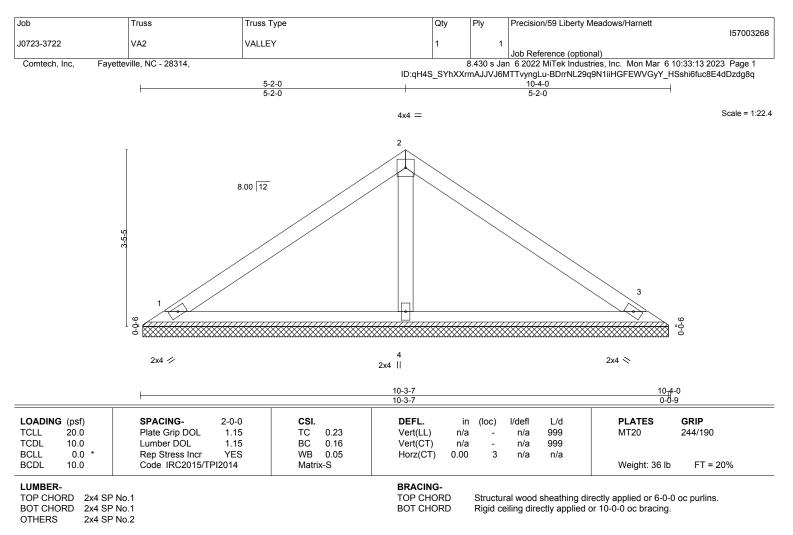
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 8, 6.



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818 Soundside Road Edenton, NC 27932



REACTIONS. (size) 1=10-2-14, 3=10-2-14, 4=10-2-14 Max Horz 1=-75(LC 8) Max Uplift 1=-23(LC 12), 3=-30(LC 13)

Max Grav 1=186(LC 1), 3=186(LC 1), 4=377(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

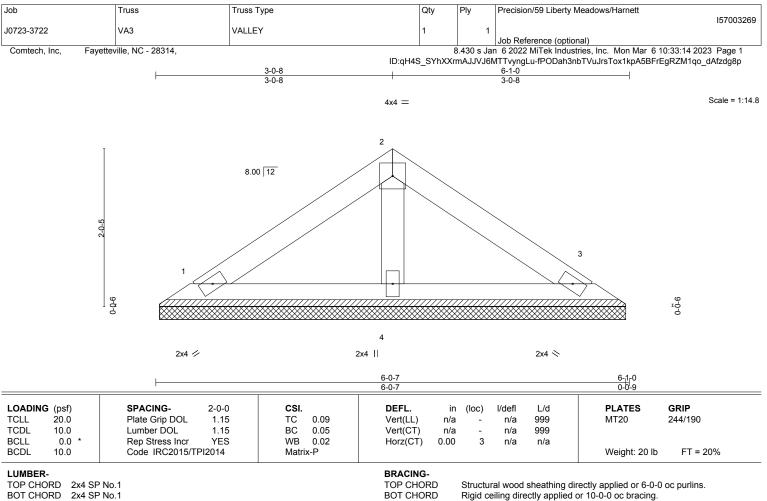
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. 1=5-11-14, 3=5-11-14, 4=5-11-14 (size) Max Horz 1=-41(LC 10)

Max Uplift 1=-17(LC 12), 3=-21(LC 13)

Max Grav 1=111(LC 1), 3=111(LC 1), 4=186(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

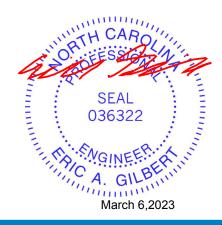
2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

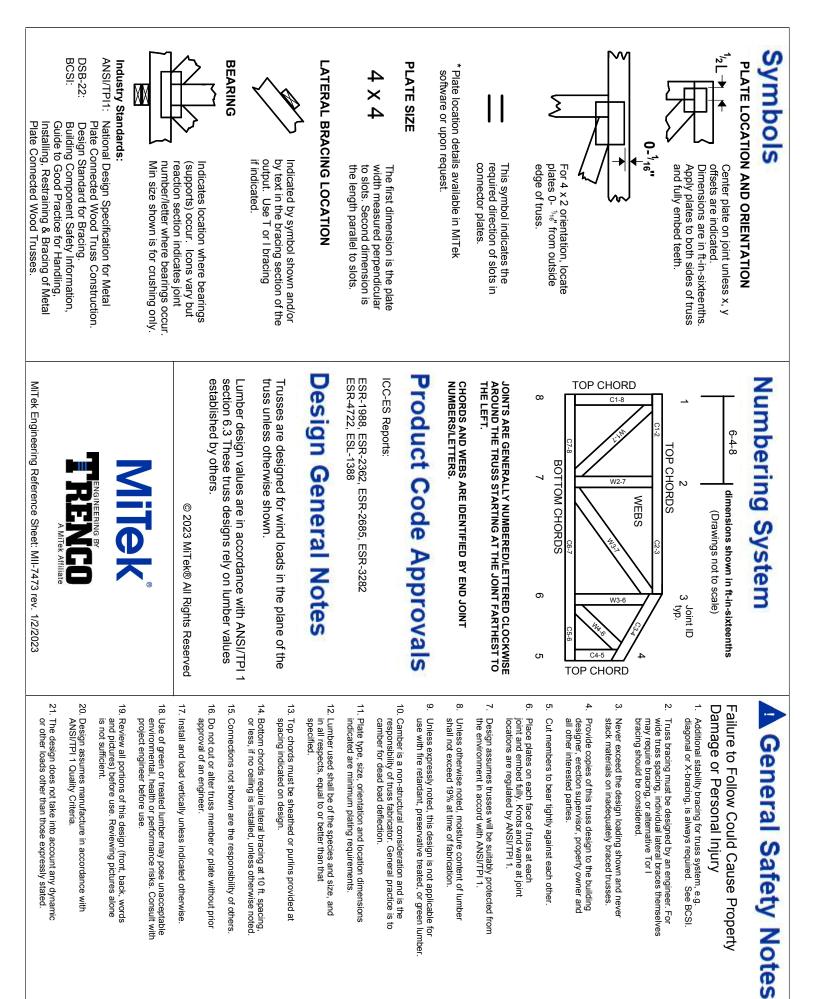
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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RE: J0723-3723 Precision/59 Liberty Meadows/Harnett **Trenco** 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: J0723-3723 Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.4 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 15 individual, dated Truss Design Drawings and 0 Additional Drawings.

| No. | Seal# | Truss Name | Date |
|-----|-----------|------------|----------|
| | | | |
| 1 | 157003270 | ET1 | 3/6/2023 |
| 2 | 157003271 | ET2 | 3/6/2023 |
| 3 | 157003272 | ET3 | 3/6/2023 |
| 4 | 157003273 | F1 | 3/6/2023 |
| 5 | 157003274 | F2 | 3/6/2023 |
| 6 | 157003275 | F3 | 3/6/2023 |
| 7 | 157003276 | F4 | 3/6/2023 |
| 8 | 157003277 | F5 | 3/6/2023 |
| 9 | 157003278 | F6 | 3/6/2023 |
| 10 | 157003279 | F6A | 3/6/2023 |
| 11 | 157003280 | F7 | 3/6/2023 |
| 12 | 157003281 | F8 | 3/6/2023 |
| 13 | 157003282 | FG-1 | 3/6/2023 |
| 14 | 157003283 | FG-2 | 3/6/2023 |
| 15 | 157003284 | FG-3 | 3/6/2023 |

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

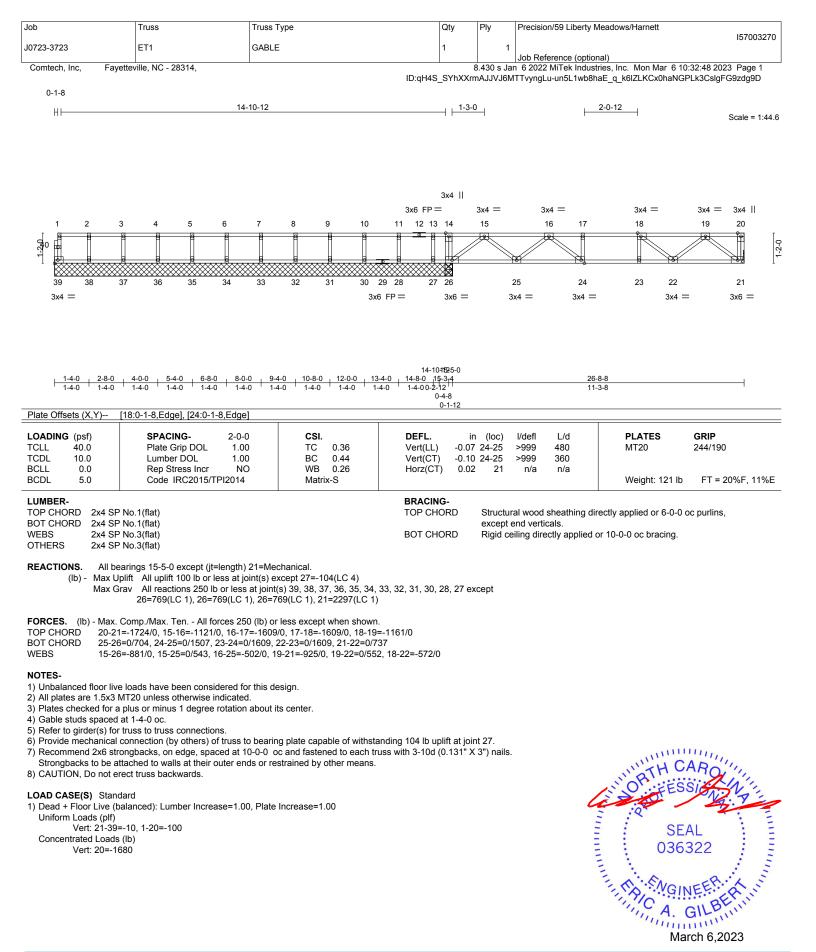
Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2023

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



| Job | Truss | Truss Type | | | Qty | Ply | Precision/59 Liberty | Meadows/Harnett | 157003271 |
|--|---|---------------------------|---------------------------------------|-------------------------------|-----------------------|-------------|---|---------------------------------|---------------------------------|
| J0723-3723 | ET2 | GABLE | | | 1 | 1 | 1 | | 157003271 |
| Comtech, Inc, Fa | yetteville, NC - 28314, | | | | | 2 4 2 0 o 1 | Job Reference (option | onal) stries, Inc. Mon Mar 6 | 10:22:50 2022 Page 1 |
| | yelleville, NC - 20514, | | | ID:qH4 | | | | BUh3HuUszNoHM56jA | |
| 0 ₁ 18 | | | | | | | | | 0 ₁ 1 ₇ 8 |
| | | | | | | | | | Scale = 1:21. |
| | | | | | | | | | |
| 1 2 | 2 3 | 4 | 5 | 6 | - | 7 | 8 | 9 | 10 11 |
| | | • | | | | | • | | |
| 22 2 | 21 20 | 19 | 18 | 17 | | 16 | 15 | 14 | 13 12 |
| 3x4 = | | 10 | 10 | | | | | | 3x4 = |
| <u> 1-4-0</u> 1-4-0 | + 2-8-0 + 4- 1-4-0 1- | 0-0 <u>5-4</u> 4-0 1-4 | -0 | <u>6-8-0</u> 1.4-0 | <u>8-0-0</u> 1-4-0 | + | 9-4-0 10- 1-4-0 1-4 | 3-0 <u>12-0-0</u> -0 14-0 | <u></u> |
| | | | | | | | | | |
| LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 | SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr | 1.00 1.00 | CSI. TC 0.06 BC 0.01 WB 0.03 | DEF Vert(Vert(Horz | (LL) n/a (CT) n/a | - | l/defl L/d n/a 999 n/a 999 n/a n/a | PLATES MT20 | GRIP 244/190 |
| BCDL 5.0 | Code IRC2015/TPI2 | | Matrix-R | | | | | Weight: 55 lb | FT = 20%F, 11%E |
| | P No.1(flat) P No.1(flat) | | | | CING- CHORD | | Iral wood sheathing c end verticals. | irectly applied or 6-0-0 |) oc purlins, |

BOT CHORD2x4 SP No.1(flat)WEBS2x4 SP No.3(flat)OTHERS2x4 SP No.3(flat)

BOT CHORD

except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 12-11-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 22, 12, 21, 20, 19, 18, 17, 16, 15, 14, 13

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

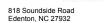
4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



| 1530000 | erty Meadows/Harnett | recision/59 Libert | Ply I | Qty | | ре | Truss Ty | Truss | lob |
|-----------------------|--|--------------------|-------|-----------------------|----------------|----------------|----------------|----------------------------|----------------------|
| 15700327 | | | 1 | 1 | | | GABLE | ET3 | 723-3723 |
| 0.20.51 2022 . Dogo 1 | optional) ndustries, Inc. Mon Mar 6 | ob Reference (opt | | | | | | eville, NC - 28314, | Comtech, Inc, Fayet |
| | d0_VcYhRThQhu1qZeHSa | | | | ID:gH4 | | | eville, NC - 20314, | contech, inc, Fayer |
| 0 ₁₁ 8 | | | | - | | | | | 0 ₁₁₇ 8 |
| Scale = 1:20 | | | | | | | | | |
| 40 44 | <u>,</u> | | - | | 0 | F | | ^ | |
| 10 11 | 9 | 8 | 7 | | 6 | 5 | 4 | 3 | 1 2 |
| 24 | | • • | • | | | <u>e</u> | <u>•</u> | | |
| | | • | • | | • | 0 | 0 | | d a |
| | | | | | | | | | |
| 13 12 3x4 = | 0 | 15 | 16 | | 17 | 18 | 19 | 20 | 22 21 3x4 = |
| 3x4 = | | | | 800 | | | | | 3x4 = |
| -0 <u>12-7-8</u> | 10-8-0 12-0 14 | 94-0 1-4-0 | | <u>8-0-0</u> 1-4-0 | 6-8-0 1-4-0 | 5-4-0 1-4-0 | 4-0-0 1-4-0 | 20 20 2-8-0 1-4-0 | |

BOT CHORD 2x4 SP No.1(flat) 2x4 SP No.3(flat) WEBS OTHERS 2x4 SP No.3(flat) BOT CHORD

except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 12-7-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 22, 12, 21, 20, 19, 18, 17, 16, 15, 14, 13

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

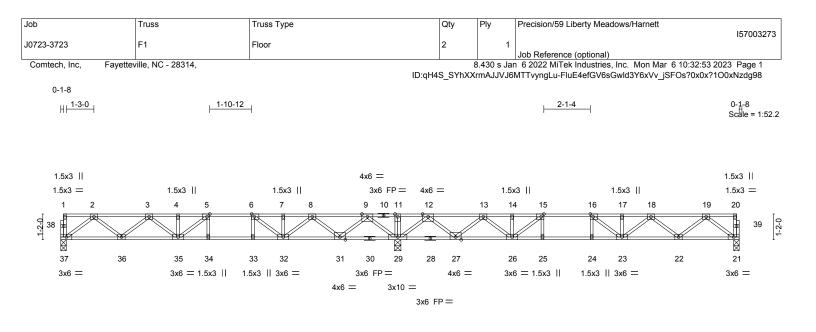
5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



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| | 15-3-4 | | | 30-9-0 15-5-12 | | | | | | | |
|---|---|--|--|-------------------|-------------------|-------------------------------|--------------------------|---|---|--|--|
| Plate Offsets (X,Y)- | Plate Offsets (X,Y) [5:0-1-8,Edge], [6:0-1-8,Edge], [15:0-1-8,Edge], [16:0-1-8,Edge] | | | | S | | | | | | |
| LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0 | SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014 | CSI. TC 0.74 BC 0.87 WB 0.54 Matrix-S | DEFL. Vert(LL) Vert(CT) Horz(CT) | -0.16 | 23-24 | l/defl >999 >853 n/a | L/d 480 360 n/a | PLATES MT20 Weight: 156 lb | GRIP 244/190 FT = 20%F, 11%E | | |
| BOT CHORD 2x4 WEBS 2x4 REACTIONS. (| SP No.1(flat) SP No.1(flat) SP No.3(flat) Size) 37=0-3-0, 29=0-3-8, 21=0-3-0 x Grav 37=728(LC 3), 29=1989(LC 1), 21= | -739(LC 4) | BRACING- TOP CHOR BOT CHOR | D | except Rigid d | t end verti ceiling dire | cals. | ectly applied or 6-0-0 o r 10-0-0 oc bracing, I ,27-29,26-27. | | | |
| TOP CHORD 2- 8- | ax. Comp./Max. Ten All forces 250 (lb) or 3=-1450/0, 3-4=-2252/0, 4-5=-2252/0, 5-6= 9=-521/736, 9-11=0/2242, 11-12=0/2242, 1 +15=-1787/207, 15-16=-2337/0, 16-17=-23 | -2276/0, 6-7=-1763/236, 7 2-13=-518/703, 13-14=-1 | 787/207, | | | | | | | | |
| BOT CHORD 36 31 | 5-37=0/901, 35-36=0/1969, 34-35=0/2276, 1-32=-456/1270, 29-31=-1091/0, 27-29=-11 4-25=0/2337, 23-24=0/2337, 22-23=0/2008 | 33-34=0/2276, 32-33=0/22 10/0, 26-27=-424/1283, 25 | 276, | | | | | | | | |
| WEBS 2- 8- 1 | 37=-1128/0, 2-36=0/714, 3-36=-676/0, 3-39 31=-1065/0, 19-21=-1146/0, 19-22=0/730, 2-27=0/1128, 13-27=-1079/0, 8-32=0/707, 5-26=-1020/0, 16-23=-156/376 | 5=0/362, 9-29=-1522/0, 9-3 18-22=-693/0, 18-23=0/38 | 0, 12-29=-1537/0 | 3 | | | | | | | |
| , | r live loads have been considered for this de | esign. | | | | | | | | | |

All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

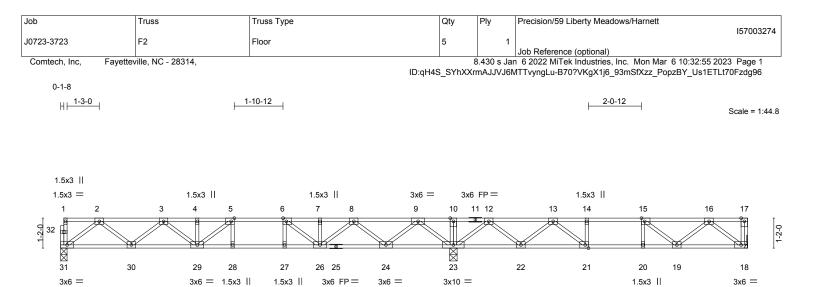
Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



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A MiTek Aft 818 Soundside Road Edenton, NC 27932



3x6 =

| l | 15-3-4 15-3-4 | | | | | 26-8-8 | | | | |
|---|--|--|--|------------------------------|-------------------------|-------------------------------|--------------------------|--|---|--|
| Plate Offsets (X, | | 8,Edge], [21:0-1-8,Edge] | | | | | 1 | | | |
| LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0 | SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014 | CSI. TC 0.60 BC 0.83 WB 0.50 Matrix-S | DEFL. Vert(LL) Vert(CT) Horz(CT) | in -0.15 -0.20 0.03 | (loc) 28 28 18 | l/defl >999 >909 n/a | L/d 480 360 n/a | PLATES MT20 Weight: 135 lb | GRIP 244/190 FT = 20%F, 11%E | |
| LUMBER- TOP CHORD 2 BOT CHORD 2 | 2x4 SP No.1(flat) 2x4 SP No.1(flat) 2x4 SP No.3(flat) | | BRACING- TOP CHOF BOT CHOF | RD | except | end vert | icals. | rectly applied or 6-0-0 or 6-0-0 oc bracing. | , | |
| REACTIONS. | (size) 31=0-3-0, 23=0-3-8, 18=Mechanica Max Grav 31=741(LC 3), 23=1724(LC 1), 18= | | | | U | U | , | Ŭ | | |
| FORCES. (Ib) - TOP CHORD | Max. Comp./Max. Ten All forces 250 (lb) or 17-18=-1835/0, 2-3=-1483/0, 3-4=-2315/0, 4- 7-8=-1892/0, 8-9=-677/263, 9-10=0/1758, 10 13-14=-1242/104, 14-15=-1242/104, 15-16=- | -5=-2315/0, 5-6=-2372/0, 6 -12=0/1758, 12-13=-482/6 | 6-7=-1892/0, | | | | | | | |
| BOT CHORD | 13-141242/104, 14-131242/104, 13-16 30-31=0/919, 29-30=0/2017, 28-29=0/2372, 24-26=-29/1412, 23-24=-586/0, 22-23=-886/0 19-20=-104/1242, 18-19=0/652 | 27-28=0/2372, 26-27=0/23 | , | | | | | | | |
| WEBS | 2-31=-1150/0, 2-30=0/735, 3-30=-696/0, 3-29 8-24=-1014/0, 12-23=-1169/0, 12-22=0/754, 16-19=-46/407, 8-26=0/663, 6-26=-851/0, 5-7 14-21=-279/0 | 13-22=-766/0, 16-18=-818 | 3/0, | | | | | | | |
| 2) All plates are 3 | oor live loads have been considered for this de 3x4 MT20 unless otherwise indicated. d for a plus or minus 1 degree rotation about i | • | | | | | | | 110. | |

Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

 Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 18-31=-10, 1-17=-100

Concentrated Loads (lb) Vert: 17=-1800



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TRENCO A Mitek Atfiliate

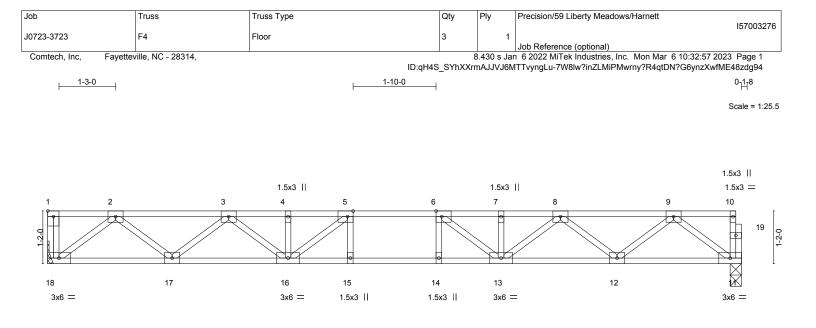
| [| | | | | | | |
|--|--|--|---|-------------------------------|--|--|--|
| Job | | Truss Type | Qty | Ply | Precision/59 Liberty | /leadows/Harnett | 157003275 |
| J0723-3723 | | GABLE | 1 | 1 | Job Reference (option | | 0.22.56 2022 Dans 4 |
| Comtech, Inc, Fay 0-1-8 ∐⊢ <u>1-3-0</u> | etteville, NC - 28314, | -12 | | | | iries, Inc. Mon Mar 61 101ErnCLeDEUCXdL32 | |
| H | | | | | | | Scale = 1:44.5 |
| | 3x4 = 3x4 = 3x4 = 3x4 = 3x4 = 5 3x4 = 3x6 = | 34 33 | 3x4 = 6 FP = 3x4 9 10 11 32 31 30 x4 = 3x6 FP = 3x6 = | | 13 14 15 10 10 10 10 10 10 10 10 10 10 | 16 17 16 17 25 24 | 3x4 18 19 20 19 20 23 22 21 3x4 |
| | <u>15-1-</u> 15-1- | | 16-7-4 15-3-4 0-1-12 1-4-0 | | | -11-4 23-3-4 24-7-4 -4-0 1-4-0 1-4-0 | |
| | [5:0-1-8,Edge], [6:0-1-8,Edge] | | | | | _ | |
| LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0 | SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014 | CSI. TC 0.27 BC 0.56 WB 0.41 Matrix-S | Vert(LL) -0.16 | (loc) 34-35 34-35 21 | l/defl L/d >999 480 >842 360 n/a n/a | PLATES MT20 Weight: 127 lb | GRIP 244/190 FT = 20%F, 11%E |
| BOT CHORD 2x4 SP WEBS 2x4 SP | No.1(flat) No.1(flat) No.3(flat) No.3(flat) | | BRACING- TOP CHORD BOT CHORD | except | al wood sheathing di end verticals. eiling directly applied o | rectly applied or 6-0-0 or 10-0-0 oc bracing. | oc purlins, |
| (lb) - Max U | arings 11-7-0 except (jt=length) 3 plift All uplift 100 lb or less at joir rav All reactions 250 lb or less a 30=992(LC 1), 30=992(LC 1) | nt(s) 29 | 25, 24, 23, 22 except 38 | =821(LC | 3), | | |
| TOP CHORD 2-3=- | Comp./Max. Ten All forces 250 1684/0, 3-4=-2691/0, 4-5=-2691/0 1613/0 | | | | | | |
| 30-3 WEBS 2-38= | 3=0/1023, 36-37=0/2310, 35-36=0. 2=0/954 =-1281/0, 2-37=0/860, 3-37=-815/0 |), 3-36=0/487, 10-30=-1196/0, | | | | | |
| NOTES- 1) Unbalanced floor livv 2) All plates are 1.5x3 l 3) Plates checked for a 4) Gable studs spaced 5) Provide mechanical 6) Recommend 2x6 str | connection (by others) of truss to ongbacks, on edge, spaced at 10- ttached to walls at their outer ends | this design. bout its center. bearing plate capable of withsta 0-0 oc and fastened to each tr | uss with 3-10d (0.131" X | · · / | 6 | 036 | ARO SSIDE V SAL S322 NEER RATION |
| | | | | | | | arch 6,2023 |

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RENGINEERING BY A MITRIK Atfiliate

March 6,2023

⁸¹⁸ Soundside Road Edenton, NC 27932



| | | | <u>15-4-0</u> 15-4-0 | | | | | |
|---|--|--|---|---|-------------------------------|--------------------------|--|---|
| Plate Offsets (X,Y) | [1:Edge,0-1-8], [5:0-1-8,Edge], [6:0-1-8 | ,Edge] | | | | | | |
| LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0 | SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014 | CSI. TC 0.35 BC 0.66 WB 0.41 Matrix-S | DEFL. Vert(LL) Vert(CT) Horz(CT) | in (loc) -0.16 14-15 -0.22 14-15 0.04 11 | l/defl >999 >839 n/a | L/d 480 360 n/a | PLATES MT20 Weight: 79 lb | GRIP 244/190 FT = 20%F, 11%E |
| BOT CHORD 2x4 S WEBS 2x4 S REACTIONS. (s | SP No.1(flat) SP No.1(flat) SP No.3(flat) ize) 18=Mechanical, 11=0-3-0 Grav 18=830(LC 1), 11=823(LC 1) | | BRACING- TOP CHOR BOT CHOR | except | end vert | icals. | rectly applied or 6-0-0 or 10-0-0 oc bracing. |) oc purlins, |
| TOP CHORD 2-3 8-9 BOT CHORD 17- | x. Comp./Max. Ten All forces 250 (lb) o =-1691/0, 3-4=-2704/0, 4-5=-2704/0, 5-6=)=-1691/0 -18=0/1028, 16-17=0/2320, 15-16=0/2966 -12=0/1027 | -2966/0, 6-7=-2704/0, 7-8 | 8=-2704/0, | 20, | | | | |
| WEBS 2-1 | 8=-1289/0, 2-17=0/864, 3-17=-818/0, 3-1 | 6=0/490, 5-16=-596/25, 9 | -11=-1285/0, | | | | | |

9-12=0/865, 8-12=-819/0, 8-13=0/490, 6-13=-596/25

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

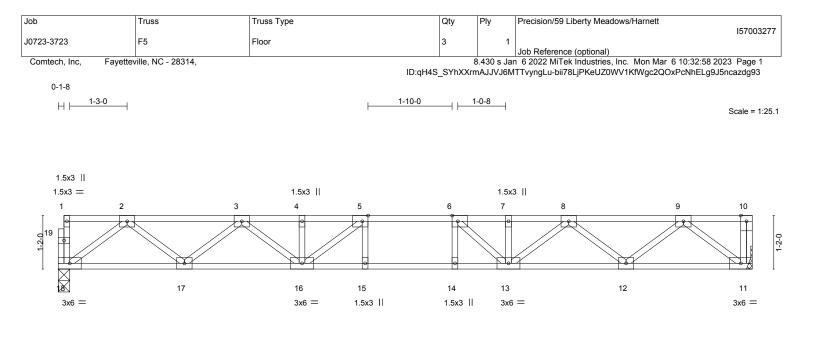
4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oclapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of frusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCI8. Building component forth. Information, and information. Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



| Plate Offsets (X,Y) | [5:0-1-8,Edge], [6:0-1-8,Edge] | | 15-1-8 15-1-8 | | | |
|---|---|--|--|---|------------------------|------------------------|
| LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 | SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES | CSI. TC 0.36 BC 0.66 WB 0.40 | DEFL. ir Vert(LL) -0.15 Vert(CT) -0.21 Horz(CT) 0.04 | 5 15 >999 480 14-15 >867 360 | PLATES MT20 | GRIP 244/190 |
| BCDL 5.0 | Code IRC2015/TPI2014 | Matrix-S | | | Weight: 78 lb | FT = 20%F, 11%E |
| | ? No.1(flat) No.1(flat) | | BRACING- TOP CHORD | Structural wood sheathing dir except end verticals. | ectly applied or 6-0-0 | oc purlins, |
| | | | | Rigid ceiling directly applied of | or 10-0-0 oc bracing. | |
| REACTIONS. (size Max G | e) 18=0-3-0, 11=Mechanical rav 18=812(LC 1), 11=818(LC 1) | | | | | |
| FORCES. (Ib) - Max. | Comp./Max. Ten All forces 250 (lb) or | less except when shown. | | | | |

TOP CHORD 2-3=-1662/0, 3-4=-2649/0, 4-5=-2649/0, 5-6=-2884/0, 6-7=-2641/0, 7-8=-2641/0, 8-9=-1663/0 BOT CHORD 17-18=0/1012, 16-17=0/2278, 15-16=0/2884, 14-15=0/2884, 13-14=0/2884, 12-13=0/2278, 11-12=0/1012 WEBS 2-18=-1267/0, 2-17=0/846, 3-17=-802/0, 3-16=0/475, 5-16=-562/38, 9-11=-1270/0, 9-12=0/847, 8-12=-800/0, 8-13=0/464, 6-13=-587/29

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

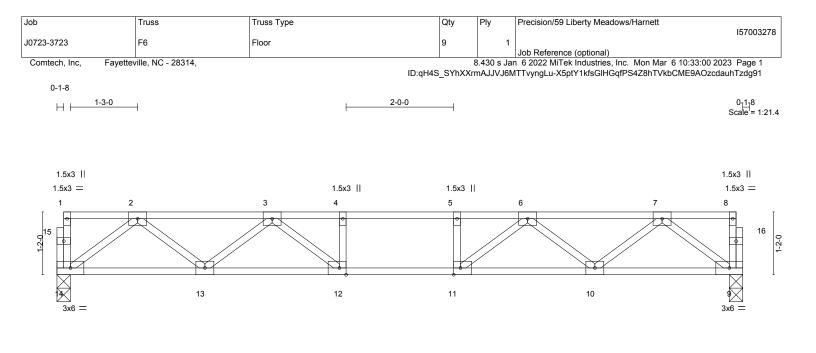
6) CAUTION, Do not erect truss backwards.



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| <u> </u> | | | <u>12-9-0</u> 12-9-0 | | | |
|---|---|--|------------------------------------|---|--|---|
| Plate Offsets (X,Y) | [11:0-1-8,Edge], [12:0-1-8,Edge] | | | | | |
| LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0 | SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014 | CSI. TC 0.35 BC 0.44 WB 0.30 Matrix-S | Vert(LL) -0.09 | n (loc) l/defl L/d 9 12-13 >999 480 2 12-13 >999 360 3 9 n/a n/a | MT20 | GRIP 244/190 FT = 20%F, 11%E |
| BOT CHORD 2x4 SF | ^{>} No.1(flat) ^{>} No.1(flat) ^{>} No.3(flat) :e) 14=0-3-0, 9=0-3-0 | | BRACING- TOP CHORD BOT CHORD | except end verticals. | hing directly applied or 6-0-0 pplied or 10-0-0 oc bracing. |) oc purlins, |

CTIONS. (size) 14=0-3-0, 9=0-3-0 Max Grav 14=681(LC 1), 9=681(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1333/0, 3-4=-2022/0, 4-5=-2022/0, 5-6=-2022/0, 6-7=-1333/0

BOT CHORD 13-14=0/841, 12-13=0/1790, 11-12=0/2022, 10-11=0/1790, 9-10=0/841

WEBS 2-14=-1053/0, 2-13=0/640, 3-13=-595/0, 3-12=0/499, 7-9=-1053/0, 7-10=0/640,

6-10=-595/0, 6-11=0/499

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

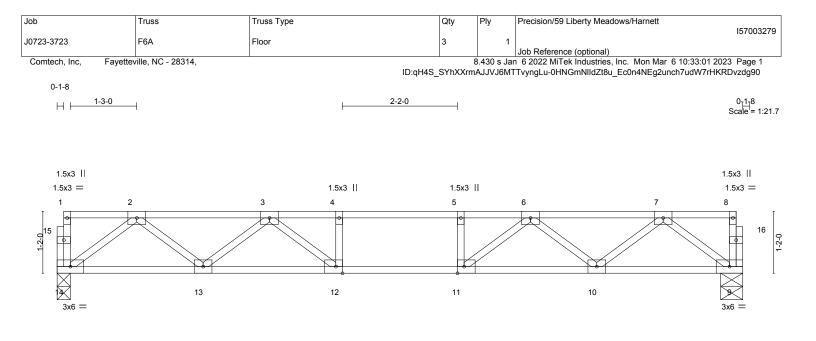
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



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| | | | <u>12-11-0</u> 12-11-0 | | | |
|---|---|--|------------------------------------|---|---------------------------------|---|
| Plate Offsets (X,Y) | [11:0-1-8,Edge], [12:0-1-8,Edge] | | | | 1 | |
| LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0 | SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014 | CSI. TC 0.39 BC 0.47 WB 0.31 Matrix-S | Vert(LL) -0.1 | n (loc) l/defl L/d 0 12-13 >999 480 3 12-13 >999 360 3 9 n/a n/a | PLATES MT20 Weight: 64 lb | GRIP 244/190 FT = 20%F, 11%E |
| BOT CHORD 2x4 SF | ² No.1(flat) ² No.1(flat) ² No.3(flat) | | BRACING- TOP CHORD BOT CHORD | Structural wood sheathing di except end verticals. Rigid ceiling directly applied | <i>y</i> |) oc purlins, |

Max Grav 14=690(LC 1), 9=690(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1356/0, 3-4=-2072/0, 4-5=-2072/0, 5-6=-2072/0, 6-7=-1356/0

BOT CHORD 13-14=0/854, 12-13=0/1823, 11-12=0/2072, 10-11=0/1823, 9-10=0/854

WEBS 2-14=-1069/0, 2-13=0/654, 3-13=-609/0, 3-12=0/526, 4-12=-251/0, 5-11=-251/0,

7-9=-1069/0, 7-10=0/654, 6-10=-609/0, 6-11=0/526

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

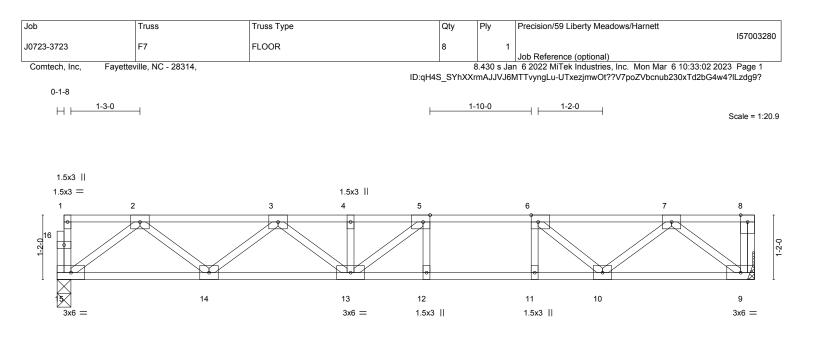
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



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| F | | | | 12-7-8 12-7-8 | | | |
|---|---------------------------------------|---|---|------------------------------------|--|--|---|
| Plate Offs | ets (X,Y) | [5:0-1-8,Edge], [6:0-1-8,Edge] | | | | | |
| LOADING TCLL TCDL BCLL BCDL | i (psf) 40.0 10.0 0.0 5.0 | SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014 | CSI. TC 0.48 BC 0.78 WB 0.32 Matrix-S | Vert(LL) -0.14 | n (loc) l/defl L/d 4 12-13 >999 480 8 12-13 >821 360 2 9 n/a n/a | PLATES MT20 Weight: 65 lb | GRIP 244/190 FT = 20%F, 11%E |
| LUMBER- TOP CHO BOT CHO WEBS | RD 2x4 SF RD 2x4 SF | P No.1(flat) P No.1(flat) P No.3(flat) | | BRACING- TOP CHORD BOT CHORD | Structural wood sheathing dire except end verticals. Rigid ceiling directly applied or | , ,, | oc purlins, |

REACTIONS. (size) 15=0-3-0, 9=Mechanical

Max Grav 15=674(LC 1), 9=681(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1313/0, 3-4=-1995/0, 4-5=-1995/0, 5-6=-1895/0, 6-7=-1327/0

BOT CHORD 14-15=0/831, 13-14=0/1770, 12-13=0/1895, 11-12=0/1895, 10-11=0/1895, 9-10=0/807

2-15=-1040/0, 2-14=0/628, 3-14=-594/0, 3-13=0/288, 5-13=-224/289, 7-9=-1012/0,

WEBS 7-10=0/677, 6-10=-745/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

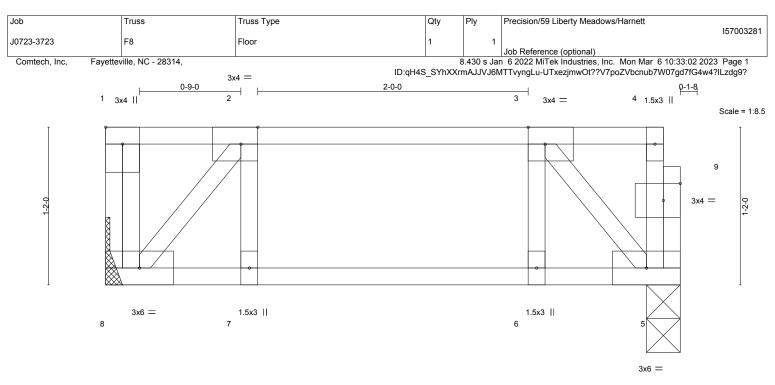
Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.



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| | ļ | | 4-3-0 | | | |
|---|---|--|---|---|---------------------------------|---|
| | | | 4-3-0 | | | |
| Plate Offsets (X,Y) | [1:Edge,0-1-8], [2:0-1-8,Edge], [3:0-1-8, | Edge], [9:0-1-8,0-1-8] | | | | |
| LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0 | SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014 | CSI. TC 0.13 BC 0.06 WB 0.06 Matrix-S | DEFL. i Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) 0.00 | 0 6 >999 360 | PLATES MT20 Weight: 24 lb | GRIP 244/190 FT = 20%F, 11%E |
| BOT CHORD 2x4 SF | ² No.1(flat) ² No.1(flat) ² No.3(flat) | | BRACING- TOP CHORD BOT CHORD | Structural wood sheathing di except end verticals. Rigid ceiling directly applied | <i>y</i> | oc purlins, |

REACTIONS. (size) 8=Mechanical, 5=0-3-0 Max Grav 8=220(LC 1), 5=214(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-8=-273/0, 3-5=-272/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

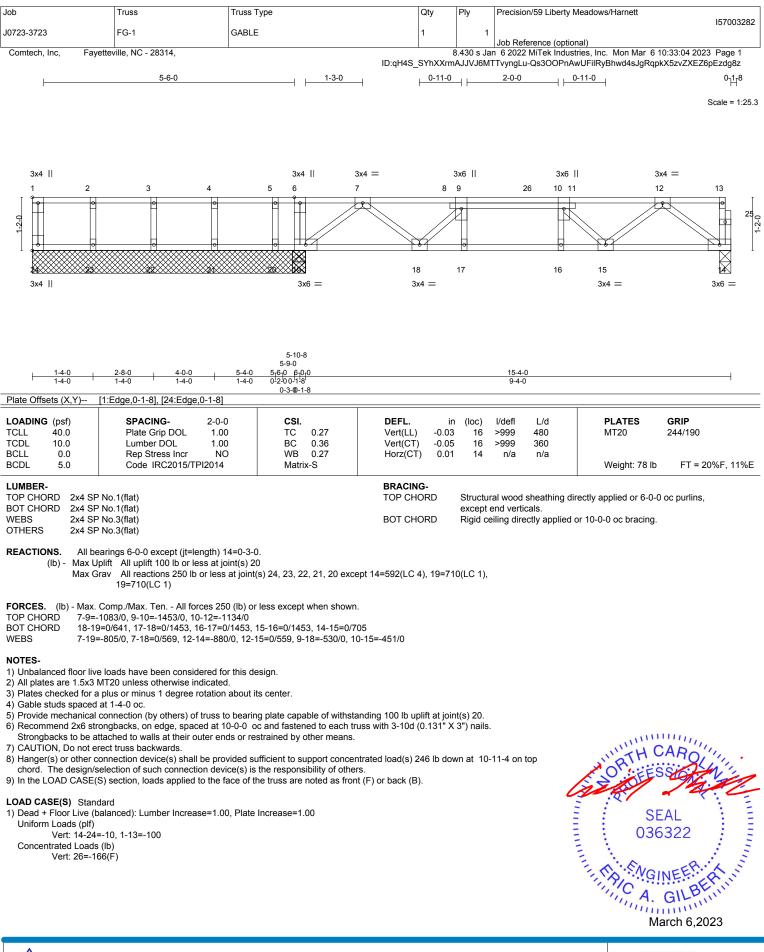
Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



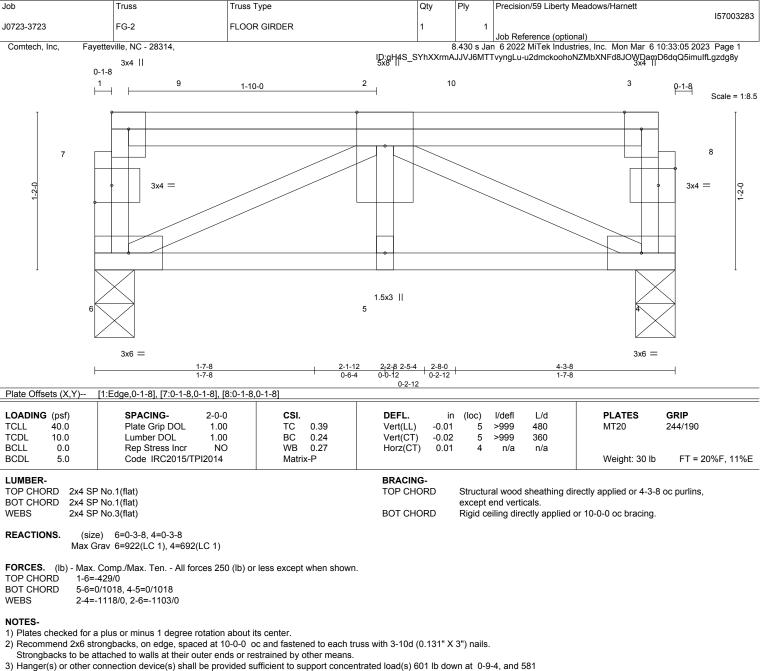
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Ib down at 2-9-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.

4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 4-6=-10, 1-3=-100 Concentrated Loads (lb)

Vert: 9=-601(B) 10=-581(B)

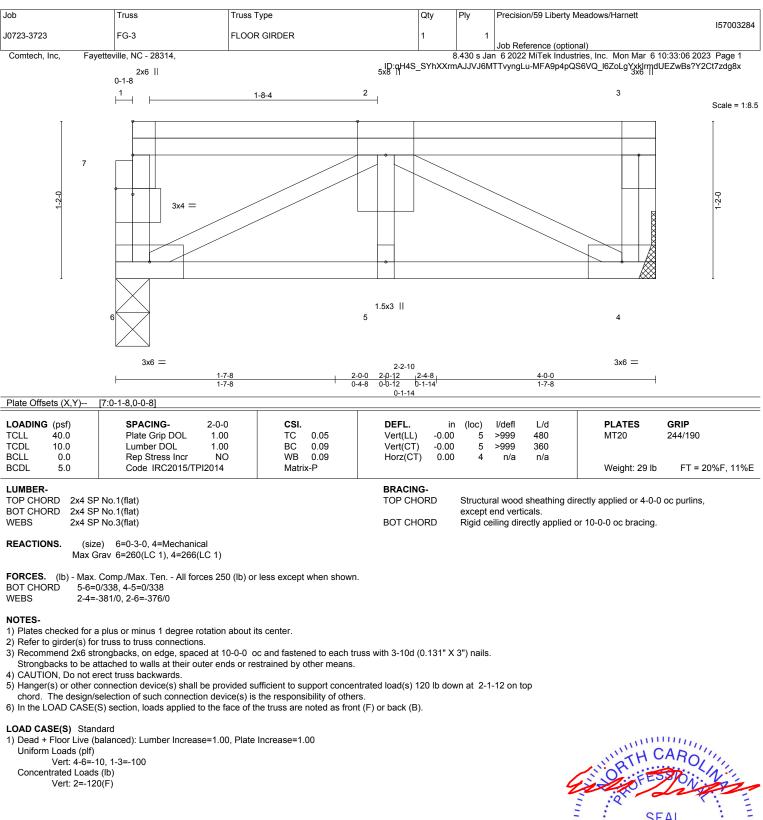


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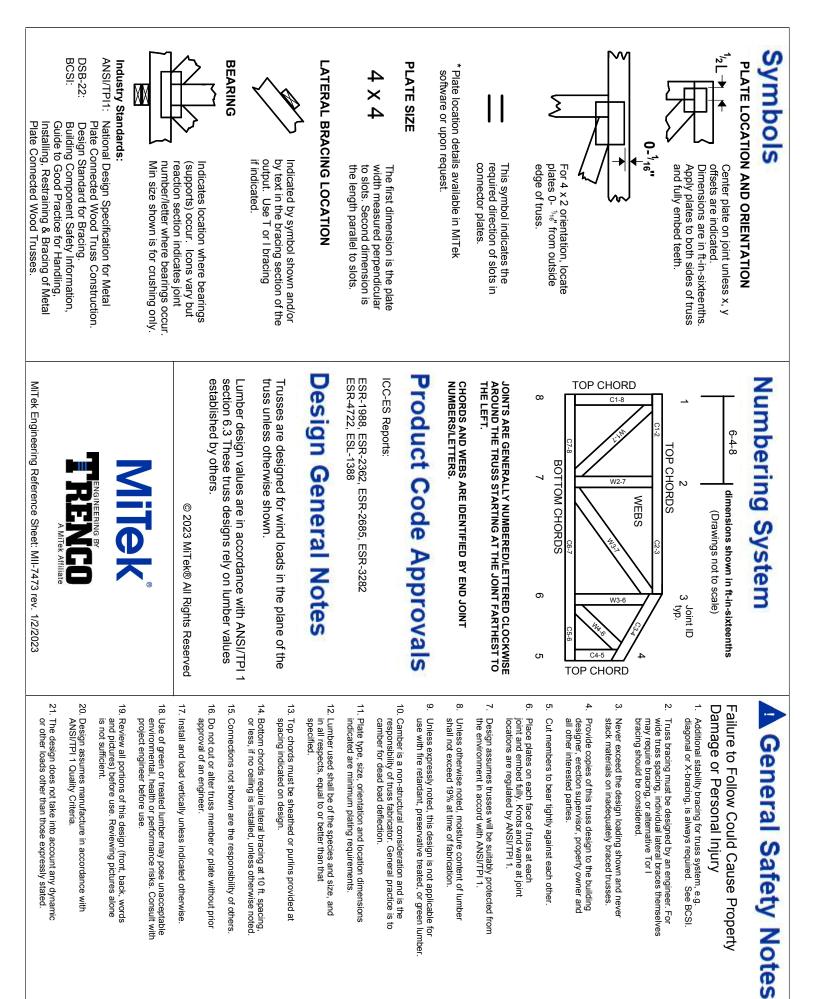
Edenton, NC 27932

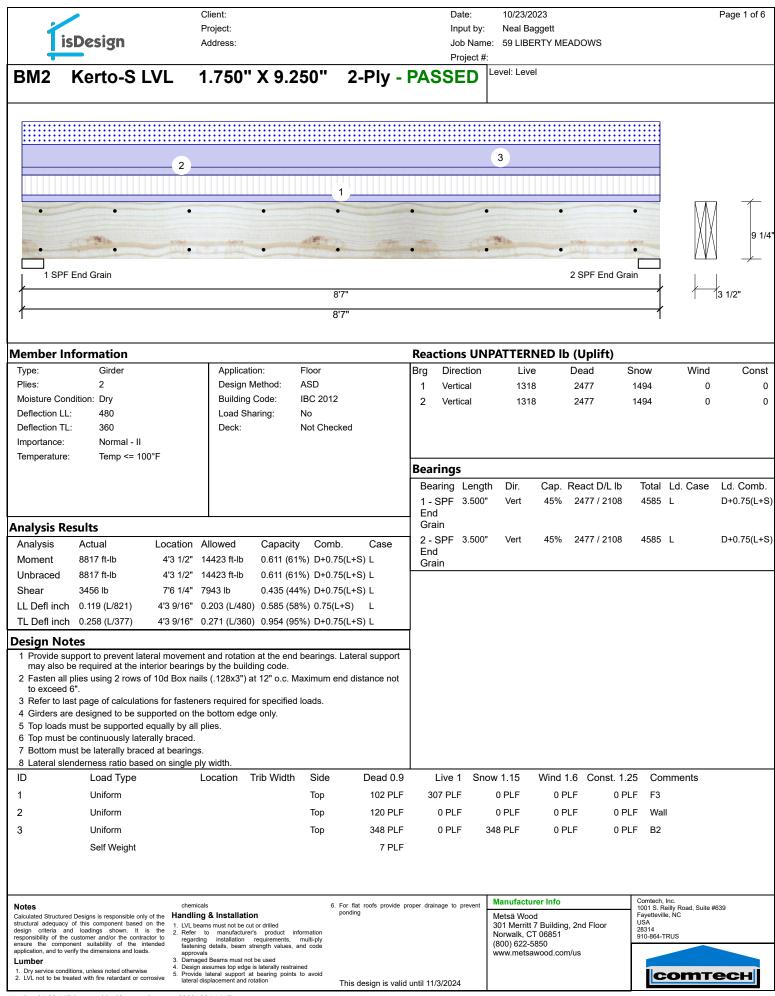




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Version 21.80.417 Powered by iStruct[™] Dataset: 23091201.1447

| | | Client: | | | Date: | 10/23/2023 | Page 2 of 6 |
|-----------------------------|--|--|---|----------------------------------|--------------------|---|--|
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| | y Analysis | | | | | | |
| | I plies using 2 rows | | 128x3") at 12" o.c. | . Maximum enc | l distance n | ot to exceed 6". | |
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| Yield Limit p | | 81.9 lb. | | | | | |
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| Notes | | chemicals | | flat roofs provide proper onding | rainage to prevent | Manufacturer Info | Comtech, Inc. 1001 S. Reilly Road, Suite #639 |
| structural adeq | ictured Designs is responsible only of t juacy of this component based on t | he 1. LVL beams must not be cut | or drilled | | | Metsä Wood 301 Merritt 7 Building, 2nd Floor | Fayetteville, NC USA 28314 |
| responsibility of | a and loadings shown. It is the off the customer and/or the contractor component suitability of the intender | to regarding installation | s product information requirements, multi-ply | | | Norwalk, CT 06851 (800) 622-5850 | 28314 910-864-TRUS |
| application, and | d to verify the dimensions and loads. | fastening details, beam st approvals Damaged Beams must not | | | | www.metsawood.com/us | |
| | conditions, unless noted otherwise | Design assumes top edge i Provide lateral support at | s laterally restrained bearing points to avoid | | | | соттесн |
| 2. LVL not to b | be treated with fire retardant or corrosi | lateral displacement and ro | | is design is valid until | 11/3/2024 | | Connech |

| Í is | 5Design | F | Client: Project: Address: | | | | | Date: Input b Job Na | - | 10/23/20 Neal Bag 59 LIBEF | gett | | | | Page 3 of |
|---|--|---------------------------------------|---------------------------------------|----------------------------|--------------------------|-----------------------|--------------|----------------------------|-------|----------------------------------|---|--------------|-------------------|--------------------|-----------|
| | | P | duress: | | | | | Projec | | 29 LIBE | | ADOW5 | | | |
| GDH | Kerto-S L | VL 1. | 750" እ | (11.8 | 575" 2 | 2-Ply - I | PASS | SED | Lev | vel: Level | | | | | |
| | | 2 | | | | | | | | | | | | | |
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| | and the second s | | • | | att the second | | · | i - | | • | National Anna anna anna anna anna anna anna a | | | M | 14.7/0 |
| | | STATES AND A STATES | and the second | | | • | - | • | • | • | | | · · - | | 11 7/8" |
| 1 SPF Er | nd Grain | | | | | | | | | | | 2 SPF Er | nd Grain | | |
|] | | | | | 16'7 | | | | | | | | | 1 13 | 1/2" |
| I | | | | | 16'7 | [** | | | | | | | | I | |
| lember In | formation | | | | | | Reac | tions U | JNPA | TTERN | IED lb | (Uplift) | | | |
| Туре: | Girder | | Applicati | | Floor | | Brg | Directio | | Live | | Dead | Snow | Wind | Con |
| Plies: Moisture Con | 2 Idition: Drv | | Design M Building | | ASD IBC 2012 | | 1 | Vertical Vertical | | 0 | | 1154 1154 | 1078 1078 | 0 | |
| Deflection LL | , | | Load Sha | | No | | 2 | vertical | | 0 | | 1154 | 1078 | 0 | |
| Deflection TL | : 240 | | Deck: | | Not Checke | ed | | | | | | | | | |
| Importance: | Normal - II | | | | | | | | | | | | | | |
| Temperature: | Temp <= 10 | 0°F | | | | | Deer | • | | | | | | | |
| | | | | | | | Bear | - | | | | | | | |
| | | | | | | | | ring Ler | - | Dir. | • | React D/L I | | | Ld. Com |
| | | | | | | | 1 - S End | SPF 3.5 | 00" | Vert | 22% | 1154 / 107 | 8 2232 | 2 L | D+S |
| nalysis Re | sults | | I | | | | Gra | | | | | | | | |
| Analysis | Actual | Location A | llowed | Capaci | ty Comb. | Case | | SPF 3.5 | 00" | Vert | 22% | 1154 / 107 | 8 2232 | 2 L | D+S |
| Moment | 8751 ft-lb | | 2897 ft-lb | | 8%) D+S | L | End | | | | | | | | |
| Unbraced | 8751 ft-lb | 8'3 1/2" 8 | 756 ft-lb | 0.999 | D+S | L | Gia | | | | | | | | |
| | | | | (100%) | | | | | | | | | | | |
| Shear | 1897 lb | 15'3 5/8" 1 | | | 9%) D+S | L | | | | | | | | | |
| | 0.214 (L/904) | 8'3 9/16" 0 | ``` | | , | L | | | | | | | | | |
| TL Defl inch | 0.444 (L/436) | 8'3 9/16" 0 | .806 (L/240) | 0.550 (5 | 5%) D+S | L | - | | | | | | | | |
| esign No | | | | | | | 4 | | | | | | | | |
| | pport to prevent lat be required at the in | | | | d bearings. Li | ateral support | | | | | | | | | |
| 2 Fasten all | plies using 2 rows o | • | | • | Maximum end | d distance not | | | | | | | | | |
| to exceed 3 Refer to la | 6". st page of calculatio | ons for fastener | s required fo | or specifie | d loads | | | | | | | | | | |
| | e designed to be su | | • | • | | | | | | | | | | | |
| | must be supported | | | | | | | | | | | | | | |
| | be laterally braced a list be laterally brace | | | 0.C. | | | | | | | | | | | |
| | nderness ratio base | | 0 | | | | | | | | | | | | |
| ID | Load Type | L | ocation 1 | rib Width | n Side | Dead 0.9 |) I | _ive 1 S | Snow | 1.15 | Wind 1 | .6 Const. | 1.25 C | omments | |
| 1 | Uniform | | | | Тор | 120 PLF | - | 0 PLF | 120 | PLF | 0 PI | _F (| PLF C | I-GE | |
| 2 | Tie-In | 0-0-0 te | o 16-7-0 C | -6-0 | Тор | 20 PSF | - | 0 PSF | 20 | PSF | 0 P\$ | SF C | PSF R/ | AKE OH | |
| | Self Weight | | | | | 9 PLF | - | | | | | | | | |
| | | | | | | | | | | | | | | | |
| lotes | | chemical | s | | | or flat roofs provide | proper drain | age to preve | nt Ma | anufactur | er Info | | Comtec 1001 S. | Reilly Road, Suite | #639 |
| Calculated Structured tructural adequacy | d Designs is responsible only of this component based | on the 1 IVI beam | & Installatio | | ро | onding | | | | etsä Wood 1 Merritt 7 | | , 2nd Floor | Fayette USA | ville, NC | |
| esign criteria an esponsibility of the | d loadings shown. It i customer and/or the contra | s the 2. Refer t ctor to regarding | o manufacturer' installation | s product requirements, | information multi-ply | | | | No | orwalk, CT | 06851 | , 1 1001 | 28314 910-864 | -TRUS | |
| nsure the compo opplication, and to ve | nent suitability of the in erify the dimensions and loads | tended fastening approvals | details, beam st | rength values, | | | | | | 00) 622-58 ww.metsav | | ı/us | | | |
| umber | 141 | Design a | d Beams must not ssumes top edge i | s laterally restr | ained | | | | | | | | | | |
| . Dry service condi | ated with fire retardant or co | 5 Provide | lateral support at | bearing point | s to avoid | | | | | | | | | COMT | |

Version 21.80.417 Powered by iStruct[™] Dataset: 23091201.1447

| Client: | Date: | 10/23/2023 | Page 4 of 6 |
|---|--|--|---|
| Project: | Input by | | |
| isDesign Address: | Job Nan | ne: 59 LIBERTY MEADOWS | |
| | Project | <u><u><u>+</u></u></u> | |
| GDH Kerto-S LVL 1.750" X 11.875" | 2-Ply - PASSED | Level: Level | |
| | Z-I IY - I ACCED | | |
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| | | | . 11 7/8" |
| 1 SPF End Grain | | 2 SPF End (| |
| | | 2 SFF Ellu | |
| / | 16'7" | | 3 1/2" |
| | | | |
| 1 | 16'7" | | |
| | | | |
| | | | |
| Multi-Ply Analysis | | | |
| Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" | o.c. Maximum end distance r | not to exceed 6". | |
| Capacity 0.0 % | | | |
| Load 0.0 PLF | | | |
| Yield Limit per Foot 163.7 PLF | | | |
| Yield Limit per Fastener 81.9 lb. | | | |
| Yield Mode IV | | | |
| Edge Distance 1 1/2" | | | |
| Min. End Distance 3" | | | |
| Load Combination | | | |
| Duration Factor 1.00 | | | |
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| | | Manufacturer Info | Comtech, Inc. |
| Notes chemicals Calculated Structured Designs is responsible only of the Handling & Installation | 6. For flat roofs provide proper drainage to prevent ponding | Metsä Wood | 1001 S. Reilly Road, Suite #639 Fayetteville, NC |
| structural adequacy of this component based on the 1. LVL beams must not be cut or drilled | | 301 Merritt 7 Building, 2nd Floor | USA 28314 |
| design criteria and loadings shown. It is the 2. Refer to manufacturer's product information responsibility of the customer and/or the contractor to regarding installation requirements, multi-ply | | Norwalk, CT 06851 | 28314 910-864-TRUS |
| application, and to verify the dimensions and loads. fastening details, beam strength values, and code approvals | | (800) 622-5850 www.metsawood.com/us | |
| Lumber 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained | | | |
| 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive 3. Provide lateral support at beam points to avoid lateral displacement and rotation | This design is well-twell 44/9/0004 | | соттесн |
| | This design is valid until 11/3/2024 | L | |

| | • | С | lient: | | | | | Dat | te: | 10/23/20 |)23 | | | | Page 5 of |
|--|--|--|--|---|------------------------------------|-------------------|------------|------------|----------|--|----------------------------|--------------|--|--------------------|-----------|
| | Destars | | roject: | | | | | - | ut by: | Neal Ba | | | | | |
| IS | Design | A | ddress: | | | | | | Name | e: 59 LIBE | RTY ME | ADOWS | | | |
| | | // / - | | 4 0 0 0 | | | | | oject #: | Level: Leve | 1 | | | | |
| BM1 k | Kerto-S LV | /L 1./ | (50" X 1 | 4.000 | 2-1 | PIY - P | AS | SED | | | • | | | | |
| 1 SPF | 2 | 1 | • | 2 SPF | | | | | | | | | | | 1'2" |
| / | ł | 5'6 1/2" | | | | | | | | | | | | | 3 1/2" |
| <u>/</u> | | 5'6 1/2" | | \longrightarrow | | | | | | | | | | | |
| I | C | JU 1/2 | | I | | | | | | | | | | | |
| Member Inf | formation | | | | | | Rea | ction | s UN | PATTERI | NED Ib | (Uplift) | | | |
| Туре: | Girder | | Application: | Floo | or | | Brg | Direc | | Live | | Dead | Snow | Wind | Con |
| Plies: | 2 | | Design Meth | | | | 1 | Vertic | | 1721 | | 604 | 0 | 0 | |
| Moisture Cond | | | Building Coo | | 2012 | | 2 | Vertic | cal | 1721 | | 604 | 0 | 0 | |
| Deflection LL: | 480 | | Load Sharin | • | | | | | | | | | | | |
| Deflection TL: | | | Deck: | Not | Checked | | | | | | | | | | |
| Importance: Temperature: | Normal - II Temp <= 100 | ∘⊏ | | | | | | | | | | | | | |
| remperature. | Temp <= 100 | F | | | | | Bea | rings | | | | | | | |
| | | | | | | | | aring l | | n Dir. | Cap. I | React D/L lb | Total | Ld. Case | Ld. Com |
| | | | | | | | | SPF 3 | - | Vert | 45% | 604 / 1721 | 2324 | | D+L |
| | | | | | | | | SPF 3 | | Vert | 45% | 604 / 1721 | 2324 | | D+L |
| Analysis Re | sults | | | | | | | | | | - | | - | | |
| Analysis | Actual | Location A | llowed C | apacity | Comb. | Case | 7 | | | | | | | | |
| Moment | 2710 ft-lb | 2'9 1/4" 20 | 6999 ft-lb 0 | .100 (10%) | D+L | L | | | | | | | | | |
| Unbraced | 2710 ft-lb | 2'9 1/4" 18 | 8950 ft-lb 0 | .143 (14%) | D+L | L | | | | | | | | | |
| Shear | 2080 lb | 1'5 1/2" 10 | 0453 lb 0 | 199 (20%) | D+L | L | | | | | | | | | |
| LL Defl inch | 0.011 (L/5785) | 2'9 1/4" 0. | .127 (L/480) 0 | .083 (8%) | L | L | | | | | | | | | |
| TL Defl inch | 0.014 (L/4282) | 2'9 1/4" 0. | .169 (L/360) 0 | .084 (8%) | D+L | L | | | | | | | | | |
| Design Not | es | | | | | | Τ | | | | | | | | |
| may also be 2 Fasten all p to exceed 6 3 Refer to las 4 Girders are 5 Top must be 6 Bottom must | poprt to prevent latele e required at the inte lies using 3 rows of ". t page of calculation designed to be sup e laterally braced at t be laterally braced derness ratio based | erior bearings 10d Box nails ns for fastener: ported on the end bearings. d at end bearir | by the building (.128x3") at 12 s required for s bottom edge or ngs. | code. 2" o.c. Maxin pecified load | num end di | | | | | | | | | | |
| ID | Load Type | | | Width S | Side | Dead 0.9 | | Live 1 | Sno | w 1.15 | Wind 1 | .6 Const. 1. | 25 Con | nments | |
| 1 | Uniform | | | | lear Face | 105 PLF | 3 | 15 PLF | | 0 PLF | 0 PI | | LF F4 | | |
| 2 | Uniform | | | | ar Face | 102 PLF | | 06 PLF | | 0 PLF | 0 PI | | LF F5 | | |
| - | Self Weight | | | 1 | | 11 PLF | 5 | | | . LI | 511 | . 01 | | | |
| Notes | | chemicals | | | | t roofs provide p | proper dra | inage to p | vrevent | Manufactu | rer Info | | Comtech, I 1001 S. Re | illy Road, Suite # | 639 |
| Calculated Structured structural adequacy of design criteria and responsibility of the c ensure the compon application, and to veri Lumber 1. Dry service conditi | Designs is responsible only of of this component based or loadings shown. It is ustomer and/or the contract ent suitability of the inter suitability of the inter fy the dimensions and loads. ons, unless noted otherwise ted with fire retardant or corr | 1. LVL beam the nded 2. Refer to regarding fastening approvals 3. Damaged 4. Design as 5. Provide li | Beams must not be us sumes top edge is late ateral support at bea | roduct informati rements, multi- th values, and co sed erally restrained ring points to av | pondin ion ply ide oid | g | | | Ī | Metsä Woo 301 Merritt Norwalk, C (800) 622-5 www.metsa | 7 Building 06851 850 | | Fayetteville USA 28314 910-864-TF | e, NC | |
| | | lateral dis | placement and rotation | 1 | | design is valio | l until 11 | /3/2024 | | | | | | | |

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| P | | Client: | | Date: | 10/23/2023 | Page 6 of 6 |
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| 1 | isDesign | Project: Address: | | Input by: Job Nam | | |
| Ţ. | | | | Project # | | |
| BM1 | Kerto-S LV | ′L 1.750'' X 14. | 000" 2-Ply | - PASSED | Level: Level | |
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| 1 SPF | : | 2 SP | F/ | | | |
| ſ | Ę | 5'6 1/2" | | | | 3 1/2" |
| 1 | 5 | '6 1/2" | | | | |
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| - | Analysis | | | | | |
| Fasten all Capacity | plies using 3 rows | s of 10d Box nails (.128x3' 85.5 % |) at 12" o.c Maxi | num end distance r | ot to exceed 6". | |
| oad | | 210.0 PLF | | | | |
| ′ield Limit pe ′ield Limit pe | | 245.6 PLF 81.9 lb. | | | | |
| ield Mode | | IV | | | | |
| dge Distand lin. End Dist | | 1 1/2" 3" | | | | |
| oad Combin | ation | D+L | | | | |
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| Notes | | chemicals | | provide proper drainage to prevent | Manufacturer Info | Comtech, Inc. 1001 S. Reilly Road, Suite #639 |
| Calculated Struct structural adequa | acy of this component based or | of the Handling & Installation | ponding | - , | Metsä Wood 301 Merritt 7 Building, 2nd Floor | Fayetteville, NC USA |
| design criteria responsibility of | and loadings shown. It is the customer and/or the contract nponent suitability of the inte | the 2. Refer to manufacturer's product or to regarding installation requirement | s, multi-ply | | Norwalk, CT 06851 (800) 622-5850 | 28314 910-864-TRUS |
| application, and to Lumber | ponent suitability of the interponent suitability of the inter | approvals 3. Damaged Beams must not be used | | | www.metsawood.com/us | |
| 1. Dry service co | onditions, unless noted otherwise treated with fire retardant or corre | Design assumes top edge is laterally re Provide lateral support at bearing pre- | ints to avoid | | | соттесн |
| | | incrai displacement and rotation | This design | is valid until 11/3/2024 | | |

This design is valid until 11/3/2024

CSD DESIGN